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SHEET

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STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION

DIVISION OF HIGHWAYS

GEOTECHNICAL ENGINEERING UNIT

**STRUCTURE
SUBSURFACE INVESTIGATION**

PROJ. REFERENCE NO. 33751.1.1 (B-4531) F.A. PROJ. BRZ-1343(1)
 COUNTY GREENE & PITT
 PROJECT DESCRIPTION BRIDGE NO. 36 ON SR 1343 OVER LITTLE
 CONTENTNEA CREEK

INVENTORY

STATE	STATE PROJECT REFERENCE NO.	SHEET NO.	TOTAL SHEETS
N.C.	33751.1.1 (B-4531)	1	8
STATE PROJ. NO.	F.A. PROJ. NO.	DESCRIPTION	
33751.1.1	BRZ-1343(1)	P.E.	
		RW & UTIL.	

CAUTION NOTICE

THE SUBSURFACE INFORMATION AND THE SUBSURFACE INVESTIGATION ON WHICH IT IS BASED WERE MADE FOR THE PURPOSE OF STUDY, PLANNING, AND DESIGN, AND NOT FOR CONSTRUCTION OR PAY PURPOSES. THE VARIOUS FIELD BORING LOGS, ROCK CORES, AND SOIL TEST DATA AVAILABLE MAY BE REVIEWED OR INSPECTED IN RALEIGH BY CONTACTING THE N. C. DEPARTMENT OF TRANSPORTATION, GEOTECHNICAL ENGINEERING UNIT AT 1919 250-4088. NEITHER THE SUBSURFACE PLANS AND REPORTS, NOR THE FIELD BORING LOGS, ROCK CORES, OR SOIL TEST DATA ARE PART OF THE CONTRACT.

GENERAL SOIL AND ROCK STRATA DESCRIPTIONS AND INDICATED BOUNDARIES ARE BASED ON A GEOTECHNICAL INTERPRETATION OF ALL AVAILABLE SUBSURFACE DATA AND MAY NOT NECESSARILY REFLECT THE ACTUAL SUBSURFACE CONDITIONS BETWEEN BORINGS OR BETWEEN SAMPLED STRATA WITHIN THE BOREHOLE. THE LABORATORY SAMPLE DATA AND THE IN SITU IN-PLACE TEST DATA CAN BE RELIED ON ONLY TO THE DEGREE OF RELIABILITY INHERENT IN THE STANDARD TEST METHOD. THE OBSERVED WATER LEVELS OR SOIL MOISTURE CONDITIONS INDICATED IN THE SUBSURFACE INVESTIGATIONS ARE AS RECORDED AT THE TIME OF THE INVESTIGATION. THESE WATER LEVELS OR SOIL MOISTURE CONDITIONS MAY VARY CONSIDERABLY WITH TIME ACCORDING TO CLIMATIC CONDITIONS INCLUDING TEMPERATURES, PRECIPITATION, AND WIND, AS WELL AS OTHER NON-CLIMATIC FACTORS.

THE BIDDER OR CONTRACTOR IS CAUTIONED THAT DETAILS SHOWN ON THE SUBSURFACE PLANS ARE PRELIMINARY ONLY AND IN MANY CASES THE FINAL DESIGN DETAILS ARE DIFFERENT. FOR BIDDING AND CONSTRUCTION PURPOSES, REFER TO THE CONSTRUCTION PLANS AND DOCUMENTS FOR FINAL DESIGN INFORMATION ON THIS PROJECT. THE DEPARTMENT DOES NOT WARRANT OR GUARANTEE THE SUFFICIENCY OR ACCURACY OF THE INVESTIGATION MADE, NOR THE INTERPRETATIONS MADE, OR OPINION OF THE DEPARTMENT AS TO THE TYPE OF MATERIALS AND CONDITIONS TO BE ENCOUNTERED. THE BIDDER OR CONTRACTOR IS CAUTIONED TO MAKE SUCH INDEPENDENT SUBSURFACE INVESTIGATIONS AS HE DEEMS NECESSARY TO SATISFY HIMSELF AS TO CONDITIONS TO BE ENCOUNTERED ON THIS PROJECT. THE CONTRACTOR SHALL HAVE NO CLAIM FOR ADDITIONAL COMPENSATION OR FOR AN EXTENSION OF TIME FOR ANY REASON RESULTING FROM THE ACTUAL CONDITIONS ENCOUNTERED AT THE SITE DIFFERING FROM THOSE INDICATED IN THE SUBSURFACE INFORMATION.

PERSONNEL

JRS

RES

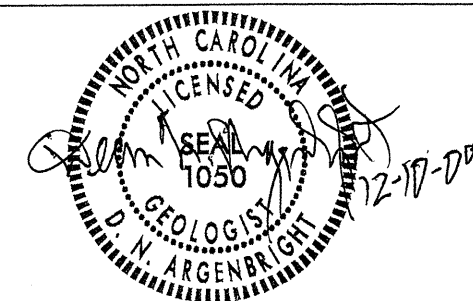
JME

INVESTIGATED BY J.R. SWARTLEY

CHECKED BY D.N. ARGENBRIGHT

SUBMITTED BY D.N. ARGENBRIGHT

DATE DECEMBER 2009



DRAWN BY: C.R. SUMNER

NOTE - THE INFORMATION CONTAINED HEREIN IS NOT IMPLIED OR GUARANTEED BY THE N. C. DEPARTMENT OF TRANSPORTATION AS BEING ACCURATE NOR IT IS CONSIDERED TO BE PART OF THE PLANS, SPECIFICATIONS, OR CONTRACT FOR THE PROJECT.

NOTE - BY HAVING REQUESTED THIS INFORMATION THE CONTRACTOR SPECIFICALLY WAIVES ANY CLAIMS FOR INCREASED COMPENSATION OR EXTENSION OF TIME BASED ON DIFFERENCES BETWEEN THE CONDITIONS INDICATED HEREIN AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

ID: B-4531

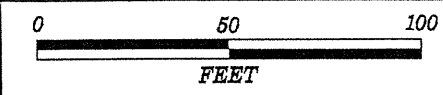
PROJECT: 33751.1.1

NORTH CAROLINA DEPARTMENT OF TRANSPORTATION
DIVISION OF HIGHWAYS
GEOTECHNICAL ENGINEERING UNIT

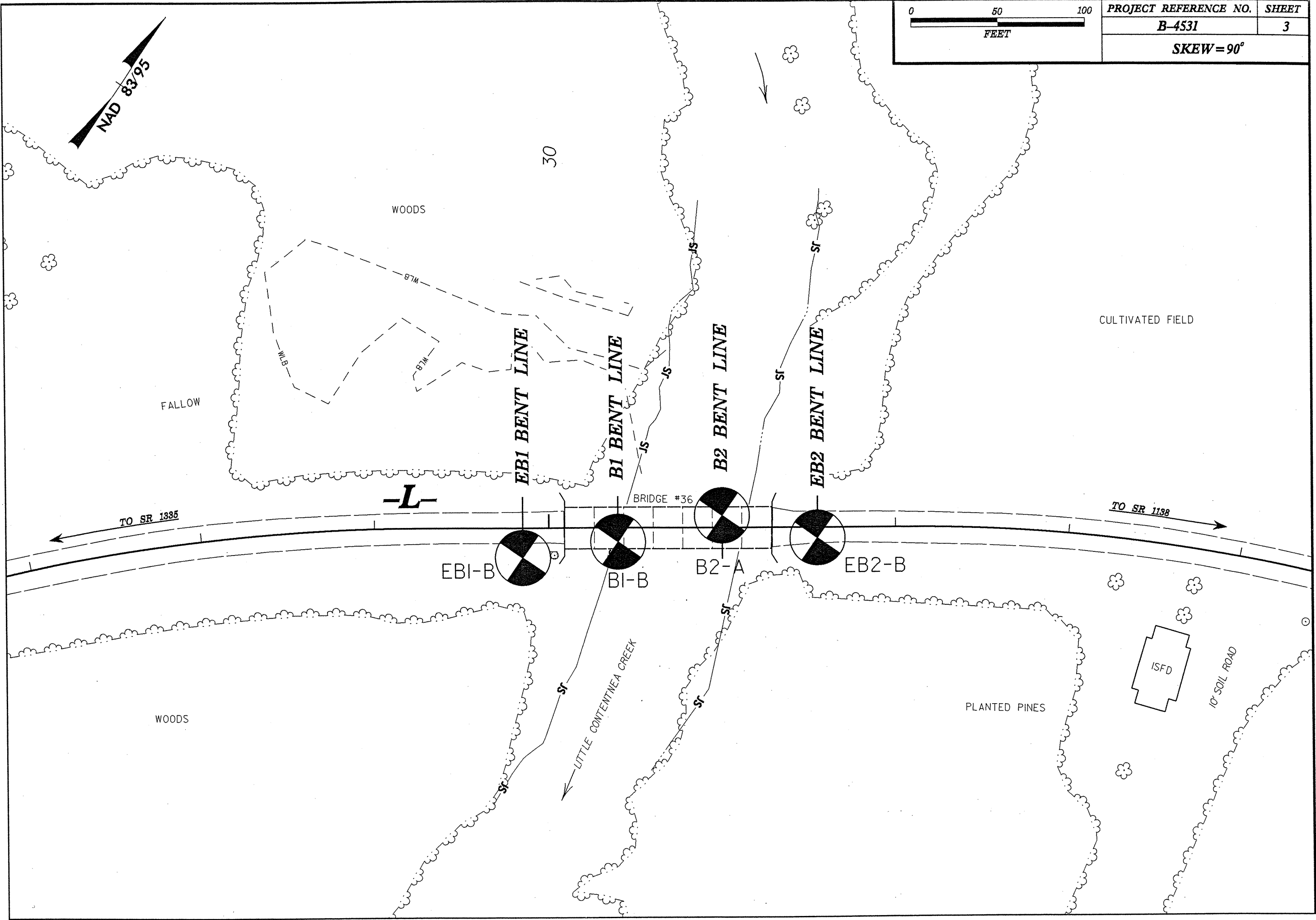
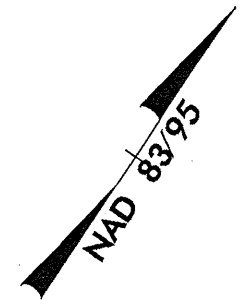
SUBSURFACE INVESTIGATION

SOIL AND ROCK LEGEND, TERMS, SYMBOLS, AND ABBREVIATIONS

SOIL DESCRIPTION	GRADATION	ROCK DESCRIPTION	TERMS AND DEFINITIONS
SOIL IS CONSIDERED TO BE THE UNCONSOLIDATED, SEMI-CONSOLIDATED, OR WEATHERED EARTH MATERIALS THAT CAN BE PENETRATED WITH A CONTINUOUS FLIGHT POWER AUGER, AND YIELD LESS THAN 100 BLOWS PER FOOT ACCORDING TO STANDARD PENETRATION TEST (ASHTO T206, ASTM D-1586). SOIL CLASSIFICATION IS BASED ON THE ASHTO SYSTEM. BASIC DESCRIPTIONS GENERALLY SHALL INCLUDE: CONSISTENCY, COLOR, TEXTURE, MOISTURE, ASHTO CLASSIFICATION, AND OTHER PERTINENT FACTORS SUCH AS MINERALOGICAL COMPOSITION, ANGULARITY, STRUCTURE, PLASTICITY, ETC. EXAMPLE: <i>VERY STIFF, GRAY, SILTY CLAY, MOST WITH INTERBEDDED FINE SAND LAYERS, HIGH PLASTIC, A-7-6</i>	WELL GRADED - INDICATES A GOOD REPRESENTATION OF PARTICLE SIZES FROM FINE TO COARSE. UNIFORM - INDICATES THAT SOIL PARTICLES ARE ALL APPROXIMATELY THE SAME SIZE. (ALSO POORLY GRADED) GAP-GRADED - INDICATES A MIXTURE OF UNIFORM PARTICLES OF TWO OR MORE SIZES. ANGULARITY OF GRAINS THE ANGULARITY OR ROUNDNESS OF SOIL GRAINS IS DESIGNATED BY THE TERMS: ANGULAR , SUBANGULAR , SUBROUNDED , OR ROUNDED .	HARD ROCK IS NON-COASTAL PLAIN MATERIAL THAT IF TESTED, WOULD YIELD SPT REFUSAL. AN INFERRED ROCK LINE INDICATES THE LEVEL AT WHICH NON-COASTAL PLAIN MATERIAL WOULD YIELD SPT REFUSAL. SPT REFUSAL IS PENETRATION BY A SPLIT SPOON SAMPLER EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS. IN NON-COASTAL PLAIN MATERIAL, THE TRANSITION BETWEEN SOIL AND ROCK IS OFTEN REPRESENTED BY A ZONE OF WEATHERED ROCK. ROCK MATERIALS ARE TYPICALLY DIVIDED AS FOLLOWS: WEATHERED ROCK (WR) - NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT N VALUES > 100 BLOWS PER FOOT IF TESTED. CRYSTALLINE ROCK (CR) - FINE TO COARSE GRAIN IGNEOUS AND METAMORPHIC ROCK THAT WOULD YIELD SPT REFUSAL IF TESTED. ROCK TYPE INCLUDES GRANITE, GNEISS, GABBRO, SCHIST, ETC. NON-CRYSTALLINE ROCK (NCR) - FINE TO COARSE GRAIN METAMORPHIC AND NON-COASTAL PLAIN SEDIMENTARY ROCK THAT WOULD YIELD SPT REFUSAL IF TESTED. ROCK TYPE INCLUDES PHYLLITE, SLATE, SANDSTONE, ETC. COASTAL PLAIN SEDIMENTARY ROCK (CP) - COASTAL PLAIN SEDIMENTS CEMENTED INTO ROCK, BUT MAY NOT YIELD SPT REFUSAL. ROCK TYPE INCLUDES LIMESTONE, SANDSTONE, CEMENTED SHELL BEDS, ETC.	ALLUVIUM (ALLUV.) - SOILS THAT HAVE BEEN TRANSPORTED BY WATER. AQUIFER - A WATER BEARING FORMATION OR STRATA. ARENACEOUS - APPLIED TO ROCKS THAT HAVE BEEN DERIVED FROM SAND OR THAT CONTAIN SAND. ARGILLACEOUS - APPLIED TO ALL ROCKS OR SUBSTANCES COMPOSED OF CLAY MINERALS, OR HAVING A NOTABLE PROPORTION OF CLAY IN THEIR COMPOSITION, AS SHALE, SLATE, ETC. ARTESIAN - GROUND WATER THAT IS UNDER SUFFICIENT PRESSURE TO RISE ABOVE THE LEVEL AT WHICH IT IS ENCOUNTERED, BUT WHICH DOES NOT NECESSARILY RISE TO OR ABOVE THE GROUND SURFACE. CALCAREOUS (CALC.) - SOILS THAT CONTAIN APPRECIABLE AMOUNTS OF CALCIUM CARBONATE. COLLUVIUM - ROCK FRAGMENTS MIXED WITH SOIL DEPOSITED BY GRAVITY ON SLOPE OR AT BOTTOM OF SLOPE. CORE RECOVERY (REC.) - TOTAL LENGTH OF ALL MATERIAL RECOVERED IN THE CORE BARREL DIVIDED BY TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE. DIKE - A TABULAR BODY OF IGNEOUS ROCK THAT CUTS ACROSS THE STRUCTURE OF ADJACENT ROCKS OR CUTS MASSIVE ROCK. DIP - THE ANGLE AT WHICH A STRATUM OR ANY PLANAR FEATURE IS INCLINED FROM THE HORIZONTAL. DIP DIRECTION (DIP AZIMUTH) - THE DIRECTION OR BEARING OF THE HORIZONTAL TRACE OF THE LINE OF DIP, MEASURED CLOCKWISE FROM NORTH. FAULT - A FRACTURE OR FRACTURE ZONE ALONG WHICH THERE HAS BEEN DISPLACEMENT OF THE SIDES RELATIVE TO ONE ANOTHER PARALLEL TO THE FRACTURE. FISSILE - A PROPERTY OF SPLITTING ALONG CLOSELY SPACED PARALLEL PLANES. FLOAT - ROCK FRAGMENTS ON SURFACE NEAR THEIR ORIGINAL POSITION AND DISLOGGED FROM PARENT MATERIAL. FLOOD PLAIN (FP) - LAND BORDERING A STREAM, BUILT OF SEDIMENTS DEPOSITED BY THE STREAM. FORMATION (FM) - A MAPPABLE GEOLOGIC UNIT THAT CAN BE RECOGNIZED AND TRACED IN THE FIELD. JOINT - FRACTURE IN ROCK ALONG WHICH NO APPRECIABLE MOVEMENT HAS OCCURRED. LEDGE - A SHELF-LIKE RIDGE OR PROJECTION OF ROCK WHOSE THICKNESS IS SMALL COMPARED TO ITS LATERAL EXTENT. LENS - A BODY OF SOIL OR ROCK THAT THINS OUT IN ONE OR MORE DIRECTIONS. MOTTLED (MOT.) - IRREGULARLY MARKED WITH SPOTS OF DIFFERENT COLORS. MOTTLING IN SOILS USUALLY INDICATES POOR AERATION AND LACK OF GOOD DRAINAGE. PERCHED WATER - WATER MAINTAINED ABOVE THE NORMAL GROUND WATER LEVEL BY THE PRESENCE OF AN INTERVENING IMPERVIOUS STRATUM. RESIDUAL (RES.) SOIL - SOIL FORMED IN PLACE BY THE WEATHERING OF ROCK. ROCK QUALITY DESIGNATION (RQD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE. SAPROLITE (SAP.) - RESIDUAL SOIL THAT RETAINS THE RELIC STRUCTURE OR FABRIC OF THE PARENT ROCK. SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, THAT HAS BEEN EMPLACED PARALLEL TO THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS. SLICKENSIDE - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT OR SLIP PLANE. STANDARD PENETRATION TEST (PENETRATION RESISTANCE) (SPT) - NUMBER OF BLOWS IN OR BPF OF A 140 LB. HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF 1 FOOT INTO SOIL WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT REFUSAL IS PENETRATION EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS. STRATA CORE RECOVERY (SREC) - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY TOTAL LENGTH OF STRATUM AND EXPRESSED AS A PERCENTAGE. STRATA ROCK QUALITY DESIGNATION (SRQD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS WITHIN A STRATUM EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF STRATA AND EXPRESSED AS A PERCENTAGE. TOPSOIL (TS) - SURFACE SOILS USUALLY CONTAINING ORGANIC MATTER.
SOIL LEGEND AND AASHTO CLASSIFICATION GENERAL CLASS. GRANULAR MATERIALS (<= 35% PASSING #200) SILT-CLAY MATERIALS (> 35% PASSING #200) ORGANIC MATERIALS GROUP CLASS. A-1, A-3, A-2, A-4, A-5, A-6, A-7, A-1, A-2, A-3, A-4, A-5, A-6, A-7 SYMBOL [Grid of patterns for soil classification]	MINERALOGICAL COMPOSITION MINERAL NAMES SUCH AS QUARTZ, FELDSPAR, MICA, TALC, KAOLIN, ETC. ARE USED IN DESCRIPTIONS WHENEVER THEY ARE CONSIDERED OF SIGNIFICANCE. COMPRESSIBILITY SLIGHTLY COMPRESSIBLE LIQUID LIMIT LESS THAN 31 MODERATELY COMPRESSIBLE LIQUID LIMIT EQUAL TO 31-50 HIGHLY COMPRESSIBLE LIQUID LIMIT GREATER THAN 50 PERCENTAGE OF MATERIAL ORGANIC MATERIAL GRANULAR SOILS SILT-CLAY SOILS OTHER MATERIAL TRACE OF ORGANIC MATTER 2 - 3% 3 - 5% TRACE 1 - 10% LITTLE ORGANIC MATTER 3 - 5% 5 - 12% LITTLE 10 - 20% MODERATELY ORGANIC 5 - 10% 12 - 20% SOME 20 - 35% HIGHLY ORGANIC >10% >20% HIGHLY 35% AND ABOVE	WEATHERING FRESH ROCK FRESH, CRYSTALS BRIGHT, FEW JOINTS MAY SHOW SLIGHT STAINING, ROCK RINGS UNDER HAMMER IF CRYSTALLINE. VERY SLIGHT (V SLI.) ROCK GENERALLY FRESH, JOINTS STAINED, SOME JOINTS MAY SHOW THIN CLAY COATINGS IF OPEN. CRYSTALS ON A BROKEN SPECIMEN FACE SHINE BRIGHTLY. ROCK RINGS UNDER HAMMER BLOWS IF OF A CRYSTALLINE NATURE. SLIGHT (SLI.) ROCK GENERALLY FRESH, JOINTS STAINED AND DISCOLORATION EXTENDS INTO ROCK UP TO 1 INCH. OPEN JOINTS MAY CONTAIN CLAY. IN GRANITOID ROCKS SOME OCCASIONAL FELDSPAR CRYSTALS ARE DULL AND DISCOLORED. CRYSTALLINE ROCKS RING UNDER HAMMER BLOWS. MODERATE (MOD.) SIGNIFICANT PORTIONS OF ROCK SHOW DISCOLORATION AND WEATHERING EFFECTS. IN GRANITOID ROCKS, MOST FELDSPARS ARE DULL AND DISCOLORED, SOME SHOW CLAY. ROCK HAS DULL SOUND UNDER HAMMER BLOWS AND SHOWS SIGNIFICANT LOSS OF STRENGTH AS COMPARED WITH FRESH ROCK. MODERATELY SEVERE (MOD. SEV.) ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. IN GRANITOID ROCKS, ALL FELDSPARS DULL AND DISCOLORED AND A MAJORITY SHOW KAOLINIZATION. ROCK SHOWS SEVERE LOSS OF STRENGTH AND CAN BE EXCAVATED WITH A GEOLOGIST'S PICK. ROCK GIVES "CLUNK" SOUND WHEN STRUCK. <i>IF TESTED, WOULD YIELD SPT REFUSAL</i> SEVERE (SEV.) ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. ROCK FABRIC CLEAR AND EVIDENT BUT REDUCED IN STRENGTH TO STRONG SOIL. IN GRANITOID ROCKS ALL FELDSPARS ARE KAOLINIZED TO SOME EXTENT. SOME FRAGMENTS OF STRONG ROCK USUALLY REMAIN. <i>IF TESTED, YIELDS SPT N VALUES > 100 BPF</i> VERY SEVERE (V SEV.) ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. ROCK FABRIC ELEMENTS ARE DISCERNIBLE BUT THE MASS IS EFFECTIVELY REDUCED TO SOIL STATUS, WITH ONLY FRAGMENTS OF STRONG ROCK REMAINING. SAPROLITE IS AN EXAMPLE OF ROCK WEATHERED TO A DEGREE SUCH THAT ONLY MINOR VESTIGES OF THE ORIGINAL ROCK FABRIC REMAIN. <i>IF TESTED, YIELDS SPT N VALUES < 100 BPF</i> COMPLETE ROCK REDUCED TO SOIL. ROCK FABRIC NOT DISCERNIBLE, OR DISCERNIBLE ONLY IN SMALL AND SCATTERED CONCENTRATIONS. QUARTZ MAY BE PRESENT AS DIKES OR STRINGERS. SAPROLITE IS ALSO AN EXAMPLE.	
GROUND WATER WATER LEVEL IN BORE HOLE IMMEDIATELY AFTER DRILLING STATIC WATER LEVEL AFTER 24 HOURS PERCHED WATER, SATURATED ZONE, OR WATER BEARING STRATA SPRING OR SEEP	MISCELLANEOUS SYMBOLS ROADWAY EMBANKMENT (RE) WITH SOIL DESCRIPTION SOIL SYMBOL ARTIFICIAL FILL (AF) OTHER THAN ROADWAY EMBANKMENT INFERRED SOIL BOUNDARY INFERRED ROCK LINE ALLUVIAL SOIL BOUNDARY DIP & DIP DIRECTION OF ROCK STRUCTURES TEST BORING WITH CORE AUGER BORING CORE BORING MONITORING WELL PIEZOMETER INSTALLATION SLOPE INDICATOR INSTALLATION CONE PENETROMETER TEST SOUNDING ROD	ROCK HARDNESS VERY HARD CANNOT BE SCRATCHED BY KNIFE OR SHARP PICK. BREAKING OF HAND SPECIMENS REQUIRES SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK. HARD CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY. HARD HAMMER BLOWS REQUIRED TO DETACH HAND SPECIMEN. MODERATELY HARD CAN BE SCRATCHED BY KNIFE OR PICK. GOUGES OR GROOVES TO 0.25 INCHES DEEP CAN BE EXCAVATED BY HARD BLOW OF A GEOLOGIST'S PICK. HAND SPECIMENS CAN BE DETACHED BY MODERATE BLOWS. MEDIUM HARD CAN BE GROUVED OR GOUGED 0.05 INCHES DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT. CAN BE EXCAVATED IN SMALL CHIPS TO PIECES 1 INCH MAXIMUM SIZE BY HARD BLOWS OF THE POINT OF A GEOLOGIST'S PICK. SOFT CAN BE GROUVED OR GOUGED READILY BY KNIFE OR PICK. CAN BE EXCAVATED IN FRAGMENTS FROM CHIPS TO SEVERAL INCHES IN SIZE BY MODERATE BLOWS OF A PICK POINT. SMALL, THIN PIECES CAN BE BROKEN BY FINGER PRESSURE. VERY SOFT CAN BE CARVED WITH KNIFE. CAN BE EXCAVATED READILY WITH POINT OF PICK. PIECES 1 INCH OR MORE IN THICKNESS CAN BE BROKEN BY FINGER PRESSURE. CAN BE SCRATCHED READILY BY FINGERNAIL.	
CONSISTENCY OR DENSITY PRIMARY SOIL TYPE COMPACTNESS OR CONSISTENCY RANGE OF STANDARD PENETRATION RESISTANCE (N-VALUE) RANGE OF UNCONFINED COMPRESSIVE STRENGTH (TONS/FT ²)	ABBREVIATIONS AR - AUGER REFUSAL BT - BORING TERMINATED CL - CLAY CPT - CONE PENETRATION TEST CSE - COARSE DMT - DILATOMETER TEST DPT - DYNAMIC PENETRATION TEST e - VOID RATIO F - FINE FOSS. - FOSSILIFEROUS FRAC. - FRACTURED, FRACTURES FRAGS. - FRAGMENTS HL - HIGHLY MED. - MEDIUM MICA - MICACEOUS MOD. - MODERATELY NP - NON PLASTIC ORG. - ORGANIC PMT - PRESSUREMETER TEST SAP. - SAPROLITIC SD. - SAND, SANDY SL. - SILT, SILTY SLI. - SLIGHTLY TCR - TRICONE REFUSAL w - MOISTURE CONTENT V - VERY VST - VANE SHEAR TEST WEA. - WEATHERED W - UNIT WEIGHT W - DRY UNIT WEIGHT SAMPLE ABBREVIATIONS S - BULK SS - SPLIT SPOON ST - SHELL TUBE SR - ROCK RT - RECOMPACTED TRIAXIAL CBR - CALIFORNIA BEARING RATIO		
TEXTURE OR GRAIN SIZE U.S. STD. SIEVE SIZE (OPENING (MM)) 4 10 40 60 200 270 4.76 2.00 0.42 0.25 0.075 0.053 BOULDER (BLDR.) COBBLE (COB.) GRAVEL (GR.) COARSE SAND (CSE. SD.) FINE SAND (F. SD.) SILT (SL.) CLAY (CL.) GRAIN SIZE MM 305 75 2.0 0.25 0.05 0.005 IN. 12 3			
SOIL MOISTURE - CORRELATION OF TERMS SOIL MOISTURE SCALE (ATTERBERG LIMITS) FIELD MOISTURE DESCRIPTION GUIDE FOR FIELD MOISTURE DESCRIPTION LL - LIQUID LIMIT - SATURATED - (SAT.) USUALLY LIQUID; VERY WET, USUALLY FROM BELOW THE GROUND WATER TABLE PL - PLASTIC LIMIT - WET - (W) SEMISOLID; REQUIRES DRYING TO ATTAIN OPTIMUM MOISTURE OM - OPTIMUM MOISTURE - MOIST - (M) SOLID; AT OR NEAR OPTIMUM MOISTURE SL - SHRINKAGE LIMIT - DRY - (D) REQUIRES ADDITIONAL WATER TO ATTAIN OPTIMUM MOISTURE			
PLASTICITY PLASTICITY INDEX (PI) DRY STRENGTH NONPLASTIC 0-5 VERY LOW LOW PLASTICITY 6-15 SLIGHT MED. PLASTICITY 16-25 MEDIUM HIGH PLASTICITY 26 OR MORE HIGH	EQUIPMENT USED ON SUBJECT PROJECT DRILL UNITS: MOBILE B-51, BK-51, CME-45C, CME-550, PORTABLE HOIST, CME-45B ADVANCING TOOLS: CLAY BITS, 6" CONTINUOUS FLIGHT AUGER, 8" HOLLOW AUGERS, HARD FACED FINGER BITS, TUNG-CARBIDE INSERTS, CASING w/ ADVANCER, TRICONE 2 1/16" STEEL TEETH, TRICONE " " TUNG-CARB., CORE BIT HAMMER TYPE: AUTOMATIC, MANUAL CORE SIZE: B, N, H HAND TOOLS: POST HOLE DIGGER, HAND AUGER, SOUNDING ROD, VANE SHEAR TEST	FRACTURE SPACING TERM SPACING THICKNESS VERY WIDE MORE THAN 10 FEET VERY THICKLY BEDDED > 4 FEET WIDE 3 TO 10 FEET THICKLY BEDDED 1.5 - 4 FEET MODERATELY CLOSE 1 TO 3 FEET THINLY BEDDED 0.16 - 1.5 FEET CLOSE 0.16 TO 1 FEET VERY THINLY BEDDED 0.03 - 0.16 FEET VERY CLOSE LESS THAN 0.16 FEET THICKLY LAMINATED 0.008 - 0.03 FEET THINLY LAMINATED < 0.008 FEET	INDURATION FOR SEDIMENTARY ROCKS, INDURATION IS THE HARDENING OF THE MATERIAL BY CEMENTING, HEAT, PRESSURE, ETC. FRIABLE RUBBING WITH FINGER FREES NUMEROUS GRAINS; GENTLE BLOW BY HAMMER DISINTEGRATES SAMPLE. MODERATELY INDURATED GRAINS CAN BE SEPARATED FROM SAMPLE WITH STEEL PROBE; BREAKS EASILY WHEN HIT WITH HAMMER. INDURATED GRAINS ARE DIFFICULT TO SEPARATE WITH STEEL PROBE; DIFFICULT TO BREAK WITH HAMMER. EXTREMELY INDURATED SHARP HAMMER BLOWS REQUIRED TO BREAK SAMPLE; SAMPLE BREAKS ACROSS GRAINS.
COLOR DESCRIPTIONS MAY INCLUDE COLOR OR COLOR COMBINATIONS (TAN, RED, YELLOW-BROWN, BLUE-GRAY). MODIFIERS SUCH AS LIGHT, DARK, STREAKED, ETC. ARE USED TO DESCRIBE APPEARANCE.			NOTES: BENCH MARK: RR SPIKE IN 18" HARDWOOD -BL- STA. 26+30, 152' LT ELEVATION: 44.67 FT.

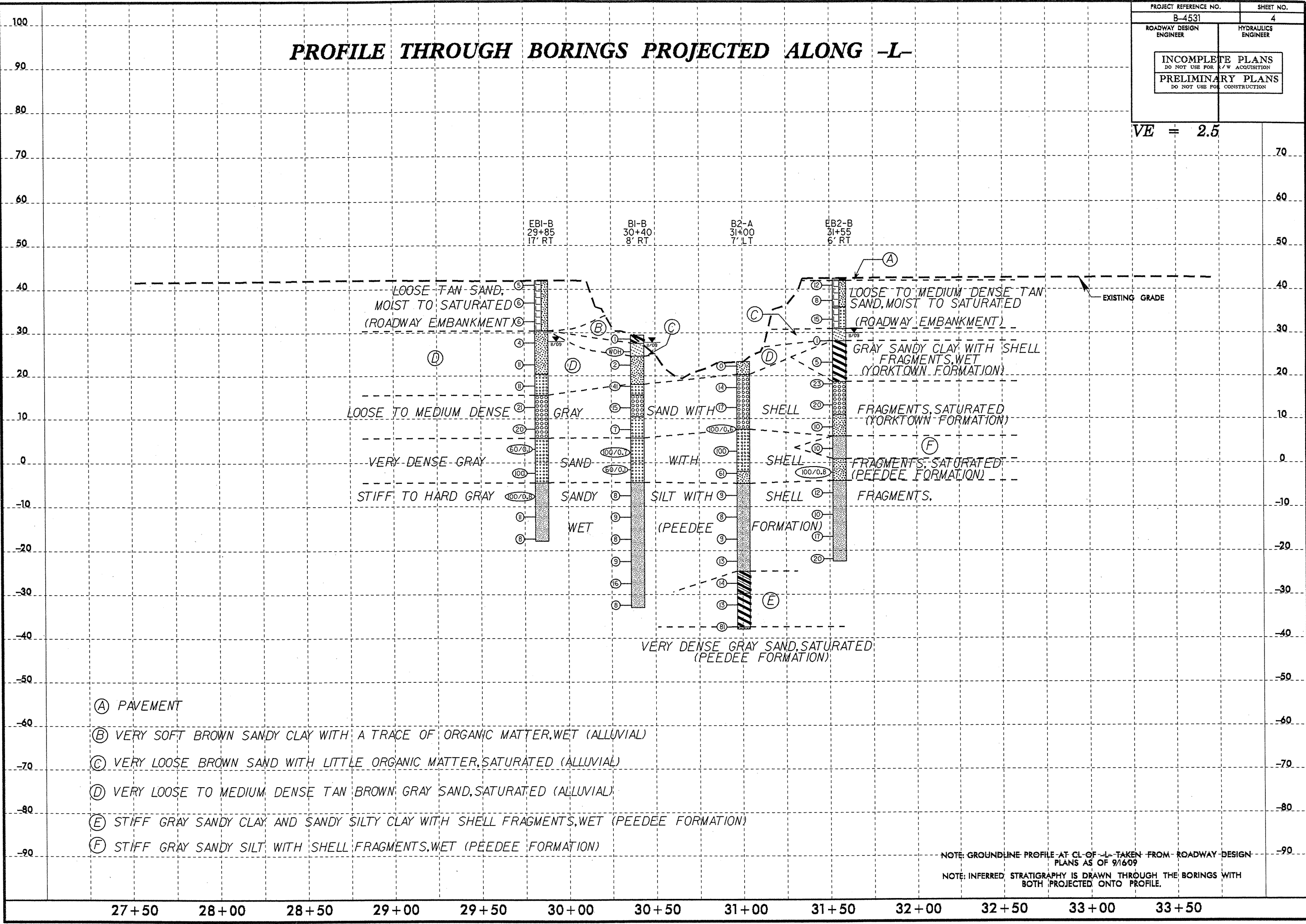


PROJECT REFERENCE NO.	SHEET
B-4531	3
SKEW = 90°	



PROFILE THROUGH BORINGS PROJECTED ALONG -L-

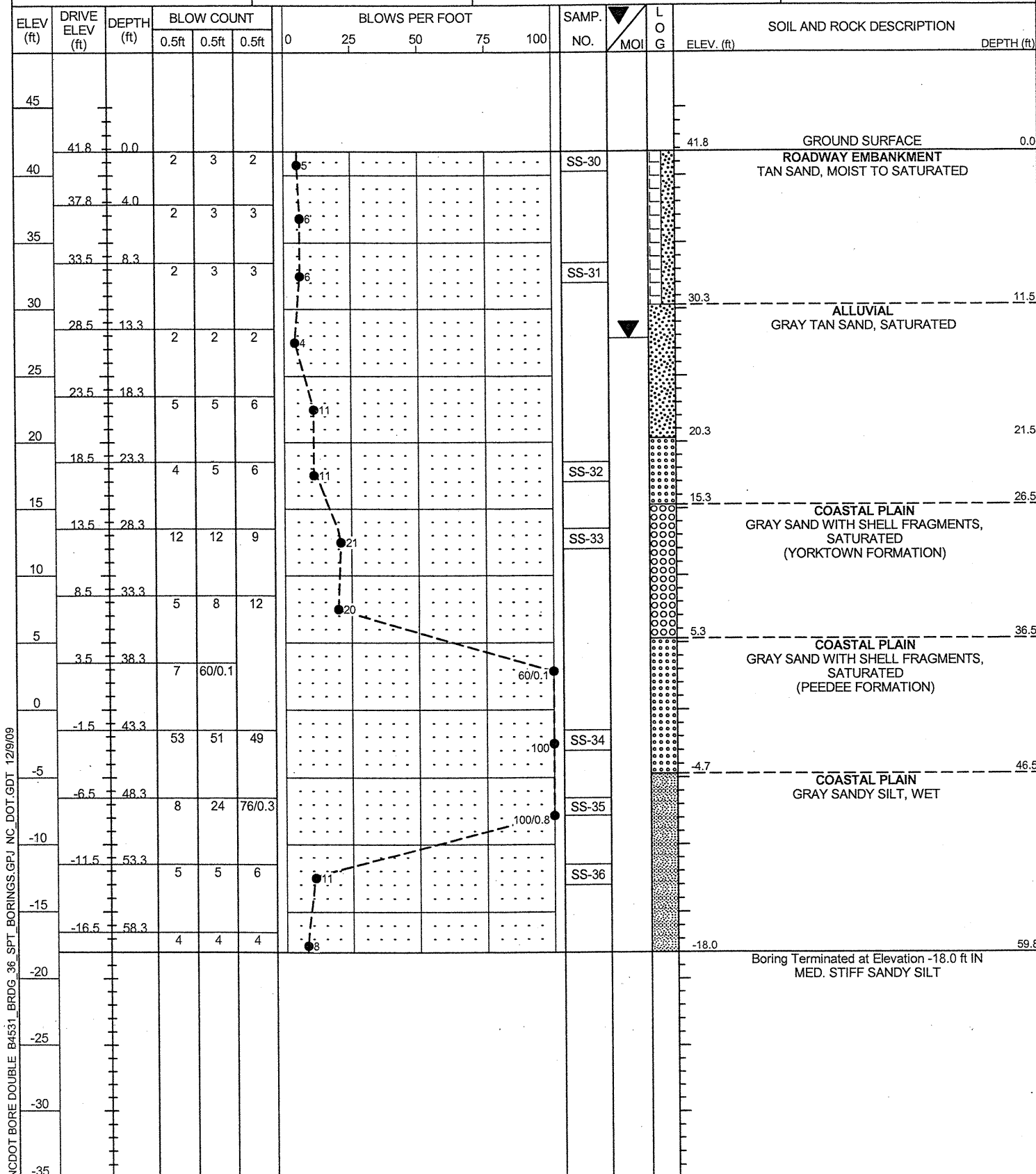
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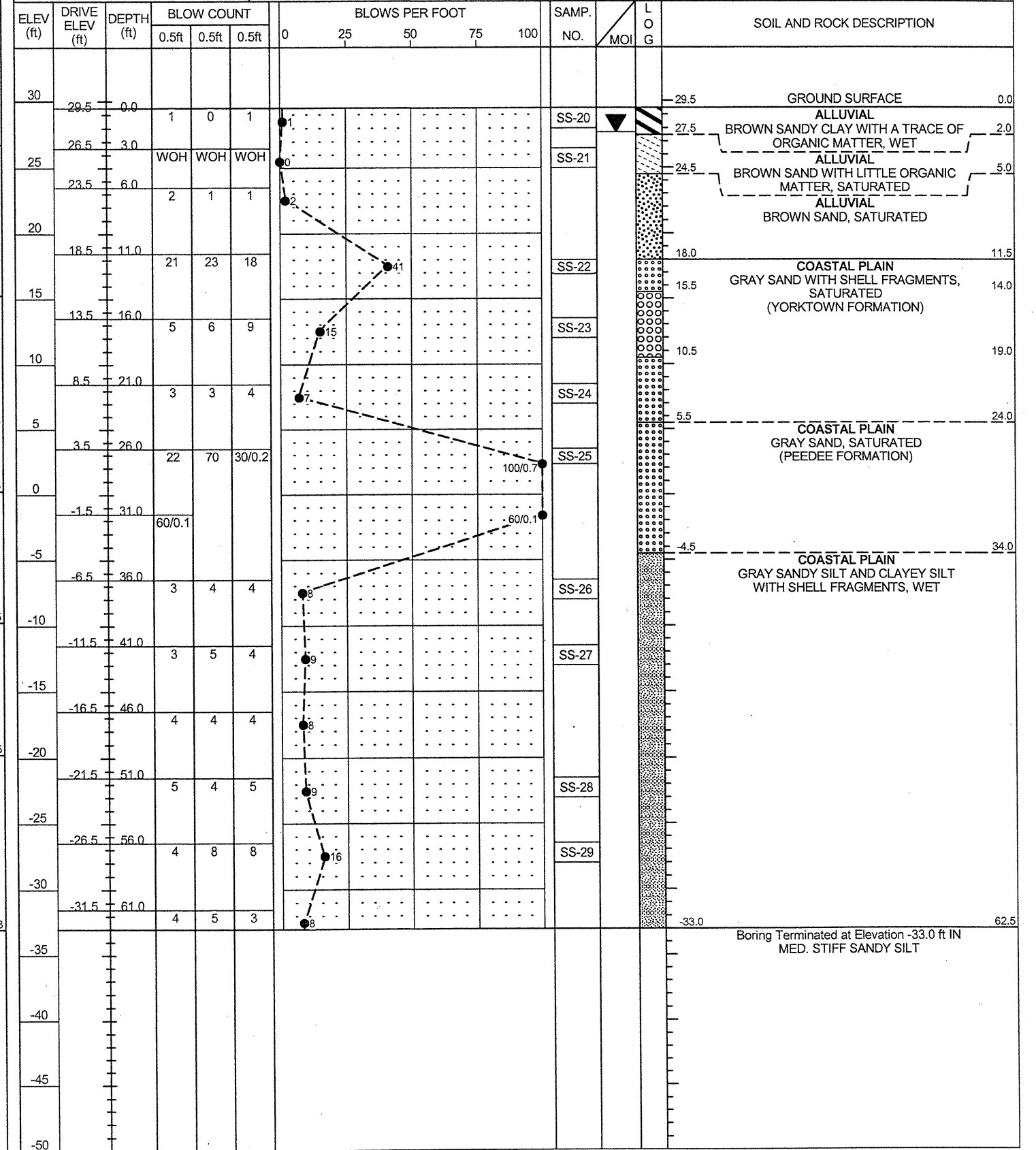
27+50 28+00 28+50 29+00 29+50 30+00 30+50 31+00 31+50 32+00 32+50 33+00 33+50

NCDOT GEOTECHNICAL ENGINEERING UNIT
BORELOG REPORT

PROJECT NO. 33751.1.1	ID. B-4531	COUNTY PITT/GREENE	GEOLOGIST Swartley, J. R.
SITE DESCRIPTION BRIDGE NO. 36 ON -L- (SR 1343) OVER LITTLE CONTENTNEA CREEK			GROUND WTR (ft)
BOHRING NO. EB1-B	STATION 29+85	OFFSET 17ft RT	ALIGNMENT -L-
COLLAR ELEV. 41.8 ft	TOTAL DEPTH 59.8 ft	NORTHING 649,226	EASTING 2,439,456
DRILL MACHINE CME-45B	DRILL METHOD Mud Rotary	HAMMER TYPE Automatic	
START DATE 11/05/09	COMP. DATE 11/06/09	SURFACE WATER DEPTH N/A	DEPTH TO ROCK N/A



PROJECT NO. 33751.1.1	ID. B-4531	COUNTY PITT/GREENE	GEOLOGIST Swartley, J. R.
SITE DESCRIPTION BRIDGE NO. 36 ON -L- (SR 1343) OVER LITTLE CONTENTNEA CREEK			GROUND WTR (ft)
BOHRING NO. B1-B	STATION 30+40	OFFSET 8ft RT	ALIGNMENT -L-
COLLAR ELEV. 29.5 ft	TOTAL DEPTH 62.5 ft	NORTHING 649,265	EASTING 2,439,496
DRILL MACHINE CME-45B	DRILL METHOD Mud Rotary	HAMMER TYPE Automatic	
START DATE 11/04/09	COMP. DATE 11/05/09	SURFACE WATER DEPTH N/A	DEPTH TO ROCK N/A



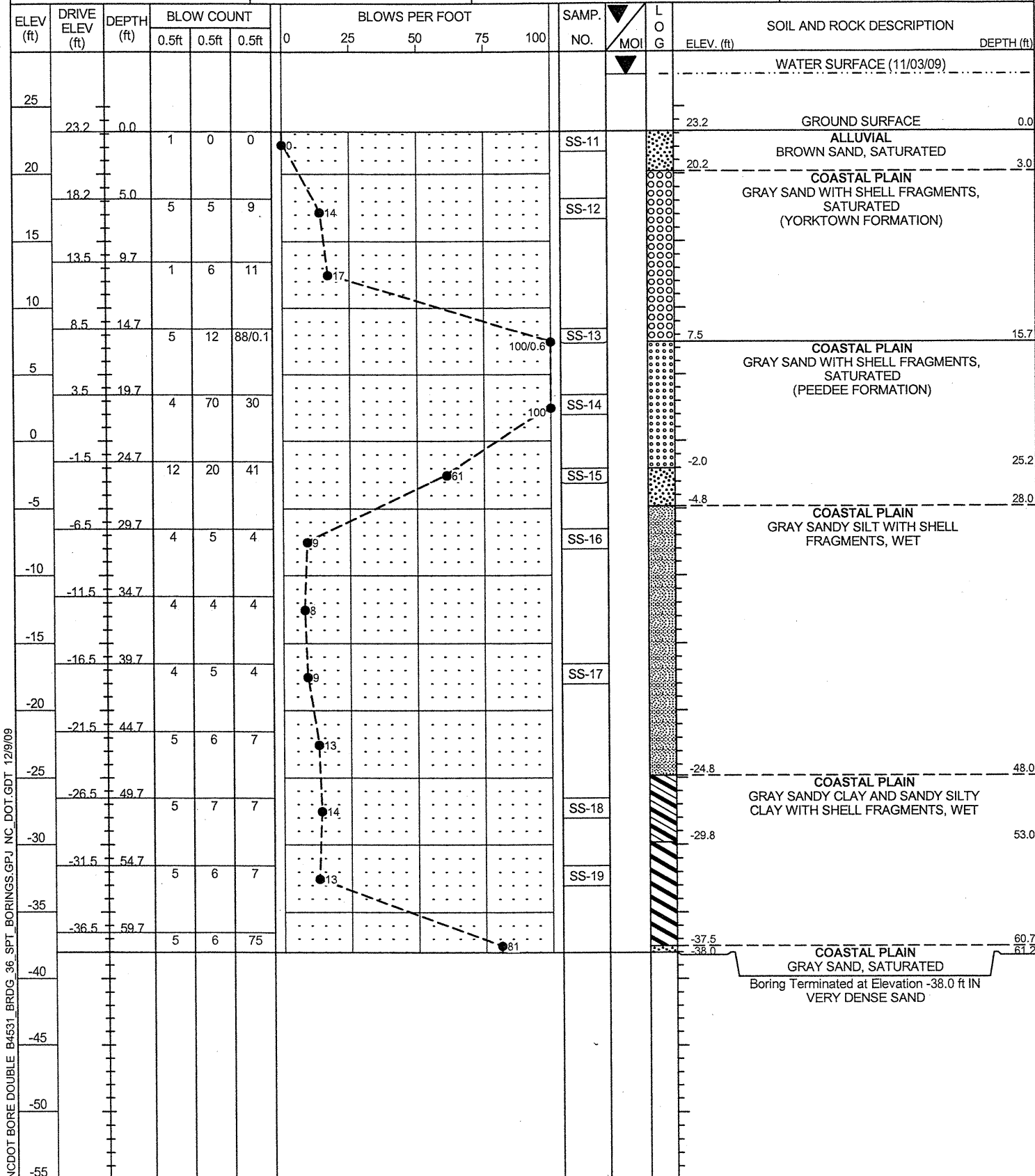
NCDOT BORE DOUBLE B4531_BRDG_36_BRDG_36_SPT_BORINGS.GPJ NC_DOT_GDT 12/9/09



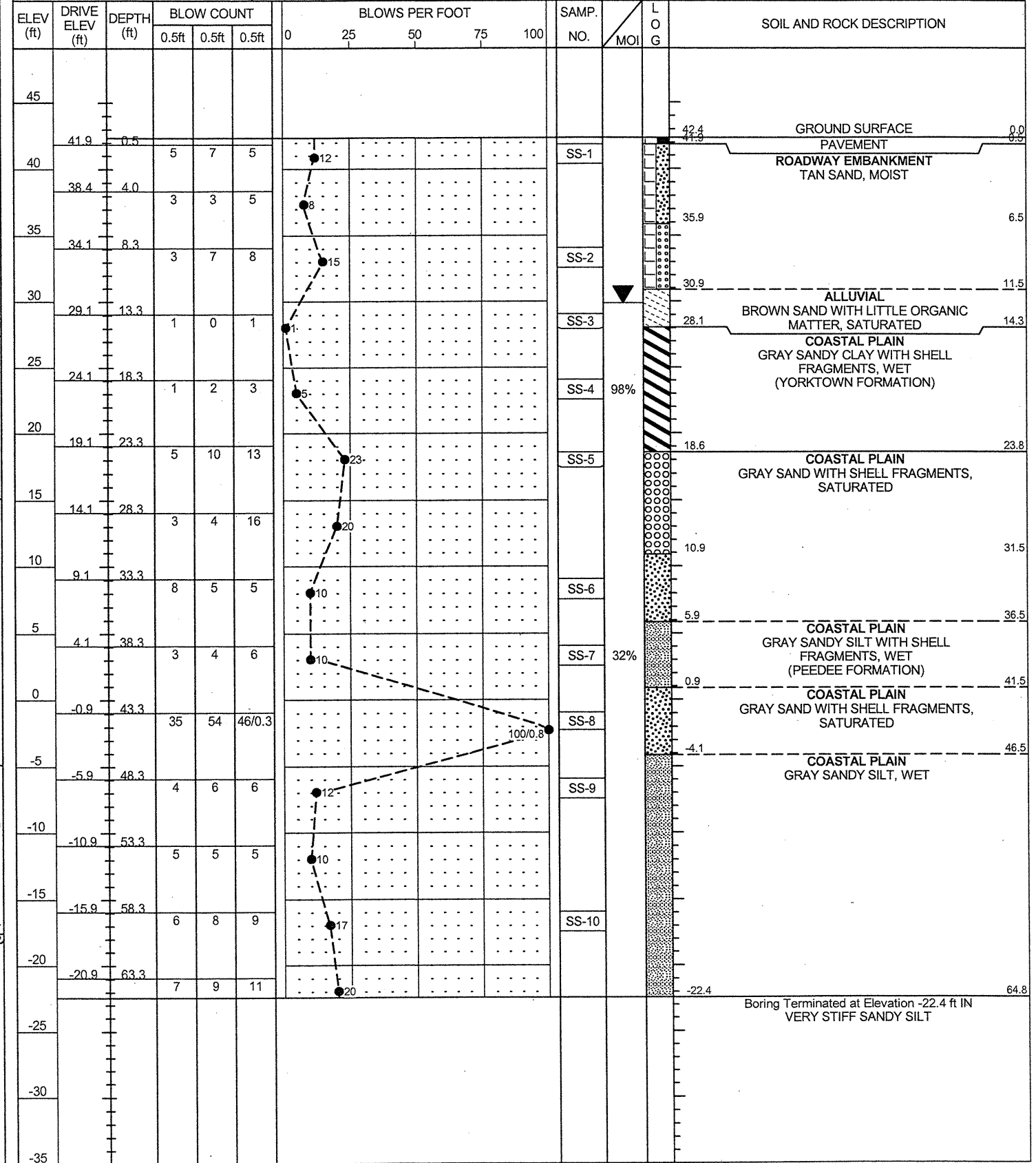
NCDOT GEOTECHNICAL ENGINEERING UNIT

BORELOG REPORT

PROJECT NO. 33751.1.1	ID. B-4531	COUNTY PITT/GREENE	GEOLOGIST Swartley, J. R.
SITE DESCRIPTION BRIDGE NO. 36 ON -L- (SR 1343) OVER LITTLE CONTENTNEA CREEK			GROUND WTR (ft)
BORING NO. B2-A	STATION 31+00	OFFSET 7ft LT	ALIGNMENT -L-
COLLAR ELEV. 23.2 ft	TOTAL DEPTH 61.2 ft	NORTHING 649,311	EASTING 2,439,537
DRILL MACHINE CME-45B	DRILL METHOD Mud Rotary	HAMMER TYPE Automatic	
START DATE 11/03/09	COMP. DATE 11/04/09	SURFACE WATER DEPTH 4.2ft	DEPTH TO ROCK N/A



PROJECT NO. 33751.1.1	ID. B-4531	COUNTY PITT/GREENE	GEOLOGIST Swartley, J. R.
SITE DESCRIPTION BRIDGE NO. 36 ON -L- (SR 1343) OVER LITTLE CONTENTNEA CREEK			GROUND WTR (ft)
BORING NO. EB2-B	STATION 31+55	OFFSET 6ft RT	ALIGNMENT -L-
COLLAR ELEV. 42.4 ft	TOTAL DEPTH 64.8 ft	NORTHING 649,332	EASTING 2,439,590
DRILL MACHINE CME-45B	DRILL METHOD Mud Rotary	HAMMER TYPE Automatic	
START DATE 11/03/09	COMP. DATE 11/03/09	SURFACE WATER DEPTH N/A	DEPTH TO ROCK N/A



NCDOT BORE DOUBLE B4531 BRDG 36 SPT BORINGS.GPJ NC DOT.GDT 12/9/09

B-4531

33751.1.1

BRIDGE NO. 36 ON SR 1343 OVER LITTLE CONTENTNEA CREEK

SOIL TEST RESULTS EB1-B															
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	LL	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C. SAND	F. SAND	SILT	CLAY	10	40	200		
SS-30	17 RT	29+85	1.0-1.5	A-2-4(0)	19	NP	33.4	57.0	2.5	7.1	100	89	13	-	-
SS-31	17 RT	29+85	8.3-9.8	A-2-4(0)	22	NP	4.9	83.3	3.7	8.1	100	100	17	-	-
SS-32	17 RT	29+85	23.3-24.8	A-3(0)	26	NP	56.5	39.2	1.2	3.0	97	91	5	-	-
SS-33	17 RT	29+85	28.3-29.8	A-1-b(0)	22	NP	87.6	8.3	0.1	4.0	94	28	5	-	-
SS-34	17 RT	29+85	43.3-44.8	A-3(0)	24	NP	52.7	41.5	0.8	5.1	95	57	7	-	-
SS-35	17 RT	29+85	48.3-49.6	A-4(0)	27	5	6.7	66.5	8.6	18.2	99	95	40	-	-
SS-36	17 RT	29+35	53.3-54.8	A-4(0)	27	3	0.4	73.6	9.8	16.2	100	100	50	-	-

SOIL TEST RESULTS B1-B															
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	LL	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C. SAND	F. SAND	SILT	CLAY	10	40	200		
SS-20	8 RT	30+40	1.0-1.5	ORG. ONLY										-	4.8
SS-21	8 RT	30+40	3.0-4.5	ORG. ONLY										-	3.7
SS-22	8 RT	30+40	11.5-12.5	A-3(0)	21	NP	62.5	32.7	0.8	4.0	97	71	5	-	-
SS-23	8 RT	30+40	16.0-17.5	A-1-b(0)	25	NP	82.1	10.3	1.5	6.1	88	32	8	-	-
SS-24	8 RT	30+40	21.0-22.5	A-3(0)	20	NP	63.4	24.3	5.3	7.1	79	56	10	-	-
SS-25	8 RT	30+40	26.0-27.2	A-3(0)	24	NP	38.0	57.0	0.9	4.0	96	71	6	-	-
SS-26	8 RT	30+40	36.0-37.5	A-4(2)	32	9	2.8	63.9	11.0	22.2	100	98	48	-	-
SS-27	8 RT	30+40	41.0-42.5	A-4(0)	27	3	0.4	72.8	10.6	16.2	100	100	51	-	-
SS-28	8 RT	30+40	51.0-52.5	A-4(1)	29	5	15.0	56.0	8.8	20.2	98	88	52	-	-
SS-29	8 RT	30+40	56.0-57.5	A-4(0)	29	6	19.4	52.8	9.6	18.2	98	84	44	-	-

SOIL TEST RESULTS B2-A															
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	LL	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C. SAND	F. SAND	SILT	CLAY	10	40	200		
SS-11	7 LT	31+00	1.0-1.5	A-2-4(0)	22	NP	32.0	58.5	5.5	4.0	100	85	13	-	-
SS-12	7 LT	31+00	5.0-6.5	A-1-b(0)	23	NP	76.1	14.0	3.8	6.1	85	41	10	-	-
SS-13	7 LT	31+00	14.7-15.7	A-1-b(0)	23	NP	74.1	17.4	4.5	4.0	67	36	6	-	-
SS-14	7 LT	31+00	19.7-21.2	A-3(0)	21	NP	35.5	55.0	5.5	4.0	85	65	10	-	-
SS-15	7 LT	31+00	25.2-26.2	A-2-4(0)	28	NP	5.3	84.8	7.9	2.0	100	98	20	-	-
SS-16	7 LT	31+00	29.7-31.2	A-4(0)	28	4	1.2	65.4	15.2	18.2	98	97	48	-	-
SS-17	7 LT	31+00	39.7-41.2	A-4(0)	26	2	0.4	69.6	13.8	16.2	100	100	55	-	-
SS-18	7 LT	31+00	49.7-51.2	A-6(6)	35	14	2.6	54.3	16.8	26.3	96	95	60	-	-
SS-19	7 LT	31+00	54.7-56.2	A-7-6(22)	46	29	4.9	37.4	23.3	34.4	100	97	79	-	-

SOIL TEST RESULTS EB2-B															
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	LL	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C. SAND	F. SAND	SILT	CLAY	10	40	200		
SS-1	6 RT	31+55	1.0-1.5	A-2-4(0)	18	NP	25.8	58.4	5.7	10.1	100	87	20	-	-
SS-2	6 RT	31+55	8.3-9.8	A-3(0)	22	NP	77.5	18.9	1.5	2.0	99	52	4	-	-
SS-3	6 RT	31+55	13.3-14.3	ORG. ONLY										-	4.8
SS-4	6 RT	31+55	18.3-19.8	A-7-6(18)	55	37	25.1	10.9	11.3	52.6	91	79	59	97.7	-
SS-5	6 RT	31+55	23.8-24.8	A-1-b(0)	21	NP	70.9	13.7	7.4	8.1	82	45	13	-	-
SS-6	6 RT	31+55	33.3-34.8	A-2-4(0)	20	NP	64.8	22.4	3.7	9.1	81	55	11	-	-
SS-7	6 RT	31+55	38.3-39.8	A-4(0)	29	4	1.6	72.4	10.8	15.2	100	99	41	31.9	-
SS-8	6 RT	31+55	43.3-44.6	A-2-4(0)	29	NP	6.8	83.3	5.9	4.1	100	97	18	-	-
SS-9	6 RT	31+55	48.3-49.8	A-4(0)	27	2	0.8	69.4	13.6	16.2	100	100	48	-	-
SS-10	6 RT	31+55	58.3-59.8	A-4(0)	27	2	0.8	67.6	15.4	16.2	98	97	61	-	-



**FIELD
 SCOUR REPORT**

WBS: 33751.1.1 TIP: B-4531 COUNTY: PITT/GREENE

DESCRIPTION(1): BRIDGE NO. 36 ON SR 1343 OVER LITTLE CONENTNEA CREEK

EXISTING BRIDGE

Information from: Field Inspection Microfilm _____ (reel _____ pos: _____)
 Other (explain) _____

Bridge No.: 36 Length: 120 Total Bents: 8 Bents in Channel: 5 Bents in Floodplain: 3
 Foundation Type: TIMBER AND STEEL PILES

EVIDENCE OF SCOUR(2)

Abutments or End Bent Slopes: NONE NOTED

Interior Bents: NONE NOTED

Channel Bed: NONE NOTED

Channel Bank: NONE NOTED

EXISTING SCOUR PROTECTION

Type(3): WOODEN WINGWALLS

Extent(4): 6' OUTSIDE EDGE OF BRIDGE

Effectiveness(5): EFFECTIVE

Obstructions(6): NONE

INSTRUCTIONS

- 1 Describe the specific site's location, including route number and body of water crossed.
- 2 Note scour evidence at existing end bents or abutments (e.g. undermining, sloughing, degradations).
- 3 Note existing scour protection (e.g. rip rap).
- 4 Describe extent of existing scour protection.
- 5 Describe whether or not the scour protection appears to be working.
- 6 Note obstructions such as dams, fallen trees, debris at bents, etc.
- 7 Describe the channel bed material based on observation and/or samples. Include any lab results with report.
- 8 Describe the channel bank material based on observation and/or samples. Include any lab results with report.
- 9 Describe the material covering the banks (e.g. grass, trees, rip rap, none).
- 10 Determine the approximate floodplain width from field observation or a topographic map.
- 11 Describe the material covering the floodplain (e.g. grass, trees, crops).
- 12 Use professional judgement to specify if the stream is degrading, aggrading, or static.
- 13 Describe potential and direction of the stream to migrate laterally during the bridge's life (approx. 100 years).
- 14 Give the design scour elevation (DSE) expected over the life of the bridge (approx. 100 years). This elevation can be given as a range across the site, or for each bent. Discuss the relationship between the Hydraulics Unit theoretical scour and the DSE. If the DSE is dependent on scour counter measures, explain (e.g. rip rap armoring on slopes). The DSE is based on the erodability of materials, giving consideration to the influence of joints, foliation, bedding characteristics, % core recovery, % RQD, differential weathering, shear strength, observations at existing structures, other tests deemed appropriate, and overall geologic conditions at the site.

DESIGN INFORMATION

Channel Bed Material(7): SAND WITH LITTLE ORGANIC MATTER AND SAND

Channel Bank Material(8): SANDY CLAY WITH A TRACE OF ORGANIC MATTER AND SAND WITH LITTLE ORGANIC MATTER

Channel Bank Cover(9): TREES AND SHRUBS

Floodplain Width(10): 1000'

Floodplain Cover(11): TREES AND SHRUBS

Stream is(12): Aggrading Degrading _____ Static _____

Channel Migration Tendency(13): SLIGHTLY TO THE EAST

Observations and Other Comments: _____

DESIGN SCOUR ELEVATIONS(14)

Feet Meters _____

BENTS

B1	B2	B3	B4										
18	19.2												

Comparison of DSE to Hydraulics Unit theoretical scour:
 THE GEOTECHNICAL ENGINEERING UNIT AND THE HYDRAULIC UNIT AGREE THAT THE DSE SHOULD BE RAISED 13.5± FEET FROM THE THEORETICAL SCOUR ELEVATION PROPOSED IN THE BSR.

SOIL ANALYSIS RESULTS FROM CHANNEL BED AND BANK MATERIAL

Bed or Bank													
Sample No.													
Retained #4													
Passed #10													
Passed #40													
Passed #200													
Coarse Sand													
Fine Sand													
Silt													
Clay													
LL													
PI													
AASHTO													
Station													
Offset													
Depth													

See Sheet # 7,
 "Soil Test Results",
 for samples:
 SS-11 (CHANNEL BED)
 SS-20, SS-3 (CHANNEL BANK)

Reported by: Janeth Swathley Date: 11/05/09

STATE	STATE PROJECT REFERENCE NO.	SHEET	TOTAL SHEETS
N.C.	33751.1.1 (B-4531)	1	8
STATE PROJ. NO.	F.A. PROJ. NO.	DESCRIPTION	
33751.1.1	BRZ-1343(1)	P.E. RAW & UTIL.	

CONTENTS

<u>SHEET</u>	<u>DESCRIPTION</u>
1	TITLE SHEET
2	LEGEND
3	SITE PLAN
4	PROFILE
5-6	BORELOGS
7	SOIL TEST RESULTS
8	SCOUR REPORT

STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
DIVISION OF HIGHWAYS
GEOTECHNICAL ENGINEERING UNIT

STRUCTURE
SUBSURFACE INVESTIGATION

PROJ. REFERENCE NO. 33751.1.1 (B-4531) F.A. PROJ. BRZ-1343(1)
COUNTY GREENE & PITT
PROJECT DESCRIPTION BRIDGE NO. 35 OVER LITTLE CONTENTNEA CREEK
OVERFLOW ON SR 1343

CAUTION NOTICE

THE SUBSURFACE INFORMATION AND THE SUBSURFACE INVESTIGATION ON WHICH IT IS BASED WERE MADE FOR THE PURPOSE OF STUDY, PLANNING, AND DESIGN AND NOT FOR CONSTRUCTION OR PAY PURPOSES. THE VARIOUS FIELD BORING LOGS, ROCK CORES, AND SOIL TEST DATA AVAILABLE MAY BE REVIEWED OR INSPECTED IN RALEIGH BY CONTACTING THE N. C. DEPARTMENT OF TRANSPORTATION, GEOTECHNICAL ENGINEERING UNIT AT (919) 250-4088. NEITHER THE SUBSURFACE PLANS AND REPORTS, NOR THE FIELD BORING LOGS, ROCK CORES, OR SOIL TEST DATA ARE PART OF THE CONTRACT.

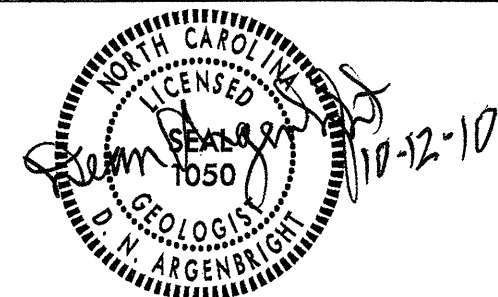
GENERAL SOIL AND ROCK STRATA DESCRIPTIONS AND INDICATED BOUNDARIES ARE BASED ON A GEOTECHNICAL INTERPRETATION OF ALL AVAILABLE SUBSURFACE DATA AND MAY NOT NECESSARILY REFLECT THE ACTUAL SUBSURFACE CONDITIONS BETWEEN BORINGS OR BETWEEN SAMPLED STRATA WITHIN THE BOREHOLE. THE LABORATORY SAMPLE DATA AND THE IN SITU (IN-PLACE) TEST DATA CAN BE RELIED ON ONLY TO THE DEGREE OF RELIABILITY INHERENT IN THE STANDARD TEST METHOD. THE OBSERVED WATER LEVELS OR SOIL MOISTURE CONDITIONS INDICATED IN THE SUBSURFACE INVESTIGATIONS ARE AS RECORDED AT THE TIME OF THE INVESTIGATION. THESE WATER LEVELS OR SOIL MOISTURE CONDITIONS MAY VARY CONSIDERABLY WITH TIME ACCORDING TO CLIMATIC CONDITIONS INCLUDING TEMPERATURES, PRECIPITATION, AND WIND, AS WELL AS OTHER NON-CLIMATIC FACTORS.

THE BIDDER OR CONTRACTOR IS CAUTIONED THAT DETAILS SHOWN ON THE SUBSURFACE PLANS ARE PRELIMINARY ONLY AND IN MANY CASES THE FINAL DESIGN DETAILS ARE DIFFERENT. FOR BIDDING AND CONSTRUCTION PURPOSES, REFER TO THE CONSTRUCTION PLANS AND DOCUMENTS FOR FINAL DESIGN INFORMATION ON THIS PROJECT. THE DEPARTMENT DOES NOT WARRANT OR GUARANTEE THE SUFFICIENCY OR ACCURACY OF THE INVESTIGATION MADE, NOR THE INTERPRETATIONS MADE, OR OPINION OF THE DEPARTMENT AS TO THE TYPE OF MATERIALS AND CONDITIONS TO BE ENCOUNTERED. THE BIDDER OR CONTRACTOR IS CAUTIONED TO MAKE SUCH INDEPENDENT SUBSURFACE INVESTIGATIONS AS HE DEEMS NECESSARY TO SATISFY HIMSELF AS TO CONDITIONS TO BE ENCOUNTERED ON THIS PROJECT. THE CONTRACTOR SHALL HAVE NO CLAIM FOR ADDITIONAL COMPENSATION OR FOR AN EXTENSION OF TIME FOR ANY REASON RESULTING FROM THE ACTUAL CONDITIONS ENCOUNTERED AT THE SITE DIFFERING FROM THOSE INDICATED IN THE SUBSURFACE INFORMATION.

PERSONNEL

JRS
RES
JME

INVESTIGATED BY J.R. SWARTLEY
CHECKED BY D.N. ARGENBRIGHT
SUBMITTED BY D.N. ARGENBRIGHT
DATE OCTOBER 2010

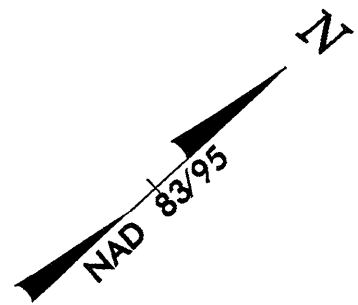


PROJECT: 33751.1.1 ID: B-4531

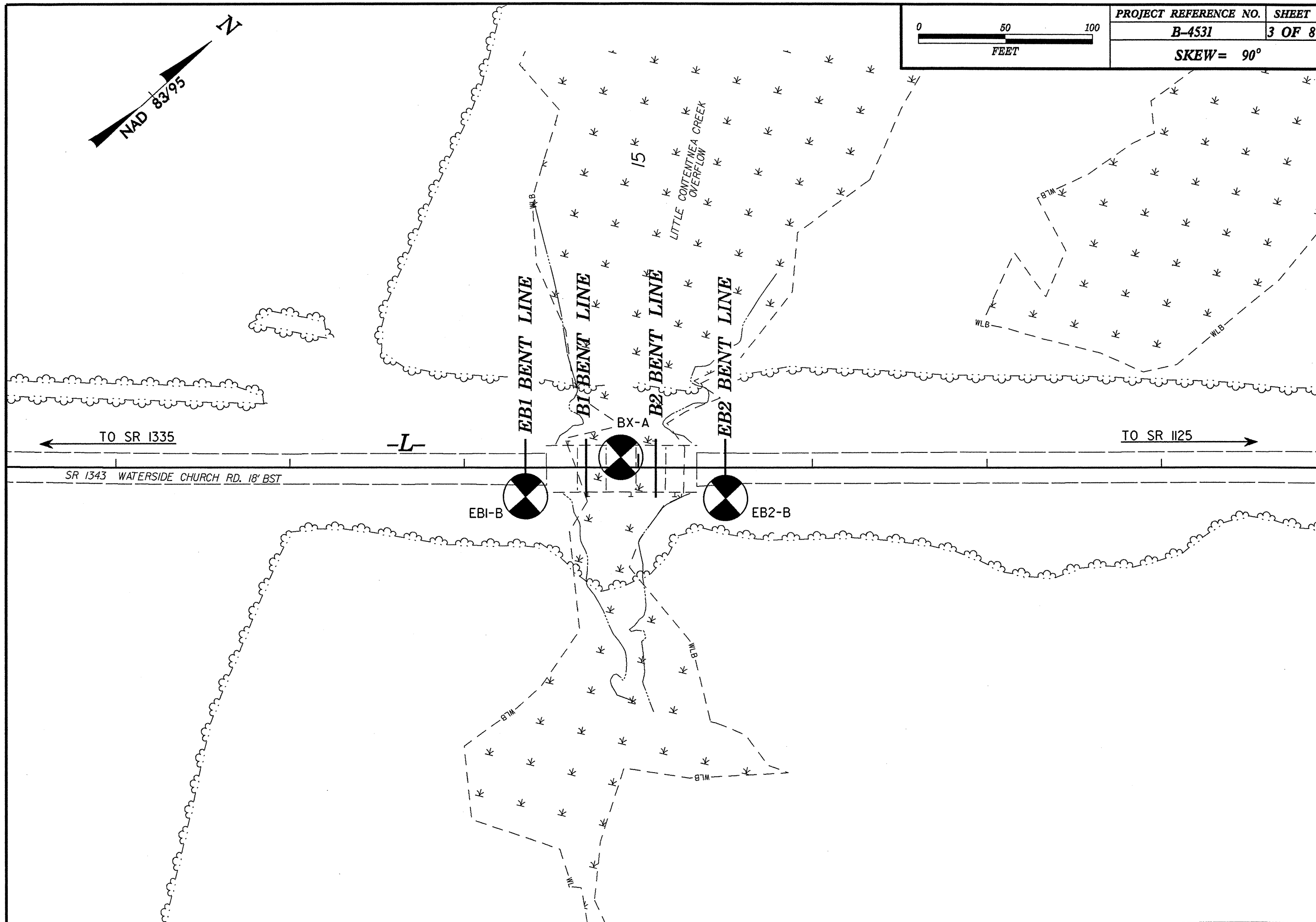
DRAWN BY: C.R. SUMNER

NOTE - THE INFORMATION CONTAINED HEREIN IS NOT IMPLIED OR GUARANTEED BY THE N. C. DEPARTMENT OF TRANSPORTATION AS BEING ACCURATE NOR IS IT CONSIDERED TO BE PART OF THE PLANS, SPECIFICATIONS, OR CONTRACT FOR THE PROJECT.

NOTE - BY HAVING REQUESTED THIS INFORMATION THE CONTRACTOR SPECIFICALLY WAIVES ANY CLAIMS FOR INCREASED COMPENSATION OR EXTENSION OF TIME BASED ON DIFFERENCES BETWEEN THE CONDITIONS INDICATED HEREIN AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

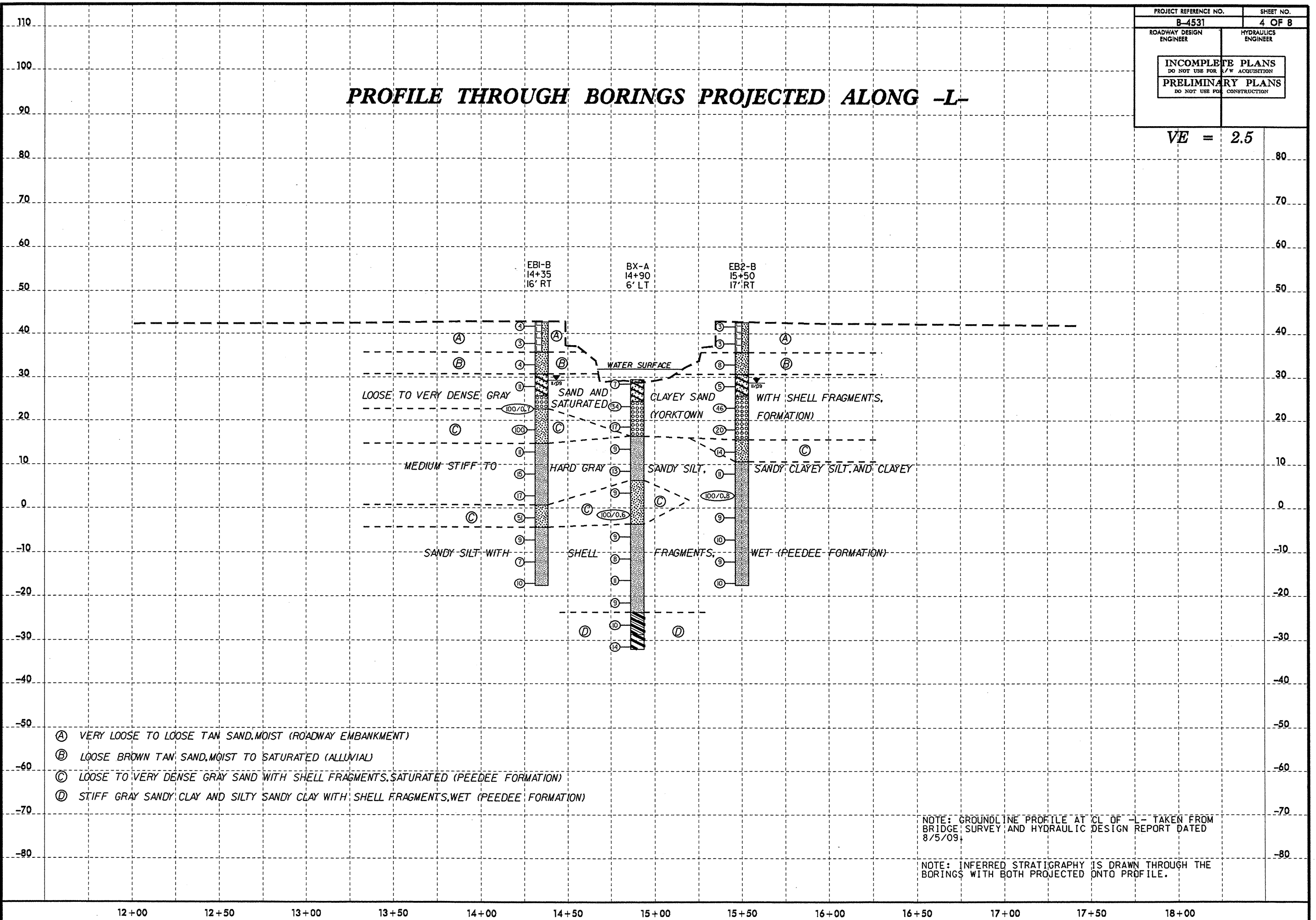


PROJECT REFERENCE NO.	SHEET
B-4531	3 OF 8
SKEW = 90°	



VE = 2.5

PROFILE THROUGH BORINGS PROJECTED ALONG -L-

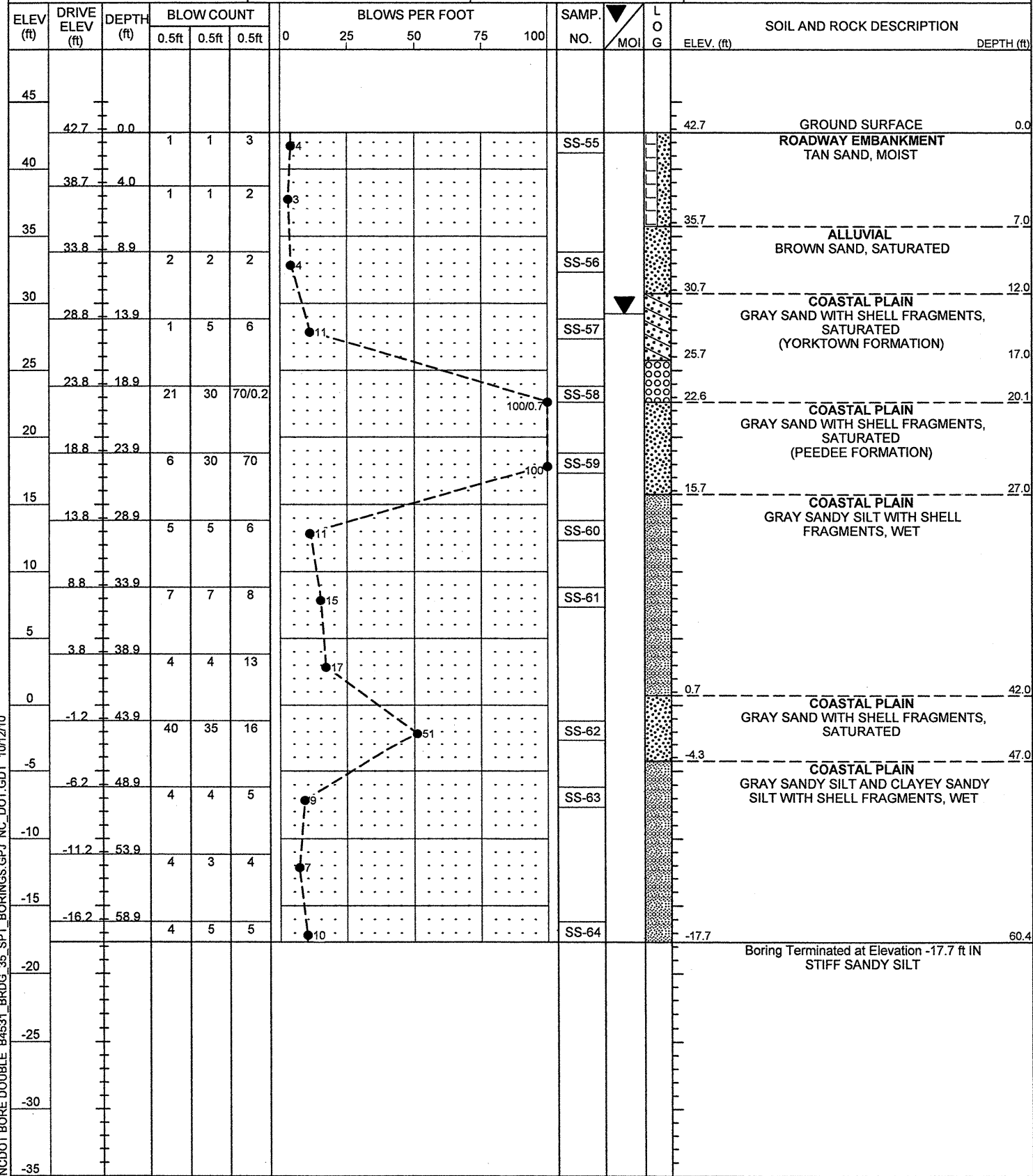


- Ⓐ VERY LOOSE TO LOOSE TAN SAND, MOIST (ROADWAY EMBANKMENT)
- Ⓑ LOOSE BROWN TAN SAND, MOIST TO SATURATED (ALLUVIAL)
- Ⓒ LOOSE TO VERY DENSE GRAY SAND WITH SHELL FRAGMENTS, SATURATED (PEEDEE FORMATION)
- Ⓓ STIFF GRAY SANDY CLAY AND SILTY SANDY CLAY WITH SHELL FRAGMENTS, WET (PEEDEE FORMATION)

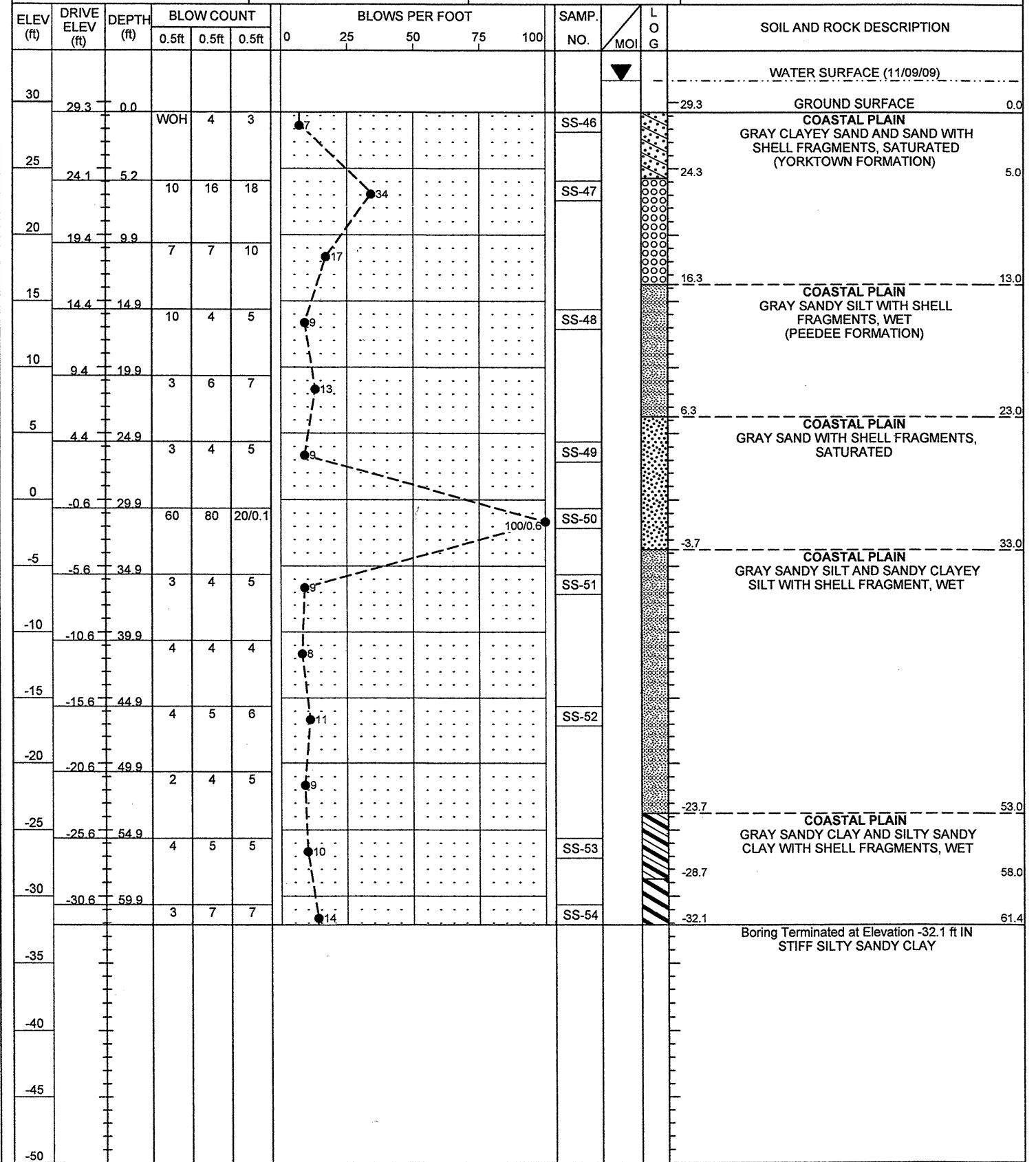
NOTE: GROUNDLINE PROFILE AT CL OF -L- TAKEN FROM BRIDGE SURVEY AND HYDRAULIC DESIGN REPORT DATED 8/5/09.

NOTE: INFERRED STRATIGRAPHY IS DRAWN THROUGH THE BORINGS WITH BOTH PROJECTED ONTO PROFILE.

PROJECT NO. 33751.1.1	ID. B-4531	COUNTY GREENE	GEOLOGIST Swartley, J. R.
SITE DESCRIPTION BRIDGE NO. 35 ON -L- (SR 1343) OVER LITTLE CONTENTNEA CREEK OVERFLOW			GROUND WTR (ft)
BORING NO. EB1-B	STATION 14+35	OFFSET 16 ft RT	ALIGNMENT -L-
COLLAR ELEV. 42.7 ft	TOTAL DEPTH 60.4 ft	NORTHING 648,106	EASTING 2,438,395
DRILL RIG/HAMMER EFF./DATE CME-45B		DRILL METHOD Mud Rotary	HAMMER TYPE Automatic
DRILLER Smith, R. E.	START DATE 11/10/09	COMP. DATE 11/10/09	SURFACE WATER DEPTH N/A



PROJECT NO. 33751.1.1	ID. B-4531	COUNTY GREENE	GEOLOGIST Swartley, J. R.
SITE DESCRIPTION BRIDGE NO. 35 ON -L- (SR 1343) OVER LITTLE CONTENTNEA CREEK OVERFLOW			GROUND WTR (ft)
BORING NO. BX-A	STATION 14+90	OFFSET 6 ft LT	ALIGNMENT -L-
COLLAR ELEV. 29.3 ft	TOTAL DEPTH 61.4 ft	NORTHING 648,162	EASTING 2,438,415
DRILL RIG/HAMMER EFF./DATE CME-45B		DRILL METHOD Mud Rotary	HAMMER TYPE Automatic
DRILLER Smith, R. E.	START DATE 11/09/09	COMP. DATE 11/09/09	SURFACE WATER DEPTH 2.4ft



NCDOT BORE DOUBLE B4531_BRDG_35_SPT_BORINGS.GPJ NC_DOT_GDT_10/12/10



NCDOT GEOTECHNICAL ENGINEERING UNIT

BORELOG REPORT

PROJECT NO. 33751.1.1	ID. B-4531	COUNTY GREENE	GEOLOGIST Swartley, J. R.
SITE DESCRIPTION BRIDGE NO. 35 ON -L- (SR 1343) OVER LITTLE CONTENTNEA CREEK OVERFLOW			GROUND WTR (ft)
BORING NO. EB2-B	STATION 15+50	OFFSET 17 ft RT	ALIGNMENT -L-
COLLAR ELEV. 42.6 ft	TOTAL DEPTH 60.3 ft	NORTHING 648,191	EASTING 2,438,473
DRILL RIG/HAMMER EFF./DATE CME-45B		DRILL METHOD Mud Rotary	HAMMER TYPE Automatic
DRILLER Smith, R. E.	START DATE 11/08/09	COMP. DATE 11/09/09	SURFACE WATER DEPTH N/A

ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	BLOW COUNT			BLOWS PER FOOT					SAMP. NO.	L O MOI G	SOIL AND ROCK DESCRIPTION	DEPTH (ft)	
			0.5ft	0.5ft	0.5ft	0	25	50	75	100					
45															
42.6	42.6	0.0												GROUND SURFACE	0.0
40			WOH	2	1									ROADWAY EMBANKMENT TAN SAND, MOIST	
38.6	38.6	4.0													
35			1	2	1										
33.8	33.8	8.8												ALLUVIAL TAN SAND, MOIST	7.0
30			3	3	5										
28.8	28.8	13.8												COASTAL PLAIN GRAY SAND WITH SHELL FRAGMENTS, SATURATED (YORKTOWN FORMATION)	12.0
25			2	2	3										
23.8	23.8	18.8													
20			12	18	28										
18.8	18.8	23.8													
15			7	8	12										
13.8	13.8	28.8												COASTAL PLAIN GRAY SAND WITH SHELL FRAGMENTS, SATURATED (PEEDEE FORMATION)	27.0
10			5	6	8										
8.8	8.8	33.8													
5			4	5	6									COASTAL PLAIN GRAY SANDY SILT AND SANDY CLAYEY SILT WITH SHELL FRAGMENTS, WET	32.0
3.8	3.8	38.8													
0			5	42	58/0.3										
-1.3	-1.3	43.8													
-5			6	4	5										
-6.3	-6.3	48.8													
-10			4	5	5										
-11.3	-11.3	53.8													
-15			4	4	5										
-16.3	-16.3	58.8													
-17.8			4	5	5										
														Boring Terminated at Elevation -17.8 ft IN STIFF SANDY SILT	60.3

NCDOT BORE DOUBLE B4531_BRDG_35_SPT_BORINGS.GPJ NC_DOT_GDT 10/12/10

B-4531

33751.1.1

BRIDGE NO. 35 ON SR 1343 OVER LITTLE CONTENTNEA CREEK OVERFLOW

EB1-B SOIL TEST RESULTS															
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C.SAND	F.SAND	SILT	CLAY	10	40	200		
SS-55	16 RT	14+35	1.0-1.5	A-2-4(0)	19	NP	26.3	62.7	3.9	7.1	98	91	14	-	-
SS-56	16 RT	14+35	8.9-10.4	-	-	-	-	-	-	-	-	-	-	-	1.4
SS-57	16 RT	14+35	13.9-15.4	A-2-6(0)	33	17	62.1	14.7	3.9	19.2	81	48	19	-	-
SS-58	16 RT	14+35	18.9-20.1	A-1-b(0)	21	NP	81.3	10.4	2.2	6.1	89	40	8	-	-
SS-59	16 RT	14+35	23.9-25.4	A-2-4(0)	30	NP	7.1	75.4	10.5	7.1	98	93	32	-	-
SS-60	16 RT	14+35	28.9-30.4	A-4(0)	30	4	4.4	66.1	11.3	18.2	99	96	45	-	-
SS-61	16 RT	14+35	33.9-35.4	A-4(0)	27	NP	1.8	80.2	7.9	10.1	97	96	37	-	-
SS-62	16 RT	14+35	43.9-45.4	A-2-4(0)	24	NP	11.6	71.7	6.6	10.1	98	91	27	-	-
SS-63	16 RT	14+35	48.9-50.4	A-4(0)	27	5	1.0	68.3	10.5	20.2	100	99	41	-	-
SS-64	16 RT	14+35	58.9-60.4	A-4(0)	24	NP	7.9	65.5	12.5	14.1	100	94	49	-	-

BX-A SOIL TEST RESULTS															
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C.SAND	F.SAND	SILT	CLAY	10	40	200		
SS-46	6 LT	14+90	1.0-1.5	A-2-6(1)	40	23	56.1	14.8	4.9	24.3	87	58	26	-	-
SS-47	6 LT	14+90	5.2-6.7	A-1-b(0)	22	NP	82.9	10.6	1.4	5.1	86	38	6	-	-
SS-48	6 LT	14+90	14.9-16.4	A-4(1)	29	2	3.0	32.4	56.5	8.1	99	97	72	-	-
SS-49	6 LT	14+90	24.9-26.4	A-2-4(0)	25	1	2.6	75.6	7.6	14.2	99	97	34	-	-
SS-50	6 LT	14+90	29.9-31.0	A-2-4(0)	27	NP	3.2	88.2	4.6	4.0	98	97	16	-	-
SS-51	6 LT	14+90	34.9-36.4	A-4(1)	28	5	0.8	66.3	10.6	22.2	100	99	52	-	-
SS-52	6 LT	14+90	44.9-46.4	A-4(1)	28	3	0.6	67.3	13.9	18.2	100	100	64	-	-
SS-53	6 LT	14+90	54.9-56.4	A-6(4)	33	12	2.6	59.0	14.1	24.3	100	98	54	-	-
SS-54	6 LT	14+90	59.9-61.4	A-7-6(18)	42	24	4.0	37.6	19.9	38.4	100	98	77	-	-

EB2-B SOIL TEST RESULTS															
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C.SAND	F.SAND	SILT	CLAY	10	40	200		
SS-37	17 RT	15+50	1.0-1.5	A-2-4(0)	17	NP	16.0	60.5	9.4	14.2	99	95	28	-	-
SS-38	17 RT	15+50	8.8-10.3	A-2-4(0)	16	NP	52.6	35.7	2.6	9.1	100	80	14	-	-
SS-39	17 RT	15+50	13.8-15.3	A-2-6(0)	35	18	64.1	14.3	2.4	19.2	84	49	19	-	-
SS-40	17 RT	15+50	18.8-20.3	A-1-b(0)	20	NP	77.2	14.0	2.7	6.1	88	48	9	-	-
SS-41	17 RT	15+50	28.8-30.3	A-2-4(0)	33	6	6.5	64.1	11.2	18.2	100	96	26	-	-
SS-42	17 RT	15+50	33.8-35.3	A-4(0)	25	1	0.8	78.1	9.0	12.1	100	100	40	-	-
SS-43	17 RT	15+50	43.8-45.3	A-4(0)	25	3	5.1	67.3	7.4	20.2	100	97	44	-	-
SS-44	17 RT	15+50	48.8-50.3	A-4(0)	26	3	1.0	71.8	9.0	18.2	100	99	39	-	-
SS-45	17 RT	15+50	58.8-60.3	A-4(2)	29	4	1.2	63.7	16.9	18.2	100	99	69	-	-



**FIELD
SCOUR REPORT**

WBS: 33751.1.1 TIP: B-4531 COUNTY: GREENE

DESCRIPTION(1): BRIDGE NO. 35 ON SR 1343 OVER LITTLE CONTENTNEA CREEK OVERFLOW

EXISTING BRIDGE

Information from: Field Inspection Microfilm _____ (reel _____ pos: _____)
Other (explain) _____

Bridge No.: 35 Length: 86' Total Bents: 6 Bents in Channel: 4 Bents in Floodplain: 2
Foundation Type: TIMBER PILES

EVIDENCE OF SCOUR(2)

Abutments or End Bent Slopes: NONE NOTED

Interior Bents: NONE NOTED

Channel Bed: 2 TO 3 FEET DEEP CHANNEL SCOUR UNDERNEATH ENTIRE LENGTH OF BRIDGE

Channel Bank: NONE NOTED

EXISTING SCOUR PROTECTION

Type(3): WOODEN WINGWALLS

Extent(4): EXTEND 10' OUTSIDE EDGE OF BRIDGE

Effectiveness(5): EFFECTIVE

Obstructions(6): NONE

INSTRUCTIONS

- 1 Describe the specific site's location, including route number and body of water crossed.
- 2 Note scour evidence at existing end bents or abutments (e.g. undermining, sloughing, degradations).
- 3 Note existing scour protection (e.g. rip rap).
- 4 Describe extent of existing scour protection.
- 5 Describe whether or not the scour protection appears to be working.
- 6 Note obstructions such as dams, fallen trees, debris at bents, etc.
- 7 Describe the channel bed material based on observation and/or samples. Include any lab results with report.
- 8 Describe the channel bank material based on observation and/or samples. Include any lab results with report.
- 9 Describe the material covering the banks (e.g. grass, trees, rip rap, none).
- 10 Determine the approximate floodplain width from field observation or a topographic map.
- 11 Describe the material covering the floodplain (e.g. grass, trees, crops).
- 12 Use professional judgement to specify if the stream is degrading, aggrading, or static.
- 13 Describe potential and direction of the stream to migrate laterally during the bridge's life (approx. 100 years).
- 14 Give the design scour elevation (DSE) expected over the life of the bridge (approx. 100 years). This elevation can be given as a range across the site, or for each bent. Discuss the relationship between the Hydraulics Unit theoretical scour and the DSE. If the DSE is dependent on scour counter measures, explain (e.g. rip rap armoring on slopes). The DSE is based on the erodability of materials, giving consideration to the influence of joints, foliation, bedding characteristics, % core recovery, % RQD, differential weathering, shear strength, observations at existing structures, other tests deemed appropriate, and overall geologic conditions at the site.

DESIGN INFORMATION

Channel Bed Material(7): CLAYEY SAND AND SAND

Channel Bank Material(8): SAND

Channel Bank Cover(9): TREES, SHRUBS

Floodplain Width(10): 1000'

Floodplain Cover(11): TREES, SHRUBS

Stream is(12): Aggrading _____ Degrading _____ Static

Channel Migration Tendency(13): NONE

Observations and Other Comments: _____

DESIGN SCOUR ELEVATIONS(14)

Feet Meters _____

BENTS

B1	B2												
22.8	22.8												

Comparison of DSE to Hydraulics Unit theoretical scour:
THE GEOTECHNICAL ENGINEERING UNIT AGREES WITH THE HYDRAULIC UNIT'S DSE NOTED ABOVE

SOIL ANALYSIS RESULTS FROM CHANNEL BED AND BANK MATERIAL

Bed or Bank													
Sample No.													
Retained #4													
Passed #10													
Passed #40													
Passed #200													
Coarse Sand													
Fine Sand													
Silt													
Clay													
LL													
PI													
AASHTO													
Station													
Offset													
Depth													

See Sheet 7,
"Soil Test Results",
for samples:
SS-38 (CHANNEL BANK)
SS-46, SS-38 (CHANNEL BED)

Reported by: *Dean Argenbright*
Dean Argenbright

Date: 11/8/2009