

NOTE: SEE SHEET 2A FOR PLAN SHEET LAYOUT AT TIME OF INVESTIGATION

STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
DIVISION OF HIGHWAYS
GEOTECHNICAL ENGINEERING UNIT

STATE	STATE PROJECT REFERENCE NO.	SHEET NO.	TOTAL SHEETS
N.C.	B-4463	1	18
STATE PROJ. NO.	F.A. PROJ. NO.	DESCRIPTION	
33713.1.2	STP-0032(8)	P.E.	
33713.2.1	STP-0032(8)	R.W. & UTILITY	
33713.3.1	BRSTP-0032(8)	CONST.	

CONTENTS

LINE	STATION	PLAN	PROFILE
- L -	12+52 TO 29+69	4-5	6-7
- L-DETOUR -	12+52 TO 29+67	4-5	8-9

CROSS SECTIONS	STATION	SHEET
- L -	18+50 TO 23+50	10-18
- L-DETOUR -	18+50 TO 23+50	10-18

ROADWAY
SUBSURFACE INVESTIGATION

PROJ. REFERENCE NO. 33713.1.2 (B-4463) F.A. PROJ. STP-0032(8)
COUNTY CHOWAN
PROJECT DESCRIPTION BRIDGE NO. 12 ON -L- (NC 32) OVER QUEEN ANNE CREEK

INVENTORY

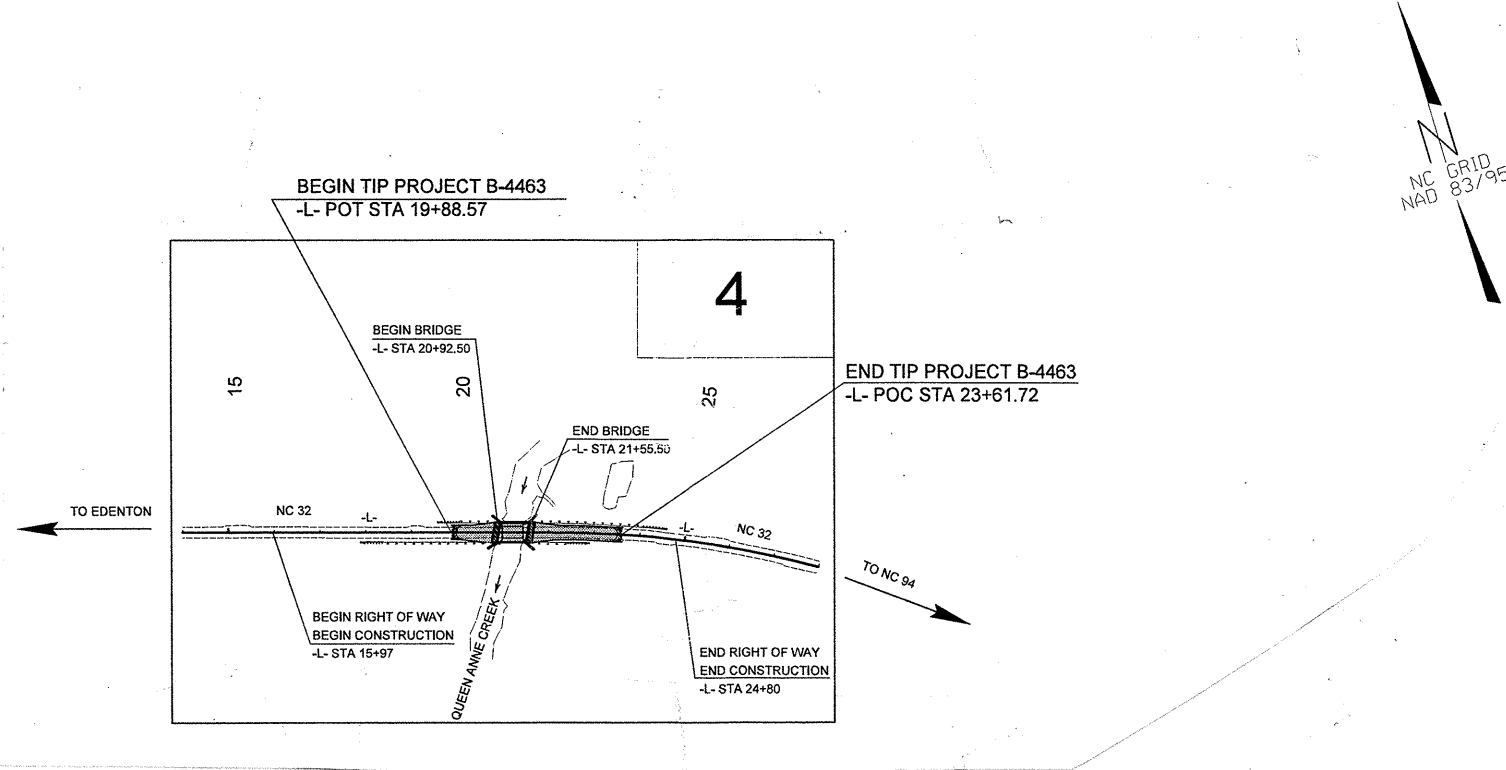
CAUTION NOTICE
THE SUBSURFACE INFORMATION AND THE SUBSURFACE INVESTIGATION ON WHICH IT IS BASED WERE MADE FOR THE PURPOSE OF STUDY, PLANNING, AND DESIGN, AND NOT FOR CONSTRUCTION OR PAY PURPOSES. THE VARIOUS FIELD BORING LOGS, ROCK CORES, AND SOIL TEST DATA AVAILABLE MAY BE REVIEWED OR INSPECTED IN RALEIGH BY CONTACTING THE N.C. DEPARTMENT OF TRANSPORTATION, GEOTECHNICAL ENGINEERING UNIT AT (919) 250-4088. NEITHER THE SUBSURFACE PLANS AND REPORTS, NOR THE FIELD BORING LOGS, ROCK CORES, OR SOIL TEST DATA ARE PART OF THE CONTRACT.

GENERAL SOIL AND ROCK STRATA DESCRIPTIONS AND INDICATED BOUNDARIES ARE BASED ON A GEOTECHNICAL INTERPRETATION OF ALL AVAILABLE SUBSURFACE DATA AND MAY NOT NECESSARILY REFLECT THE ACTUAL SUBSURFACE CONDITIONS BETWEEN BORINGS OR BETWEEN SAMPLED STRATA WITHIN THE BOREHOLE. THE LABORATORY SAMPLE DATA AND THE IN SITU (IN-PLACE) TEST DATA CAN BE RELIED ON ONLY TO THE DEGREE OF RELIABILITY INHERENT IN THE STANDARD TEST METHOD. THE OBSERVED WATER LEVELS OR SOIL MOISTURE CONDITIONS INDICATED IN THE SUBSURFACE INVESTIGATIONS ARE AS RECORDED AT THE TIME OF THE INVESTIGATION. THESE WATER LEVELS OR SOIL MOISTURE CONDITIONS MAY VARY CONSIDERABLY WITH TIME ACCORDING TO CLIMATIC CONDITIONS INCLUDING TEMPERATURES, PRECIPITATION, AND WIND, AS WELL AS OTHER NON-CLIMATIC FACTORS.

THE BIDDER OR CONTRACTOR IS CAUTIONED THAT DETAILS SHOWN ON THE SUBSURFACE PLANS ARE PRELIMINARY ONLY AND IN MANY CASES THE FINAL DESIGN DETAILS ARE DIFFERENT. FOR BIDDING AND CONSTRUCTION PURPOSES, REFER TO THE CONSTRUCTION PLANS AND DOCUMENTS FOR FINAL DESIGN INFORMATION ON THIS PROJECT. THE DEPARTMENT DOES NOT WARRANT OR GUARANTEE THE SUFFICIENCY OR ACCURACY OF THE INVESTIGATION MADE, NOR THE INTERPRETATIONS MADE, OR OPINION OF THE DEPARTMENT AS TO THE TYPE OF MATERIALS AND CONDITIONS TO BE ENCOUNTERED. THE BIDDER OR CONTRACTOR IS CAUTIONED TO MAKE SUCH INDEPENDENT SUBSURFACE INVESTIGATIONS AS HE DEEMS NECESSARY TO SATISFY HIMSELF AS TO CONDITIONS TO BE ENCOUNTERED ON THIS PROJECT. THE CONTRACTOR SHALL HAVE NO CLAIM FOR ADDITIONAL COMPENSATION OR FOR AN EXTENSION OF TIME FOR ANY REASON RESULTING FROM THE ACTUAL CONDITIONS ENCOUNTERED AT THE SITE DIFFERING FROM THOSE INDICATED IN THE SUBSURFACE INFORMATION.

ID: B-4463

CONTRACT: C202908



PERSONNEL

CMW

SCD

JRS

RES

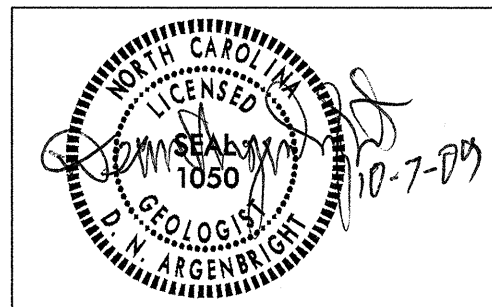
JME

INVESTIGATED BY C.M. WRIKE

CHECKED BY D.N. ARGENBRIGHT

SUBMITTED BY D.N. ARGENBRIGHT

DATE OCTOBER 2009



DRAWN BY: C.R. SUMNER, C.M. WRIKE

NOTE - THE INFORMATION CONTAINED HEREIN IS NOT IMPLIED OR GUARANTEED BY THE N.C. DEPARTMENT OF TRANSPORTATION AS BEING ACCURATE NOR IS IT CONSIDERED TO BE PART OF THE PLANS, SPECIFICATIONS, OR CONTRACT FOR THE PROJECT.

NOTE - BY HAVING REQUESTED THIS INFORMATION THE CONTRACTOR SPECIFICALLY WAIVES ANY CLAIMS FOR INCREASED COMPENSATION OR EXTENSION OF TIME BASED ON DIFFERENCES BETWEEN THE CONDITIONS INDICATED HEREIN AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

NORTH CAROLINA DEPARTMENT OF TRANSPORTATION
DIVISION OF HIGHWAYS
GEOTECHNICAL ENGINEERING UNIT

PROJECT REFERENCE NO. B-4463
SHEET NO. 2 OF 18

SUBSURFACE INVESTIGATION

SOIL AND ROCK LEGEND, TERMS, SYMBOLS, AND ABBREVIATIONS

SOIL DESCRIPTION				GRADATION				ROCK DESCRIPTION				TERMS AND DEFINITIONS			
SOIL IS CONSIDERED TO BE THE UNCONSOLIDATED, SEMI-CONSOLIDATED, OR WEATHERED EARTH MATERIALS THAT CAN BE PENETRATED WITH A CONTINUOUS FLIGHT POWER AUGER, AND YIELD LESS THAN 100 BLOWS PER FOOT ACCORDING TO THE STANDARD PENETRATION TEST (AASHTO T206, ASTM D-1586). SOIL CLASSIFICATION IS BASED ON THE AASHTO SYSTEM. BASIC DESCRIPTIONS GENERALLY SHALL INCLUDE: CONSISTENCY, COLOR, TEXTURE, MOISTURE, AASHTO CLASSIFICATION, AND OTHER PERTINENT FACTORS SUCH AS MINERALOGICAL COMPOSITION, ANGULARITY, STRUCTURE, PLASTICITY, ETC. EXAMPLES: VERY STIFF, GRAY, SILTY CLAY, MOST WITH INTERBEDDED FINE SAND LAYERS, HIGHLY PLASTIC, A-7-6				WELL GRADED - INDICATES A GOOD REPRESENTATION OF PARTICLE SIZES FROM FINE TO COARSE. UNIFORM - INDICATES THAT SOIL PARTICLES ARE ALL APPROXIMATELY THE SAME SIZE. (ALSO POORLY GRADED) GAP-GRADED - INDICATES A MIXTURE OF UNIFORM PARTICLES OF TWO OR MORE SIZES.				HARD ROCK IS NON-COASTAL PLAIN MATERIAL THAT IF TESTED, WOULD YIELD SPT REFUSAL. AN INFERRED ROCK LINE INDICATES THE LEVEL AT WHICH NON-COASTAL PLAIN MATERIAL WOULD YIELD SPT REFUSAL. SPT REFUSAL IS PENETRATION BY A SPLIT SPOON SAMPLER EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS. IN NON-COASTAL PLAIN MATERIAL, THE TRANSITION BETWEEN SOIL AND ROCK IS OFTEN REPRESENTED BY A ZONE OF WEATHERED ROCK. ROCK MATERIALS ARE TYPICALLY DIVIDED AS FOLLOWS:				ALLUVIUM (ALLUV.) - SOILS THAT HAVE BEEN TRANSPORTED BY WATER. AQUIFER - A WATER BEARING FORMATION OR STRATA. ARENACEOUS - APPLIED TO ROCKS THAT HAVE BEEN DERIVED FROM SAND OR THAT CONTAIN SAND. ARGILLACEOUS - APPLIED TO ALL ROCKS OR SUBSTANCES COMPOSED OF CLAY MINERALS, OR HAVING A NOTABLE PROPORTION OF CLAY IN THEIR COMPOSITION, AS SHALE, SLATE, ETC. ARTESIAN - GROUND WATER THAT IS UNDER SUFFICIENT PRESSURE TO RISE ABOVE THE LEVEL AT WHICH IT IS ENCOUNTERED, BUT WHICH DOES NOT NECESSARILY RISE TO OR ABOVE THE GROUND SURFACE. CALCAREOUS (CALC.) - SOILS THAT CONTAIN APPRECIABLE AMOUNTS OF CALCIUM CARBONATE. COLLUVIUM - ROCK FRAGMENTS MIXED WITH SOIL DEPOSITED BY GRAVITY ON SLOPE OR AT BOTTOM OF SLOPE. CORE RECOVERY (REC.) - TOTAL LENGTH OF ALL MATERIAL RECOVERED IN THE CORE BARREL DIVIDED BY TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE. DIKE - A TABULAR BODY OF IGNEOUS ROCK THAT CUTS ACROSS THE STRUCTURE OF ADJACENT ROCKS OR CUTS MASSIVE ROCK. DIP - THE ANGLE AT WHICH A STRATUM OR ANY PLANAR FEATURE IS INCLINED FROM THE HORIZONTAL. DIP DIRECTION (DIP AZIMUTH) - THE DIRECTION OR BEARING OF THE HORIZONTAL TRACE OF THE LINE OF DIP, MEASURED CLOCKWISE FROM NORTH. FAULT - A FRACTURE OR FRACTURE ZONE ALONG WHICH THERE HAS BEEN DISPLACEMENT OF THE SIDES RELATIVE TO ONE ANOTHER PARALLEL TO THE FRACTURE. FISSILE - A PROPERTY OF SPLITTING ALONG CLOSELY SPACED PARALLEL PLANES. FLOAT - ROCK FRAGMENTS ON SURFACE NEAR THEIR ORIGINAL POSITION AND DISLODGED FROM PARENT MATERIAL. FLOOD PLAIN (FP) - LAND BORDERING A STREAM, BUILT OF SEDIMENTS DEPOSITED BY THE STREAM. FORMATION (FM) - A MAPPABLE GEOLOGIC UNIT THAT CAN BE RECOGNIZED AND TRACED IN THE FIELD. JOINT - FRACTURE IN ROCK ALONG WHICH NO APPRECIABLE MOVEMENT HAS OCCURRED. LEDGE - A SHELF-LIKE RIDGE OR PROJECTION OF ROCK WHOSE THICKNESS IS SMALL COMPARED TO ITS LATERAL EXTENT. LENS - A BODY OF SOIL OR ROCK THAT THINS OUT IN ONE OR MORE DIRECTIONS. MOTTLED (MOT.) - IRREGULARLY MARKED WITH SPOTS OF DIFFERENT COLORS. MOTTLING IN SOILS USUALLY INDICATES POOR AERATION AND LACK OF GOOD DRAINAGE. PERCHED WATER - WATER MAINTAINED ABOVE THE NORMAL GROUND WATER LEVEL BY THE PRESENCE OF AN INTERVENING IMPERVIOUS STRATUM. RESIDUAL (RES.) SOIL - SOIL FORMED IN PLACE BY THE WEATHERING OF ROCK. ROCK QUALITY DESIGNATION (ROQD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE. SAPROLITE (SAP.) - RESIDUAL SOIL THAT RETAINS THE RELIC STRUCTURE OR FABRIC OF THE PARENT ROCK. SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, THAT HAS BEEN EMPLACED PARALLEL TO THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS. SLICKENSIDE - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT OR SLIP PLANE. STANDARD PENETRATION TEST (PENETRATION RESISTANCE) (SPT) - NUMBER OF BLOWS (N OR BPF) OF A 140 LB. HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF 1 FOOT INTO SOIL WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT REFUSAL IS PENETRATION EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS. STRATA CORE RECOVERY (SREC) - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY TOTAL LENGTH OF STRATUM AND EXPRESSED AS A PERCENTAGE. STRATA ROCK QUALITY DESIGNATION (SROQD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS WITHIN A STRATUM EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF STRATA AND EXPRESSED AS A PERCENTAGE. TOPSOIL (TS.) - SURFACE SOILS USUALLY CONTAINING ORGANIC MATTER.			
SOIL LEGEND AND AASHTO CLASSIFICATION				MINERALOGICAL COMPOSITION				WEATHERING				WEATHERING			
GENERAL CLASS. GRANULAR MATERIALS (<= 35% PASSING #200) SILT-CLAY MATERIALS (> 35% PASSING #200) ORGANIC MATERIALS GROUP CLASS. A-1, A-3, A-2, A-4, A-5, A-6, A-7, A-1, A-2, A-3, A-4, A-5, A-6, A-7 SYMBOL [Diagrams showing soil symbols for various groups] % PASSING: 10, 40, 200 (Diagrams showing sieve analysis curves) LIQUID LIMIT PLASTIC INDEX (Diagrams showing LL and PI charts) GROUP INDEX (Diagrams showing group index calculations) USUAL TYPES OF MAJOR MATERIALS (Diagrams showing soil types) GENERATING AS A SUBGRADE (Diagrams showing subgrade ratings)				MINERAL NAMES SUCH AS QUARTZ, FELDSPAR, MICA, TALC, KAOLIN, ETC. ARE USED IN DESCRIPTIONS WHENEVER THEY ARE CONSIDERED OF SIGNIFICANCE. COMPRESSIBILITY: SLIGHTLY COMPRESSIBLE, MODERATELY COMPRESSIBLE, HIGHLY COMPRESSIBLE LIQUID LIMIT LESS THAN 31, LIQUID LIMIT EQUAL TO 31-50, LIQUID LIMIT GREATER THAN 50 PERCENTAGE OF MATERIAL: ORGANIC MATERIAL, GRANULAR SOILS, SILT-CLAY SOILS, OTHER MATERIAL TRACE OF ORGANIC MATTER, LITTLE ORGANIC MATTER, MODERATELY ORGANIC, HIGHLY ORGANIC GROUND WATER: WATER LEVEL IN BORE HOLE IMMEDIATELY AFTER DRILLING, STATIC WATER LEVEL AFTER 24 HOURS, PERCHED WATER, SATURATED ZONE, OR WATER BEARING STRATA, SPRING OR SEEP				WEATHERED ROCK (WR) [Diagram] CRYSTALLINE ROCK (CR) [Diagram] NON-CRYSTALLINE ROCK (NCR) [Diagram] COASTAL PLAIN SEDIMENTARY ROCK (CP) [Diagram]				FRESH: ROCK FRESH, CRYSTALS BRIGHT, FEW JOINTS MAY SHOW SLIGHT STAINING, ROCK RINGS UNDER HAMMER IF CRYSTALLINE. VERY SLIGHT (V SL.): ROCK GENERALLY FRESH, JOINTS STAINED, SOME JOINTS MAY SHOW THIN CLAY COATINGS IF OPEN, CRYSTALS ON A BROKEN SPECIMEN FACE SHINE BRIGHTLY, ROCK RINGS UNDER HAMMER BLOWS IF OF A CRYSTALLINE NATURE. SLIGHT (SL.): ROCK GENERALLY FRESH, JOINTS STAINED AND DISCOLORATION EXTENDS INTO ROCK UP TO 1 INCH, OPEN JOINTS MAY CONTAIN CLAY, IN GRANITOID ROCKS SOME OCCASIONAL FELDSPAR CRYSTALS ARE DULL AND DISCOLORED, CRYSTALLINE ROCKS RING UNDER HAMMER BLOWS. MODERATE (MOD.): SIGNIFICANT PORTIONS OF ROCK SHOW DISCOLORATION AND WEATHERING EFFECTS, IN GRANITOID ROCKS, MOST FELDSPARS ARE DULL AND DISCOLORED, SOME SHOW CLAY, ROCK HAS DULL SOUND UNDER HAMMER BLOWS AND SHOWS SIGNIFICANT LOSS OF STRENGTH AS COMPARED WITH FRESH ROCK. MODERATELY SEVERE (MOD. SEV.): ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED, IN GRANITOID ROCKS, ALL FELDSPARS DULL AND DISCOLORED AND A MAJORITY SHOW KAOLINIZATION, ROCK SHOWS SEVERE LOSS OF STRENGTH AND CAN BE EXCAVATED WITH A GEOLOGIST'S PICK, ROCK GIVES 'CLUNK' SOUND WHEN STRUCK, IF TESTED, WOULD YIELD SPT REFUSAL. SEVERE (SEV.): ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED, ROCK FABRIC CLEAR AND EVIDENT BUT REDUCED IN STRENGTH TO STRONG SOIL, IN GRANITOID ROCKS ALL FELDSPARS ARE KAOLINIZED TO SOME EXTENT, SOME FRAGMENTS OF STRONG ROCK USUALLY REMAIN, IF TESTED, YIELDS SPT N VALUES > 100 BPF. VERY SEVERE (V SEV.): ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED, ROCK FABRIC ELEMENTS ARE DISCERNIBLE BUT THE MASS IS EFFECTIVELY REDUCED TO SOIL STATUS, WITH ONLY FRAGMENTS OF STRONG ROCK REMAINING, SAPROLITE IS AN EXAMPLE OF ROCK WEATHERED TO A DEGREE SUCH THAT ONLY MINOR VESTIGES OF THE ORIGINAL ROCK FABRIC REMAIN, IF TESTED, YIELDS SPT N VALUES < 100 BPF. COMPLETE: ROCK REDUCED TO SOIL, ROCK FABRIC NOT DISCERNIBLE, OR DISCERNIBLE ONLY IN SMALL AND SCATTERED CONCENTRATIONS, QUARTZ MAY BE PRESENT AS DIKES OR STRINGERS, SAPROLITE IS ALSO AN EXAMPLE.			
CONSISTENCY OR DENSITY				MISCELLANEOUS SYMBOLS				ROCK HARDNESS				ROCK HARDNESS			
PRIMARY SOIL TYPE, COMPACTNESS OR CONSISTENCY, RANGE OF STANDARD PENETRATION RESISTANCE (N-VALUE), RANGE OF UNCONFINED COMPRESSIVE STRENGTH (TONS/FT ²)				ROADWAY EMBANKMENT (RE) WITH SOIL DESCRIPTION, SOIL SYMBOL, ARTIFICIAL FILL (AF) OTHER THAN ROADWAY EMBANKMENT, INFERRED SOIL BOUNDARY, INFERRED ROCK LINE, ALLUVIAL SOIL BOUNDARY, DIP & DIP DIRECTION OF ROCK STRUCTURES, SOUNDING ROD, SPT TEST BORING, AUGER BORING, CORE BORING, MONITORING WELL, PIEZOMETER INSTALLATION, SLOPE INDICATOR INSTALLATION, SPT N-VALUE, SPT REFUSAL, SAMPLE DESIGNATIONS, BULK SAMPLE, SPLIT SPOON SAMPLE, SHELBY TUBE SAMPLE, ROCK SAMPLE, RECOMPACTED TRIAXIAL SAMPLE, CALIFORNIA BEARING RATIO SAMPLE				VERY HARD: CANNOT BE SCRATCHED BY KNIFE OR SHARP PICK, BREAKING OF HAND SPECIMENS REQUIRES SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK. HARD: CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY, HARD HAMMER BLOWS REQUIRED TO DETACH HAND SPECIMEN. MODERATELY HARD: CAN BE SCRATCHED BY KNIFE OR PICK, GOUGES OR GROOVES TO 0.25 INCHES DEEP CAN BE EXCAVATED BY HARD BLOW OF A GEOLOGIST'S PICK, HAND SPECIMENS CAN BE DETACHED BY MODERATE BLOWS. MEDIUM HARD: CAN BE GROOVED OR GOUGED 0.05 INCHES DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT, CAN BE EXCAVATED IN SMALL CHIPS TO PIECES 1 INCH MAXIMUM SIZE BY HARD BLOWS OF THE POINT OF A GEOLOGIST'S PICK. SOFT: CAN BE GROOVED OR GOUGED READILY BY KNIFE OR PICK, CAN BE EXCAVATED IN FRAGMENTS FROM CHIPS TO SEVERAL INCHES IN SIZE BY MODERATE BLOWS OF A PICK POINT, SMALL, THIN PIECES CAN BE BROKEN BY FINGER PRESSURE. VERY SOFT: CAN BE CARVED WITH KNIFE, CAN BE EXCAVATED READILY WITH POINT OF PICK, PIECES 1 INCH OR MORE IN THICKNESS CAN BE BROKEN BY FINGER PRESSURE, CAN BE SCRATCHED READILY BY FINGERNAIL.							
TEXTURE OR GRAIN SIZE				ABBREVIATIONS				EQUIPMENT USED ON SUBJECT PROJECT				EQUIPMENT USED ON SUBJECT PROJECT			
U.S. STD. SIEVE SIZE OPENING (MM), BOULDER (BLD.), COBBLE (COB.), GRAVEL (GR.), COARSE SAND (CSE, SD.), FINE SAND (F SD.), SILT (SL.), CLAY (CL.)				AR - AUGER REFUSAL, BT - BORING TERMINATED, CL - CLAY, CPT - CONE PENETRATION TEST, CSE - COARSE, DMT - DILATOMETER TEST, DPT - DYNAMIC PENETRATION TEST, F - FINE, FOSS. - FOSSILIFEROUS, FRAC. - FRACTURED, FRACTURES, FRAGS. - FRAGMENTS, HI. - HIGHLY, MED. - MEDIUM, MICA - MICAEOUS, MOD. - MODERATELY, NP - NON PLASTIC, ORG. - ORGANIC, PMT - PRESSUREMETER TEST, SAP. - SAPROLITIC, SD. - SAND, SANDY, SL. - SILT, SILTY, SLL - SLIGHTLY, TCR - TRICONE REFUSAL, W - MOISTURE CONTENT, V - VERY, VST - VANE SHEAR TEST, WEA. - WEATHERED, Wt - UNIT WEIGHT, Wt - DRY UNIT WEIGHT				DRILL UNITS: MOBILE B-, BK-51, CME-45C, CME-750, PORTABLE HOIST, CME-45B ADVANCING TOOLS: CLAY BITS, 6" CONTINUOUS FLIGHT AUGER, 8" HOLLOW AUGERS, HARD FACED FINGER BITS, TUNG-CARBIDE INSERTS, CASING w/ ADVANCER, TRICONE 2 1/8" STEEL TEETH, TRICONE TUNG-CARB., CORE BIT HAMMER TYPE: AUTOMATIC, MANUAL CORE SIZE: B, N, H HAND TOOLS: POST HOLE DIGGER, HAND AUGER, SOUNDING ROD, VANE SHEAR TEST				FRACTURE SPACING: TERM, SPACING, BEDDING: TERM, THICKNESS			
SOIL MOISTURE - CORRELATION OF TERMS				INDURATION				INDURATION				INDURATION			
SOIL MOISTURE SCALE (ATTERBERG LIMITS), FIELD MOISTURE DESCRIPTION, GUIDE FOR FIELD MOISTURE DESCRIPTION, LIQUID LIMIT, PLASTIC LIMIT, OPTIMUM MOISTURE SHRINKAGE LIMIT				FOR SEDIMENTARY ROCKS, INDURATION IS THE HARDENING OF THE MATERIAL BY CEMENTING, HEAT, PRESSURE, ETC. FRIABLE: RUBBING WITH FINGER FREES NUMEROUS GRAINS; GENTLE BLOW BY HAMMER DISINTEGRATES SAMPLE. MODERATELY INDURATED: GRAINS CAN BE SEPARATED FROM SAMPLE WITH STEEL PROBE; BREAKS EASILY WHEN HIT WITH HAMMER. INDURATED: GRAINS ARE DIFFICULT TO SEPARATE WITH STEEL PROBE; DIFFICULT TO BREAK WITH HAMMER. EXTREMELY INDURATED: SHARP HAMMER BLOWS REQUIRED TO BREAK SAMPLE; SAMPLE BREAKS ACROSS GRAINS.				APPROXIMATE LIMITS OF SURFICIAL ORGANIC DEPOSITS [Diagram]							
PLASTICITY				COLOR				COLOR				COLOR			
NONPLASTIC, LOW PLASTICITY, MED. PLASTICITY, HIGH PLASTICITY, PLASTICITY INDEX (PI), DRY STRENGTH, VERY LOW, SLIGHT, MEDIUM, HIGH				DESCRIPTIONS MAY INCLUDE COLOR OR COLOR COMBINATIONS (TAN, RED, YELLOW-BROWN, BLUE-GRAY). MODIFIERS SUCH AS LIGHT, DARK, STREAKED, ETC. ARE USED TO DESCRIBE APPEARANCE.				DESCRIPTIONS MAY INCLUDE COLOR OR COLOR COMBINATIONS (TAN, RED, YELLOW-BROWN, BLUE-GRAY). MODIFIERS SUCH AS LIGHT, DARK, STREAKED, ETC. ARE USED TO DESCRIBE APPEARANCE.				DESCRIPTIONS MAY INCLUDE COLOR OR COLOR COMBINATIONS (TAN, RED, YELLOW-BROWN, BLUE-GRAY). MODIFIERS SUCH AS LIGHT, DARK, STREAKED, ETC. ARE USED TO DESCRIBE APPEARANCE.			

See Sheet 1-A For Index of Sheets

STATE OF NORTH CAROLINA
DIVISION OF HIGHWAYS

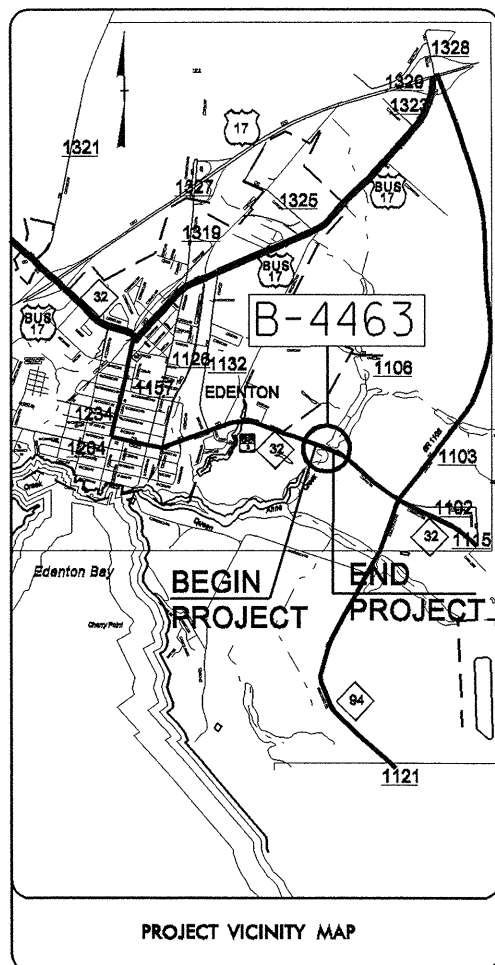
CHOWAN COUNTY

LOCATION: BRIDGE NO. 12 ON NC 32.
OVER QUEEN ANNE CREEK

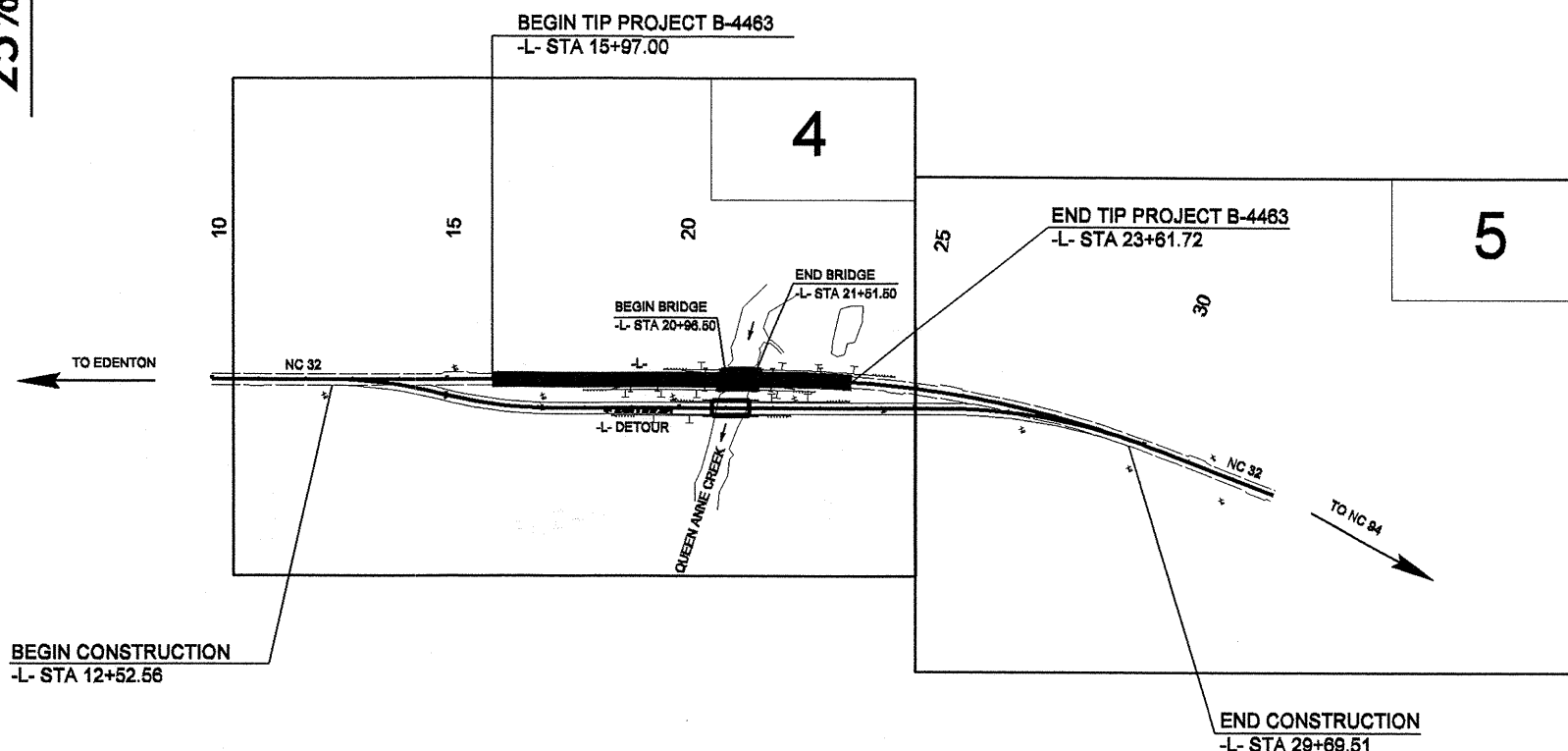
TYPE OF WORK: GRADING, PAVING, DRAINAGE, AND STRUCTURE

STATE	STATE PROJECT REFERENCE NO.	SHEET NO.	TOTAL SHEETS
N.C.	B-4463	2A	18
STATE PROJ. NO.	F.A. PROJ. NO.	DESCRIPTION	
33713.1.2	STP-0032(8)	P.E.	

TIP PROJECT: B-4463



25% REVIEW PLANS



-THIS PROJECT IS NOT WITHIN ANY MUNICIPAL BOUNDARIES

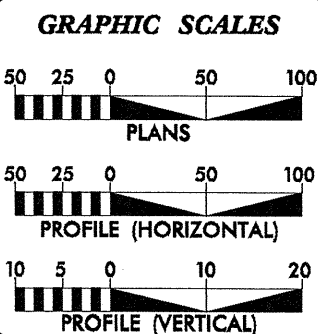
-CLEARING ON THIS PROJECT SHALL BE TO THE LIMITS ESTABLISHED BY METHOD----

PRELIMINARY PLANS
DO NOT USE FOR CONSTRUCTION

INCOMPLETE PLANS
DO NOT USE FOR R/W ACQUISITION

NCDOT Contact: Cathy S. Houser, PE
Roadway Design-Engineering Coordination

CONTRACT:



DESIGN DATA

ADT 2011	= 6280
ADT 2030	= 9200
DHV	= 10 %
D	= 60 %
T	= 5% (TTST 2%, DUAL 3%)
V	= 60 MPH
FUNC CLASS	= MAJOR COLLECTOR

PROJECT LENGTH

LENGTH ROADWAY TIP PROJECT B-4463	= 0.135 MILE
LENGTH STRUCTURE TIP PROJECT B-4463	= 0.010 MILE
TOTAL LENGTH TIP PROJECT B-4463	= 0.145 MILE

Prepared In the Office of
DYER, RIDDLE, MILLS & PRECOURT, INC. (DRMP)
7506 EAST INDEPENDENCE BLVD., SUITE 105
CHARLOTTE, NORTH CAROLINA 28227
(704) 332-2289

2006 STANDARD SPECIFICATIONS

RIGHT OF WAY DATE:
JANUARY 15, 2010

LETTING DATE:
JANUARY 18, 2011

Ronald C. Smith, PE
PROJECT ENGINEER

A. Matthew Thigpen, PE
PROJECT DESIGN ENGINEER

HYDRAULICS ENGINEER

SIGNATURE: _____

ROADWAY DESIGN ENGINEER

SIGNATURE: _____

P.E.

**DIVISION OF HIGHWAYS
STATE OF NORTH CAROLINA**

STATE HIGHWAY DESIGN ENGINEER

P.E.



STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION

BEVERLY EAVES PERDUE
GOVERNOR

EUGENE A. CONTI, JR.
SECRETARY

October 7, 2009

STATE PROJECT: 33713.1.2 (B-4463)
F.A. PROJECT: STP-0032 (4)
COUNTY: Chowan
DESCRIPTION: Bridge No. 12 on NC 32 over Queen Anne Creek

SUBJECT: Geotechnical Report – Inventory

Project Description

The proposed project is located in Chowan County along NC 32 at the existing Queen Anne Creek crossing approximately 2 miles east of Edenton. Based on the current plans, proposed construction consists of minor widening of NC 32 and construction of an on-site detour to the south to accommodate bridge replacement.

Standard penetration test (SPT) and hand auger borings were performed at various offset locations from -L- and -L-DETOUR- alignments. SPT borings were completed with an ATV mounted CME-45B drill machine and advanced by rotary drill methods using bentonite drilling fluid. Representative samples were collected for visual classification in the field and submitted for laboratory analysis by the Materials and Tests Unit. Vane shear tests were performed in soft organic deposits. Also, Shelby tubes were collected in the soft organic deposits.

The investigation of subsurface conditions was confined to areas of proposed construction and included the following alignments. Profiles and selected cross sections will be submitted with this project.

<u>Alignment</u>	<u>Station</u>
-L-	12+52 to 29+69
-L-DETOUR-	12+52 to 29+67

Physiography and Geology

The project is located in Chowan County within the Coastal Plain Physiographic Province. This area is underlain by roadway embankment, alluvial sediments, undivided coastal plain soils, and the Yorktown Formation. Topography along the project is flat to gradually sloping. Ground elevations range from -6± feet below sea level along the channel bed of Queen Anne Creek to 13± feet above sea level along the existing NC 32 roadway embankment. Surface water along the project flows directly into Queen Anne Creek.

Ground Water

Ground water data was collected during February and March 2009, during which period the area experienced normal precipitation conditions. Ground water elevations were found to be from at mean sea level to 3± feet above mean sea level. The project area is subject to wind-driven tidal fluctuations.

Soils

Soils encountered during this investigation are separated into 4 categories: roadway embankment, undivided coastal plain soils, alluvial soils, and the Yorktown Formation soils.

Roadway embankment soils are made up of 1.5± feet to 6.5± feet of loose tan gray silty sand (A-2-4).

Undivided coastal plain soils consist of 2.5± feet to 6.6± feet of medium stiff to stiff tan sandy silt (A-4), underlain by loose gray silty sand (A-2-4).

Alluvial soils in the flood plain consist of 2.0± feet to 15.0± feet of soft brown muck (A-5), 8.0± feet to 15.0± feet of soft brown moderately organic sandy silt (A-4), and 1.5± feet of loose dark brown moderately organic silty sand (A-2-4). These deposits are underlain by loose to dense gray sand (A-2-4, A-3). Laboratory analysis of representative samples collected within the organic deposits returned a natural moisture content of 24.0 percent to 42.0 percent. The representative samples returned an organic content of 9.0 percent to 24.0 percent. The Vane Shear tests range from 0 to 1378 psf.

The Yorktown Formation soils are made up of silty sand (A-2-4), sandy silt (A-4), and sandy silty clay (A-6, A-7-6). Shell fragments are typically found in this formation. The Yorktown Formation is encountered at -30± feet to -35± feet below sea level.

MAILING ADDRESS:
NC DEPARTMENT OF TRANSPORTATION
GEOTECHNICAL ENGINEERING UNIT
1589 MAIL SERVICE CENTER
RALEIGH NC 27699-1589

TELEPHONE: 919-250-4088
FAX: 919-250-4237
WEBSITE: WWW.NCDOT.GOV/DOH

LOCATION:
CENTURY CENTER COMPLEX
ENTRANCE B-2
1020 BIRCH RIDGE DRIVE
RALEIGH NC

Undisturbed Samples

Undisturbed thin wall Shelby tube samples were collected at the following locations and submitted for testing.

<u>Sample No.</u>	<u>Station</u>	<u>Depth</u>	<u>Test</u>
ST-1	-L- Sta. 19+50 45' RT	0.0'-2.0'	Consolidation, Triaxial
ST-2	-L- Sta. 22+75 58' RT	0.0'-2.0'	Consolidation, Triaxial

Prepared by,



Cynthia Wrike
Engineering Geologist

EARTHWORK BALANCE SHEET

Volumes in Cubic Yards

PROJECT B-4463

COUNTY Chowan

DATE 2-Apr-10

COMPILED BY: Garrett McCaffety



3B of 18
SHEET 1 OF 1 SHEETS

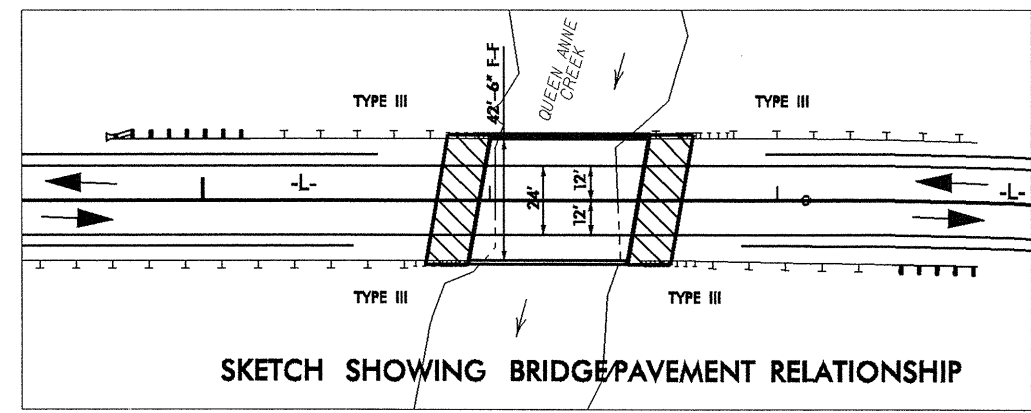
STATION	STATION	EXCAVATION					EMBANKMENT				BORROW	WASTE			
		TOTAL UNCLASS.	ROCK	UNDERCUT	UNSUIT. UNCLASS.	SUITABLE UNCLASS.	TOTAL	ROCK	EARTH	EMBANK. +30%		ROCK	SUITABLE	UNSUIT.	TOTAL
-L- 17+71.27	20+92.50 (BEG. BRIDGE)	15				15	219		219	286	271				
	SUBTOTAL	15				15	219		219	286	271				
-L- 21+55.50 (END BRIDGE)	23+61.72	18				18	186		186	241	223				
	SUBTOTAL	18				18	186		186	241	223				
	SUBTOTAL														
	SUBTOTAL														
	SUBTOTAL														
	TOTAL	33				33	405		405	527	494				
MATERIAL FOR SHOULDER CONSTRUCTION															
LOSS DUE TO CLEARING & GRUBBING															
ADDITIONAL UNDERCUT															
ROCK WASTE TO REPLACE BORROW															
ADJUST FOR ROCK WASTE															
WASTE IN LIEU OF BORROW															
	PROJECT TOTAL	33				33	405		405	527	494				
EST. 5% TO REPLACE TOP SOIL ON BORROW PIT											25				
	GRAND TOTAL	33				33	405		405	527	519				
	SAY	40				40					520				
Per Geotech Recommendation															
						400									

NOTE: EARTHWORK QUANTITIES ARE CALCULATED BY THE ROADWAY DESIGN UNIT. THESE EARTHWORK QUANTITIES ARE BASED IN PART ON SUBSURFACE DATA PROVIDED BY THE GEOTECHNICAL ENGINEERING UNIT.

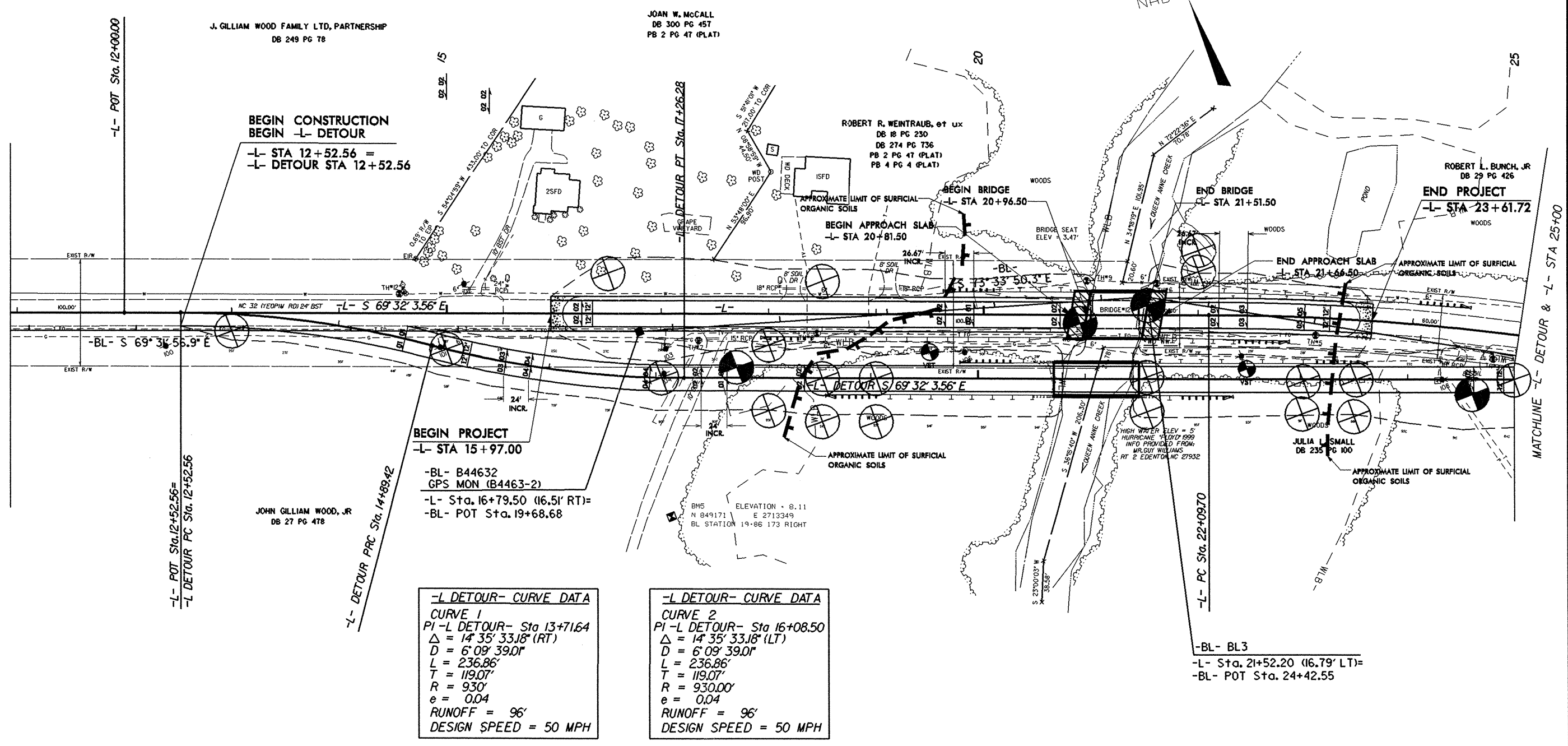
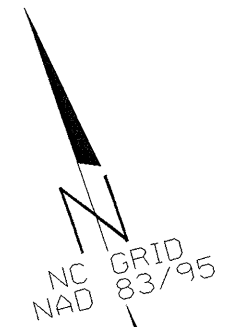
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PROJECT REFERENCE NO. B-4463	SHEET NO. 4
R/W SHEET NO.	
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
INCOMPLETE PLANS DO NOT USE FOR ACQUISITION PRELIMINARY PLANS DO NOT USE FOR CONSTRUCTION	
 DRMP ENGINEER-PLANNERS-DESIGNERS DYER, REOLE, MILLS & PRECOURT, INC. 1506 EAST INDEPENDENCE BLVD., SUITE 105 CHARLOTTE, NORTH CAROLINA 28227 (704) 332-2289	
 MA Engineering CONSULTANTS, INC. 1000 S. COLLETTA DRIVE CHARLOTTE, NORTH CAROLINA 28227 (704) 332-2289	



SKETCH SHOWING BRIDGE/PAVEMENT RELATIONSHIP



BEGIN CONSTRUCTION
BEGIN -L- DETOUR
 -L- STA 12+52.56 =
 -L- DETOUR STA 12+52.56

BEGIN PROJECT
 -L- STA 15+97.00
 -BL- B44632
 GPS MON (B4463-2)
 -L- Sta. 16+79.50 (16.51' RT)=
 -BL- POT Sta. 19+68.68

-L DETOUR- CURVE DATA
CURVE 1
 PI -L DETOUR- Sta 13+71.64
 $\Delta = 14^\circ 35' 33.18''$ (RT)
 $D = 6' 09' 39.01''$
 $L = 236.86'$
 $T = 119.07'$
 $R = 930'$
 $e = 0.04$
 RUNOFF = 96'
 DESIGN SPEED = 50 MPH

-L DETOUR- CURVE DATA
CURVE 2
 PI -L DETOUR- Sta 16+08.50
 $\Delta = 14^\circ 35' 33.18''$ (LT)
 $D = 6' 09' 39.01''$
 $L = 236.86'$
 $T = 119.07'$
 $R = 930.00'$
 $e = 0.04$
 RUNOFF = 96'
 DESIGN SPEED = 50 MPH

-BL- BL3
 -L- Sta. 21+52.20 (16.79' LT)=
 -BL- POT Sta. 24+42.55

REVISIONS

MATCHLINE -L- DETOUR & -L- STA 25+00

8/17/99

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REVISIONS

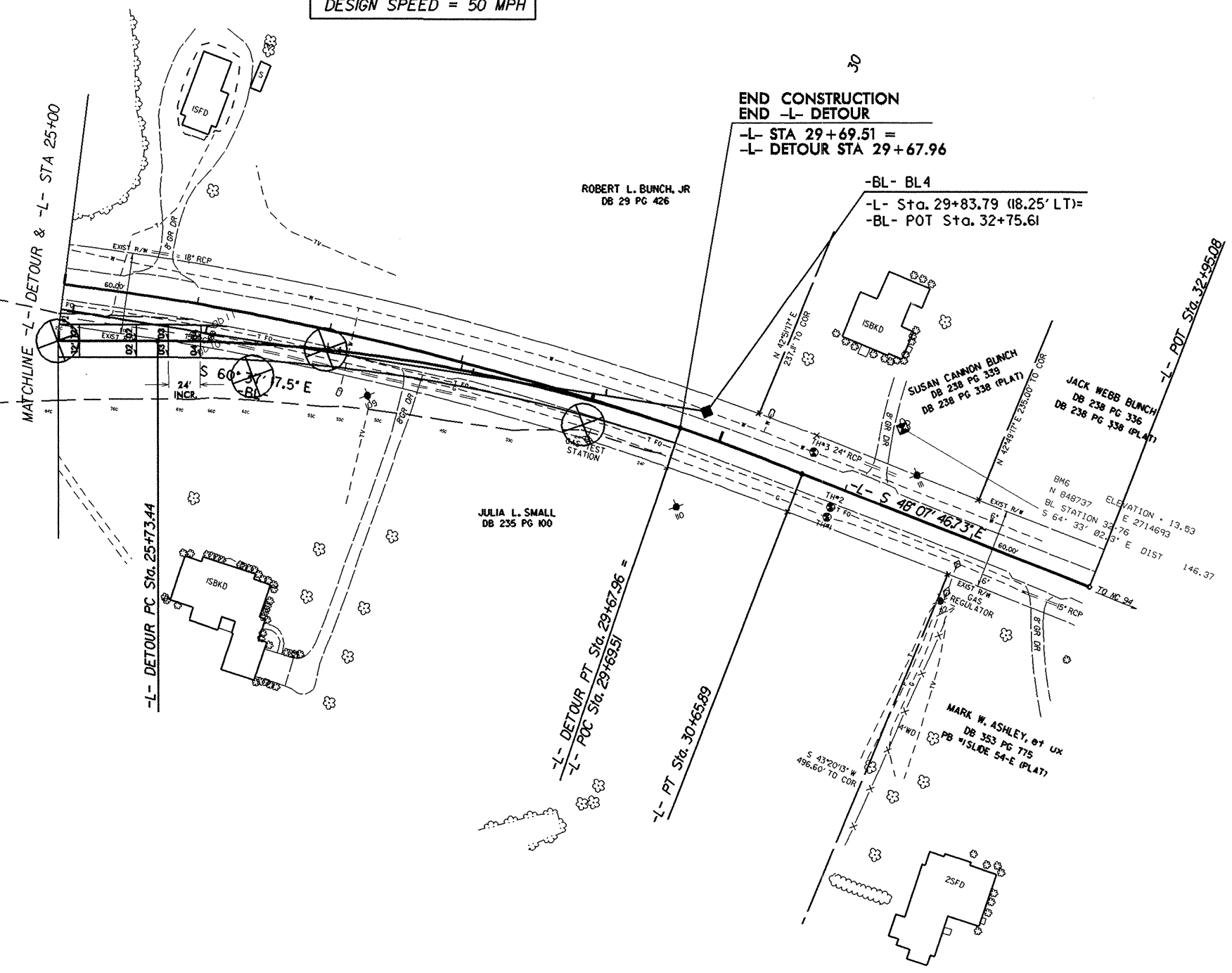
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RAW SHEET NO.	
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
INCOMPLETE PLANS DO NOT USE FOR ACQUISITION PRELIMINARY PLANS DO NOT USE FOR CONSTRUCTION	

DRMP ENGINEERS, PLANNERS, ARCHITECTS
 1506 EAST INDEPENDENCE BLVD., SUITE 105
 CHARLOTTE, NORTH CAROLINA 28227
 (704) 332-2289

MA Engineering CONSULTANTS, INC.
 1000 W. GOLF COURSE DRIVE, SUITE 107
 CHARLOTTE, NORTH CAROLINA 28202
 (704) 527-7511



-L DETOUR- CURVE DATA
 CURVE 3
 PI -L DETOUR- Sta 27+72.53
 $\Delta = 18^{\circ} 59' 43.13''$ (LT)
 $D = 448' 53.18''$
 $L = 394.52'$
 $T = 199.09'$
 $R = 1,190'$
 $e = 0.04$
 RUNOFF = 96E PLANS
 DESIGN SPEED = 50 MPH



END CONSTRUCTION
 END -L- DETOUR

-L- STA 29+69.51 =
 -L- DETOUR STA 29+67.96

-BL- BL 4
 -L- Sta. 29+83.79 (18.25' LT)=
 -BL- POT Sta. 32+75.61

ROBERT L. BUNCH, JR.
 DB 29 PG 426

JULIA L. SMALL
 DB 235 PG 100

SUSAN CANNON BLANCH
 DB 238 PG 359
 DB 238 PG 338 (PLAT)

JACK WEBB BLANCH
 DB 238 PG 336
 DB 238 PG 338 (PLAT)

MARK W. ASHLEY, et ux
 DB 353 PG 775
 PB 11SLDUE 54-E (PLAT)

MATCHLINE -L- DETOUR & -L- STA 25+00

-L- DETOUR PC Sta. 25+73.44

-L- DETOUR PT Sta. 29+67.96 =
 -L- POC Sta. 29+69.51

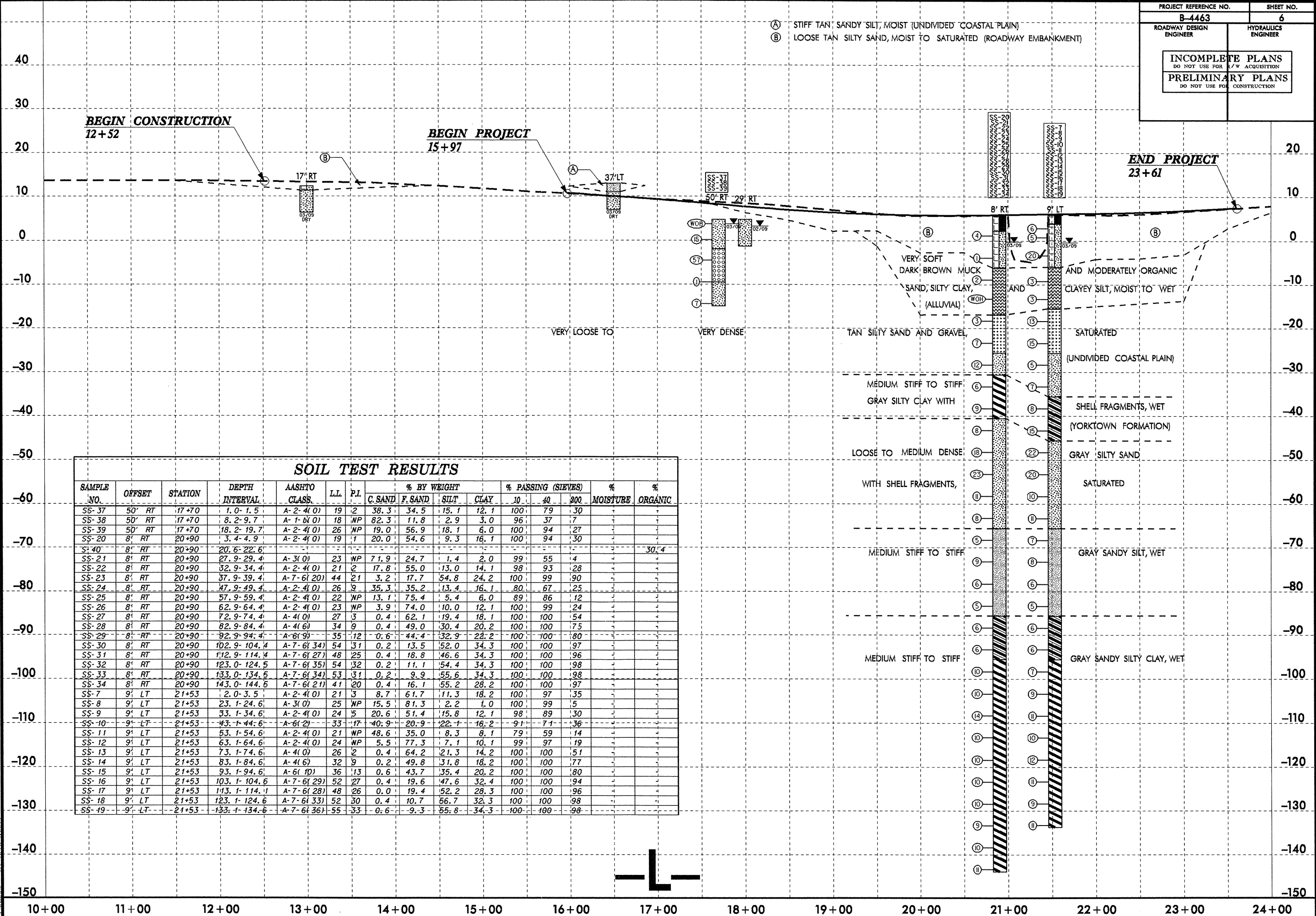
-L- PT Sta. 30+65.89

-L- POT STA. 32+95.08

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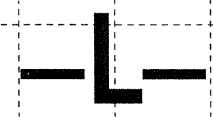
PROJECT REFERENCE NO. B-4463	SHEET NO. 6
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
INCOMPLETE PLANS DO NOT USE FOR ACQUISITION	
PRELIMINARY PLANS DO NOT USE FOR CONSTRUCTION	

- (A) STIFF TAN SANDY SILT, MOIST (UNDIVIDED COASTAL PLAIN)
- (B) LOOSE TAN SILTY SAND, MOIST TO SATURATED (ROADWAY EMBANKMENT)



SOIL TEST RESULTS

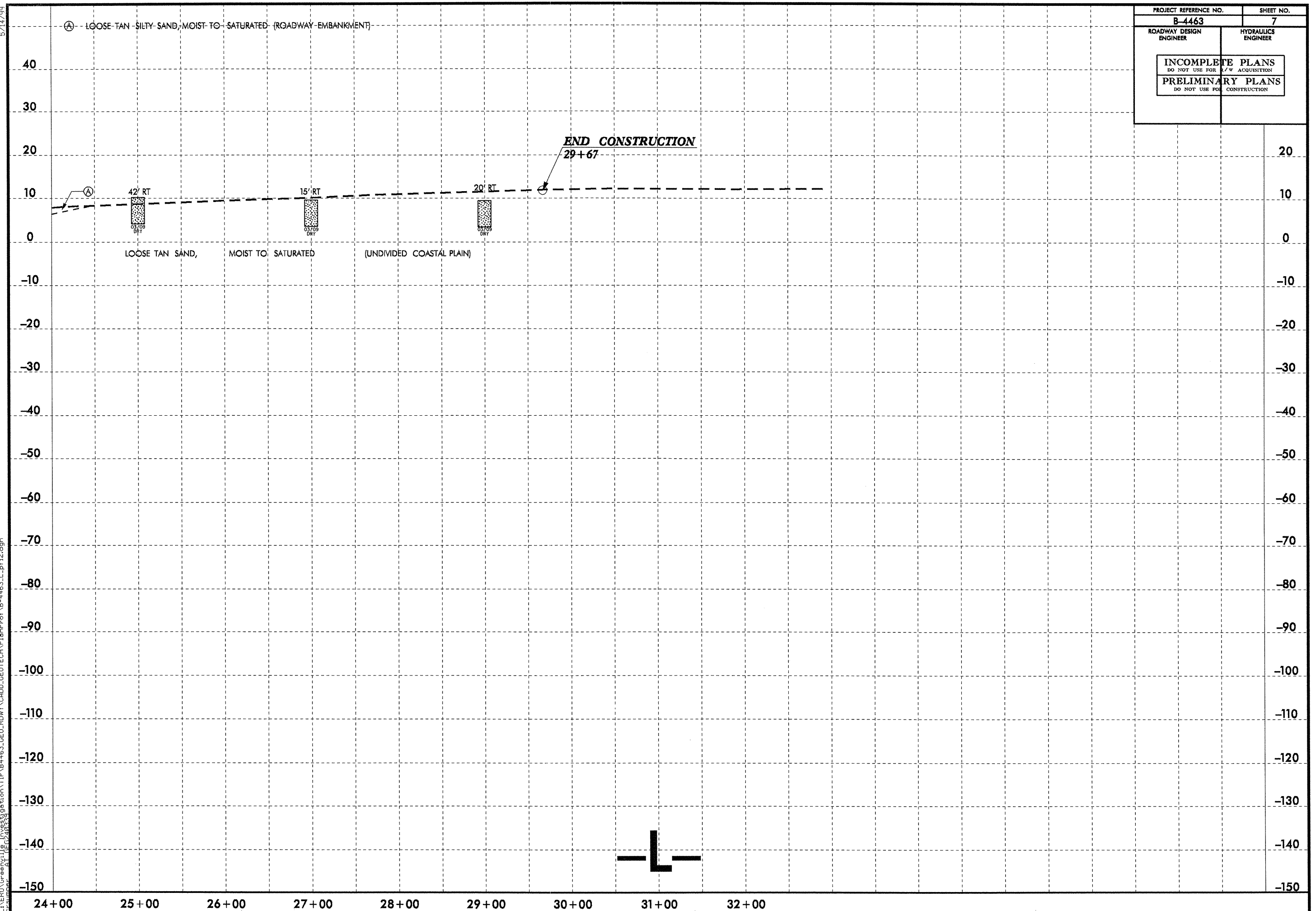
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	LL	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C. SAND	F. SAND	SILT	CLAY	#10	#40	#200		
SS-37	50' RT	17+70	1.0-1.5	A-2-4(0)	19	2	38.3	34.5	15.1	12.1	100	79	30	-	-
SS-38	50' RT	17+70	8.2-9.7	A-1-6(0)	18	NP	82.3	11.8	2.9	3.0	96	37	7	-	-
SS-39	50' RT	17+70	18.2-19.7	A-2-4(0)	26	NP	19.0	56.9	18.1	6.0	100	94	27	-	-
SS-20	8' RT	20+90	3.4-4.9	A-2-4(0)	19	1	20.0	54.6	9.3	16.1	100	94	30	-	-
SS-40	8' RT	20+90	20.6-22.6	A-2-4(0)	19	1	20.0	54.6	9.3	16.1	100	94	30	30.4	-
SS-21	8' RT	20+90	27.9-29.4	A-3(0)	23	NP	71.9	24.7	1.4	2.0	99	55	4	-	-
SS-22	8' RT	20+90	32.9-34.4	A-2-4(0)	21	2	17.8	55.0	13.0	14.1	98	93	28	-	-
SS-23	8' RT	20+90	37.9-39.4	A-7-6(20)	44	21	3.2	17.7	54.8	24.2	100	99	90	-	-
SS-24	8' RT	20+90	47.9-49.4	A-2-4(0)	26	9	35.3	35.2	13.4	16.1	80	67	25	-	-
SS-25	8' RT	20+90	57.9-59.4	A-2-4(0)	22	NP	13.1	75.4	5.4	6.0	89	86	12	-	-
SS-26	8' RT	20+90	62.9-64.4	A-2-4(0)	23	NP	3.9	74.0	10.0	12.1	100	99	24	-	-
SS-27	8' RT	20+90	72.9-74.4	A-4(0)	27	3	0.4	62.1	19.4	18.1	100	100	54	-	-
SS-28	8' RT	20+90	82.9-84.4	A-4(6)	34	9	0.4	49.0	30.4	20.2	100	100	75	-	-
SS-29	8' RT	20+90	92.9-94.4	A-6(9)	35	12	0.6	44.4	32.9	22.2	100	100	80	-	-
SS-30	8' RT	20+90	102.9-104.4	A-7-6(34)	54	31	0.2	13.5	52.0	34.3	100	100	97	-	-
SS-31	8' RT	20+90	112.9-114.4	A-7-6(27)	48	25	0.4	18.8	46.6	34.3	100	100	96	-	-
SS-32	8' RT	20+90	123.0-124.5	A-7-6(35)	54	32	0.2	11.1	54.4	34.3	100	100	98	-	-
SS-33	8' RT	20+90	133.0-134.5	A-7-6(34)	53	31	0.2	9.9	55.6	34.3	100	100	98	-	-
SS-34	8' RT	20+90	143.0-144.5	A-7-6(21)	41	20	0.4	16.1	55.2	28.2	100	100	97	-	-
SS-7	9' LT	21+53	2.0-3.5	A-2-4(0)	21	3	8.7	61.7	11.3	18.2	100	97	35	-	-
SS-8	9' LT	21+53	23.1-24.6	A-3(0)	25	NP	15.5	81.3	2.2	1.0	100	99	5	-	-
SS-9	9' LT	21+53	33.1-34.6	A-2-4(0)	24	5	20.6	51.4	15.8	12.1	98	89	30	-	-
SS-10	9' LT	21+53	43.1-44.6	A-6(2)	33	17	40.9	20.9	22.1	16.2	91	71	36	-	-
SS-11	9' LT	21+53	53.1-54.6	A-2-4(0)	21	NP	48.6	35.0	8.3	8.1	79	59	14	-	-
SS-12	9' LT	21+53	63.1-64.6	A-2-4(0)	24	NP	5.5	77.3	7.1	10.1	99	97	19	-	-
SS-13	9' LT	21+53	73.1-74.6	A-4(0)	26	2	0.4	64.2	21.3	14.2	100	100	51	-	-
SS-14	9' LT	21+53	83.1-84.6	A-4(6)	32	9	0.2	49.8	31.8	18.2	100	100	77	-	-
SS-15	9' LT	21+53	93.1-94.6	A-6(10)	36	13	0.6	43.7	35.4	20.2	100	100	80	-	-
SS-16	9' LT	21+53	103.1-104.6	A-7-6(29)	52	27	0.4	19.6	47.6	32.4	100	100	94	-	-
SS-17	9' LT	21+53	113.1-114.6	A-7-6(28)	48	26	0.0	19.4	52.2	28.3	100	100	96	-	-
SS-18	9' LT	21+53	123.1-124.6	A-7-6(33)	52	30	0.4	10.7	56.7	32.3	100	100	98	-	-
SS-19	9' LT	21+53	133.1-134.6	A-7-6(36)	55	33	0.6	9.3	55.8	34.3	100	100	98	-	-



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PROJECT REFERENCE NO. B-4463	SHEET NO. 7
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
INCOMPLETE PLANS DO NOT USE FOR ACQUISITION	
PRELIMINARY PLANS DO NOT USE FOR CONSTRUCTION	



⊕ - LOOSE TAN SILTY SAND, MOIST TO SATURATED (ROADWAY EMBANKMENT)

42' RT
03/08
DRY

15' RT
03/08
DRY

20' RT
03/08
DRY

END CONSTRUCTION
29+67

LOOSE TAN SAND, MOIST TO SATURATED (UNDIVIDED COASTAL PLAIN)

40
30
20
10
0
-10
-20
-30
-40
-50
-60
-70
-80
-90
-100
-110
-120
-130
-140
-150

24+00 25+00 26+00 27+00 28+00 29+00 30+00 31+00 32+00

20
10
0
-10
-20
-30
-40
-50
-60
-70
-80
-90
-100
-110
-120
-130
-140
-150

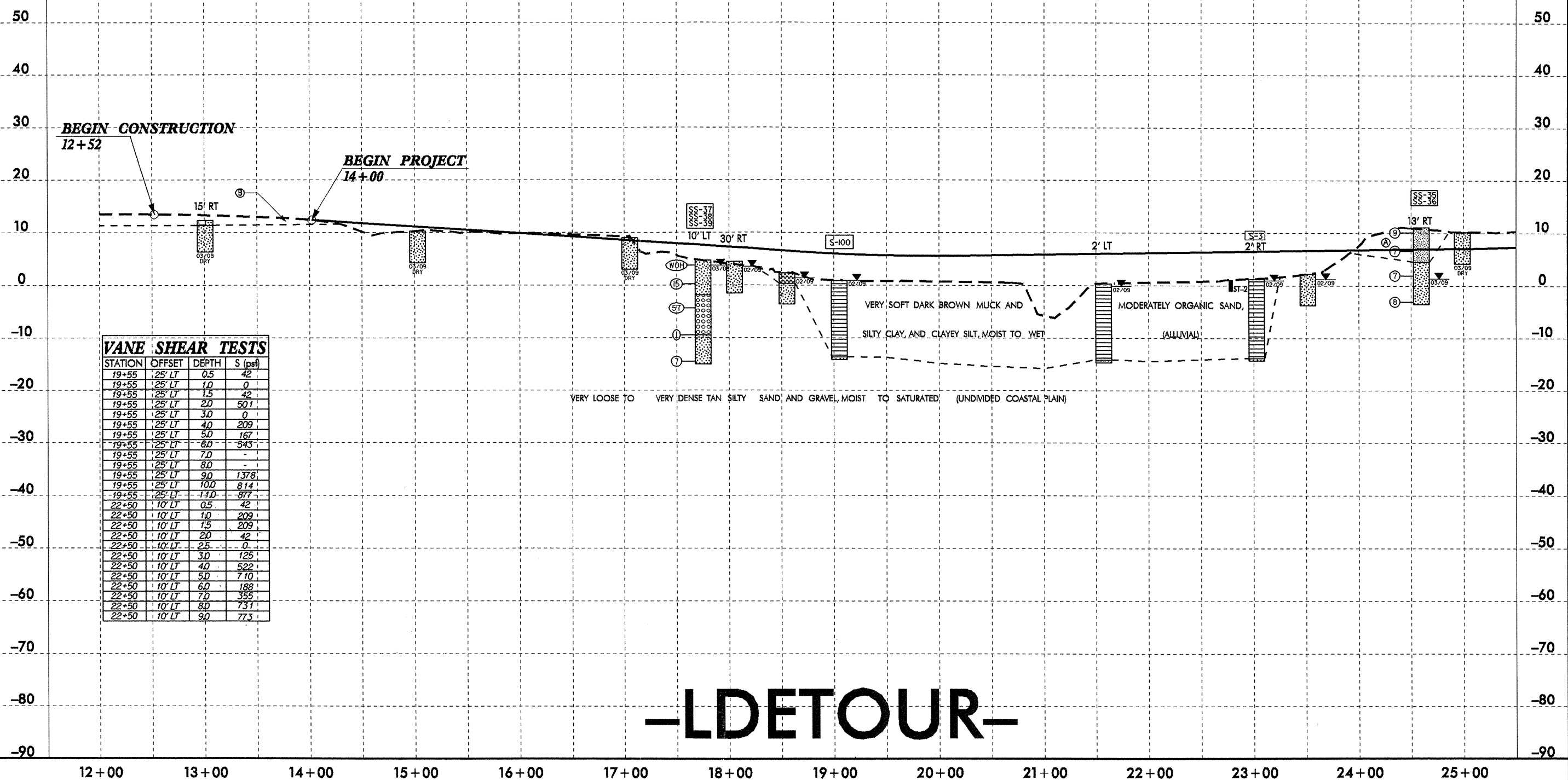
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PROJECT REFERENCE NO. B-4463	SHEET NO. 8
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
INCOMPLETE PLANS DO NOT USE FOR R/W ACQUISITION	
PRELIMINARY PLANS DO NOT USE FOR CONSTRUCTION	

SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	LL	P.I.	% BY WEIGHT				% PASSING (SIEVES)			MOISTURE %	ORGANIC %
							C. SAND	F. SAND	SILT	CLAY	10	40	200		
SS-37	10' LT	17+75	1.0-1.5	A-2-4(0)	19	2	38.3	34.5	15.1	12.1	100	79	30	-	-
SS-38	10' LT	17+75	8.2-9.7	A-1-b(0)	18	NP	82.3	11.8	2.9	3.0	96	37	7	-	-
SS-39	10' LT	17+75	18.2-19.7	A-2-4(0)	26	NP	79.0	56.9	18.1	6.0	100	94	27	-	-
S-100	CL	19+05	1.0-14.5	A-7-5(8)	48	13	13.2	27.0	25.4	34.4	100	95	64	-	13.7
S-3	2' RT	23+00	1.0-15.0	A-4(0)	28	5	21.1	36.3	17.7	24.9	100	88	47	27.9	17.2
SS-35	13' RT	24+62	1.0-1.5	A-4(0)	17	1	10.3	54.4	21.2	14.1	100	96	42	-	-
SS-36	13' RT	24+62	8.2-9.7	A-2-4(0)	19	NP	47.4	41.2	3.3	8.1	96	69	12	-	-

Ⓐ MEDIUM STIFF TO STIFF SANDY SILT, MOIST (UNDIVIDED COASTAL PLAIN)
 Ⓑ LOOSE TAN SILTY SAND, MOIST TO SATURATED (ROADWAY EMBANKMENT)

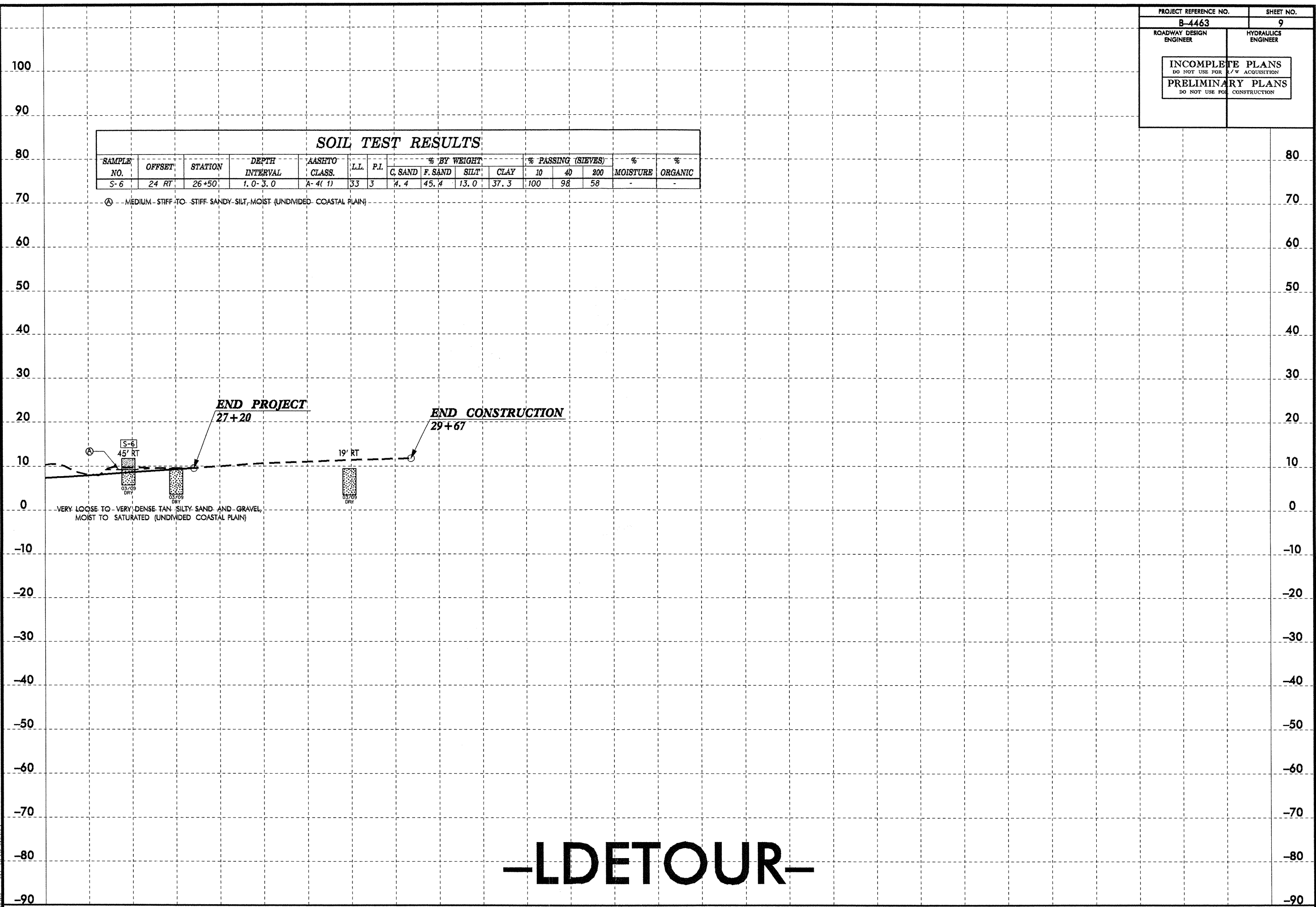


STATION	OFFSET	DEPTH	S (psi)
19+55	25' LT	0.5	42
19+55	25' LT	1.0	0
19+55	25' LT	1.5	42
19+55	25' LT	2.0	501
19+55	25' LT	3.0	0
19+55	25' LT	4.0	209
19+55	25' LT	5.0	167
19+55	25' LT	6.0	543
19+55	25' LT	7.0	-
19+55	25' LT	8.0	-
19+55	25' LT	9.0	1378
19+55	25' LT	10.0	814
19+55	25' LT	11.0	877
22+50	10' LT	0.5	42
22+50	10' LT	1.0	209
22+50	10' LT	1.5	209
22+50	10' LT	2.0	42
22+50	10' LT	2.5	0
22+50	10' LT	3.0	125
22+50	10' LT	4.0	522
22+50	10' LT	5.0	710
22+50	10' LT	6.0	188
22+50	10' LT	7.0	355
22+50	10' LT	8.0	731
22+50	10' LT	9.0	773

-LDETOUR-

SOIL TEST RESULTS															
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	LL	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C. SAND	F. SAND	SILT	CLAY	10	40	200		
S-6	24 RT	26+50	1.0-3.0	A-4(1)	33	3	4.4	45.4	13.0	37.3	100	98	58	-	-

Ⓐ MEDIUM-STIFF TO STIFF SANDY SILT, MOIST (UNDIVIDED COASTAL PLAIN)



VERY LOOSE TO VERY DENSE TAN SILTY SAND AND GRAVEL, MOIST TO SATURATED (UNDIVIDED COASTAL PLAIN)

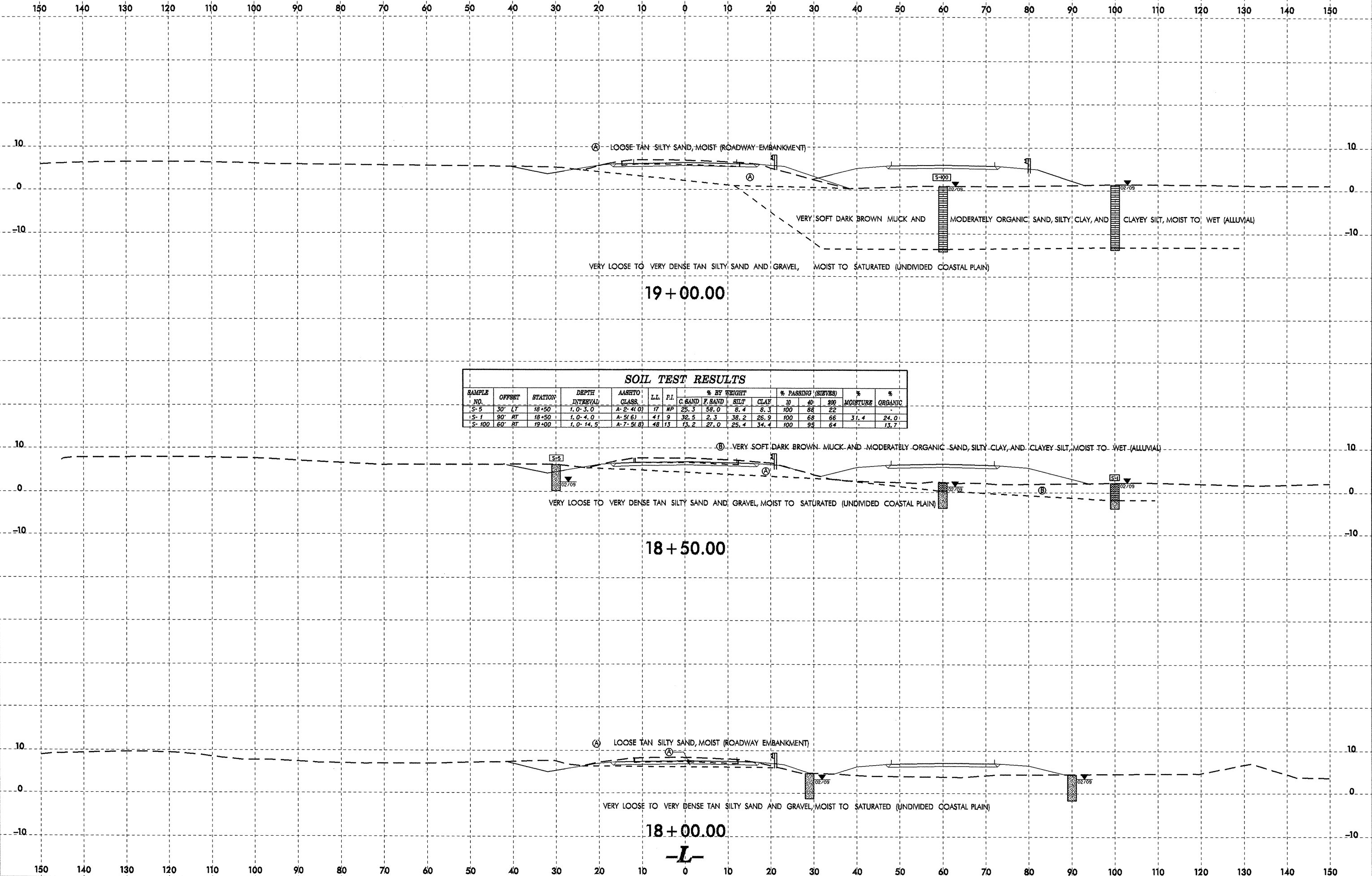
END PROJECT
27+20

END CONSTRUCTION
29+67

-LDETOUR-

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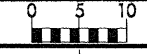
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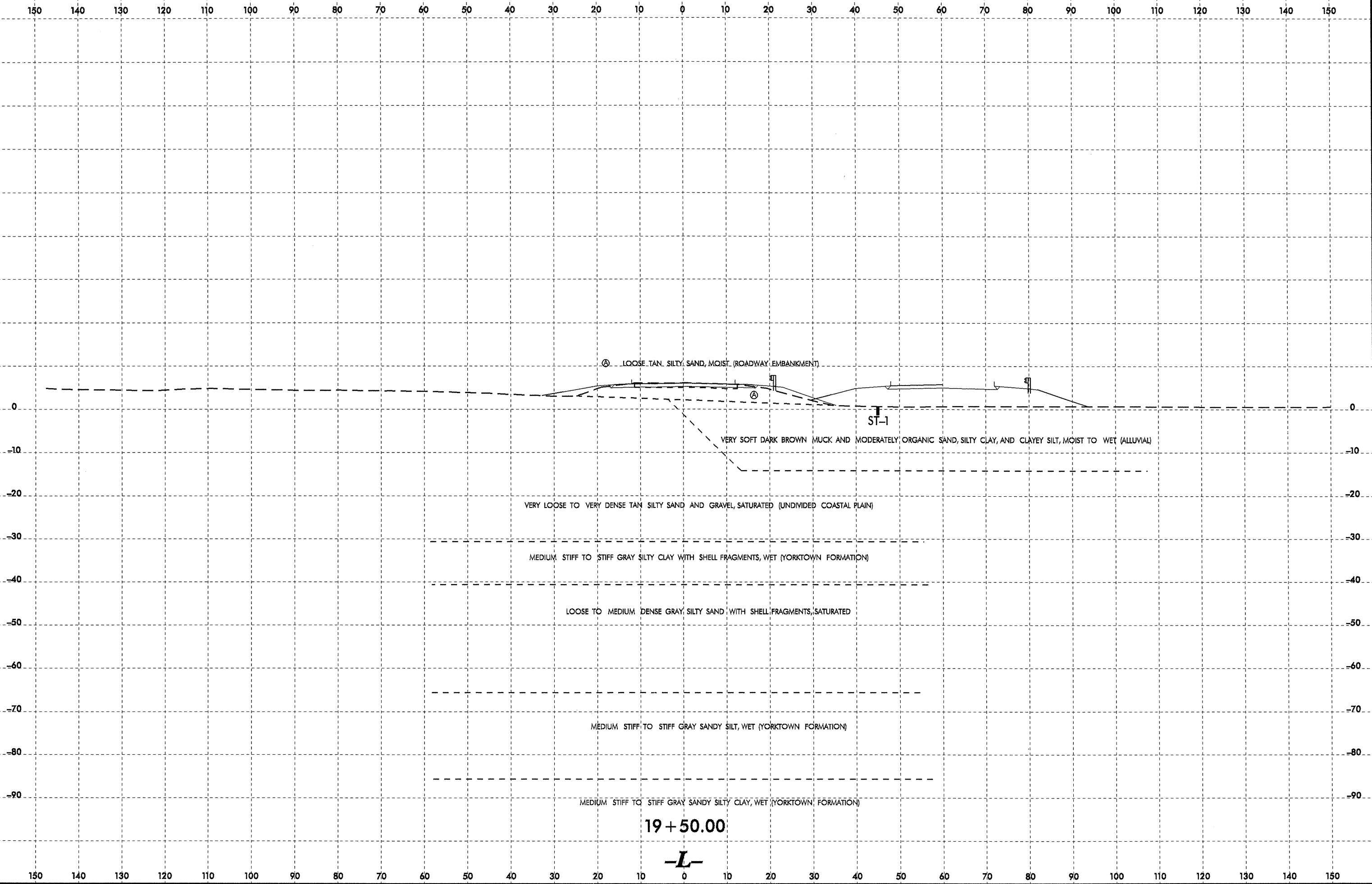
SOIL TEST RESULTS															
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	LL	F.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C. SAND	F. SAND	SILT	CLAY	10	40	200		
S-5	30' LY	18+50	1.0-3.0	A-2-4(0)	17	NP	25.3	58.0	8.4	8.3	100	88	22	-	-
S-1	90' RT	18+50	1.0-4.0	A-5(6)	41	9	32.5	2.3	38.2	26.9	100	68	66	31.4	24.0
S-100	60' RT	19+00	1.0-14.5	A-7-5(8)	48	13	75.2	27.0	25.4	34.4	100	94	64	-	13.7

8/23/11



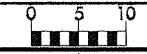
PROJ. REFERENCE NO.
B-4463

SHEET NO.
11



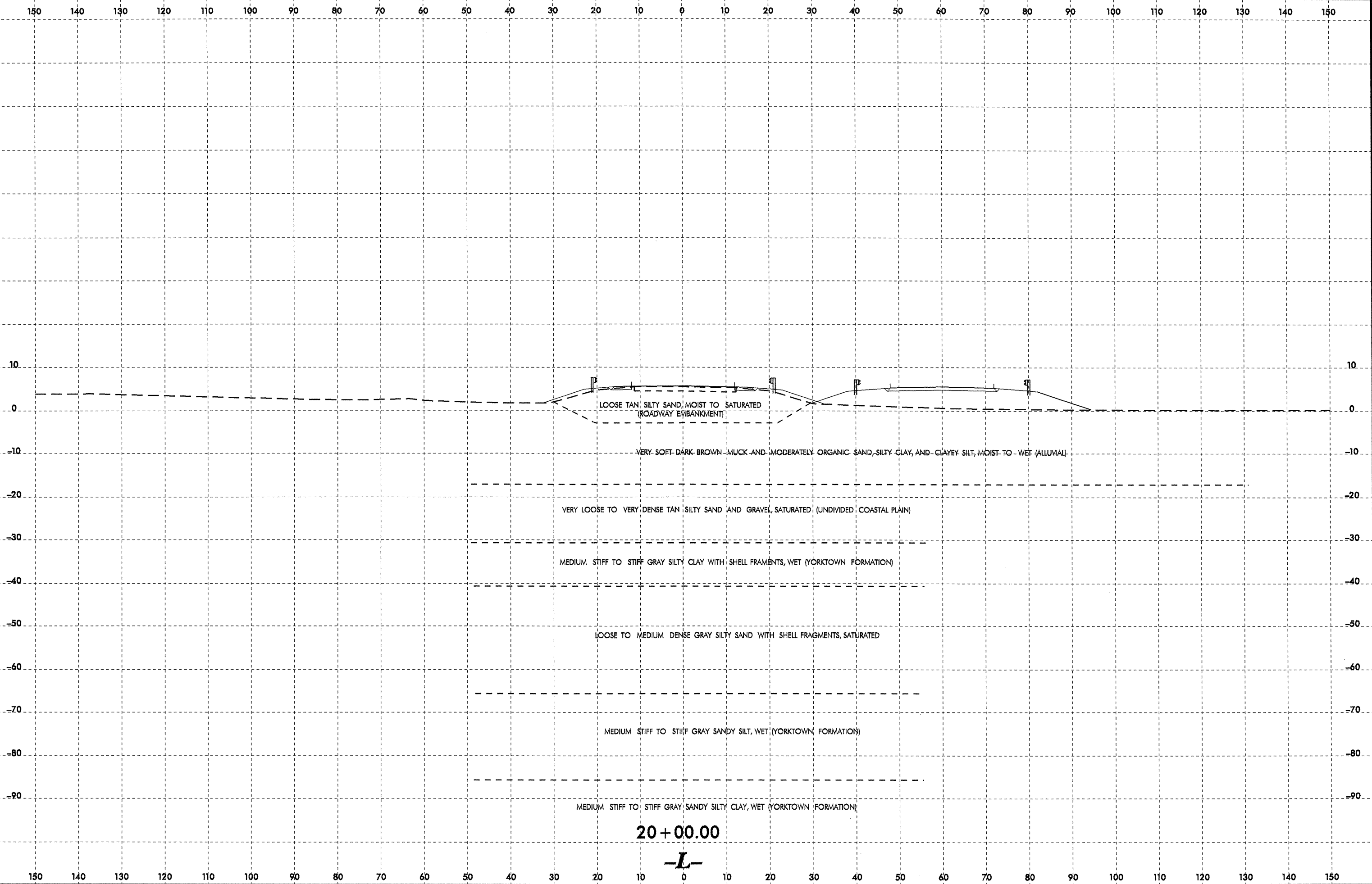
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PROJ. REFERENCE NO.
B-4463

SHEET NO.
12



LOOSE TAN SILTY SAND, MOIST TO SATURATED
(ROADWAY EMBANKMENT)

VERY SOFT DARK BROWN MUCK AND MODERATELY ORGANIC SAND, SILTY CLAY, AND CLAYEY SILT, MOIST TO WET (ALLUVIAL)

VERY LOOSE TO VERY DENSE TAN SILTY SAND AND GRAVEL, SATURATED (UNDIVIDED COASTAL PLAIN)

MEDIUM STIFF TO STIFF GRAY SILTY CLAY WITH SHELL FRAGMENTS, WET (YORKTOWN FORMATION)

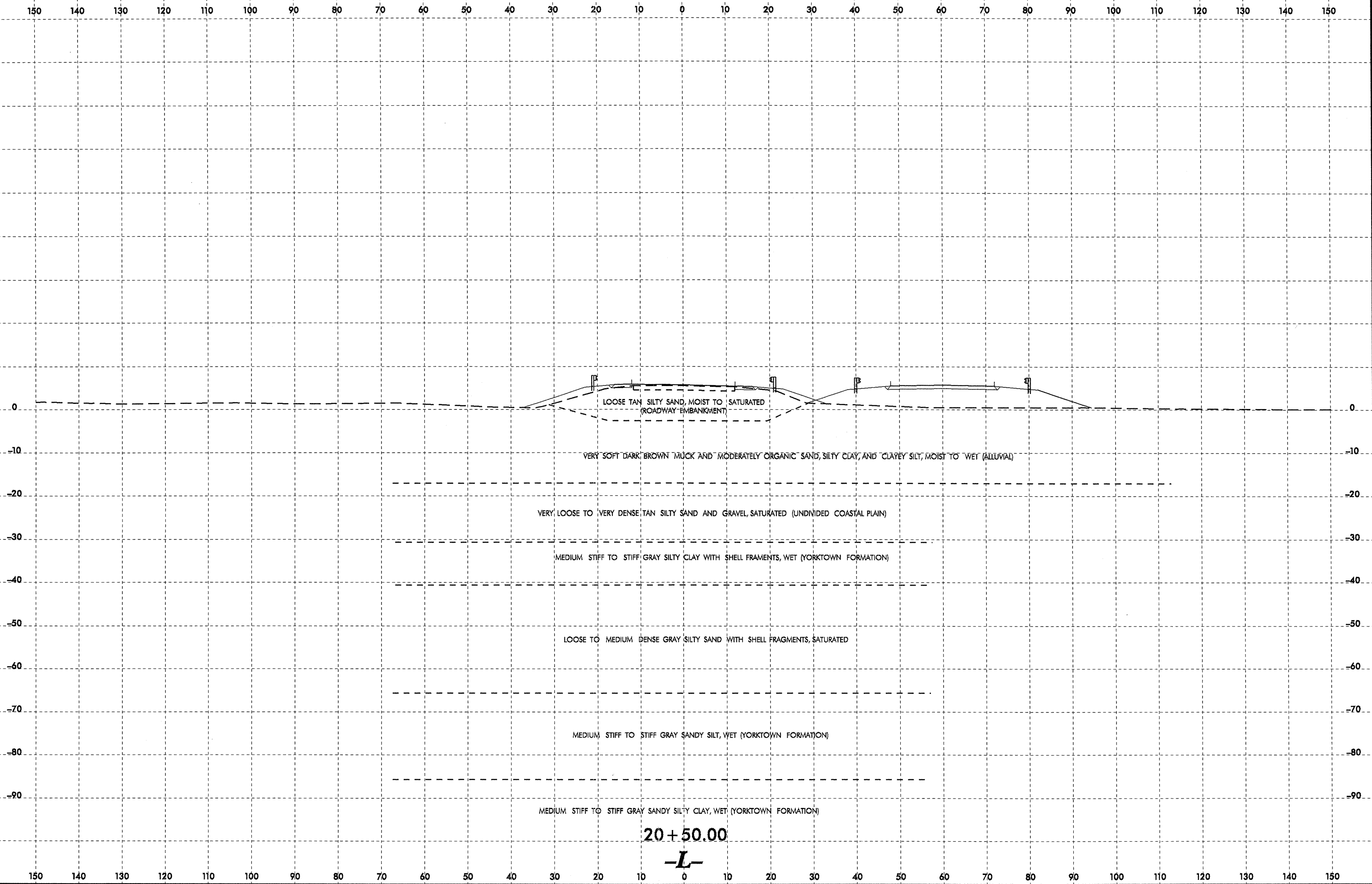
LOOSE TO MEDIUM DENSE GRAY SILTY SAND WITH SHELL FRAGMENTS, SATURATED

MEDIUM STIFF TO STIFF GRAY SANDY SILT, WET (YORKTOWN FORMATION)

MEDIUM STIFF TO STIFF GRAY SANDY SILTY CLAY, WET (YORKTOWN FORMATION)

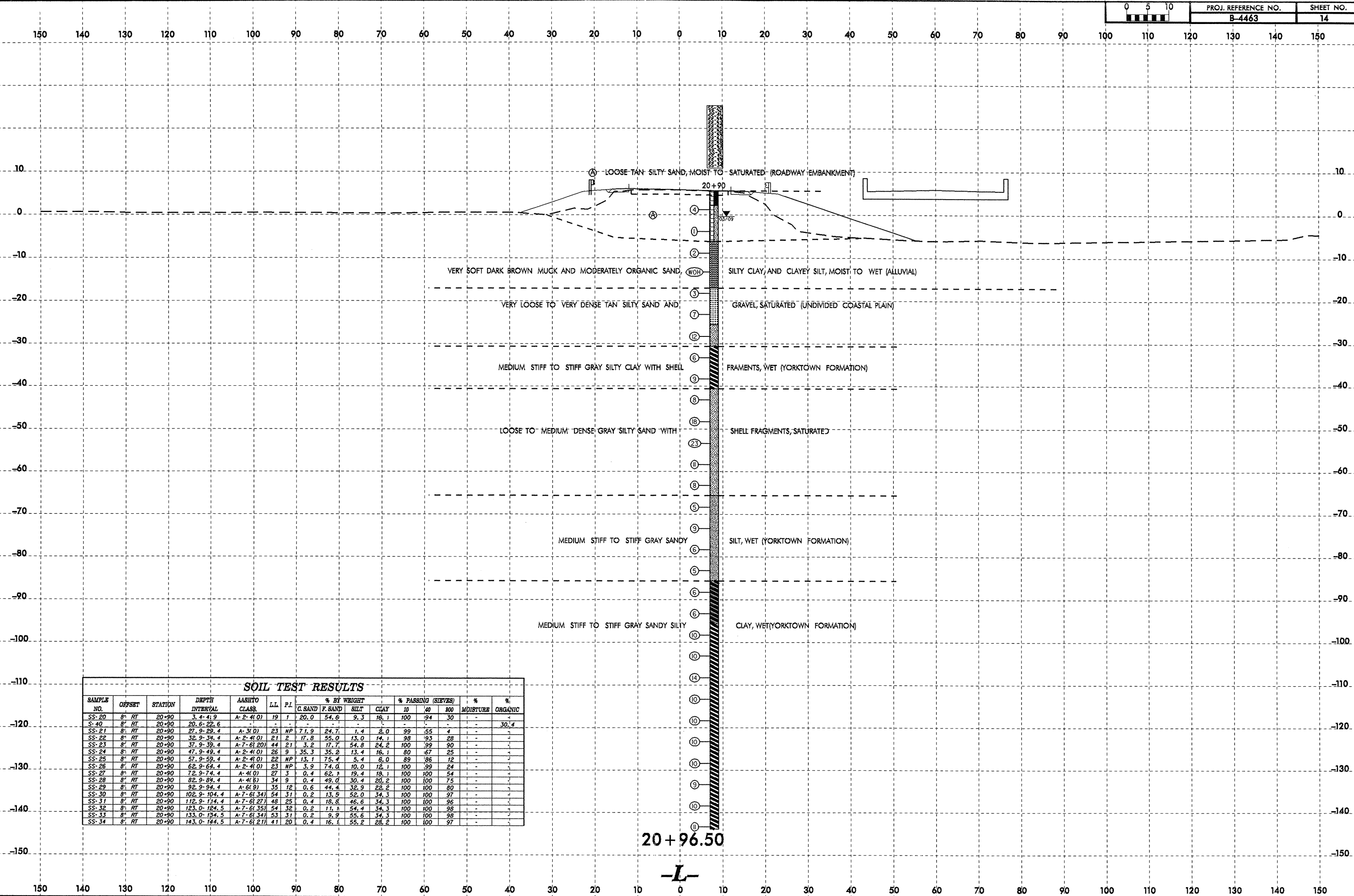
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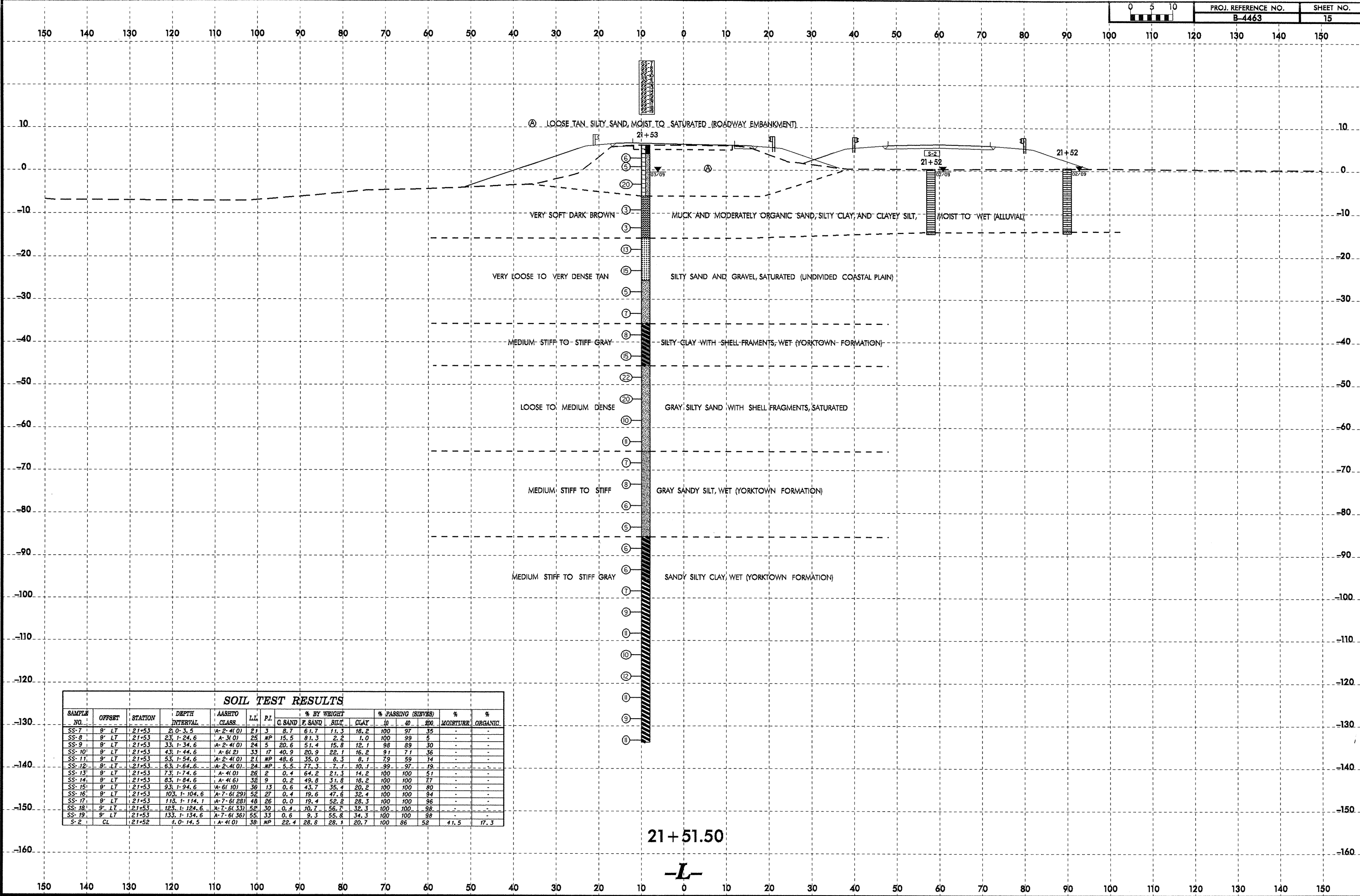
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SOIL TEST RESULTS

SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	LL	PI	% BY WEIGHT				% PASSING (SIEVES)			%	%
							C. SAND	F. SAND	SILT	CLAY	10	40	800		
SS-20	8' RT	20+90	3.4-4.9	A-2-(10)	19	1	20.0	54.6	9.3	16.1	100	94	30	-	30.4
S-40	8' RT	20+90	20.6-22.6	-	-	-	-	-	-	-	-	-	-	-	-
SS-21	8' RT	20+90	27.9-29.4	A-3(0)	23	NP	71.9	24.7	1.4	2.0	99	55	4	-	-
SS-22	8' RT	20+90	32.9-34.4	A-2-(10)	21	2	17.8	55.0	13.0	14.1	98	93	28	-	-
SS-23	8' RT	20+90	37.9-39.4	A-7-(6120)	44	21	3.2	17.7	54.8	24.2	100	99	90	-	-
SS-24	8' RT	20+90	47.9-49.4	A-2-(10)	26	9	35.3	35.2	13.4	16.1	80	67	25	-	-
SS-25	8' RT	20+90	57.9-59.4	A-2-(10)	22	NP	13.1	75.4	5.4	6.0	89	86	12	-	-
SS-26	8' RT	20+90	62.9-64.4	A-2-(10)	23	NP	3.9	74.0	10.0	12.1	100	99	24	-	-
SS-27	8' RT	20+90	72.9-74.4	A-4(0)	27	3	0.4	62.1	19.4	18.1	100	100	54	-	-
SS-28	8' RT	20+90	82.9-84.4	A-4(0)	34	9	0.4	49.0	30.4	20.2	100	100	75	-	-
SS-29	8' RT	20+90	92.9-94.4	A-6(9)	35	12	0.6	44.4	32.9	22.2	100	100	80	-	-
SS-30	8' RT	20+90	102.9-104.4	A-7-(6134)	54	31	0.2	13.9	52.0	34.3	100	100	97	-	-
SS-31	8' RT	20+90	112.9-114.4	A-7-(6127)	48	25	0.4	18.8	46.6	34.3	100	100	96	-	-
SS-32	8' RT	20+90	123.0-124.5	A-7-(6135)	54	32	0.2	11.1	54.4	34.3	100	100	98	-	-
SS-33	8' RT	20+90	133.0-134.5	A-7-(6134)	53	31	0.2	9.9	55.6	34.3	100	100	98	-	-
SS-34	8' RT	20+90	143.0-144.5	A-7-(6121)	41	20	0.4	16.1	55.2	28.2	100	100	97	-	-

20+96.50
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SOIL TEST RESULTS

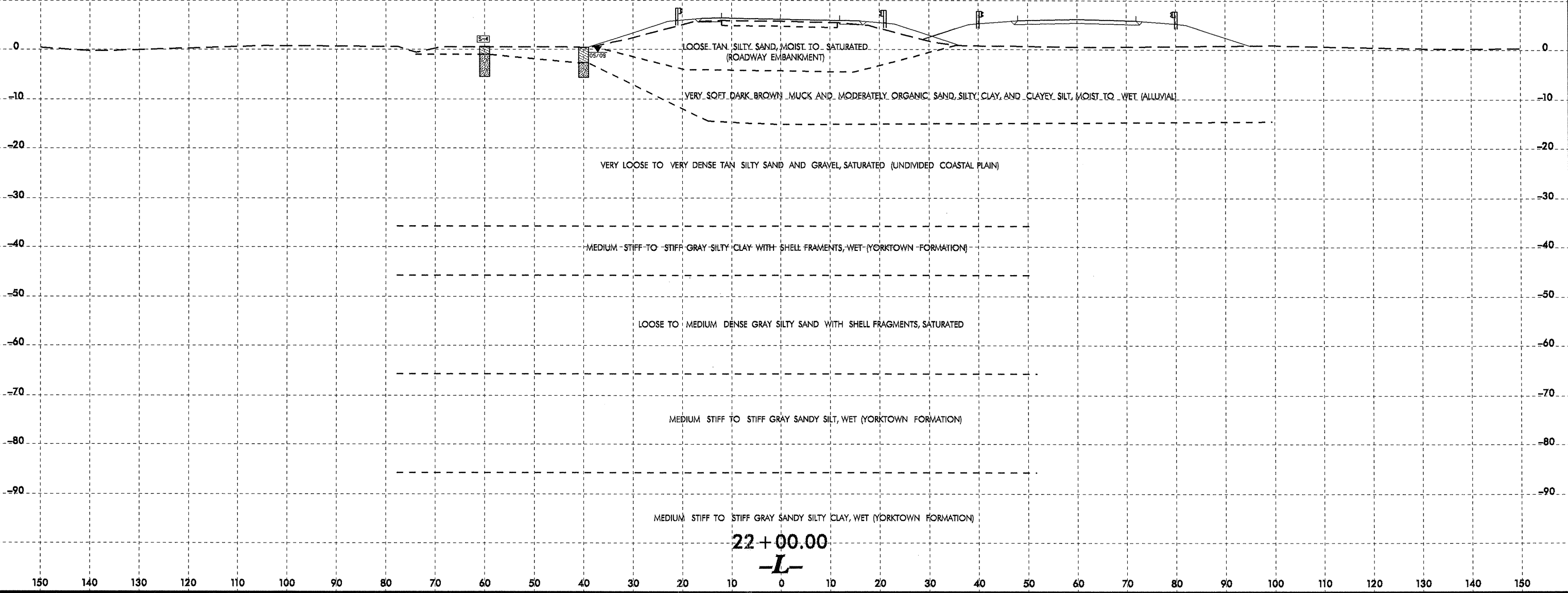
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							G SAND	F. SAND	SILT	CLAY	#10	#40	#200		
SS-7	9' LT	21+53	21.0-3.5	A-2-4(0)	21	3	8.7	61.7	11.3	18.2	100	97	35	-	-
SS-8	9' LT	21+53	23.1-24.6	A-3(0)	25	NP	15.5	81.3	2.2	1.0	100	99	5	-	-
SS-9	9' LT	21+53	33.1-34.6	A-2-4(0)	24	5	20.6	51.4	15.8	12.1	98	89	30	-	-
SS-10	9' LT	21+53	43.1-44.6	A-6(2)	33	17	40.9	20.9	22.1	16.2	91	71	36	-	-
SS-11	9' LT	21+53	53.1-54.6	A-2-4(0)	21	NP	48.6	35.0	8.3	8.1	79	59	14	-	-
SS-12	9' LT	21+53	63.1-64.6	A-2-4(0)	24	NP	5.5	77.3	7.1	10.1	99	97	19	-	-
SS-13	9' LT	21+53	73.1-74.6	A-4(0)	26	2	0.4	64.2	21.3	14.2	100	100	51	-	-
SS-14	9' LT	21+53	83.1-84.6	A-4(0)	32	9	0.2	49.8	31.8	18.2	100	100	77	-	-
SS-15	9' LT	21+53	93.1-94.6	A-6(10)	36	13	0.6	43.7	35.4	20.2	100	100	80	-	-
SS-16	9' LT	21+53	103.1-104.6	A-7-6(29)	52	27	0.4	19.6	47.6	32.4	100	100	94	-	-
SS-17	9' LT	21+53	113.1-114.1	A-7-6(28)	48	26	0.0	19.4	52.2	28.3	100	100	96	-	-
SS-18	9' LT	21+53	123.1-124.6	A-7-6(33)	52	30	0.4	10.7	56.7	32.3	100	100	98	-	-
SS-19	9' LT	21+53	133.1-134.6	A-7-6(36)	55	33	0.6	9.3	55.8	34.3	100	100	98	-	-
S-2	CL	21+52	4.0-14.5	A-4(0)	38	NP	22.4	28.8	28.1	20.7	100	86	52	41.5	17.3

21+51.50

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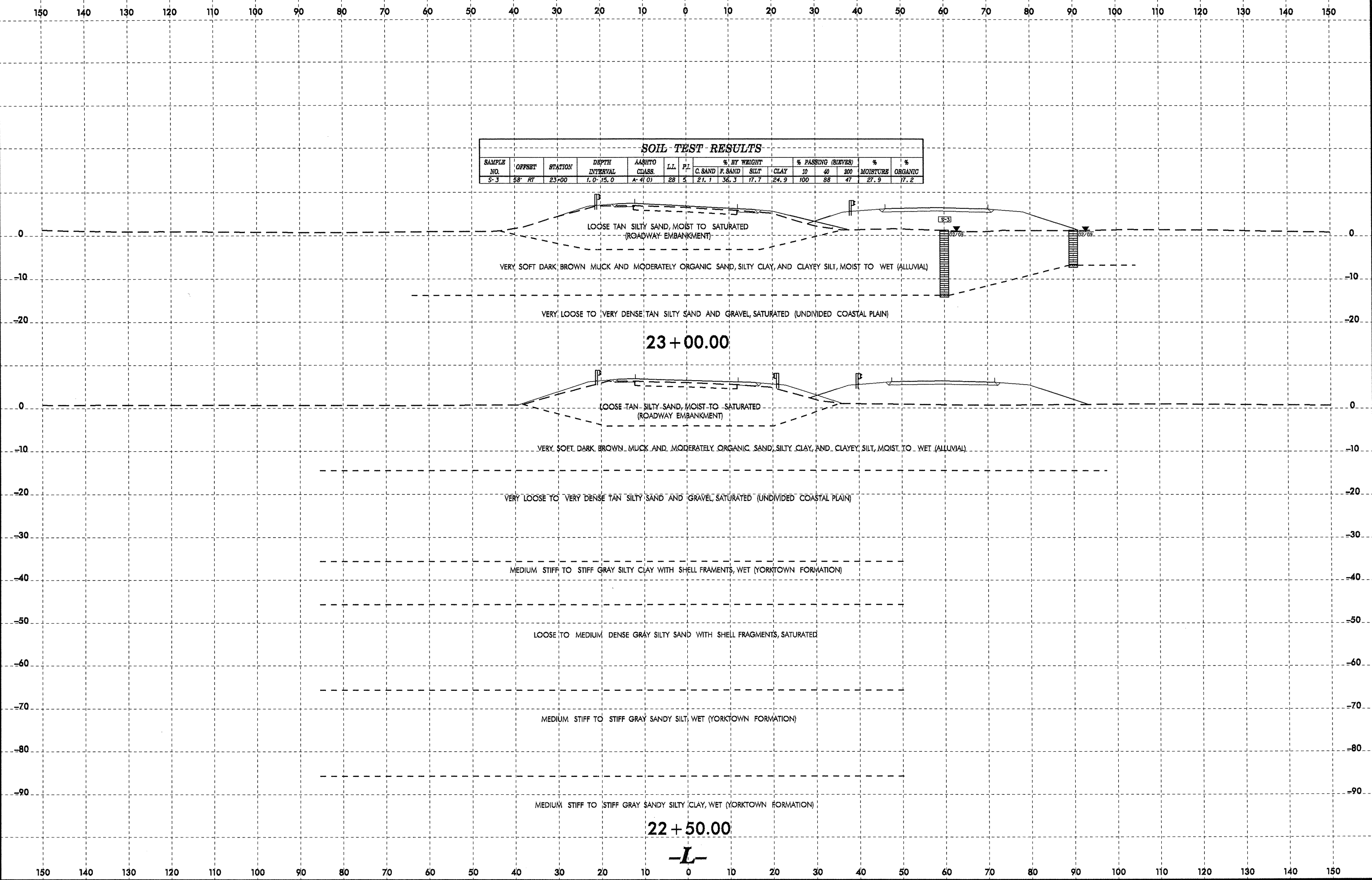
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SOIL TEST RESULTS															
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	LL	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C. SAND	F. SAND	SILT	CLAY	20	40	800		
S-4	60' LT	22+00	1.0' - 1.5'	A-2-4(0)	36	NP	26.7	42.3	6.1	24.9	98	84	32	23.6	9



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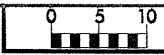
SOIL TEST RESULTS															
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	ASTM CLASS.	LL	PI	% BY WEIGHT			% PASSING (SIEVES)			% MOISTURE	% ORGANIC	
							C. SAND	F. SAND	SILT	CLAY	10	40			200
S-3	58' RT	23+00	1.0-15.0	A-4(0)	28	5	21.1	36.3	17.7	24.9	100	88	47	27.9	17.2

23 + 00.00

22 + 50.00

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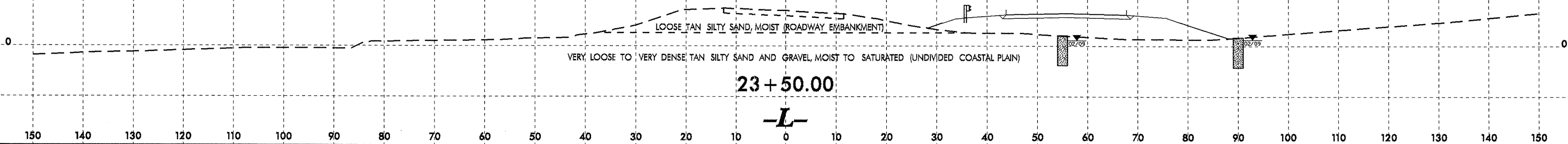
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PROJ. REFERENCE NO.	SHEET NO.
B-4463	18

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