

NOTE: SEE SHEET 2A FOR PLAN SHEET LAYOUT AT TIME OF INVESTIGATION

STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
DIVISION OF HIGHWAYS
GEOTECHNICAL ENGINEERING UNIT

STATE	STATE PROJECT REFERENCE NO.	SHEET NO.	TOTAL SHEETS
N.C.	U-5018A *	1	55
WBS NO.	F.A. PROJ. NO.	DESCRIPTION	
41431.1.1		P.E.	
41431.2.1		R/W	
41431.3.2		CONST.	

CONTENTS

LINE	STATION	PLAN	PROFILE
-L-	12+25 TO 145+84	4-14	15-24

ROADWAY
SUBSURFACE INVESTIGATION

CROSS SECTIONS

LINE	STATION	SHEET NO.
-L-	13+50 TO 53+00	25-44
-L-	57+50 TO 81+50	45-55

PROJ. REFERENCE NO. 41431.1.1 (U-5018) F.A. PROJ. STP-0043 (8)
COUNTY PITT
PROJECT DESCRIPTION NC 43 FROM US 264 TO NC 11
(MEMORIAL DRIVE)

INVENTORY

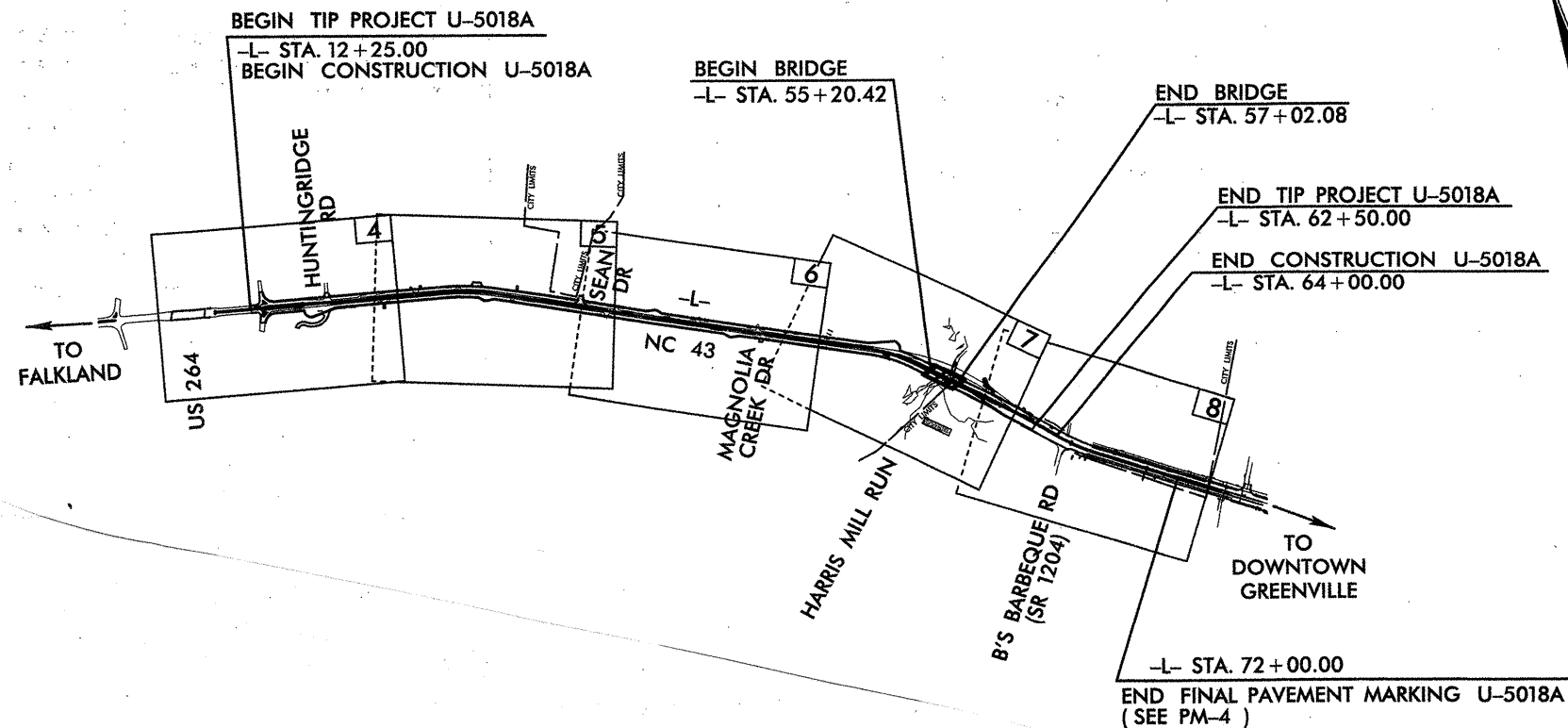
CAUTION NOTICE

THE SUBSURFACE INFORMATION AND THE SUBSURFACE INVESTIGATION ON WHICH IT IS BASED WERE MADE FOR THE PURPOSE OF STUDY, PLANNING, AND DESIGN, AND NOT FOR CONSTRUCTION OR PAY PURPOSES. THE VARIOUS FIELD BORING LOGS, ROCK CORES, AND SOIL TEST DATA AVAILABLE MAY BE REVIEWED OR INSPECTED IN RALEIGH BY CONTACTING THE N.C. DEPARTMENT OF TRANSPORTATION, GEOTECHNICAL ENGINEERING UNIT AT (919) 250-4088. NEITHER THE SUBSURFACE PLANS AND REPORTS, NOR THE FIELD BORING LOGS, ROCK CORES, OR SOIL TEST DATA ARE PART OF THE CONTRACT.

GENERAL SOIL AND ROCK STRATA DESCRIPTIONS AND INDICATED BOUNDARIES ARE BASED ON A GEOTECHNICAL INTERPRETATION OF ALL AVAILABLE SUBSURFACE DATA AND MAY NOT NECESSARILY REFLECT THE ACTUAL SUBSURFACE CONDITIONS BETWEEN BORINGS OR BETWEEN SAMPLED STRATA WITHIN THE BOREHOLE. THE LABORATORY SAMPLE DATA AND THE IN SITU (IN-PLACE) TEST DATA CAN BE RELIED ON ONLY TO THE DEGREE OF RELIABILITY INHERENT IN THE STANDARD TEST METHOD. THE OBSERVED WATER LEVELS OR SOIL MOISTURE CONDITIONS INDICATED IN THE SUBSURFACE INVESTIGATIONS ARE AS RECORDED AT THE TIME OF THE INVESTIGATION. THESE WATER LEVELS OR SOIL MOISTURE CONDITIONS MAY VARY CONSIDERABLY WITH TIME ACCORDING TO CLIMATIC CONDITIONS INCLUDING TEMPERATURES, PRECIPITATION, AND WIND, AS WELL AS OTHER NON-CLIMATIC FACTORS.

THE BIDDER OR CONTRACTOR IS CAUTIONED THAT DETAILS SHOWN ON THE SUBSURFACE PLANS ARE PRELIMINARY ONLY AND IN MANY CASES THE FINAL DESIGN DETAILS ARE DIFFERENT. FOR BIDDING AND CONSTRUCTION PURPOSES, REFER TO THE CONSTRUCTION PLANS AND DOCUMENTS FOR FINAL DESIGN INFORMATION ON THIS PROJECT. THE DEPARTMENT DOES NOT WARRANT OR GUARANTEE THE SUFFICIENCY OR ACCURACY OF THE INVESTIGATION MADE, NOR THE INTERPRETATIONS MADE, OR OPINION OF THE DEPARTMENT AS TO THE TYPE OF MATERIALS AND CONDITIONS TO BE ENCOUNTERED. THE BIDDER OR CONTRACTOR IS CAUTIONED TO MAKE SUCH INDEPENDENT SUBSURFACE INVESTIGATIONS AS HE DEEMS NECESSARY TO SATISFY HIMSELF AS TO CONDITIONS TO BE ENCOUNTERED ON THIS PROJECT. THE CONTRACTOR SHALL HAVE NO CLAIM FOR ADDITIONAL COMPENSATION OR FOR AN EXTENSION OF TIME FOR ANY REASON RESULTING FROM THE ACTUAL CONDITIONS ENCOUNTERED AT THE SITE DIFFERING FROM THOSE INDICATED IN THE SUBSURFACE INFORMATION.

TRACT: C202636 ID: U-5018A



PERSONNEL

- TCB
- JRS
- RES
- S&ME

INVESTIGATED BY TC BOTTOMS
CHECKED BY DN ARGENBRIGHT
SUBMITTED BY DN ARGENBRIGHT
DATE AUGUST, 2008

* This inventory is for U-5018, which includes U-5018A and U-5018B sections. Please refer to the respective portions for your needs.

NOTE - THE INFORMATION CONTAINED HEREIN IS NOT IMPLIED OR GUARANTEED BY THE N.C. DEPARTMENT OF TRANSPORTATION AS BEING ACCURATE NOR IS IT CONSIDERED TO BE PART OF THE PLANS, SPECIFICATIONS, OR CONTRACT FOR THE PROJECT.





NOTE - BY HAVING REQUESTED THIS INFORMATION THE CONTRACTOR SPECIFICALLY WAIVES ANY CLAIMS FOR INCREASED COMPENSATION OR EXTENSION OF TIME BASED ON DIFFERENCES BETWEEN THE CONDITIONS INDICATED HEREIN AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.



NORTH CAROLINA DEPARTMENT OF TRANSPORTATION
DIVISION OF HIGHWAYS
GEOTECHNICAL ENGINEERING UNIT

SUBSURFACE INVESTIGATION

SOIL AND ROCK LEGEND, TERMS, SYMBOLS, AND ABBREVIATIONS

SOIL DESCRIPTION		GRADATION		ROCK DESCRIPTION		TERMS AND DEFINITIONS					
SOIL IS CONSIDERED TO BE THE UNCONSOLIDATED, SEMI-CONSOLIDATED, OR WEATHERED EARTH MATERIALS THAT CAN BE PENETRATED WITH A CONTINUOUS FLIGHT POWER AUGER, AND YIELD LESS THAN 100 BLOWS PER FOOT ACCORDING TO STANDARD PENETRATION TEST (ASTM D-1586). SOIL CLASSIFICATION IS BASED ON THE AASHTO SYSTEM. BASIC DESCRIPTIONS GENERALLY SHALL INCLUDE: CONSISTENCY, COLOR, TEXTURE, MOISTURE, AASHTO CLASSIFICATION AND OTHER PERTINENT FACTORS SUCH AS MINERALOGICAL COMPOSITION, ANGULARITY, STRUCTURE, PLASTICITY, ETC. EXAMPLE: <i>VERY STIFF, GRAY, SILT CLAY, MOST WITH INTERBEDDED FINE SAND LAYERS, HIGH PLASTIC, A-7-6</i>		WELL GRADED - INDICATES A GOOD REPRESENTATION OF PARTICLE SIZES FROM FINE TO COARSE. UNIFORM - INDICATES THAT SOIL PARTICLES ARE ALL APPROXIMATELY THE SAME SIZE. (ALSO POORLY GRADED) GAP-GRADED - INDICATES A MIXTURE OF UNIFORM PARTICLES OF TWO OR MORE SIZES.		HARD ROCK IS NON-COASTAL PLAIN MATERIAL THAT IF TESTED, WOULD YIELD SPT REFUSAL, AN INFERRED ROCK LINE INDICATES THE LEVEL AT WHICH NON-COASTAL PLAIN MATERIAL WOULD YIELD SPT REFUSAL. SPT REFUSAL IS PENETRATION BY A SPLIT SPOON SAMPLER EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS. IN NON-COASTAL PLAIN MATERIAL, THE TRANSITION BETWEEN SOIL AND ROCK IS OFTEN REPRESENTED BY A ZONE OF WEATHERED ROCK. ROCK MATERIALS ARE TYPICALLY DIVIDED AS FOLLOWS:  NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT N VALUES > 100 BLOWS PER FOOT IF TESTED.  FINE TO COARSE GRAIN IGNEOUS AND METAMORPHIC ROCK THAT WOULD YIELD SPT REFUSAL IF TESTED. ROCK TYPE INCLUDES GRANITE, GNEISS, GABBRO, SCHIST, ETC.  FINE TO COARSE GRAIN METAMORPHIC AND NON-COASTAL PLAIN SEDIMENTARY ROCK THAT WOULD YIELD SPT REFUSAL IF TESTED. ROCK TYPE INCLUDES PHYLLITE, SLATE, SANDSTONE, ETC.  COASTAL PLAIN SEDIMENTS CEMENTED INTO ROCK, BUT MAY NOT YIELD SPT REFUSAL. ROCK TYPE INCLUDES LIMESTONE, SANDSTONE, CEMENTED SHELL BEDS, ETC.		ALLUVIUM (ALLUV.) - SOILS THAT HAVE BEEN TRANSPORTED BY WATER. AQUIFER - A WATER BEARING FORMATION OR STRATA. ARENACEOUS - APPLIED TO ROCKS THAT HAVE BEEN DERIVED FROM SAND OR THAT CONTAIN SAND. ARGILLACEOUS - APPLIED TO ALL ROCKS OR SUBSTANCES COMPOSED OF CLAY MINERALS, OR HAVING A NOTABLE PROPORTION OF CLAY IN THEIR COMPOSITION AS SHALE, SLATE, ETC. ARTESIAN - GROUND WATER THAT IS UNDER SUFFICIENT PRESSURE TO RISE ABOVE THE LEVEL AT WHICH IT IS ENCOUNTERED, BUT WHICH DOES NOT NECESSARILY RISE TO OR ABOVE THE GROUND SURFACE. CALCAREOUS (CALC.) - SOILS THAT CONTAIN APPRECIABLE AMOUNTS OF CALCIUM CARBONATE. COLLUVIUM - ROCK FRAGMENTS MIXED WITH SOIL DEPOSITED BY GRAVITY ON SLOPE OR AT BOTTOM OF SLOPE. CORE RECOVERY (REC.) - TOTAL LENGTH OF ALL MATERIAL RECOVERED IN THE CORE BARREL DIVIDED BY TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE. DIKE - A TABULAR BODY OF IGNEOUS ROCK THAT CUTS ACROSS THE STRUCTURE OF ADJACENT ROCKS OR CUTS MASSIVE ROCK. DIP - THE ANGLE AT WHICH A STRATUM OR ANY PLANAR FEATURE IS INCLINED FROM THE HORIZONTAL. DIP DIRECTION (DIP AZIMUTH) - THE DIRECTION OR BEARING OF THE HORIZONTAL TRACE OF THE LINE OF DIP, MEASURED CLOCKWISE FROM NORTH. FAULT - A FRACTURE OR FRACTURE ZONE ALONG WHICH THERE HAS BEEN DISPLACEMENT OF THE SIDES RELATIVE TO ONE ANOTHER PARALLEL TO THE FRACTURE. FISSILE - A PROPERTY OF SPLITTING ALONG CLOSELY SPACED PARALLEL PLANES. FLOAT - ROCK FRAGMENTS ON SURFACE NEAR THEIR ORIGINAL POSITION AND DISLODGED FROM PARENT MATERIAL. FLOOD PLAIN (FP) - LAND BORDERING A STREAM, BUILT OF SEDIMENTS DEPOSITED BY THE STREAM. FORMATION (FM.) - A MAPPABLE GEOLOGIC UNIT THAT CAN BE RECOGNIZED AND TRACED IN THE FIELD. JOINT - FRACTURE IN ROCK ALONG WHICH NO APPRECIABLE MOVEMENT HAS OCCURRED. LEDGE - A SHELF-LIKE RIDGE OR PROJECTION OF ROCK WHOSE THICKNESS IS SMALL COMPARED TO ITS LATERAL EXTENT. LENS - A BODY OF SOIL OR ROCK THAT THINS OUT IN ONE OR MORE DIRECTIONS. MOTTLED (MOT.) - IRREGULARLY MARKED WITH SPOTS OF DIFFERENT COLORS. MOTTLING IN SOILS USUALLY INDICATES POOR AERATION AND LACK OF GOOD DRAINAGE. PERCHED WATER - WATER MAINTAINED ABOVE THE NORMAL GROUND WATER LEVEL BY THE PRESENCE OF AN INTERVENING IMPERVIOUS STRATUM. RESIDUAL (RES.) SOIL - SOIL FORMED IN PLACE BY THE WEATHERING OF ROCK. ROCK QUALITY DESIGNATION (ROD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE. SAPROLITE (SAP.) - RESIDUAL SOIL THAT RETAINS THE RELIC STRUCTURE OR FABRIC OF THE PARENT ROCK. SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, THAT HAS BEEN EMPLACED PARALLEL TO THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS. SLICKENSIDE - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT OR SLIP PLANE. STANDARD PENETRATION TEST (PENETRATION RESISTANCE) (SPT) - NUMBER OF BLOWS (N OR BPF) OF A 140 LB. HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF 1 FOOT INTO SOIL WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT REFUSAL IS PENETRATION EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS. STRATA CORE RECOVERY (SREC) - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY TOTAL LENGTH OF STRATUM AND EXPRESSED AS A PERCENTAGE. STRATA ROCK QUALITY DESIGNATION (SRQD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS WITHIN A STRATUM EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF STRATA AND EXPRESSED AS A PERCENTAGE. TOPSOIL (TS.) - SURFACE SOILS USUALLY CONTAINING ORGANIC MATTER.					
SOIL LEGEND AND AASHTO CLASSIFICATION		MINERALOGICAL COMPOSITION		WEATHERING		ROCK HARDNESS					
GENERAL CLASS. GRANULAR MATERIALS (<= 35% PASSING #200) SILT-CLAY MATERIALS (> 35% PASSING #200) ORGANIC MATERIALS		MINERAL NAMES SUCH AS QUARTZ, FELDSPAR, MICA, TALC, KAOLIN, ETC. ARE USED IN DESCRIPTIONS WHENEVER THEY ARE CONSIDERED OF SIGNIFICANCE.		FRESH ROCK FRESH, CRYSTALS BRIGHT, FEW JOINTS MAY SHOW SLIGHT STAINING. ROCK RINGS UNDER HAMMER IF CRYSTALLINE. VERY SLIGHT (V SL.) ROCK GENERALLY FRESH, JOINTS STAINED, SOME JOINTS MAY SHOW THIN CLAY COATINGS IF OPEN. CRYSTALS ON A BROKEN SPECIMEN FACE SHINE BRIGHTLY. ROCK RINGS UNDER HAMMER BLOWS IF OF A CRYSTALLINE NATURE. SLIGHT (SL.) ROCK GENERALLY FRESH, JOINTS STAINED AND DISCOLORATION EXTENDS INTO ROCK UP TO 1 INCH. OPEN JOINTS MAY CONTAIN CLAY. IN GRANITOID ROCKS SOME OCCASIONAL FELDSPAR CRYSTALS ARE DULL AND DISCOLORED. CRYSTALLINE ROCKS RING UNDER HAMMER BLOWS. MODERATE (MOD.) SIGNIFICANT PORTIONS OF ROCK SHOW DISCOLORATION AND WEATHERING EFFECTS. IN GRANITOID ROCKS, MOST FELDSPARS ARE DULL AND DISCOLORED, SOME SHOW CLAY. ROCK HAS DULL SOUND UNDER HAMMER BLOWS AND SHOWS SIGNIFICANT LOSS OF STRENGTH AS COMPARED WITH FRESH ROCK. MODERATELY SEVERE (MOD. SEV.) ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. IN GRANITOID ROCKS, ALL FELDSPARS DULL AND DISCOLORED AND A MAJORITY SHOW KAOLINIZATION. ROCK SHOWS SEVERE LOSS OF STRENGTH AND CAN BE EXCAVATED WITH A GEOLOGIST'S PICK. ROCK GIVES "CLUNK" SOUND WHEN STRUCK. <i>IF TESTED, WOULD YIELD SPT REFUSAL.</i> SEVERE (SEV.) ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. ROCK FABRIC CLEAR AND EVIDENT BUT REDUCED IN STRENGTH TO STRONG SOIL. IN GRANITOID ROCKS ALL FELDSPARS ARE KAOLINIZED TO SOME EXTENT. SOME FRAGMENTS OF STRONG ROCK USUALLY REMAIN. <i>IF TESTED, WOULD YIELD SPT N VALUES > 100 BPF.</i> VERY SEVERE (V SEV.) ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. ROCK FABRIC ELEMENTS ARE DISCERNIBLE BUT THE MASS IS EFFECTIVELY REDUCED TO SOIL STATUS, WITH ONLY FRAGMENTS OF STRONG ROCK REMAINING. SAPROLITE IS AN EXAMPLE OF ROCK WEATHERED TO A DEGREE SUCH THAT ONLY MINOR VESTIGES OF THE ORIGINAL ROCK FABRIC REMAIN. <i>IF TESTED, WOULD YIELD SPT N VALUES < 100 BPF.</i> COMPLETE ROCK REDUCED TO SOIL. ROCK FABRIC NOT DISCERNIBLE, OR DISCERNIBLE ONLY IN SMALL AND SCATTERED CONCENTRATIONS. QUARTZ MAY BE PRESENT AS DIKES OR STRINGERS. SAPROLITE IS ALSO AN EXAMPLE.		SLIGHTLY COMPRESSIBLE LIQUID LIMIT LESS THAN 31 MODERATELY COMPRESSIBLE LIQUID LIMIT EQUAL TO 31-50 HIGHLY COMPRESSIBLE LIQUID LIMIT GREATER THAN 50		VERY HARD CANNOT BE SCRATCHED BY KNIFE OR SHARP PICK. BREAKING OF HAND SPECIMENS REQUIRES SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK. HARD CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY. HARD HAMMER BLOWS REQUIRED TO DETACH HAND SPECIMEN. MODERATELY HARD CAN BE SCRATCHED BY KNIFE OR PICK. GOUGES OR GROOVES TO 0.25 INCHES DEEP CAN BE EXCAVATED BY HARD BLOW OF A GEOLOGIST'S PICK. HAND SPECIMENS CAN BE DETACHED BY MODERATE BLOWS. MEDIUM HARD CAN BE GROOVED OR GOUGED 0.05 INCHES DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT. CAN BE EXCAVATED IN SMALL CHIPS TO PIECES 1 INCH MAXIMUM SIZE BY HARD BLOWS OF THE POINT OF A GEOLOGIST'S PICK. SOFT CAN BE GROOVED OR GOUGED READILY BY KNIFE OR PICK. CAN BE EXCAVATED IN FRAGMENTS FROM CHIPS TO SEVERAL INCHES IN SIZE BY MODERATE BLOWS OF A PICK POINT. SMALL, THIN PIECES CAN BE BROKEN BY FINGER PRESSURE. VERY SOFT CAN BE CARVED WITH KNIFE. CAN BE EXCAVATED READILY WITH POINT OF PICK. PIECES 1 INCH OR MORE IN THICKNESS CAN BE BROKEN BY FINGER PRESSURE. CAN BE SCRATCHED READILY BY FINGER NAIL.		ROCK QUALITY DESIGNATION (ROD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE. SAPROLITE (SAP.) - RESIDUAL SOIL THAT RETAINS THE RELIC STRUCTURE OR FABRIC OF THE PARENT ROCK. SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, THAT HAS BEEN EMPLACED PARALLEL TO THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS. SLICKENSIDE - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT OR SLIP PLANE. STANDARD PENETRATION TEST (PENETRATION RESISTANCE) (SPT) - NUMBER OF BLOWS (N OR BPF) OF A 140 LB. HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF 1 FOOT INTO SOIL WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT REFUSAL IS PENETRATION EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS. STRATA CORE RECOVERY (SREC) - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY TOTAL LENGTH OF STRATUM AND EXPRESSED AS A PERCENTAGE. STRATA ROCK QUALITY DESIGNATION (SRQD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS WITHIN A STRATUM EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF STRATA AND EXPRESSED AS A PERCENTAGE. TOPSOIL (TS.) - SURFACE SOILS USUALLY CONTAINING ORGANIC MATTER.	
PERCENTAGE OF MATERIAL		GROUND WATER		ROCK HARDNESS		BEDDING					
ORGANIC MATERIAL GRANULAR SILT - CLAY OTHER MATERIAL		WATER LEVEL IN BORE HOLE IMMEDIATELY AFTER DRILLING STATIC WATER LEVEL AFTER 24 HOURS PERCHED WATER, SATURATED ZONE, OR WATER BEARING STRATA SPRING OR SEEP		VERY HARD CANNOT BE SCRATCHED BY KNIFE OR SHARP PICK. BREAKING OF HAND SPECIMENS REQUIRES SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK. HARD CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY. HARD HAMMER BLOWS REQUIRED TO DETACH HAND SPECIMEN. MODERATELY HARD CAN BE SCRATCHED BY KNIFE OR PICK. GOUGES OR GROOVES TO 0.25 INCHES DEEP CAN BE EXCAVATED BY HARD BLOW OF A GEOLOGIST'S PICK. HAND SPECIMENS CAN BE DETACHED BY MODERATE BLOWS. MEDIUM HARD CAN BE GROOVED OR GOUGED 0.05 INCHES DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT. CAN BE EXCAVATED IN SMALL CHIPS TO PIECES 1 INCH MAXIMUM SIZE BY HARD BLOWS OF THE POINT OF A GEOLOGIST'S PICK. SOFT CAN BE GROOVED OR GOUGED READILY BY KNIFE OR PICK. CAN BE EXCAVATED IN FRAGMENTS FROM CHIPS TO SEVERAL INCHES IN SIZE BY MODERATE BLOWS OF A PICK POINT. SMALL, THIN PIECES CAN BE BROKEN BY FINGER PRESSURE. VERY SOFT CAN BE CARVED WITH KNIFE. CAN BE EXCAVATED READILY WITH POINT OF PICK. PIECES 1 INCH OR MORE IN THICKNESS CAN BE BROKEN BY FINGER PRESSURE. CAN BE SCRATCHED READILY BY FINGER NAIL.		MISCELLANEOUS SYMBOLS		TERM SPACING		THICKNESS	
TRACE OF ORGANIC MATTER 2 - 3% LITTLE ORGANIC MATTER 3 - 5% MODERATELY ORGANIC 5 - 10% HIGHLY ORGANIC >10%		ROADWAY EMBANKMENT (RE) WITH SOIL DESCRIPTION SOIL SYMBOL ARTIFICIAL FILL (AF) OTHER THAN ROADWAY EMBANKMENT INFERRED SOIL BOUNDARY INFERRED ROCK LINE ALLUVIAL SOIL BOUNDARY DIP & DIP DIRECTION OF ROCK STRUCTURES SOUNDING ROD		SPT TEST BORING AUGER BORING CORE BORING MONITORING WELL PIEZOMETER INSTALLATION SLOPE INDICATOR INSTALLATION SPT N-VALUE SPT REFUSAL		VERY WIDE MORE THAN 10 FEET WIDE 3 TO 10 FEET MODERATELY CLOSE 1 TO 3 FEET CLOSE 0.16 TO 1 FEET VERY CLOSE LESS THAN 0.16 FEET		> 4 FEET 1.5 - 4 FEET 0.16 - 1.5 FEET 0.03 - 0.16 FEET 0.003 - 0.03 FEET < 0.003 FEET			
CONSISTENCY OR DENSENESS		ABBREVIATIONS		INDURATION		BENCH MARK:					
PRIMARY SOIL TYPE COMPACTNESS OR CONSISTENCY RANGE OF STANDARD PENETRATION RESISTANCE (N-VALUE) RANGE OF UNCONFINED COMPRESSIVE STRENGTH (TONS/FT ²)		AR - AUGER REFUSAL BT - BORING TERMINATED CL - CLAY CPT - CONE PENETRATION TEST CSE - COARSE DMT - DILATOMETER TEST DPT - DYNAMIC PENETRATION TEST F - FINE FOSS. - FOSSILIFEROUS FRAC. - FRACTURED, FRACTURES FRAGS. - FRAGMENTS HI. - HIGHLY MED. - MEDIUM MICA. - MICAEOUS MOD. - MODERATELY NP - NON PLASTIC ORG. - ORGANIC PMT - PRESSUREMETER TEST SAP. - SAPROLITIC SD. - SAND, SANDY SL. - SILT, SILTY SLI. - SLIGHTLY TCR - TRICONE REFUSAL W - MOISTURE CONTENT V - VERY VST - VANE SHEAR TEST WEA. - WEATHERED γ - UNIT WEIGHT γ _d - DRY UNIT WEIGHT		FOR SEDIMENTARY ROCKS, INDURATION IS THE HARDENING OF THE MATERIAL BY CEMENTING, HEAT, PRESSURE, ETC. FRIABLE RUBBING WITH FINGER FREES NUMEROUS GRAINS; GENTLE BLOW BY HAMMER DISINTEGRATES SAMPLE. MODERATELY INDURATED GRAINS CAN BE SEPARATED FROM SAMPLE WITH STEEL PROBE; BREAKS EASILY WHEN HIT WITH HAMMER. INDURATED GRAINS ARE DIFFICULT TO SEPARATE WITH STEEL PROBE; DIFFICULT TO BREAK WITH HAMMER. EXTREMELY INDURATED SHARP HAMMER BLOWS REQUIRED TO BREAK SAMPLE; SAMPLE BREAKS ACROSS GRAINS.		ELEVATION: FT.		NOTES:			
TEXTURE OR GRAIN SIZE		EQUIPMENT USED ON SUBJECT PROJECT		INDURATION		BENCH MARK:					
U.S. STD. SIEVE SIZE OPENING (MM) 4 10 40 60 200 270 4.76 2.00 0.42 0.25 0.075 0.053		DRILL UNITS: MOBILE B- BK-51 CME-45B CME-550 PORTABLE HOIST DIEDRICH D-50 ADVANCING TOOLS: CLAY BITS 6" CONTINUOUS FLIGHT AUGER 8" HOLLOW AUGERS HARD FACED FINGER BITS TUNG-CARBIDE INSERTS CASING w/ ADVANCER TRICONE 2 5/16" STEEL TEETH TRICONE TUNG-CARB. CORE BIT HAMMER TYPE: AUTOMATIC MANUAL CORE SIZE: B N H HAND TOOLS: POST HOLE DIGGER HAND AUGER SOUNDING ROD VANE SHEAR TEST		FOR SEDIMENTARY ROCKS, INDURATION IS THE HARDENING OF THE MATERIAL BY CEMENTING, HEAT, PRESSURE, ETC. FRIABLE RUBBING WITH FINGER FREES NUMEROUS GRAINS; GENTLE BLOW BY HAMMER DISINTEGRATES SAMPLE. MODERATELY INDURATED GRAINS CAN BE SEPARATED FROM SAMPLE WITH STEEL PROBE; BREAKS EASILY WHEN HIT WITH HAMMER. INDURATED GRAINS ARE DIFFICULT TO SEPARATE WITH STEEL PROBE; DIFFICULT TO BREAK WITH HAMMER. EXTREMELY INDURATED SHARP HAMMER BLOWS REQUIRED TO BREAK SAMPLE; SAMPLE BREAKS ACROSS GRAINS.		ELEVATION: FT.		NOTES:			
SOIL MOISTURE - CORRELATION OF TERMS		EQUIPMENT USED ON SUBJECT PROJECT		INDURATION		BENCH MARK:					
SOIL MOISTURE SCALE (ATTERBERG LIMITS) FIELD MOISTURE DESCRIPTION GUIDE FOR FIELD MOISTURE DESCRIPTION		DRILL UNITS: MOBILE B- BK-51 CME-45B CME-550 PORTABLE HOIST DIEDRICH D-50 ADVANCING TOOLS: CLAY BITS 6" CONTINUOUS FLIGHT AUGER 8" HOLLOW AUGERS HARD FACED FINGER BITS TUNG-CARBIDE INSERTS CASING w/ ADVANCER TRICONE 2 5/16" STEEL TEETH TRICONE TUNG-CARB. CORE BIT HAMMER TYPE: AUTOMATIC MANUAL CORE SIZE: B N H HAND TOOLS: POST HOLE DIGGER HAND AUGER SOUNDING ROD VANE SHEAR TEST		FOR SEDIMENTARY ROCKS, INDURATION IS THE HARDENING OF THE MATERIAL BY CEMENTING, HEAT, PRESSURE, ETC. FRIABLE RUBBING WITH FINGER FREES NUMEROUS GRAINS; GENTLE BLOW BY HAMMER DISINTEGRATES SAMPLE. MODERATELY INDURATED GRAINS CAN BE SEPARATED FROM SAMPLE WITH STEEL PROBE; BREAKS EASILY WHEN HIT WITH HAMMER. INDURATED GRAINS ARE DIFFICULT TO SEPARATE WITH STEEL PROBE; DIFFICULT TO BREAK WITH HAMMER. EXTREMELY INDURATED SHARP HAMMER BLOWS REQUIRED TO BREAK SAMPLE; SAMPLE BREAKS ACROSS GRAINS.		ELEVATION: FT.		NOTES:			
PLASTICITY		EQUIPMENT USED ON SUBJECT PROJECT		INDURATION		BENCH MARK:					
NONPLASTIC LOW PLASTICITY MED. PLASTICITY HIGH PLASTICITY		DRILL UNITS: MOBILE B- BK-51 CME-45B CME-550 PORTABLE HOIST DIEDRICH D-50 ADVANCING TOOLS: CLAY BITS 6" CONTINUOUS FLIGHT AUGER 8" HOLLOW AUGERS HARD FACED FINGER BITS TUNG-CARBIDE INSERTS CASING w/ ADVANCER TRICONE 2 5/16" STEEL TEETH TRICONE TUNG-CARB. CORE BIT HAMMER TYPE: AUTOMATIC MANUAL CORE SIZE: B N H HAND TOOLS: POST HOLE DIGGER HAND AUGER SOUNDING ROD VANE SHEAR TEST		FOR SEDIMENTARY ROCKS, INDURATION IS THE HARDENING OF THE MATERIAL BY CEMENTING, HEAT, PRESSURE, ETC. FRIABLE RUBBING WITH FINGER FREES NUMEROUS GRAINS; GENTLE BLOW BY HAMMER DISINTEGRATES SAMPLE. MODERATELY INDURATED GRAINS CAN BE SEPARATED FROM SAMPLE WITH STEEL PROBE; BREAKS EASILY WHEN HIT WITH HAMMER. INDURATED GRAINS ARE DIFFICULT TO SEPARATE WITH STEEL PROBE; DIFFICULT TO BREAK WITH HAMMER. EXTREMELY INDURATED SHARP HAMMER BLOWS REQUIRED TO BREAK SAMPLE; SAMPLE BREAKS ACROSS GRAINS.		ELEVATION: FT.		NOTES:			
COLOR		EQUIPMENT USED ON SUBJECT PROJECT		INDURATION		BENCH MARK:					
DESCRIPTIONS MAY INCLUDE COLOR OR COLOR COMBINATIONS (TAN, RED, YELLOW-BROWN, BLUE-GRAY). MODIFIERS SUCH AS LIGHT, DARK, STREAKED, ETC. ARE USED TO DESCRIBE APPEARANCE.		DRILL UNITS: MOBILE B- BK-51 CME-45B CME-550 PORTABLE HOIST DIEDRICH D-50 ADVANCING TOOLS: CLAY BITS 6" CONTINUOUS FLIGHT AUGER 8" HOLLOW AUGERS HARD FACED FINGER BITS TUNG-CARBIDE INSERTS CASING w/ ADVANCER TRICONE 2 5/16" STEEL TEETH TRICONE TUNG-CARB. CORE BIT HAMMER TYPE: AUTOMATIC MANUAL CORE SIZE: B N H HAND TOOLS: POST HOLE DIGGER HAND AUGER SOUNDING ROD VANE SHEAR TEST		FOR SEDIMENTARY ROCKS, INDURATION IS THE HARDENING OF THE MATERIAL BY CEMENTING, HEAT, PRESSURE, ETC. FRIABLE RUBBING WITH FINGER FREES NUMEROUS GRAINS; GENTLE BLOW BY HAMMER DISINTEGRATES SAMPLE. MODERATELY INDURATED GRAINS CAN BE SEPARATED FROM SAMPLE WITH STEEL PROBE; BREAKS EASILY WHEN HIT WITH HAMMER. INDURATED GRAINS ARE DIFFICULT TO SEPARATE WITH STEEL PROBE; DIFFICULT TO BREAK WITH HAMMER. EXTREMELY INDURATED SHARP HAMMER BLOWS REQUIRED TO BREAK SAMPLE; SAMPLE BREAKS ACROSS GRAINS.		ELEVATION: FT.		NOTES:			

See Sheet 1-A For Index of Sheets
See Sheet 1-B For Conventional Symbols

STATE OF NORTH CAROLINA

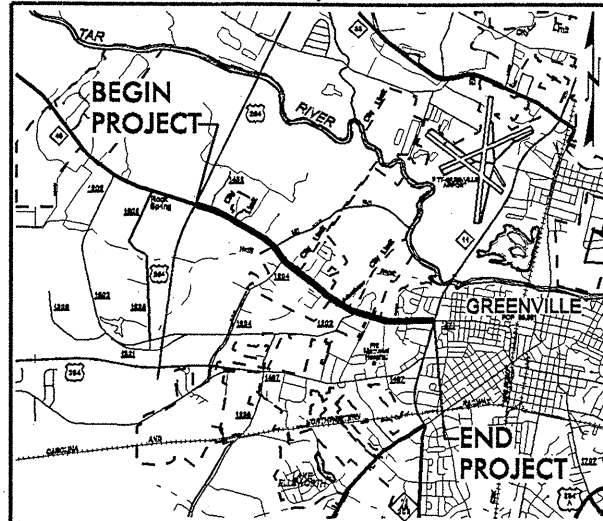
DIVISION OF HIGHWAYS

PITT COUNTY

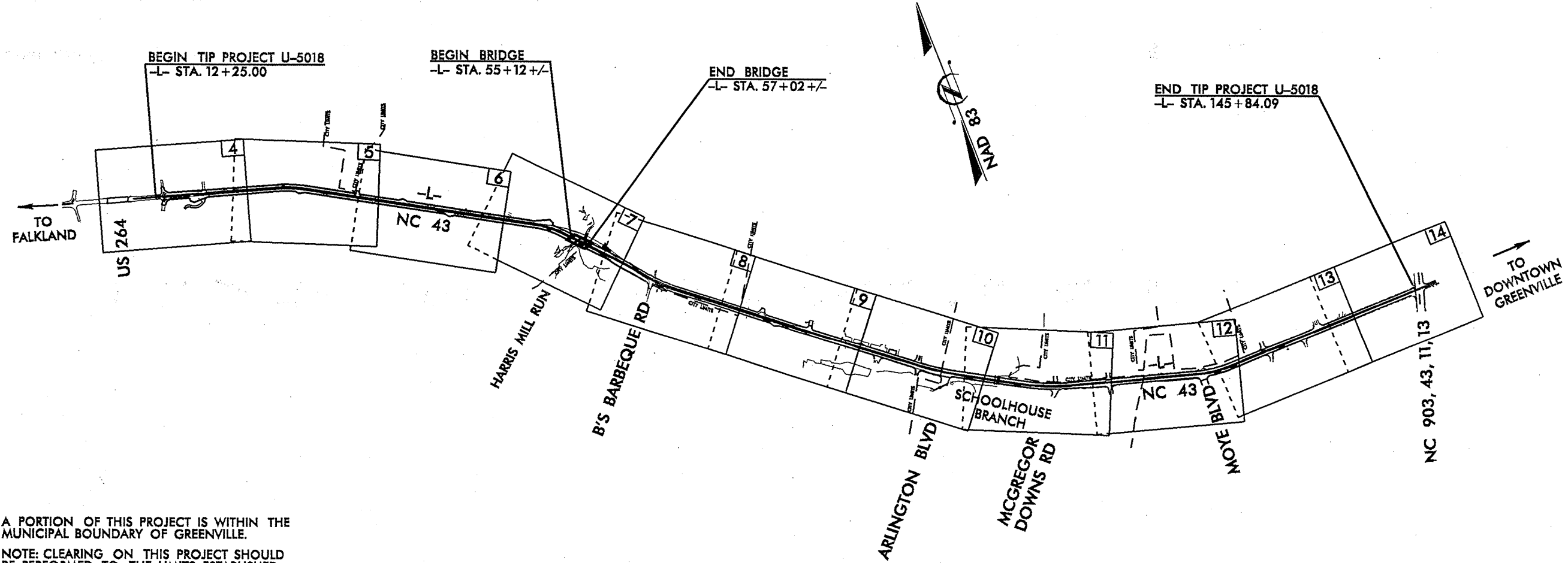
LOCATION: NC 43 FROM US 264 TO NC 11

TYPE OF WORK: GRADING, PAVING, DRAINAGE, AND STRUCTURE

STATE	STATE PROJECT APPEARANCE NO.	SHEET NO.	TOTAL SHEETS
N.C.	U-5018	2A	
WS NO.	P.A. PROJ. NO.	DESCRIPTION	
41431.1.1	STP-0043(8)	P.E.	
41431.3.1	STP-0043(8)	CONST.	



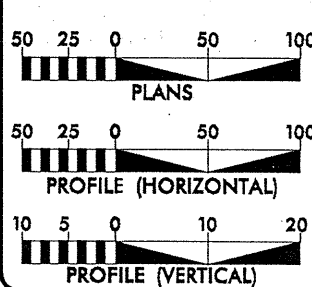
VICINITY MAP



A PORTION OF THIS PROJECT IS WITHIN THE MUNICIPAL BOUNDARY OF GREENVILLE.

NOTE: CLEARING ON THIS PROJECT SHOULD BE PERFORMED TO THE LIMITS ESTABLISHED BY METHOD III.

GRAPHIC SCALES



DESIGN DATA

ADT 2007 = 19,700
ADT 2029 = 40,600
DHV = 10 %
D = 50 %
T = 6 % *
V = 50 MPH
(* TTST 2 % + DUAL 4 %)

PROJECT LENGTH

LENGTH ROADWAY TIP PROJECT U-5018 = 2.494 MILES
LENGTH STRUCTURE TIP PROJECT U-5018 = 0.036 MILES
TOTAL LENGTH TIP PROJECT U-5018 = 2.530 MILES

Prepared In the Office of:
MULKEY
ENGINEERS & CONSULTANTS

FOR NORTH CAROLINA DEPARTMENT OF TRANSPORTATION
2006 STANDARD SPECIFICATIONS

RIGHT OF WAY DATE:
???????

LETTING DATE:
AUGUST 19, 2008

NCDOT CONTACT: JOHN ROUSE

TIM JORDAN, PE
PROJECT ENGINEER

JEFF RECK, PE
HYDRAULICS ENGINEER

HYDRAULICS ENGINEER

SIGNATURE: _____ P.E.

ROADWAY DESIGN ENGINEER

SIGNATURE: _____ P.E.

DIVISION OF HIGHWAYS
STATE OF NORTH CAROLINA



STATE HIGHWAY DESIGN ENGINEER P.E.

01-AUG-2008 09:01 i:\p01\greenville\investigation\p\us5018-geo-rdwy\cadd-geotech\planprof\us5018-geo-rdwy_title.dgn Tcb011r0ms AT GEG226166

CONTRACT: C201904 TIP PROJECT: U-5018



STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION

MICHAEL F. EASLEY
GOVERNOR

LYNDO TIPPETT
SECRETARY

August 11, 2008

STATE PROJECT: 41431.1.1 (U-5018)
F.A. PROJECT: STP-0043 (8)
COUNTY: Pitt
DESCRIPTION: NC 43 from US 264 to NC 11 (Memorial Drive)
SUBJECT: Geotechnical Inventory

Project Description

The project area lies just west of the city of Greenville along existing NC 43, beginning east of the intersection of US 264 Bypass and NC 43, (Sta. 12+25), and extending eastward approximately 2.5 miles to the intersection of NC 43 and NC 11 (Sta. 145+84.) This project consists primarily of the widening of existing NC 43 from two lanes to four, with medians and turn lanes. This will be accomplished through the addition of a new travel lane to the north and south of existing NC 43 from Sta. 12+25 to Sta. 80+00. NC 43 will be realigned to the south from Sta. 51+00 to 63+50 to accommodate the bridge construction. Additionally, slight widening and resurfacing is proposed from Sta. 80+00 to Sta. 145+84. Limited subsurface information was collected from station 80+00 to 145+84 due to the scope of proposed construction.

The geotechnical field investigation was completed from April to June of 2008. Borings were advanced with a track mounted Diedrich D-50 drill machine with an automatic hammer. Standard penetration tests were performed in selected borings. Hand auger borings were also completed. Representative soil samples were collected for visual classification in the field and for laboratory analysis by the Materials and Tests Unit.

The following alignment was investigated. Subsurface profiles and selected cross sections of this alignment are included in this report.

<u>Line</u>	<u>Station (±)</u>
-L-	12+25 to 145+84

Areas of Special Geotechnical Interest

- 1) The following sections contain cohesive soils which have the potential to cause embankment stability and/or long term settlement problems:

<u>Line</u>	<u>Station (±)</u>
-L-	13+50 to 59+75
-L-	62+75 to 65+25
-L-	67+25 to 68+75
-L-	71+25 to 73+75
-L-	77+75 to 82+00

- 2) The following sections were found to exhibit seasonal high ground water, or the potential for ground water related construction problems:

<u>Line</u>	<u>Station (±)</u>
-L-	13+75 to 18+25
-L-	20+25 to 21+00
-L-	27+50 to 37+50
-L-	39+50 to 53+00
-L-	54+50 to 57+00
-L-	65+50 to 82+00

Physiography and Geology

This project corridor is located within the Coastal Plain Physiographic Province. Topography along the project varies from nearly flat to moderately sloping and generally exhibits adequate surface drainage. Elevations ranged from 55± to 90± feet in upland areas to 20± to 55± feet in the flood plains. Surface waters from this area flow southeast into the various tributaries of Harris Mill Run, and ultimately into the Tar River.

Surficial soils in this area are generally derived from alluvial deposition and the weathering of existing formational material. Alluvial soils are restricted to areas in and around Harris Mill Run. These soils were not encountered during this roadway investigation. The upland sections are composed primarily of oxidized formational soils. These surface units are underlain by the Pliocene marine deposits of the Yorktown Formation and Cretaceous deposits of the Peedee Formation.

MAILING ADDRESS:
NC DEPARTMENT OF TRANSPORTATION
GEOTECHNICAL ENGINEERING UNIT
1589 MAIL SERVICE CENTER
RALEIGH NC 27699-1589

TELEPHONE: 919-250-4088
Fax: 919-250-4237
Website: www.ncdot.org/doh

LOCATION:
CENTURY CENTER COMPLEX
ENTRANCE B-2
1020 BIRCH RIDGE DRIVE
RALEIGH NC 27610

Ground Water

Ground water data was collected during April, May, June and August 2008, during a time of below normal precipitation. Ground water elevations ranged from 50± to 73± feet in upland areas to 18± to 50± feet in the flood plains.

Soils

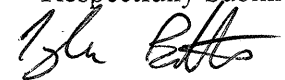
Soils within this project area have been divided into three categories, upland soils, formational soils, and roadway embankment soils.

Upland soils within this project area have been derived from weathering of the underlying formational material. These soils are characterized by various degrees of oxidation, and primarily consist of 2± or more feet of soft to stiff sandy silt and sandy clayey silt (A-4). These units typically have greater than 50 percent passing the no. 200 sieve and exhibit non-plastic to low plastic characteristics. These soils have a natural moisture content of about 34 percent. Alternating with these silt rich horizons are 1± to 8 or more feet of soft to stiff sandy clay and silty clay (A-6, A-7-6). These soils generally have more than 50 percent passing the no. 200 sieve and exhibit low to medium plastic characteristics. These cohesive units have a natural moisture content of 14 to 38 percent. Scattered within these silty and clayey units, localized sand rich horizons have been identified. Typically, these units consist of 1± to 8 or more feet of loose to medium dense sand and silty sand (A-2-4, A-3).

Formational soils encountered belong to the Pliocene age Yorktown Formation and Cretaceous age Peedee Formation. The Yorktown Formation is composed of 15 or more feet of medium dense silty sand (A-2-4), and 5 or more feet of medium dense sandy silt (A-4) layers. The Peedee Formation is composed of 15 or more feet of stiff to very stiff sandy silt and silty clay (A-4, A-7-6). The cohesive units found within this formation have greater than 50 percent passing the no. 200 sieve and exhibit low to medium plasticity indices.

Roadway Embankment material was generally encountered along existing NC 43 and consists of 1± to 7± feet of medium stiff sandy and silty clay (A-6, A-7-6) and 2± feet of loose silty sand (A-2-4).

Respectfully Submitted,



Tyler Bottoms
Engineering Geologist I

PROJECT NO. : U-5018A

COUNTY: PITT

EARTHWORK BALANCE SHEET

LOCATION	EXCAVATION				EMBANKMENT			BORROW	WASTE					
	TOTAL UNCLASS. EXCAVATION	ROCK	UNDERCUT	UNSUITABLE UNCLASS.	SUITABLE UNCLASS.	TOTAL EMBANKMENT	ROCK		EARTH EMBANKMENT	EMBANKMENT PLUS 25%	ROCK	SUITABLE	UNSUITABLE	TOTAL
-L-														
12+25.00 TO 42+00.00	13367		19209	7150	6217	3230		3230	4038			2179	26359	28538
42+00.00 TO 55+20.42	7088		8516	3833	3255	11653		11653	14566	11311			12349	12349
57+02.08 TO 62+50.00	3940		275	117	3823	5479		5479	6849	3026			392	392
TOTAL	24395		28000	11100	13295	20362		20362	25453	14337		2179	39100	41279
WASTE TO REPLACE BORROW										-2179		-2179		-2179
EST. SHOULDER MATERIAL						2535		2535	3169	3169				
ADDITIONAL UNDERCUT			2400										2400	2400
SHALLOW UNDERCUT PORTION			-150										-150	-150
PROJECT TOTAL	24395		30250	11100	13295	22897		22897	28622	15327			41350	41350
5% TO REPLACE BORROW										767				
EST. MATERIAL TO BACKFILL UNDERCUT										30650				
GRAND TOTAL	24395		30250							46744				
SAY	24700		30250							46950				

EST. DDE = 5205 CY

EST. FABRIC FOR SOIL STABILIZATION = 30,650 SY

EST. CLASS IV SELECT MATERIAL = 250 TON

EST. SHALLOW UNDERCUT = 150 TON

EMBANKMENT DOES NOT INCLUDE BACKFILL FOR UNDERCUT.

UNDERCUT SHALL BE BACKFILLED WITH BORROW EXCAVATION MEETING THE REQUIREMENT FOR SELECT GRANULAR MATERIAL CL. III

EARTHWORK QUANTITIES ARE CALCULATED BY HIGHWAY DIVISION 2. THESE EARTHWORK QUANTITIES ARE BASED IN PART ON SUBSURFACE DATA PROVIDED BY THE GEOTECHNICAL ENGINEERING UNIT.

5/28/99

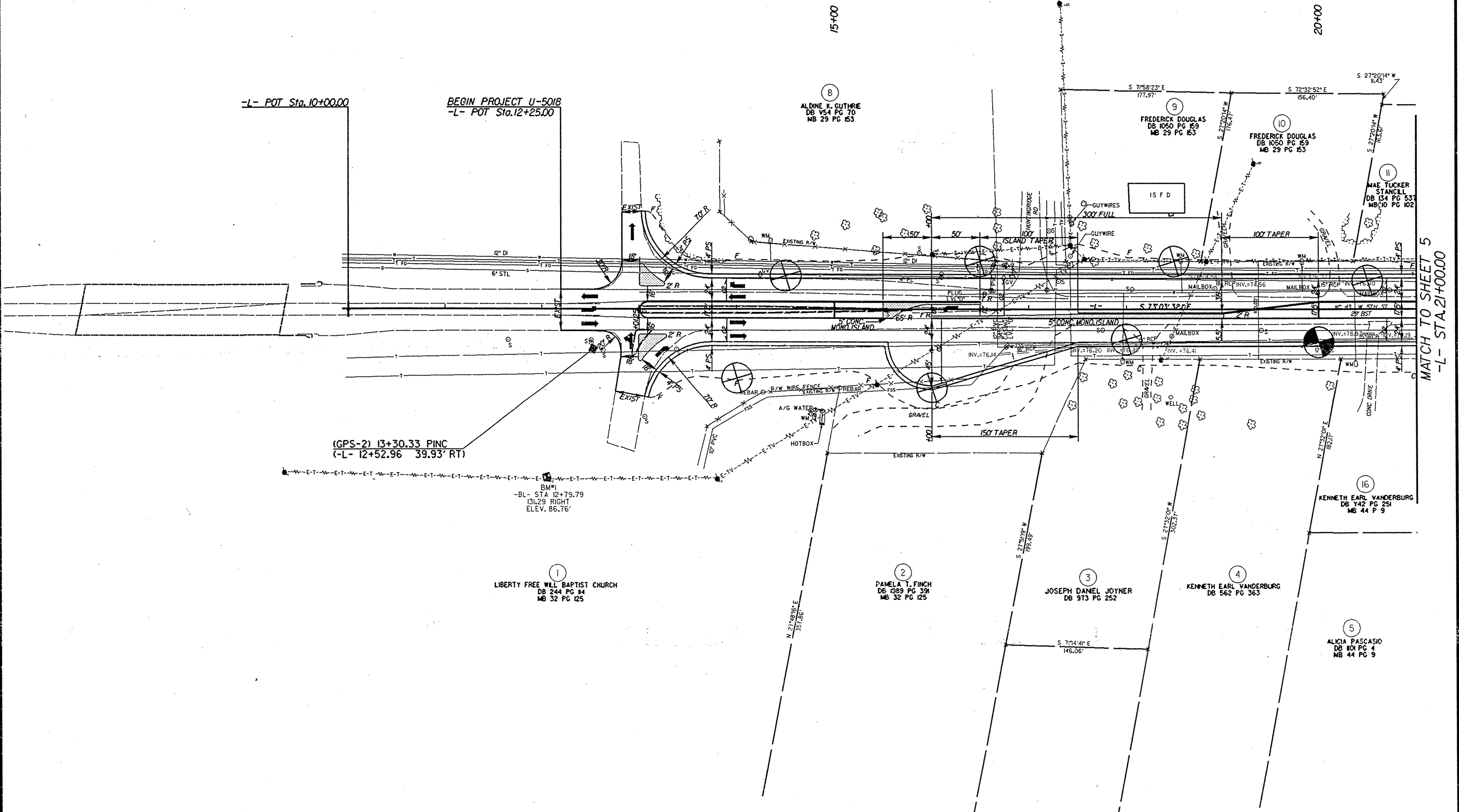
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I:\aero\greenville_investigation\sp\5018_geo...

REVISIONS

MULKEY
ENGINEERS & CONSULTANTS
P.O. BOX 16137
GREENVILLE, SC 29616
TEL: 864.681.1111
WWW.MULKEYENGINEERS.COM

PROJECT REFERENCE NO. U-5018	SHEET NO. 4
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
INCOMPLETE PLANS DO NOT USE FOR R/W ACQUISITION PRELIMINARY PLANS DO NOT USE FOR CONSTRUCTION	

FOR -L- PROFILE SEE SHEET 15



MATCH TO SHEET 5
-L- STA. 21+00.00

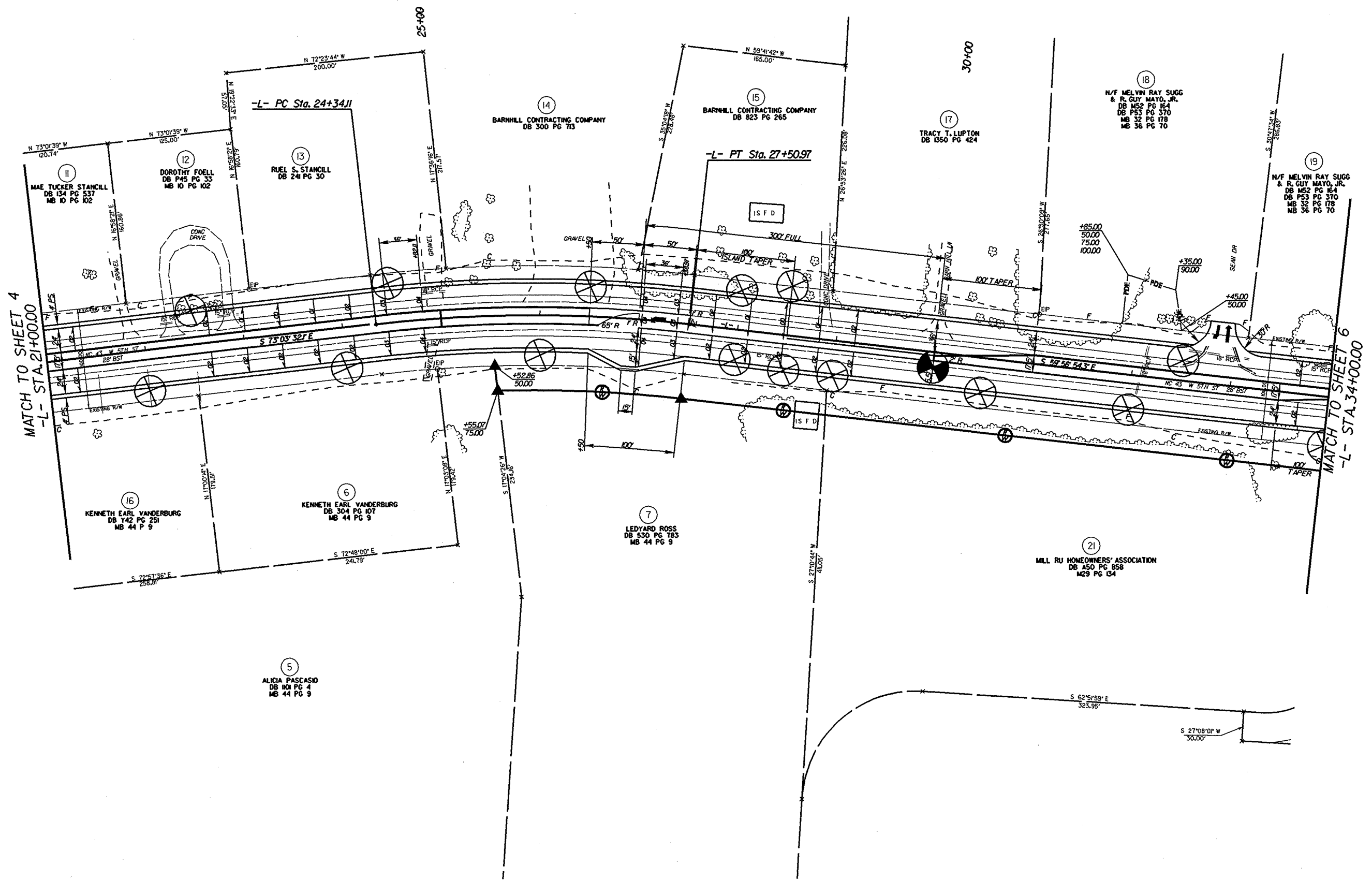
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-L-
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 $L = 316.86'$
 $T = 159.12'$
 $R = 1,384.74'$
 $SE = .04$
 $RO = 144'$



MULKEY
 ENGINEERS & CONSULTANTS
 100 BOX 18187
 KANSAS CITY, MO 64118
 (816) 451-1181 FAX
 WWW.MULKEYINC.COM

PROJECT REFERENCE NO. U-5018	SHEET NO. 5
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
INCOMPLETE PLANS DO NOT USE FOR ACQUISITION PRELIMINARY PLANS DO NOT USE FOR CONSTRUCTION	
FOR -L- PROFILE SEE SHEET 15	



MATCH TO SHEET 4
 -L- STA. 21+00.00

MATCH TO SHEET 6
 -L- STA. 34+00.00

5
 ALICIA PASCASIO
 DB 101 PG 4
 MB 44 PG 9

16
 KENNETH EARL VANDERBURG
 DB Y42 PG 251
 MB 44 P 9

6
 KENNETH EARL VANDERBURG
 DB 304 PG 107
 MB 44 PG 9

7
 LEDYARD ROSS
 DB 530 PG 783
 MB 44 PG 9

12
 DOROTHY FOELL
 DB P45 PG 33
 MB 10 PG 102

13
 RUEL S. STANCLIL
 DB 241 PG 30

14
 BARNHILL CONTRACTING COMPANY
 DB 300 PG 713

15
 BARNHILL CONTRACTING COMPANY
 DB 823 PG 265

17
 TRACY T. LUPTON
 DB 1350 PG 424

18
 N/F MELVIN RAY SUGG
 & R. GUY MAYO, JR.
 DB M52 PG 164
 DB P53 PG 370
 MB 32 PG 178
 MB 36 PG 70

19
 N/F MELVIN RAY SUGG
 & R. GUY MAYO, JR.
 DB M52 PG 164
 DB P53 PG 370
 MB 32 PG 178
 MB 36 PG 70

21
 MILL RU HOMEOWNERS' ASSOCIATION
 DB 450 PG 658
 M29 PG 134

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REVISIONS

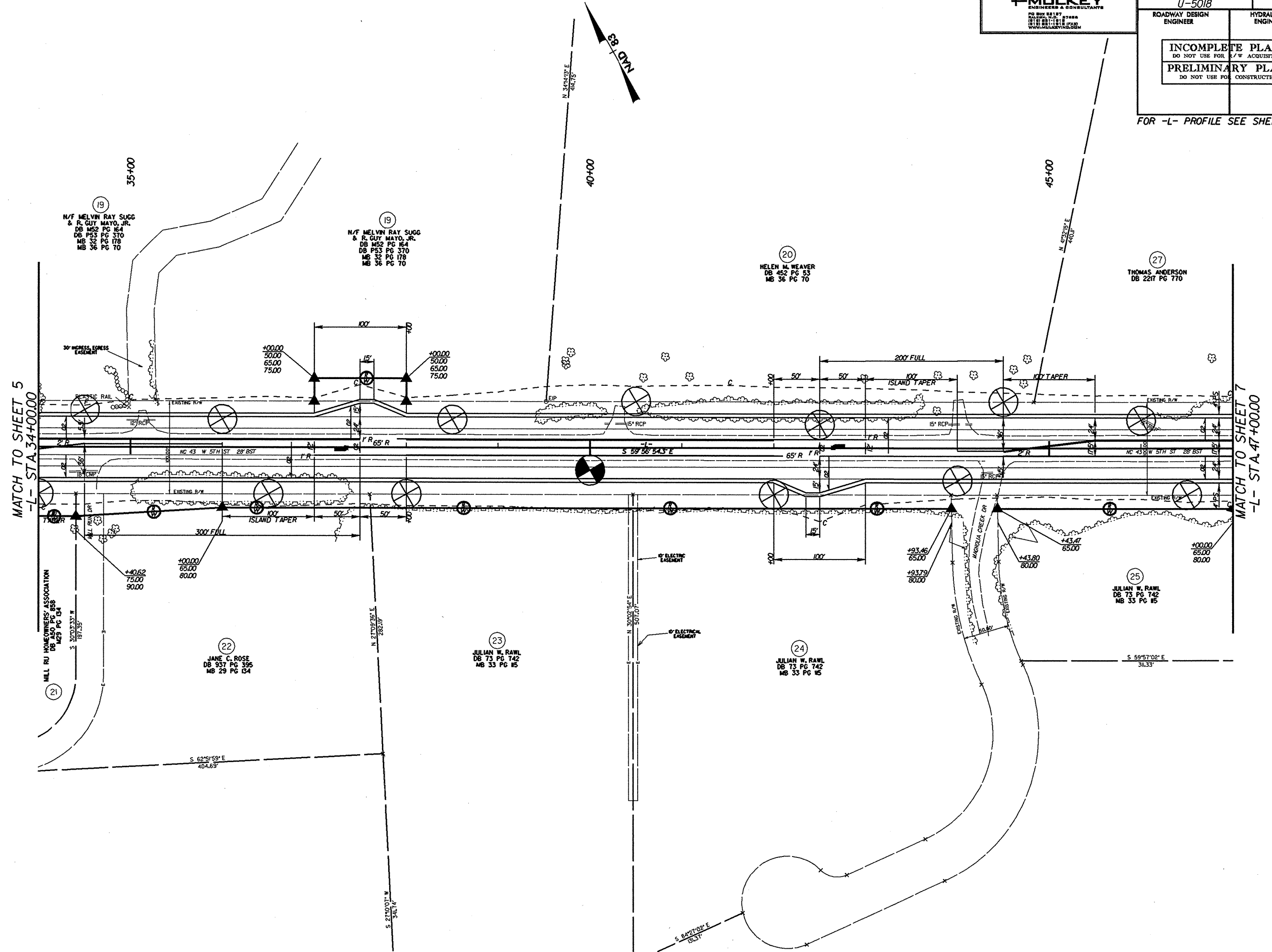


PROJECT REFERENCE NO. U-5018	SHEET NO. 6
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
INCOMPLETE PLANS DO NOT USE FOR R/W ACQUISITION	
PRELIMINARY PLANS DO NOT USE FOR CONSTRUCTION	

FOR -L- PROFILE SEE SHEET 16

MATCH TO SHEET 5
 -L- STA. 34+00.00

MATCH TO SHEET 7
 -L- STA. 47+00.00



19 N/F MELVIN RAY SUGG & R. GUY MAYO, JR.
 DB MS2 PG 164
 DS MS3 PG 370
 MB 32 PG 178
 MB 36 PG 70

19 N/F MELVIN RAY SUGG & R. GUY MAYO, JR.
 DB MS2 PG 164
 DS MS3 PG 370
 MB 32 PG 178
 MB 36 PG 70

20 HELEN M. WEAVER
 DB 452 PG 53
 MB 36 PG 70

27 THOMAS ANDERSON
 DB 2217 PG 770

21 MILL RU HOMEOWNERS' ASSOCIATION
 DB MSO PG 858
 MS29 PG 134

22 JANE C. ROSE
 DB 937 PG 395
 MB 29 PG 134

23 JULIAN W. RAWL
 DB 73 PG 742
 MB 33 PG 15

24 JULIAN W. RAWL
 DB 73 PG 742
 MB 33 PG 15

25 JULIAN W. RAWL
 DB 73 PG 742
 MB 33 PG 15

5/28/99

-L-

PI Sta 52+50.83	PI Sta 61+27.90
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$T = 142.17'$	$T = 222.17'$
$R = 950.00'$	$R = 4,940.00'$
$SE = 04$	$SE = 02$
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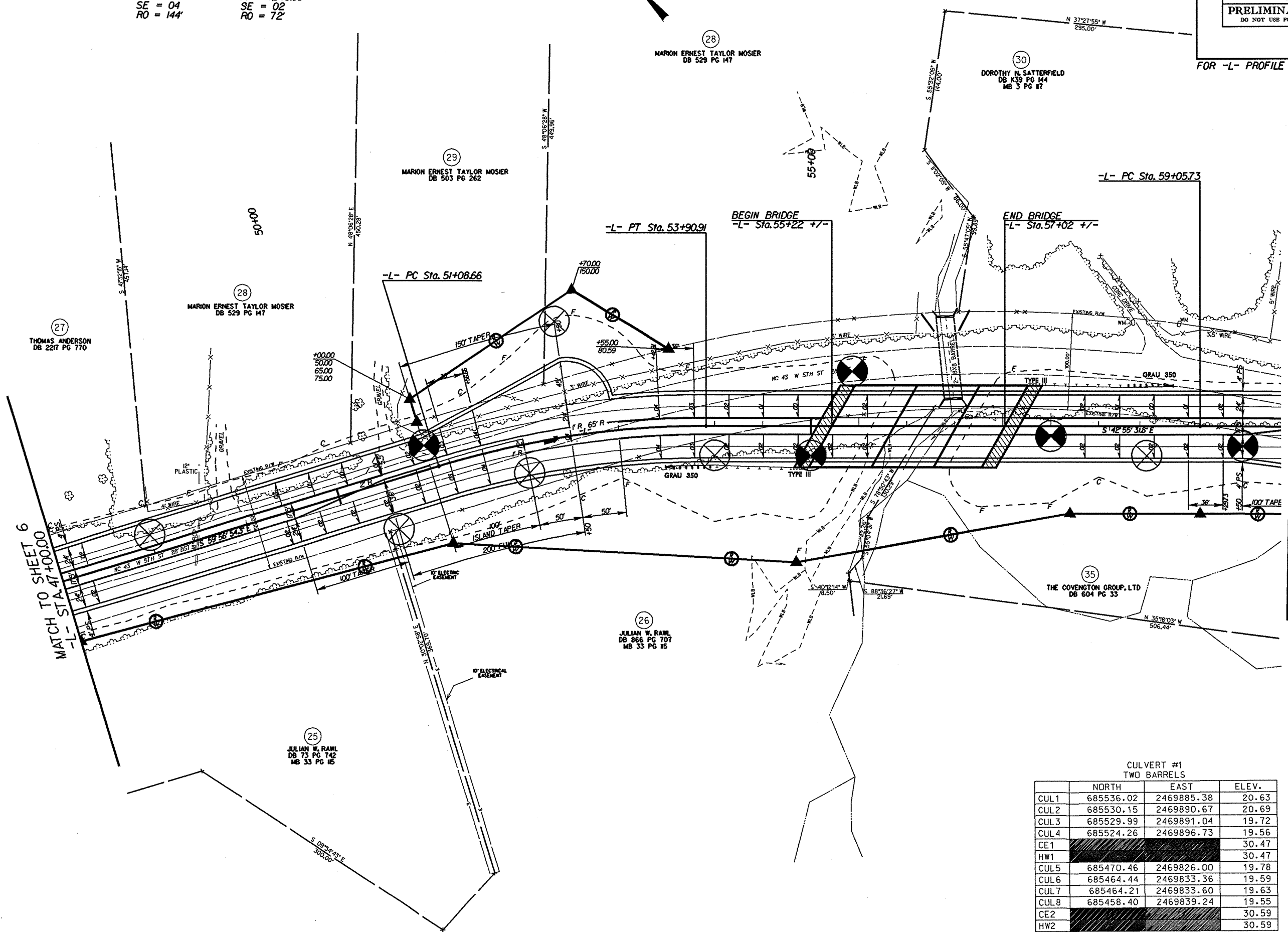
MULKEY
ENGINEERS & CONSULTANTS
PO BOX 8817
Raleigh, NC 27617
(919) 871-1111 FAX
WWW.MULKEYENGINEERS.COM

PROJECT REFERENCE NO. U-5018	SHEET NO. 7
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
INCOMPLETE PLANS DO NOT USE FOR R/W ACQUISITION PRELIMINARY PLANS DO NOT USE FOR CONSTRUCTION	

FOR -L- PROFILE SEE SHEET 16

REVISIONS

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CULVERT #1
TWO BARRELS

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CUL2	685530.15	2469890.67	20.69
CUL3	685529.99	2469891.04	19.72
CUL4	685524.26	2469896.73	19.56
CE1			30.47
HW1			30.47
CUL5	685470.46	2469826.00	19.78
CUL6	685464.44	2469833.36	19.59
CUL7	685464.21	2469833.60	19.63
CUL8	685458.40	2469839.24	19.55
CE2			30.59
HW2			30.59

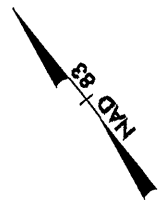
5/28/09

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REVISIONS

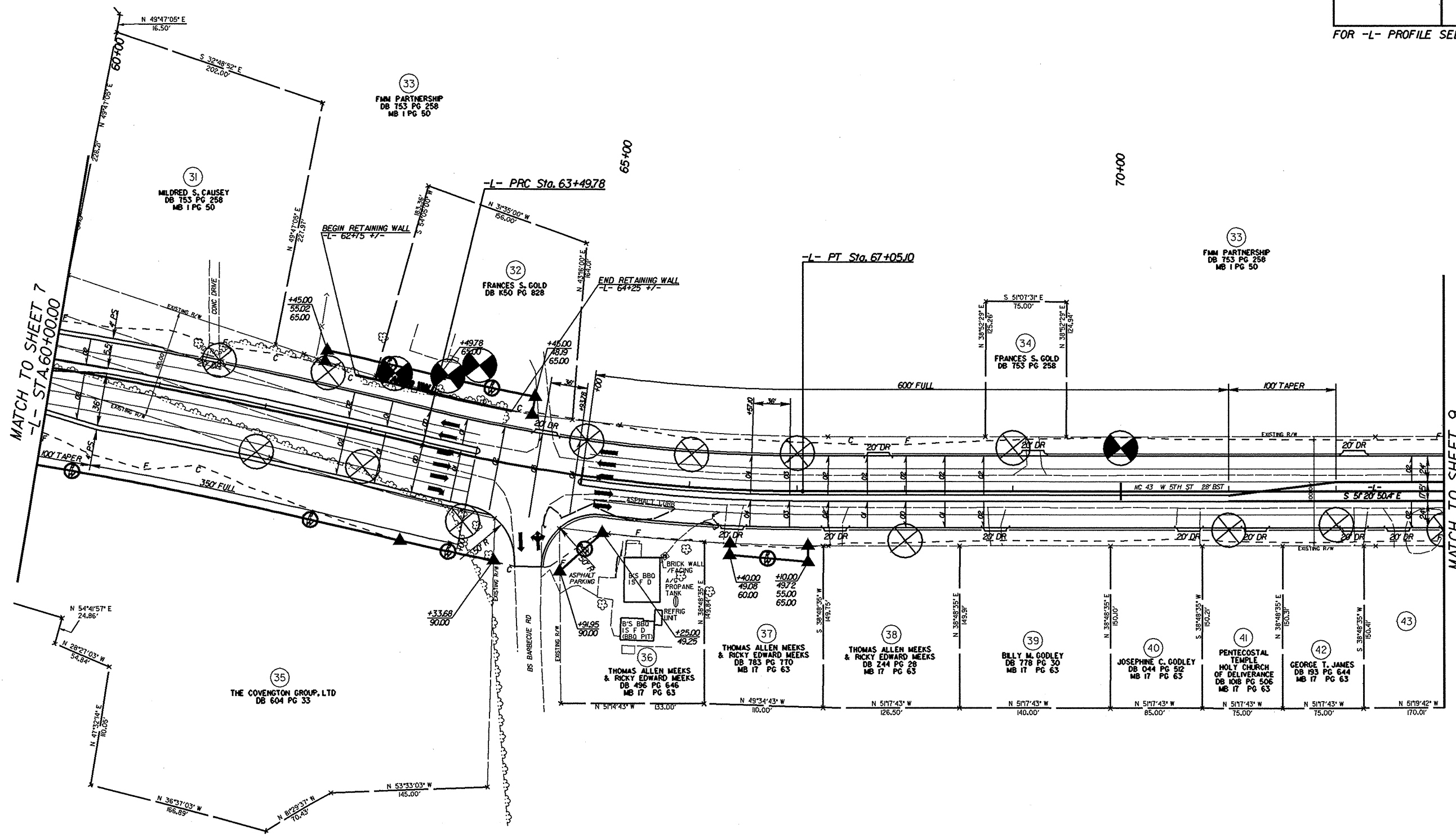
-L-

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L = 444.05'	L = 355.32'
T = 222.17'	T = 178.49'
R = 4,940.00'	R = 1,500.00'
SE = 02	SE = 04
RO = 72	RO = 144



PROJECT REFERENCE NO. U-5018	SHEET NO. 8
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
INCOMPLETE PLANS DO NOT USE FOR ACQUISITION	
PRELIMINARY PLANS DO NOT USE FOR CONSTRUCTION	

FOR -L- PROFILE SEE SHEET 17

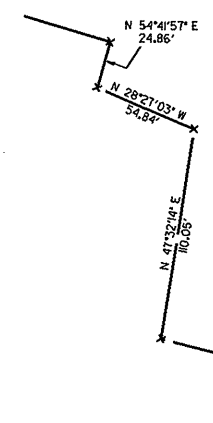


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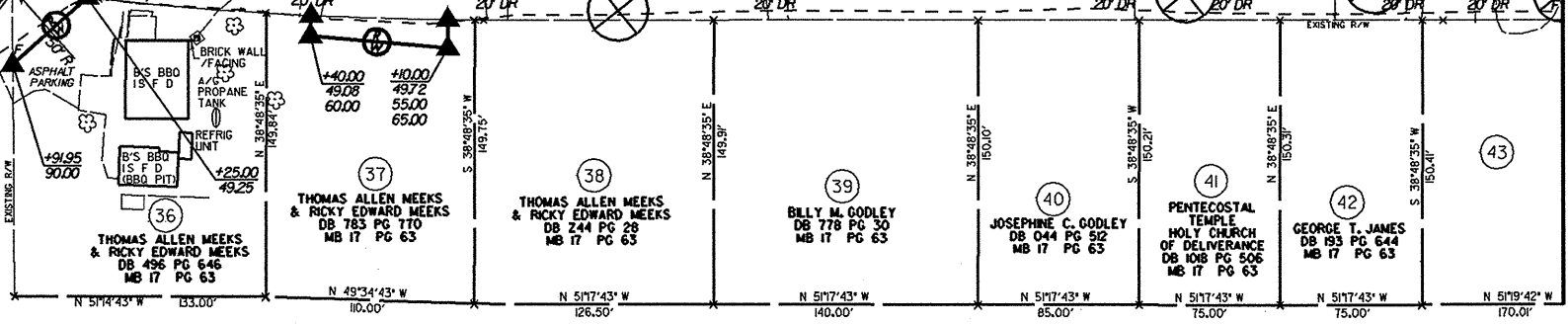
MATCH TO SHEET 9
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65+00

70+00



35
THE COVENANT GROUP, LTD
DB 604 PG 33



36
THOMAS ALLEN MEEKS
& RICKY EDWARD MEEKS
DB 496 PG 646
MB 17 PG 63

37
THOMAS ALLEN MEEKS
& RICKY EDWARD MEEKS
DB 783 PG 770
MB 17 PG 63

38
THOMAS ALLEN MEEKS
& RICKY EDWARD MEEKS
DB 244 PG 28
MB 17 PG 63

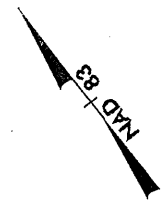
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BILLY M. GODLEY
DB 778 PG 30
MB 17 PG 63

40
JOSEPHINE C. GODLEY
DB 044 PG 512
MB 17 PG 63

41
PENTECOSTAL
TEMPLE
HOLY CHURCH
OF DELIVERANCE
DB 108 PG 506
MB 17 PG 63

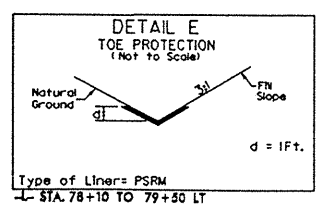
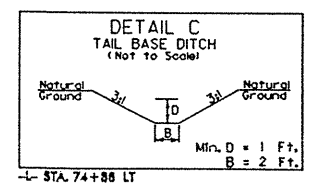
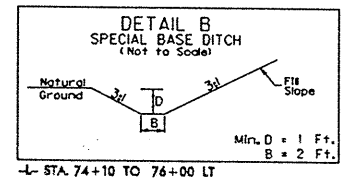
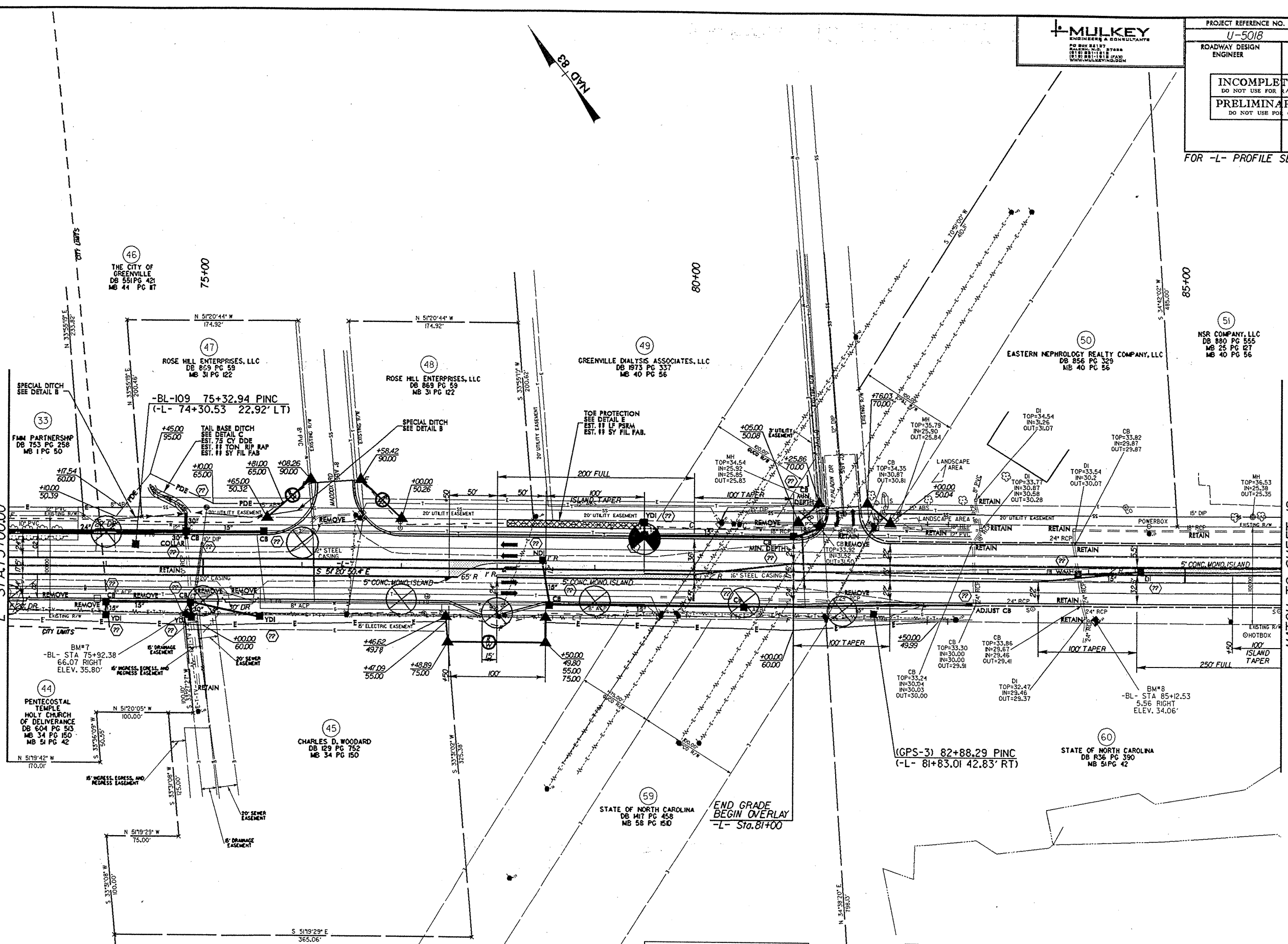
42
GEORGE T. JAMES
DB 193 PG 644
MB 17 PG 63

43



MATCH TO SHEET 8
-L- STA.73+00.00

MATCH TO SHEET 10
-L- STA.86+00.00



REVISIONS

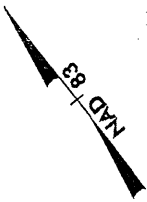
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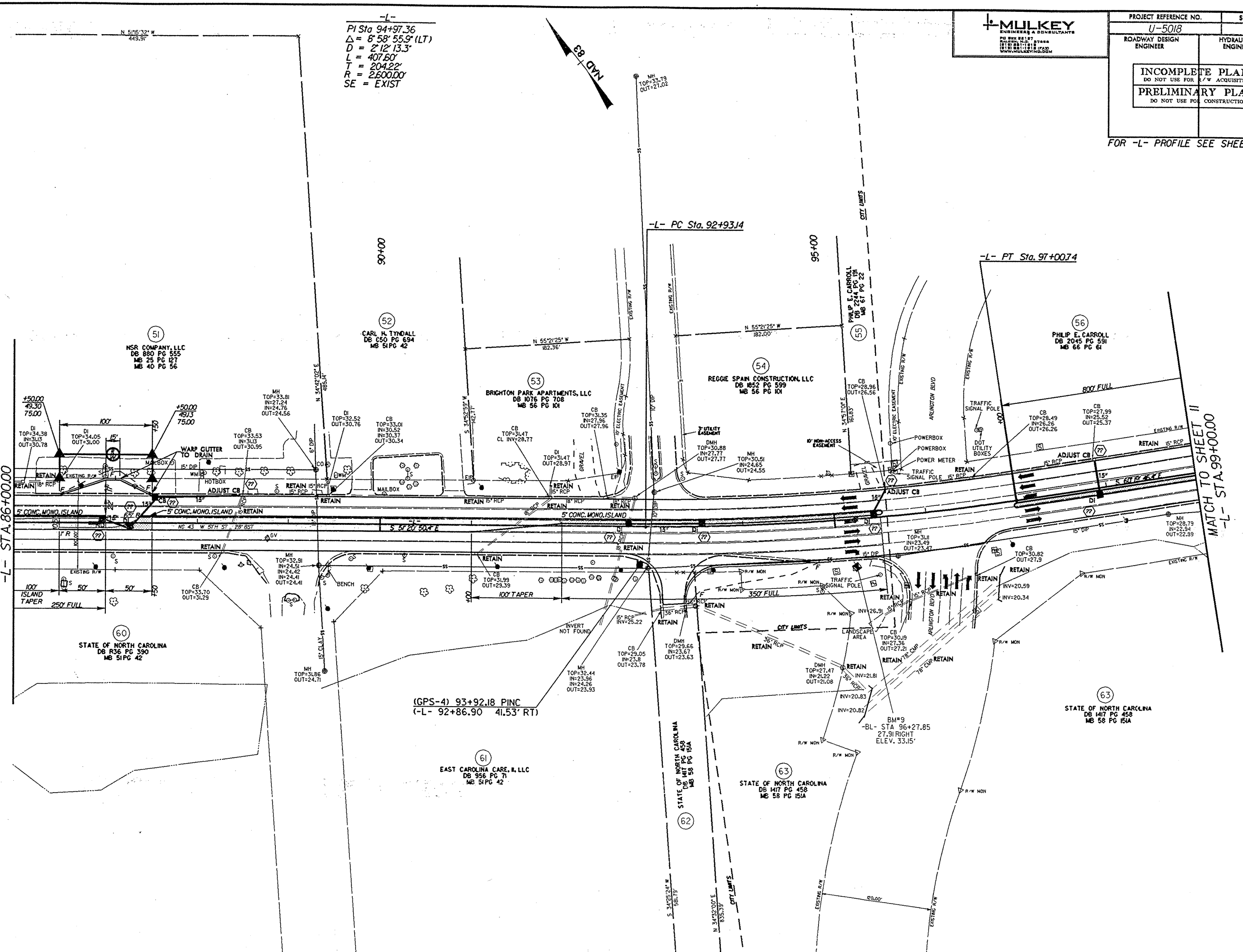
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ROADWAY DESIGN ENGINEER		HYDRAULICS ENGINEER	
INCOMPLETE PLANS DO NOT USE FOR A/C ACQUISITION PRELIMINARY PLANS DO NOT USE FOR CONSTRUCTION			
FOR -L- PROFILE SEE SHEET 18			

-L-
 PI Sta 94+97.36
 $\Delta = 8' 58" 55.9" (LT)$
 $D = 2' 12" 13.3"$
 $L = 407.60'$
 $T = 204.22'$
 $R = 2,600.00'$
 SE = EXIST



MATCH TO SHEET 9
-L- STA. 86+00.00

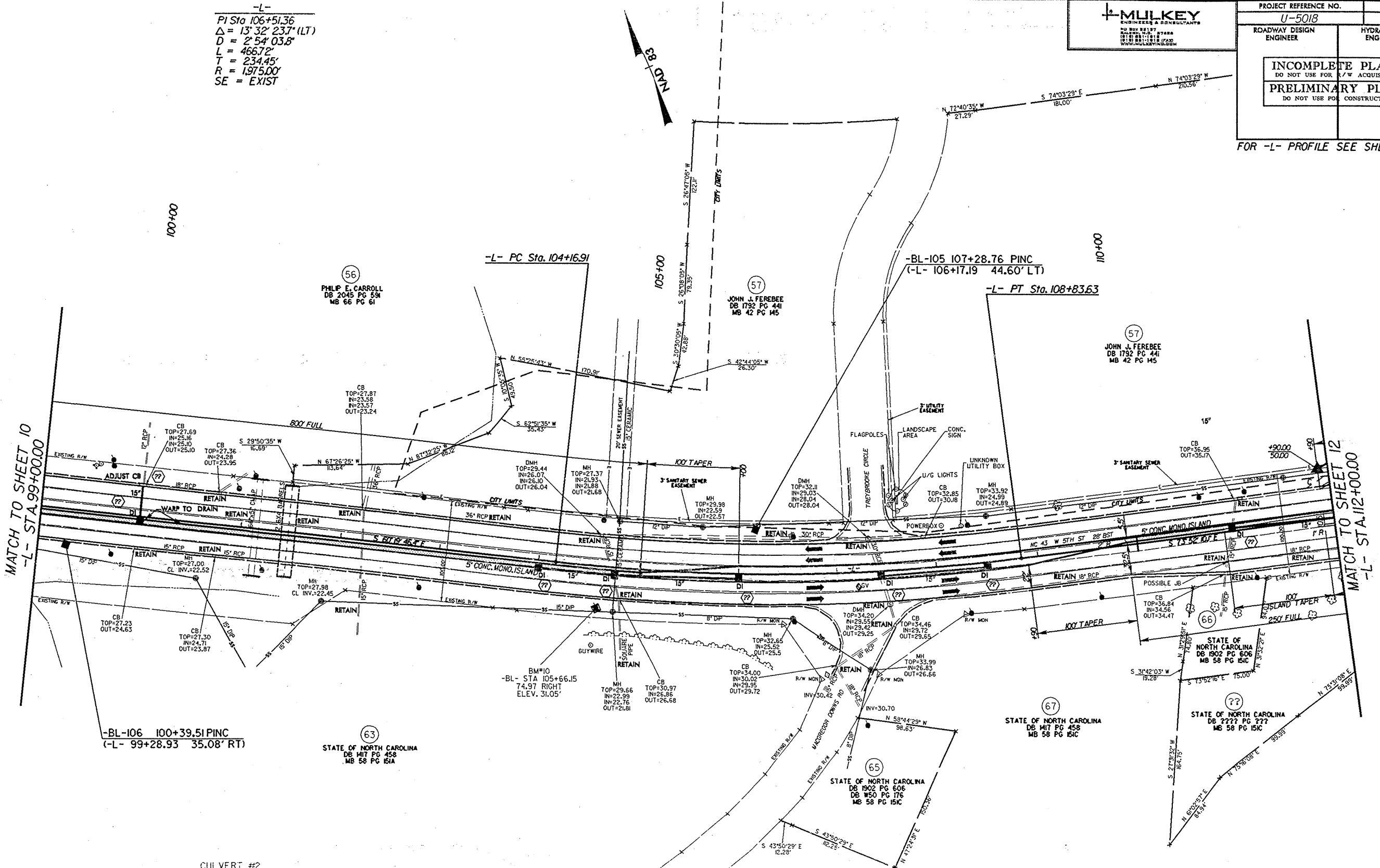
MATCH TO SHEET 11
-L- STA. 99+00.00



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 5/28/09

-L-
 PI Sta 106+51.36
 $\Delta = 13^\circ 32' 23.7''$ (LT)
 $D = 2' 54.038''$
 $L = 466.72'$
 $T = 234.45'$
 $R = 1975.00'$
 SE = EXIST



MATCH TO SHEET 10
-L- STA.99+00.00

MATCH TO SHEET 12
-L- STA.112+00.00

CULVERT #2
TWO BARRELS

	NORTH	EAST	ELEV.
CUL1	682644.09	2473287.16	20.80
CUL2	682641.55	2473292.22	20.79
CUL3	682641.10	2473292.78	20.78
CUL4	682637.95	2473297.93	20.77
CE1			24.82
HW1			26.32
CUL5	682564.13	2473242.32	21.35
CUL6	682561.20	2473247.09	21.38
CUL7	682560.65	2473247.82	21.37
CUL8	682557.95	2473252.87	21.39
CE2			25.39
HW2			26.89

REVISIONS

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5/28/99

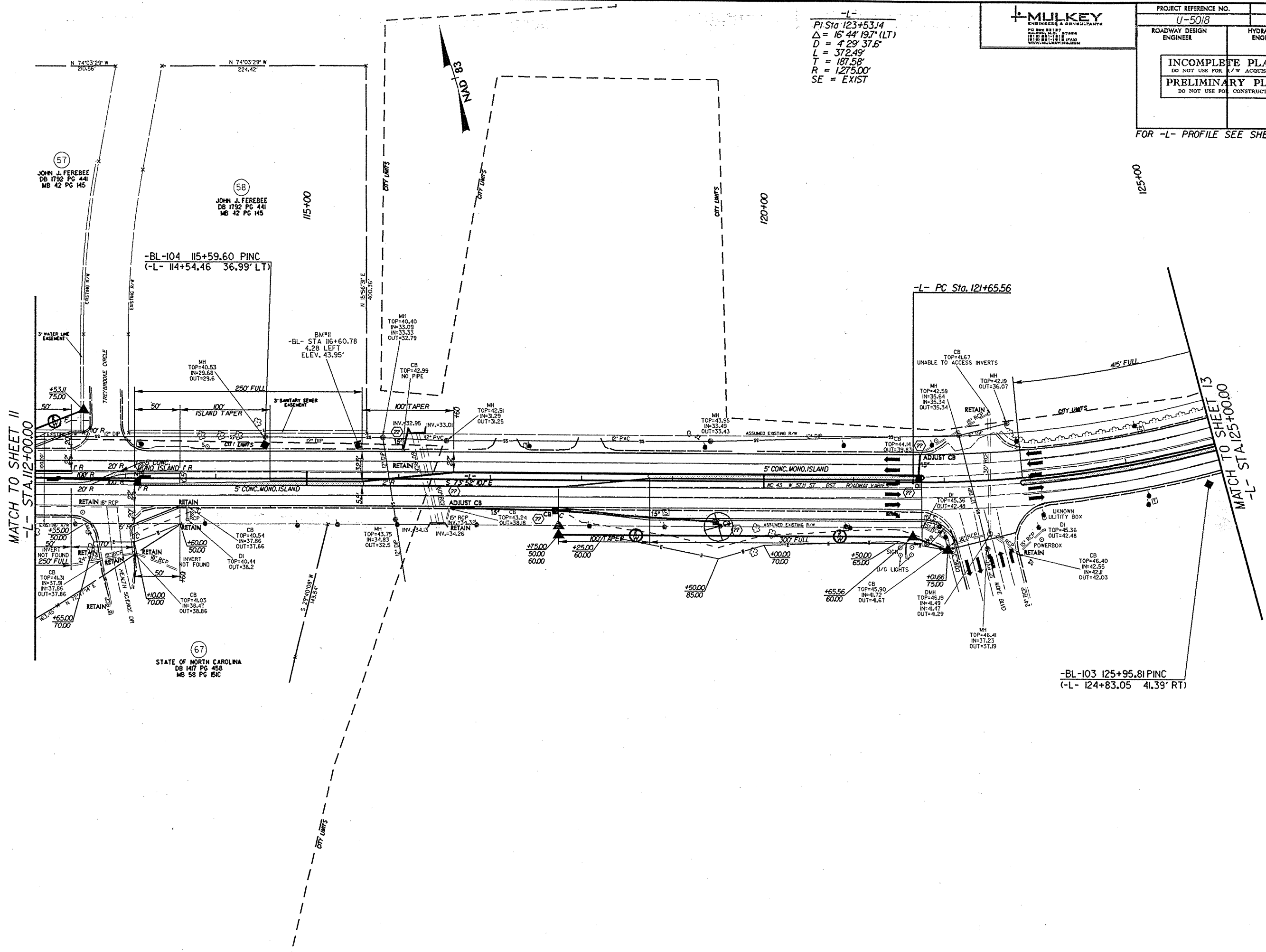
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PROJECT REFERENCE NO. U-5018	SHEET NO. 12
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
INCOMPLETE PLANS DO NOT USE FOR ACQUISITION PRELIMINARY PLANS DO NOT USE FOR CONSTRUCTION	
FOR -L- PROFILE SEE SHEET 19	

-L-
 PI Sta 123+53.14
 $\Delta = 16' 44" 19.7' (LT)$
 $D = 4' 29' 37.6"$
 $L = 372.49'$
 $T = 187.58'$
 $R = 1,275.00'$
 SE = EXIST



-BL-104 115+59.60 PINC
 (-L- 114+54.46 36.99' LT)

-L- PC Sta. 121+65.56

-BL-103 125+95.81 PINC
 (-L- 124+83.05 41.39' RT)

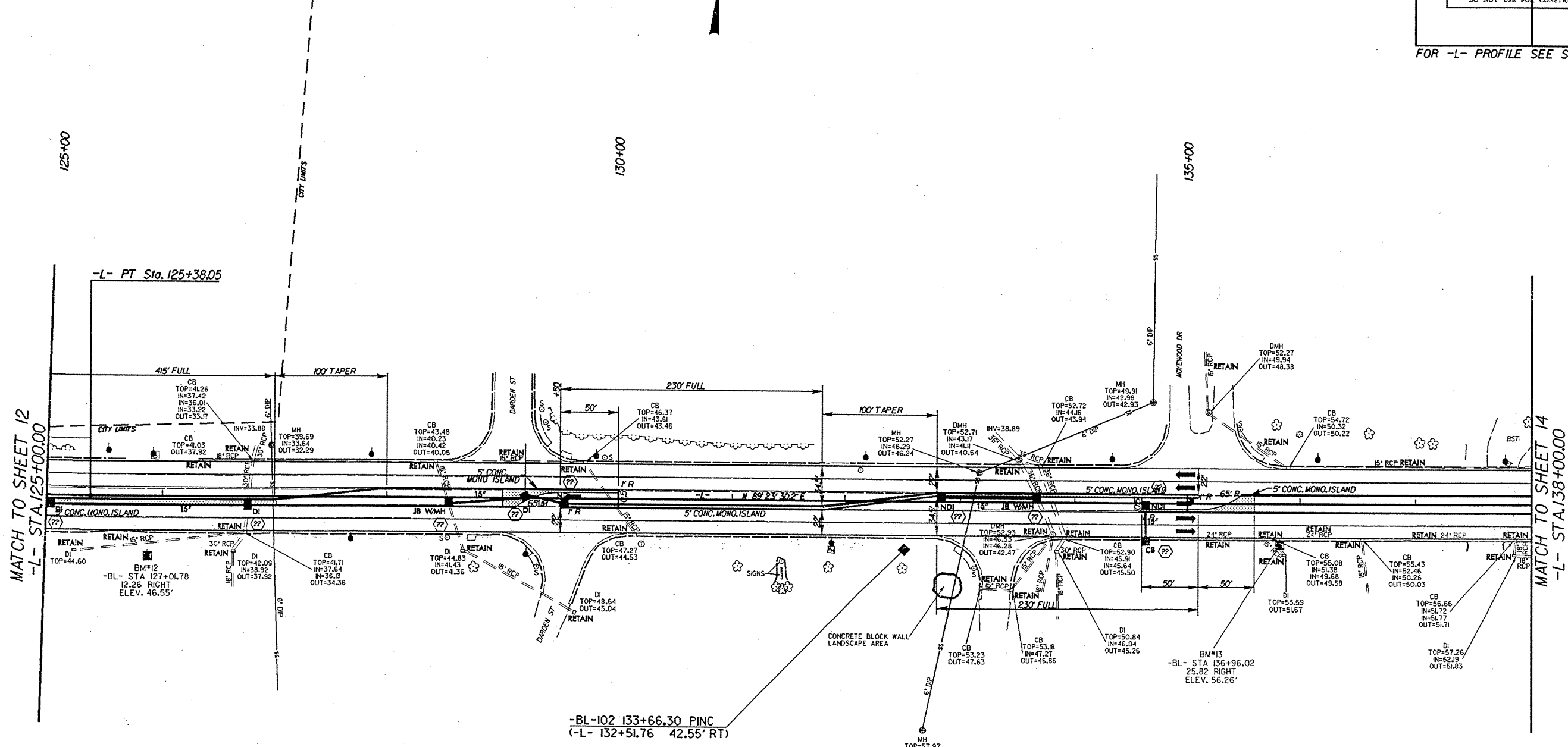
(67)
 STATE OF NORTH CAROLINA
 DB 147 PG 458
 MB 58 PG 51C



PROJECT REFERENCE NO. U-5018		SHEET NO. 13	
ROADWAY DESIGN ENGINEER		HYDRAULICS ENGINEER	
INCOMPLETE PLANS DO NOT USE FOR E/W ACQUISITION			
PRELIMINARY PLANS DO NOT USE FOR CONSTRUCTION			

FOR -L- PROFILE SEE SHEET 19

-L-
 PI Sta 123+53.14
 $\Delta = 16^\circ 44' 19.7''$ (LT)
 $D = 4' 29' 37.6''$
 $L = 372.49'$
 $T = 187.58'$
 $R = 1275.00'$
 SE = EXIST



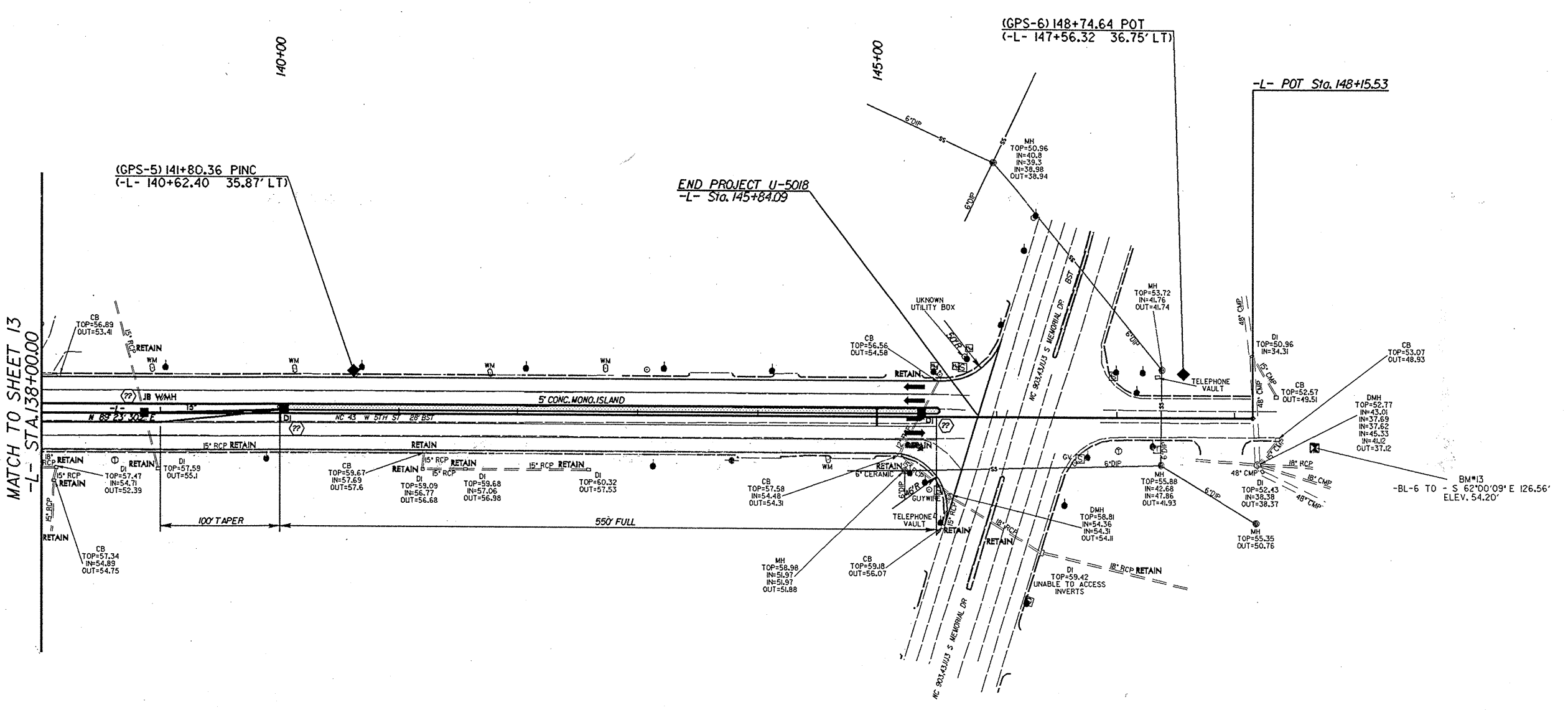
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MATCH TO SHEET 12
-L- STA 125+00.00

MATCH TO SHEET 14
-L- STA 138+00.00

PROJECT REFERENCE NO. U-5018	SHEET NO. 14
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
INCOMPLETE PLANS DO NOT USE FOR ACQUISITION	
PRELIMINARY PLANS DO NOT USE FOR CONSTRUCTION	
FOR -L- PROFILE SEE SHEET 20	

MULKEY ENGINEERS & CONSULTANTS
 40 BOX 42197
 KANSAS CITY, MO 64118
 TEL: 816-451-8700
 WWW.MULKEYINC.COM



(GPS-5) 141+80.36 P.I.N.C.
 (-L- 140+62.40 35.87' LT)

(GPS-6) 148+74.64 P.O.T.
 (-L- 147+56.32 36.75' LT)

END PROJECT U-5018
 -L- Sta. 145+84.09

-L- POT Sta. 148+15.53

MATCH TO SHEET 13
 -L- STA. 138+00.00

100' TAPER

550' FULL

BM#13
 -BL-6 TO -S 62°00'09\"/>

REVISIONS

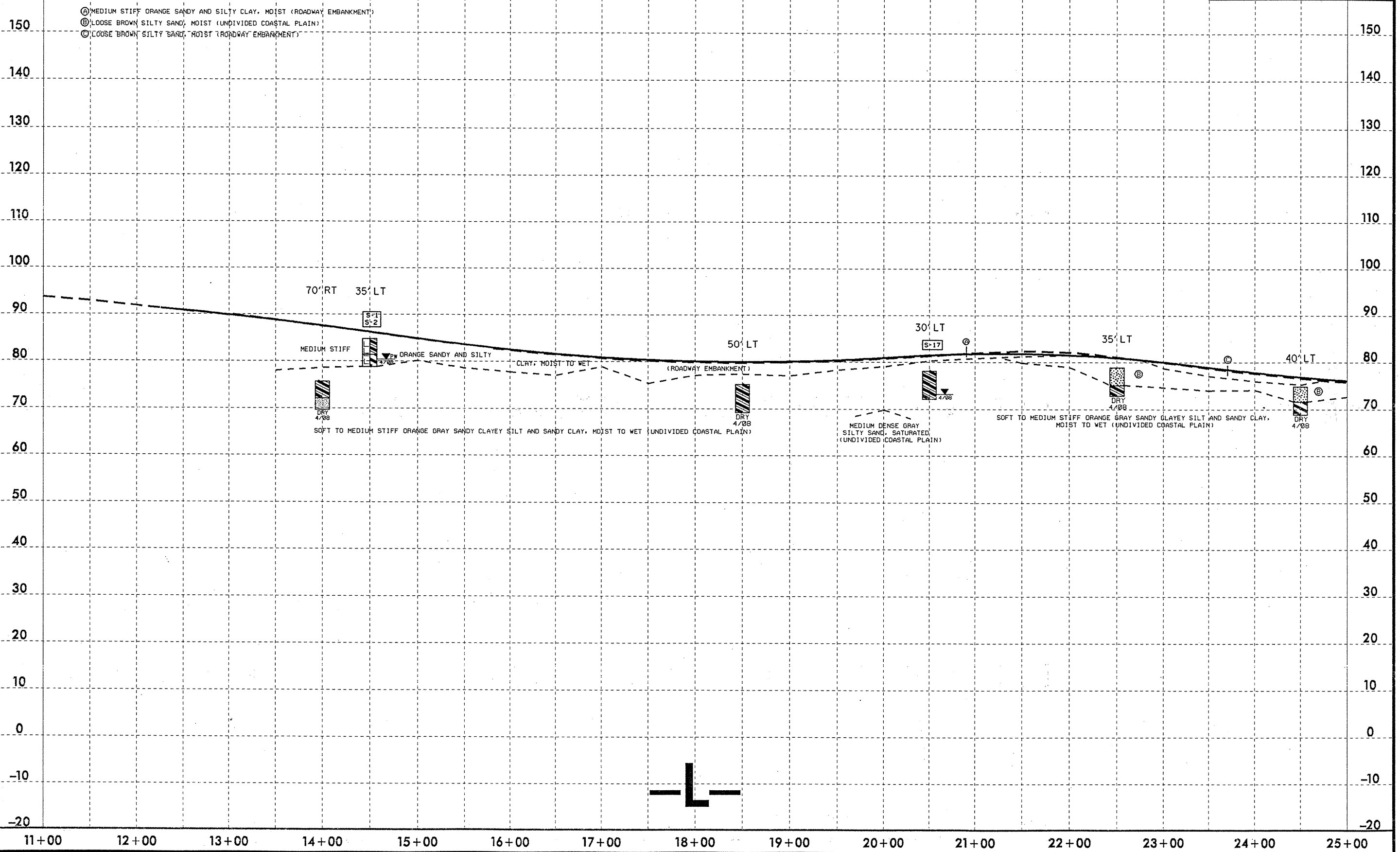
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 6/15/08 10:24:45

PROJECT REFERENCE NO.	SHEET NO.
U-5018	15
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
INCOMPLETE PLANS DO NOT USE FOR A/W ACQUISITION PRELIMINARY PLANS DO NOT USE FOR CONSTRUCTION	

SOIL TEST RESULTS															
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	ASTM CLASS	LL	PI	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C. SAND	F. SAND	SILT	CLAY	10	40	200		
S-1	35' LT	14+50.1	1.00-3.50	A-6(1)	30	12	5.1	58.1	6.1	30.7	100	98	39	18.2	-
S-2	35' LT	14+50.1	3.50-6.00	A-7-6(34)	64	43	1.2	19.0	24.7	49.1	100	96	77	-	-
S-17	30' LT	20+50.1	1.00-4.00	A-6(1)	37	11	4.9	59.5	4.9	30.7	100	98	38	-	-

- Ⓐ MEDIUM STIFF ORANGE SANDY AND SILTY CLAY, MOIST (ROADWAY EMBANKMENT)
- Ⓑ LOOSE BROWN SILTY SAND, MOIST (UNDIVIDED COASTAL PLAIN)
- Ⓒ LOOSE BROWN SILTY SAND, MOIST (ROADWAY EMBANKMENT)

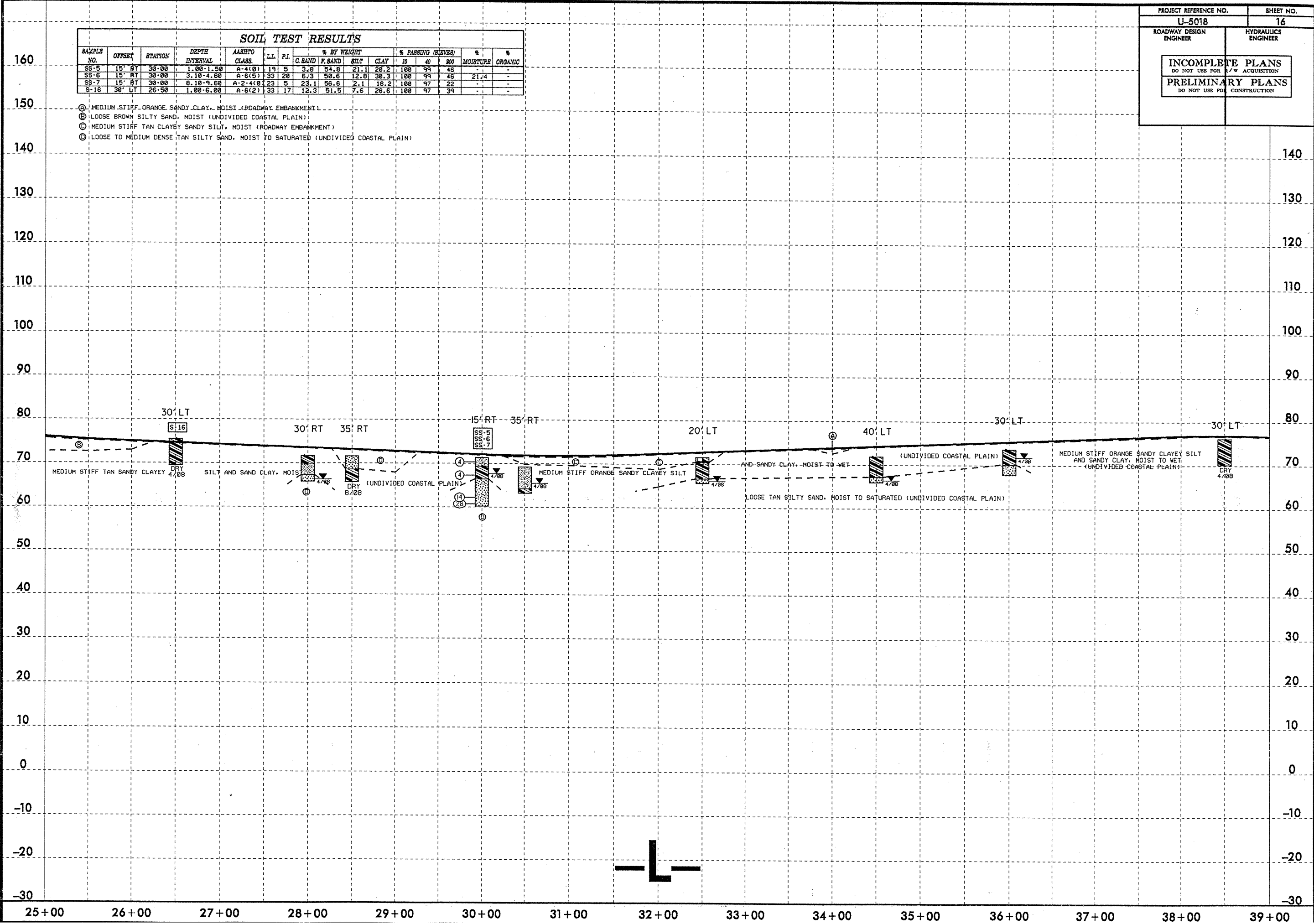


5/14/99

PROJECT REFERENCE NO.		SHEET NO.	
U-5018		16	
ROADWAY DESIGN ENGINEER		HYDRAULICS ENGINEER	
INCOMPLETE PLANS DO NOT USE FOR ACQUISITION			
PRELIMINARY PLANS DO NOT USE FOR CONSTRUCTION			

SOIL TEST RESULTS															
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	LL	P.I.	% BY WEIGHT				% PASSING (SIEVES)			MOISTURE	ORGANIC
							C. SAND	F. SAND	SILT	CLAY	10	40	200		
SS-5	15' RT	30+00	1.00-1.50	A-4(0)	19	5	3.8	54.8	21.1	20.2	100	99	46	-	-
SS-6	15' RT	30+00	3.10-4.60	A-6(5)	33	20	6.3	50.6	12.8	38.3	100	99	46	21.4	-
SS-7	15' RT	30+00	8.10-9.60	A-2-4(0)	23	5	23.1	56.6	2.1	18.2	100	97	22	-	-
S-16	30' LT	26+50	1.00-6.00	A-6(2)	33	17	12.3	51.5	7.6	28.6	100	97	39	-	-

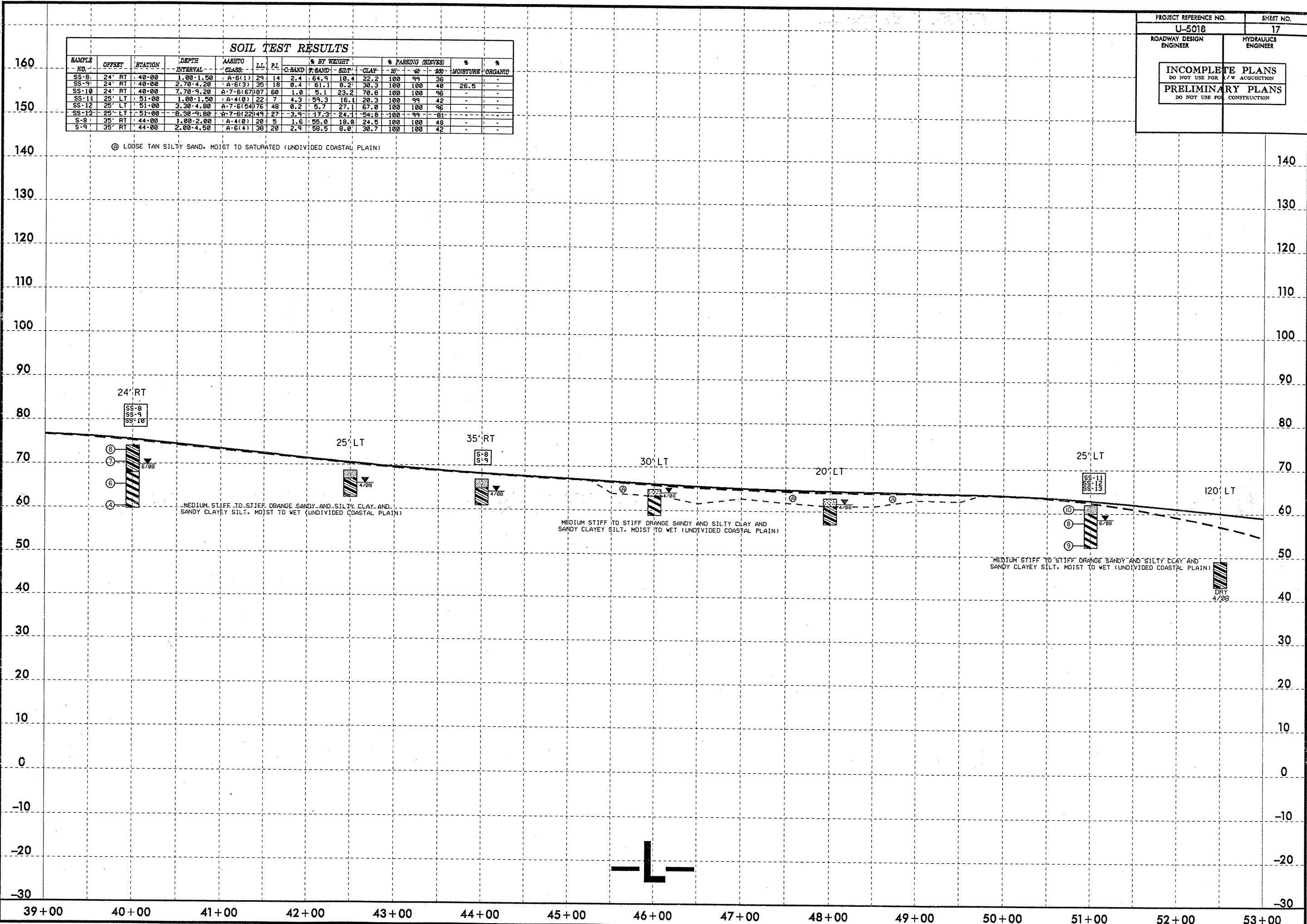
- ⊙ MEDIUM STIFF ORANGE SANDY CLAY, MOIST (ROADWAY EMBANKMENT)
- ⊙ LOOSE BROWN SILTY SAND, MOIST (UNDIVIDED COASTAL PLAIN)
- ⊙ MEDIUM STIFF TAN CLAYEY SANDY SILT, MOIST (ROADWAY EMBANKMENT)
- ⊙ LOOSE TO MEDIUM DENSE TAN SILTY SAND, MOIST TO SATURATED (UNDIVIDED COASTAL PLAIN)



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SOIL TEST RESULTS															
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	ASTM CLASS	LL	PI	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C-SAND	F-SAND	SILT	CLAY	-10	-40	-200		
SS-8	24' RT	40+00	1.00-1.50	A-6(1)	29	14	2.4	64.9	10.4	22.2	100	99	36	-	-
SS-9	24' RT	40+00	2.70-4.20	A-6(3)	36	18	0.4	61.1	8.2	30.3	100	100	48	26.5	-
SS-10	24' RT	40+00	7.70-9.20	A-7-6(67)	27	60	1.0	5.1	23.2	78.8	100	100	96	-	-
SS-11	25' LT	51+00	1.00-1.50	A-4(0)	22	7	4.3	59.3	16.1	20.3	100	99	42	-	-
SS-12	25' LT	51+00	3.30-4.80	A-7-6(54)	76	48	0.2	5.7	27.1	67.0	100	100	96	-	-
SS-13	25' LT	51+00	8.30-9.80	A-7-6(22)	49	27	3.9	17.3	24.1	54.8	100	99	81	-	-
S-8	35' RT	44+00	1.00-2.00	A-4(0)	20	5	1.6	55.0	18.8	24.5	100	100	48	-	-
S-9	35' RT	44+00	2.00-4.50	A-6(4)	38	20	2.9	58.5	8.0	30.7	100	100	42	-	-

Ⓞ LOOSE TAN SILTY SAND, MOIST TO SATURATED (UNDIVIDED COASTAL PLAIN)



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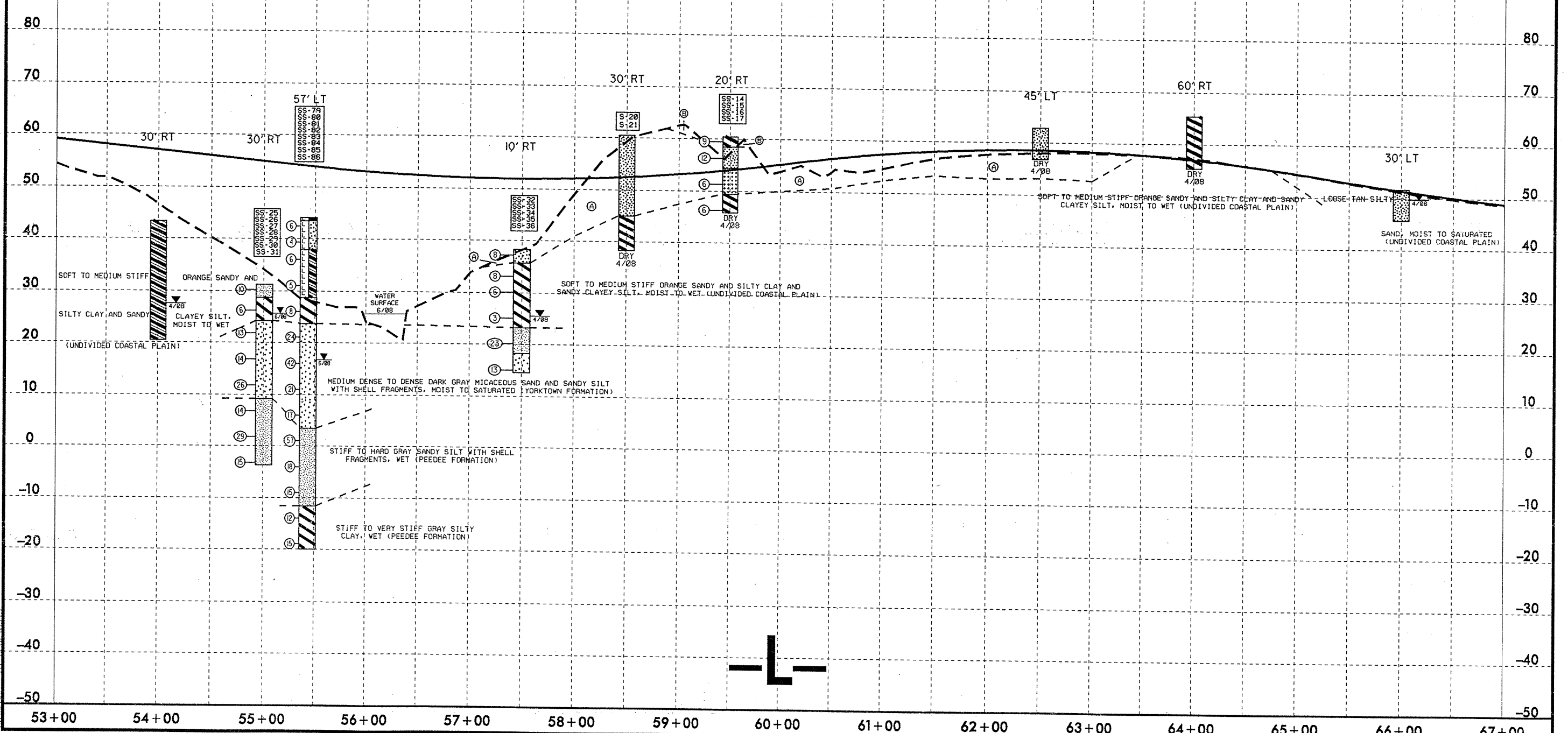
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PROJECT REFERENCE NO. U-5018	SHEET NO. 18
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
INCOMPLETE PLANS DO NOT USE FOR R/W ACQUISITION PRELIMINARY PLANS DO NOT USE FOR CONSTRUCTION	

SOIL TEST RESULTS

SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	LL	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C. SAND	F. SAND	SILT	CLAY	10	40	200		
SS-14	20 RT	59+50	1.00-1.50	A-7-6(23)	54	26	3.0	18.9	19.2	58.9	100	99	81	-	-
SS-15	20 RT	59+50	3.10-4.60	A-2-4(0)	18	NP	18.1	63.6	4.2	14.2	100	96	19	-	-
SS-16	20 RT	59+50	8.10-9.60	A-3(0)	18	NP	19.3	71.0	1.6	8.1	100	98	10	-	-
SS-17	20 RT	59+50	13.10-14.60	A-7-6(28)	59	31	0.8	20.3	22.0	56.9	100	100	82	-	-
SS-25	30 RT	55+00	1.00-1.50	A-4(0)	23	5	4.4	54.2	19.1	22.2	100	99	46	-	-
SS-26	30 RT	55+00	4.00-5.50	A-7-6(32)	60	34	2.2	15.0	28.2	54.6	100	98	86	-	-
SS-27	30 RT	55+00	8.40-9.90	A-2-4(0)	18	4	50.6	24.2	16.2	9.1	98	78	26	-	-
SS-28	30 RT	55+00	13.40-14.90	A-2-4(0)	18	NP	65.0	23.5	4.4	7.1	94	61	72	-	-
SS-29	30 RT	55+00	18.40-19.90	A-2-4(0)	27	4	0.4	72.0	15.5	12.1	100	100	35	-	-
SS-30	30 RT	55+00	23.40-24.90	A-4(2)	31	7	2.6	64.5	14.7	18.2	100	98	52	34	-
SS-31	30 RT	55+00	28.40-29.90	A-4(1)	29	5	0.2	70.2	17.5	12.1	100	100	59	-	-
SS-79	57 LT	55+43	1.00-2.00	A-2-4(0)	18	2	6.9	63.5	11.2	18.3	98	95	93	-	-
SS-80	57 LT	55+43	7.10-8.60	A-6(4)	35	18	7.7	51.9	9.8	30.5	100	98	43	-	-
SS-81	57 LT	55+43	17.10-18.60	A-7-6(23)	53	26	3.5	18.5	21.0	57.0	100	98	81	-	-
SS-82	57 LT	55+43	22.10-23.60	A-2-4(0)	25	8	52.7	28.5	4.5	14.3	77	52	16	-	-
SS-83	57 LT	55+43	32.10-33.60	A-2-4(0)	29	3	1.8	75.2	6.7	16.3	99	98	33	-	-
SS-84	57 LT	55+43	42.10-43.60	A-4(0)	26	NP	0.8	82.6	10.5	6.1	98	97	37	-	-
SS-85	57 LT	55+43	47.10-48.60	A-4(4)	33	6	1.4	60.7	17.5	20.4	100	99	70	-	-
SS-86	57 LT	55+43	57.10-58.60	A-7-6(37)	43	25	2.6	45.2	17.5	34.6	100	99	73	-	-
SS-32	10 RT	57+50	1.00-1.50	A-2-4(0)	18	NP	12.9	62.7	16.3	8.1	100	98	27	-	-
SS-33	10 RT	57+50	4.00-5.50	A-7-6(36)	65	42	1.4	20.4	23.6	54.6	100	99	81	30	-
SS-34	10 RT	57+50	12.10-13.60	A-7-6(23)	50	29	8.9	15.4	35.3	40.4	100	96	78	-	-
SS-35	10 RT	57+50	17.10-18.60	A-4(0)	16	2	46.3	24.0	20.6	9.1	98	82	60	-	-
SS-36	10 RT	57+50	22.10-23.60	A-2-4(0)	21	NP	57.9	26.5	6.5	9.1	90	61	16	-	-
S-20	30 RT	58+50	1.00-15.50	A-2-4(0)	21	NP	21.2	61.7	2.9	14.3	100	97	18	-	-
S-21	30 RT	58+50	15.50-22.20	A-7-6(22)	54	26	1.4	23.3	20.0	55.2	100	100	78	34.3	-

(A) LOOSE TO MEDIUM DENSE TAN SILTY SAND, MOIST (UNDIVIDED COASTAL PLAIN)
 (B) MEDIUM STIFF ORANGE SILTY CLAY, MOIST (UNDIVIDED COASTAL PLAIN)



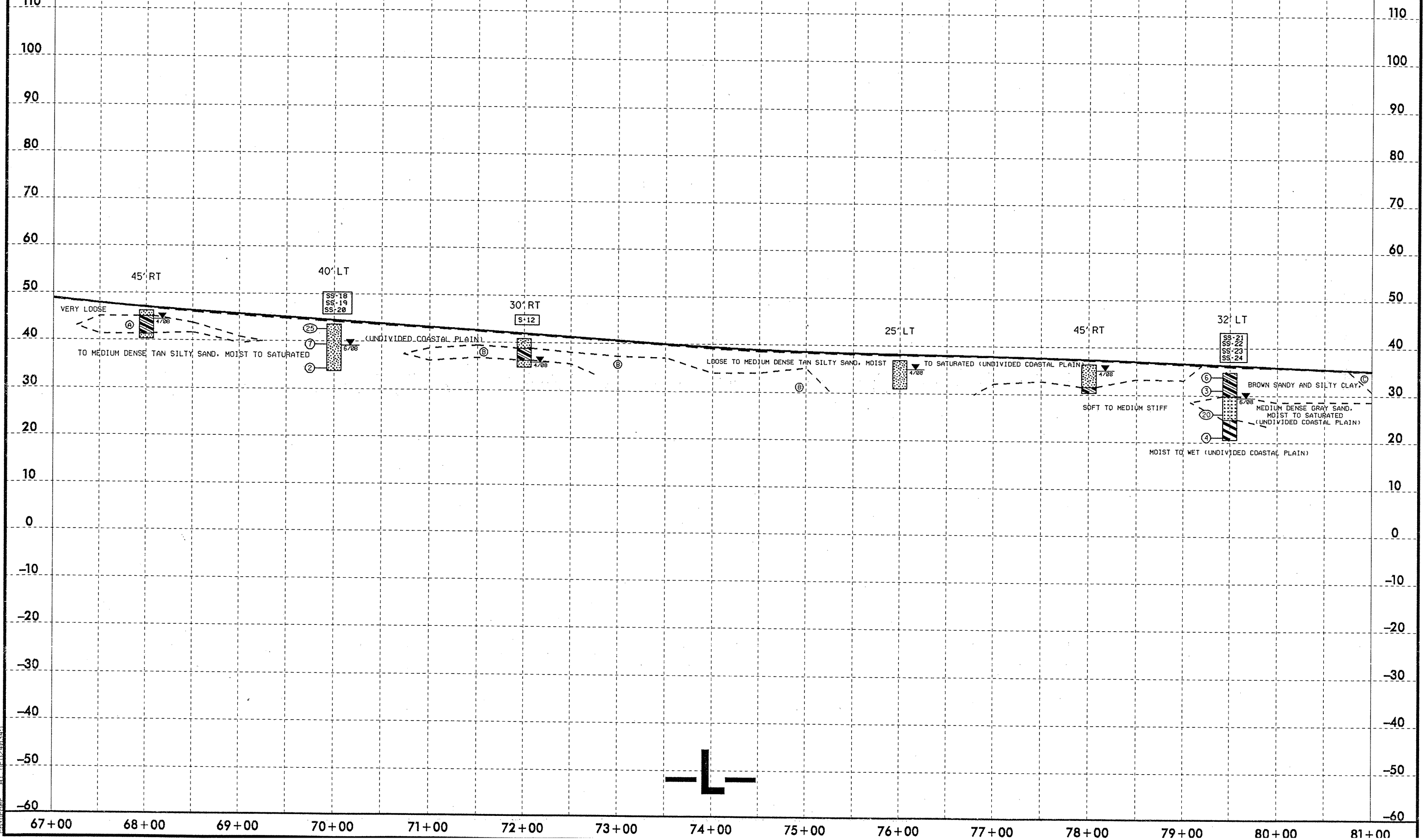
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SOIL TEST RESULTS															
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	ASTM CLASS	LL	PL	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C. SAND	F. SAND	SILT	CLAY	20	40	600		
SS-12	30 RT	72+00	2.00-4.50	A-1(1)2	24	17	2.7	55.6	10.8	20.7	100	100	45	21.1	
SS-18	40 LT	70+00	1.00-1.50	A-2-4(1)	20	21	2.8	71.9	7.0	18.3	100	100	28	-	
SS-19	40 LT	70+00	3.30-4.80	A-2-4(1)	20	NP	2.5	84.8	3.5	18.2	100	100	17	-	
SS-20	40 LT	70+00	8.30-9.80	A-2-4(1)	21	NP	1.3	73.4	6.0	19.3	100	100	29	-	
SS-21	32 LT	79+50	1.00-1.50	A-1(1)5	31	14	2.4	47.9	17.2	38.5	100	100	54	14.8	
SS-22	32 LT	79+50	2.80-4.30	A-1(1)4	28	18	2.6	47.9	19.0	30.5	100	100	55	-	
SS-23	32 LT	79+50	7.80-9.30	A-1(1)1	16	NP	46.2	49.5	2.2	2.0	100	80	6	-	
SS-24	32 LT	79+50	12.80-14.30	A-7-1(1)4	42	15	1.0	21.7	36.6	40.6	100	99	85	-	

- Ⓐ MEDIUM STIFF TAN SANDY CLAY, MOIST TO WET (UNDIVIDED COASTAL PLAIN)
- Ⓑ MEDIUM STIFF TAN SANDY CLAY, MOIST (UNDIVIDED COASTAL PLAIN)
- Ⓒ SOFT BROWN CLAYEY SANDY SILT, MOIST TO WET (UNDIVIDED COASTAL PLAIN)

PROJECT REFERENCE NO. U-5018	SHEET NO. 19
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
INCOMPLETE PLANS DO NOT USE FOR ACQUISITION PRELIMINARY PLANS DO NOT USE FOR CONSTRUCTION	



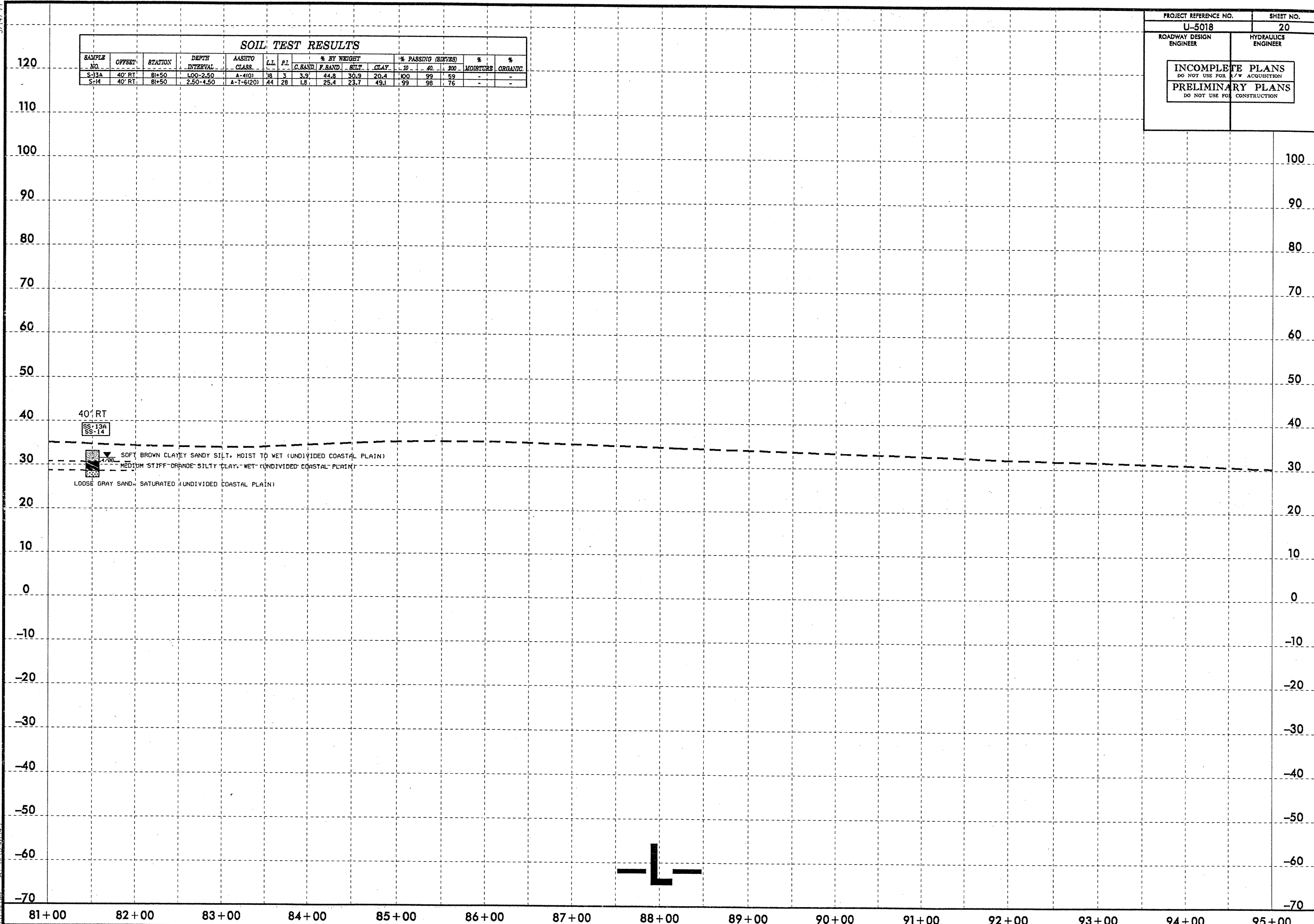
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67+00 68+00 69+00 70+00 71+00 72+00 73+00 74+00 75+00 76+00 77+00 78+00 79+00 80+00 81+00

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 11/20/08

SOIL TEST RESULTS															
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	ASHTO CLASS.	LL	PI	% BY WEIGHT				% PASSING (SIEVES)			%	%
							C. SAND	F. SAND	SILT	CLAY	#10	#40	#200		
S-13A	40' RT	81+50	1.00-2.50	A-4(0)	18	3	3.9	44.8	30.9	20.4	100	99	59	-	-
S-14	40' RT	81+50	2.50-4.50	A-7-6(20)	44	28	1.8	25.4	23.7	49.1	99	98	76	-	-

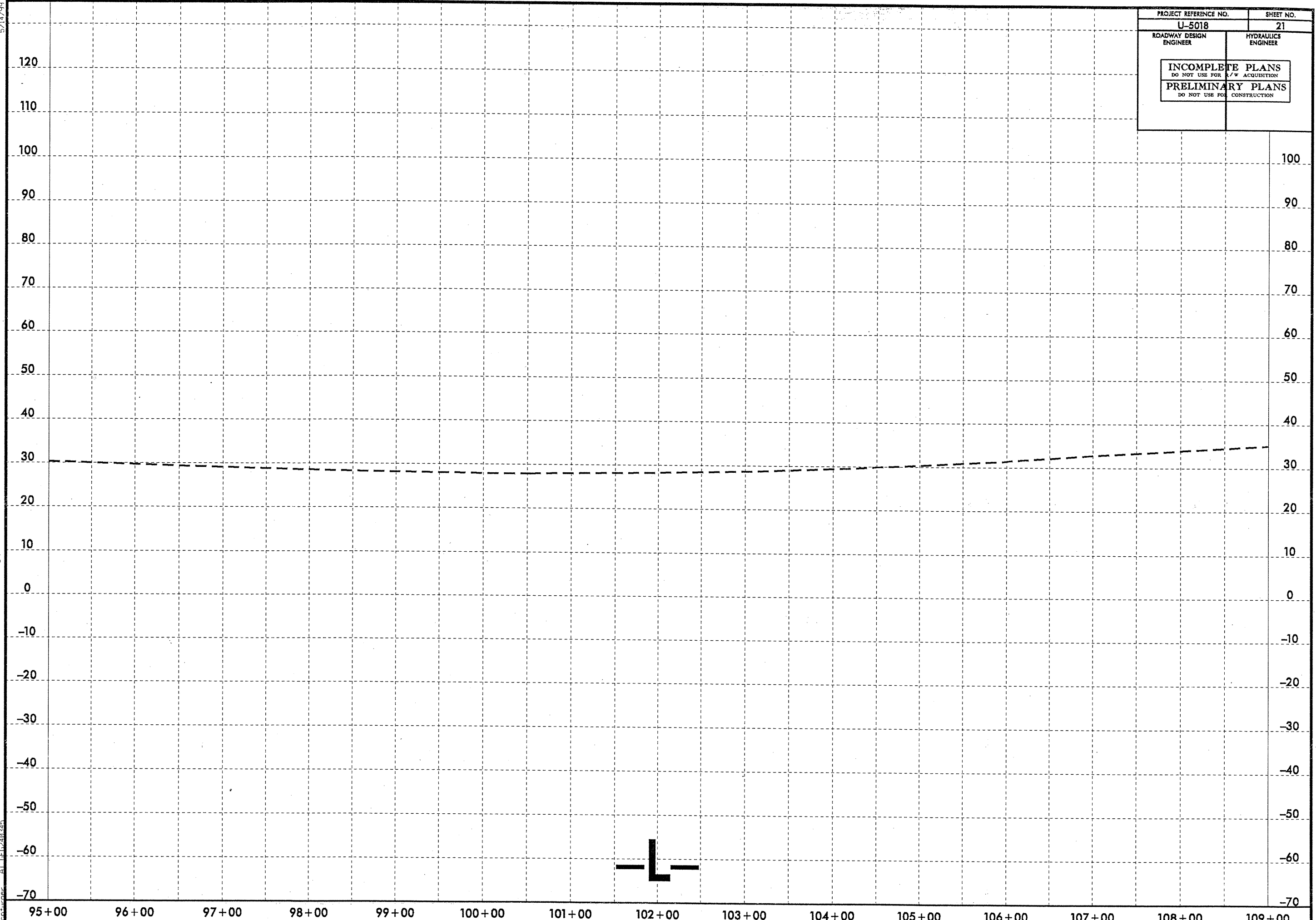
PROJECT REFERENCE NO. U-5018	SHEET NO. 20
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
INCOMPLETE PLANS DO NOT USE FOR ACQUISITION	
PRELIMINARY PLANS DO NOT USE FOR CONSTRUCTION	



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70
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30
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10
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-10
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-30
-40
-50
-60
-70
81+00 82+00 83+00 84+00 85+00 86+00 87+00 88+00 89+00 90+00 91+00 92+00 93+00 94+00 95+00

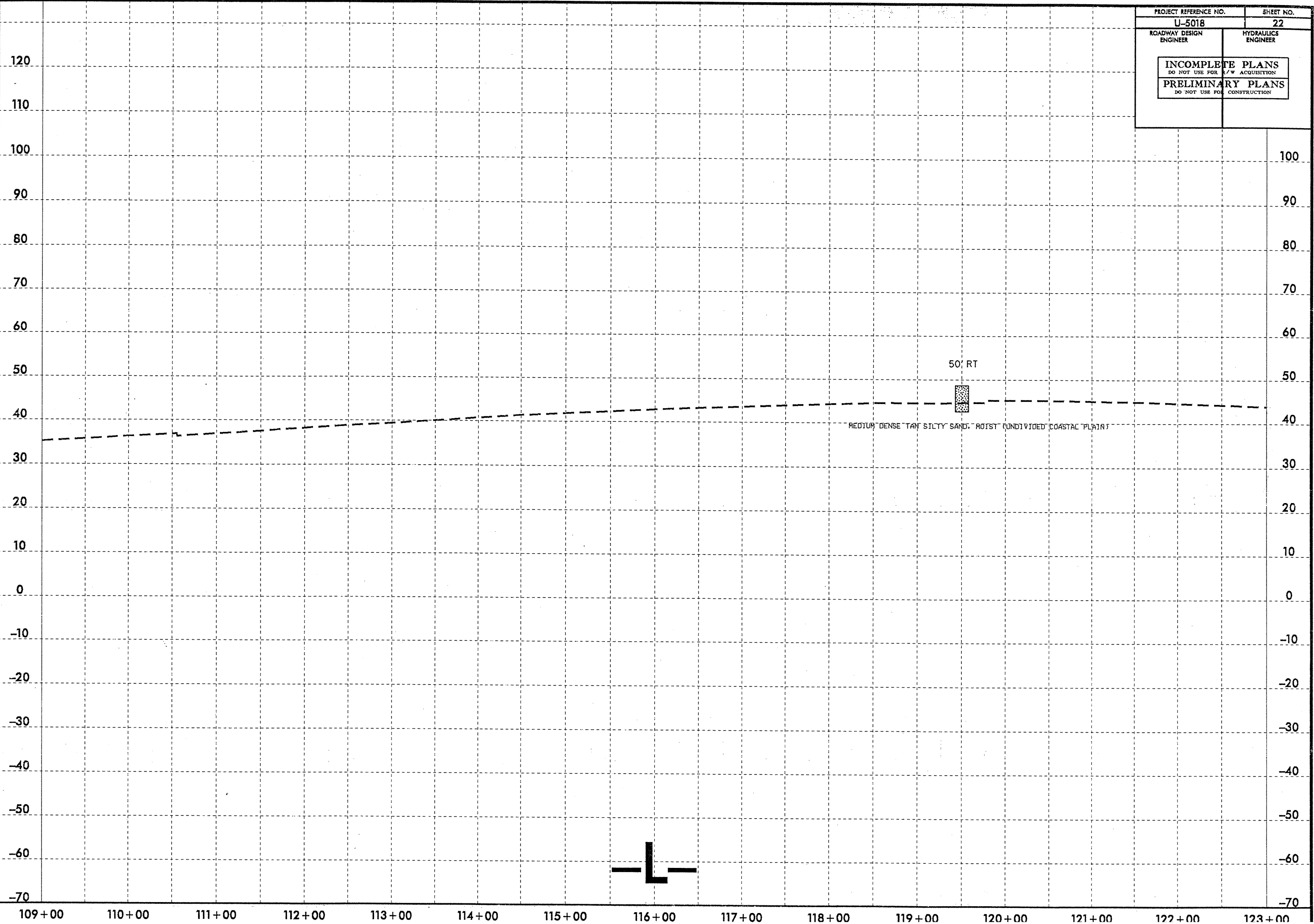
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PROJECT REFERENCE NO. U-5018	SHEET NO. 21
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PRELIMINARY PLANS DO NOT USE FOR CONSTRUCTION	

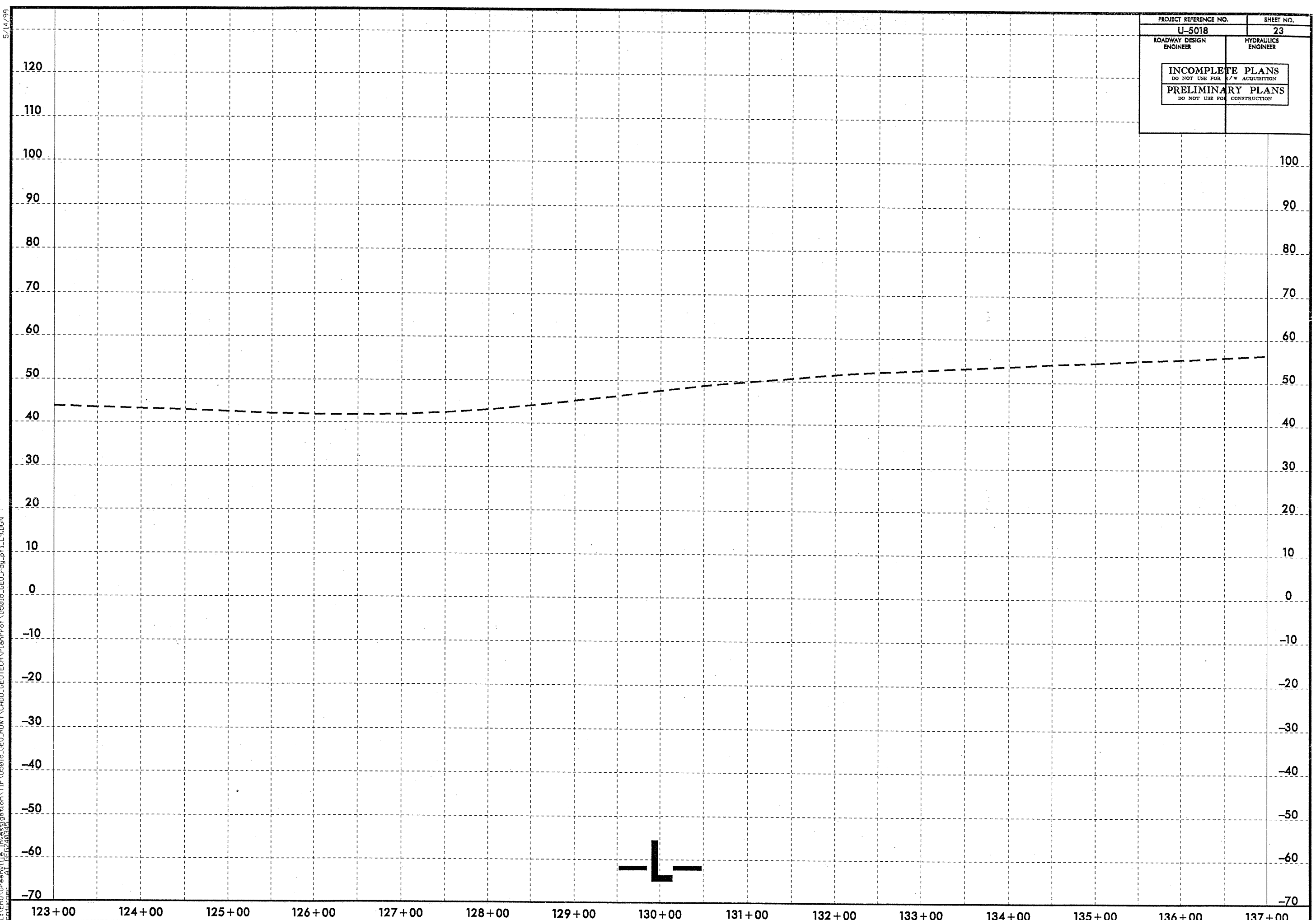


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Author: AT 08/24/08

PROJECT REFERENCE NO. U-5018	SHEET NO. 22
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INCOMPLETE PLANS DO NOT USE FOR A/W ACQUISITION	
PRELIMINARY PLANS DO NOT USE FOR CONSTRUCTION	



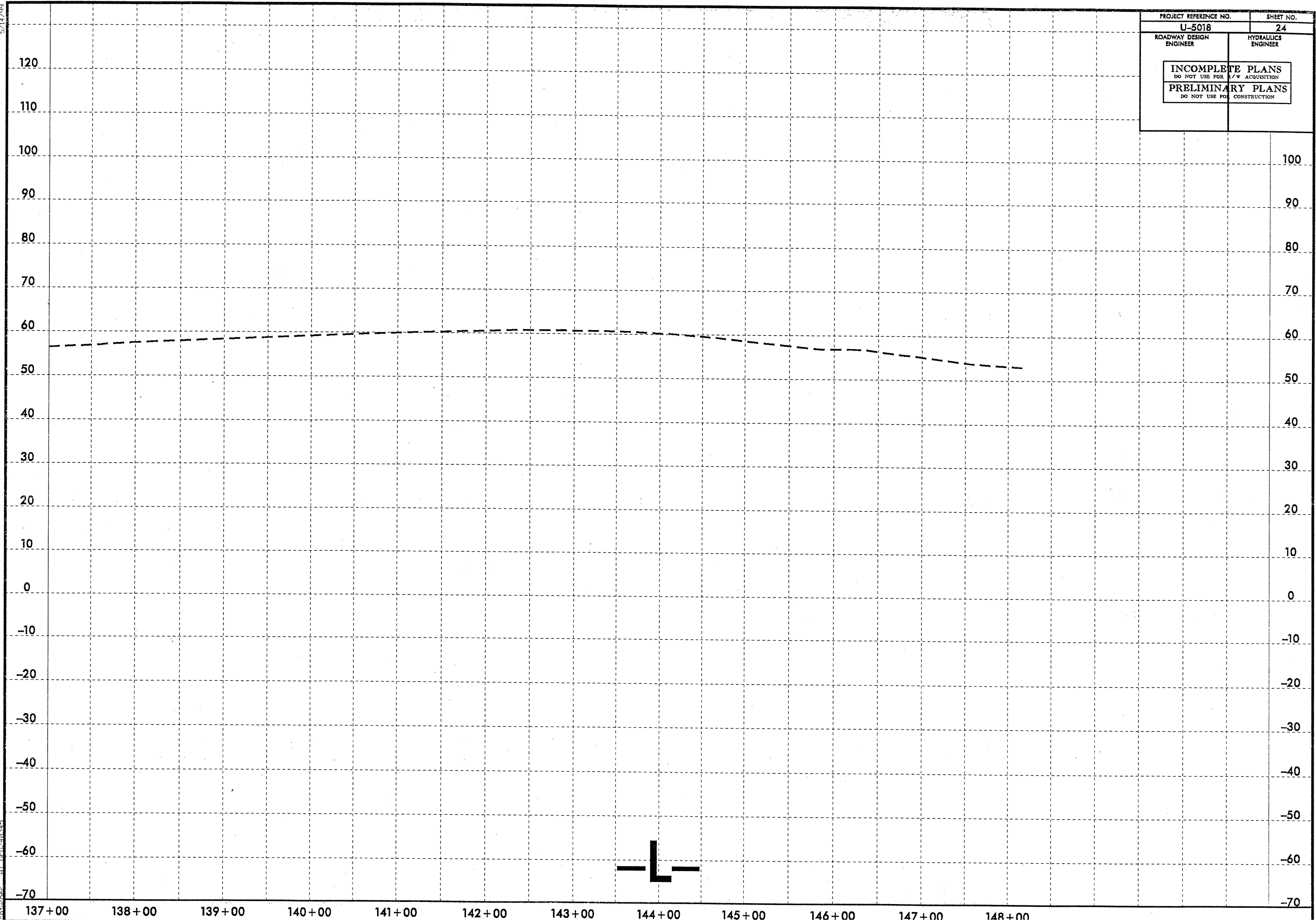
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PRELIMINARY PLANS DO NOT USE FOR CONSTRUCTION	



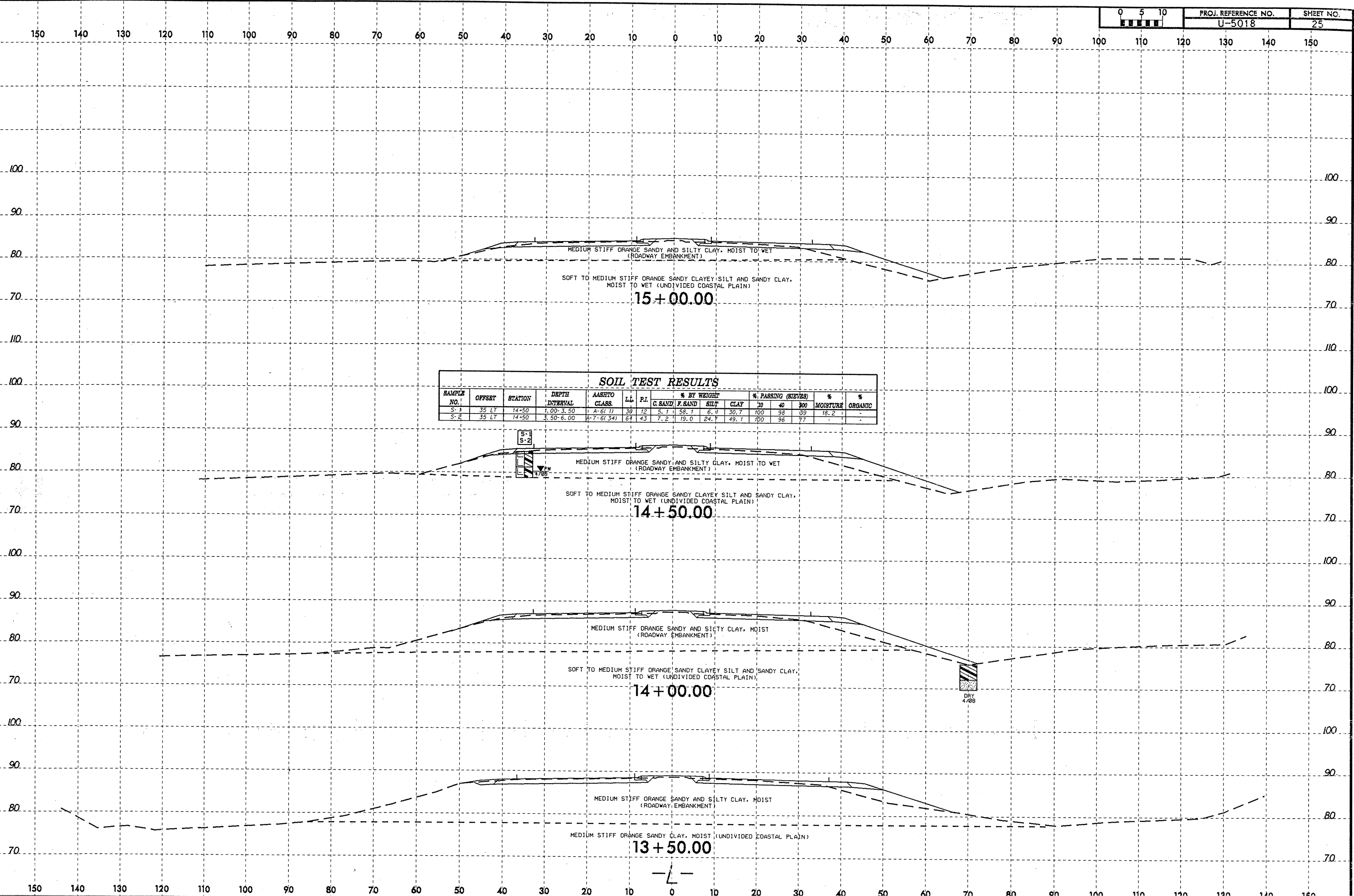
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PROJECT REFERENCE NO. U-5018	SHEET NO. 24
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
INCOMPLETE PLANS DO NOT USE FOR ACQUISITION	
PRELIMINARY PLANS DO NOT USE FOR CONSTRUCTION	



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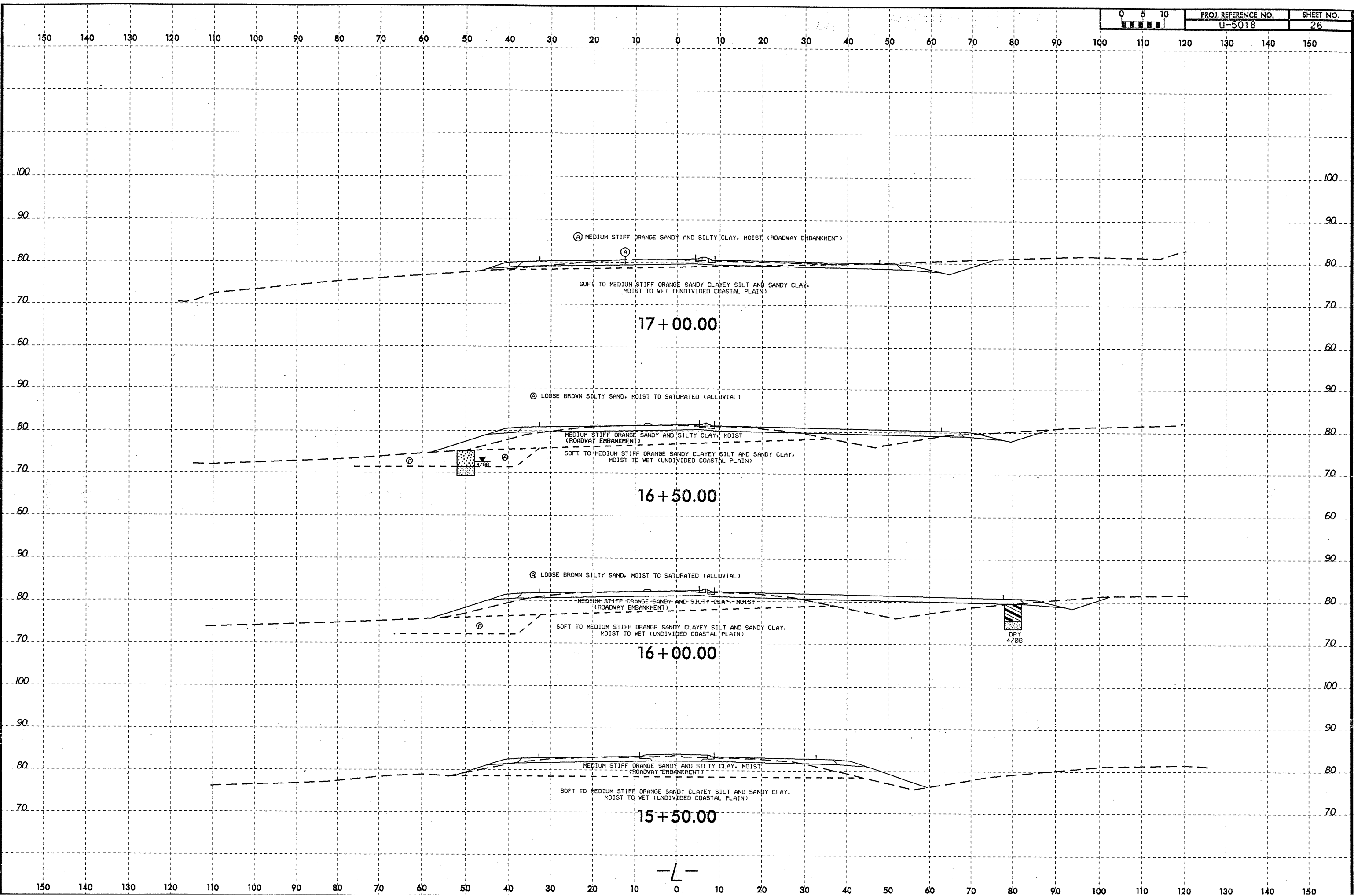


SOIL TEST RESULTS

SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	ASHTO CLASS	LL	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C. SAND	F. SAND	SILT	CLAY	#20	#40	#100		
S-1	35 LT	14+50	1.00-3.50	A-6(1)	39	12	5.1	58.1	6.0	30.7	100	98	99	18.2	-
S-2	35 LT	14+50	3.50-6.00	A-7-6(34)	64	43	7.2	19.0	24.7	49.1	100	96	77	-	-

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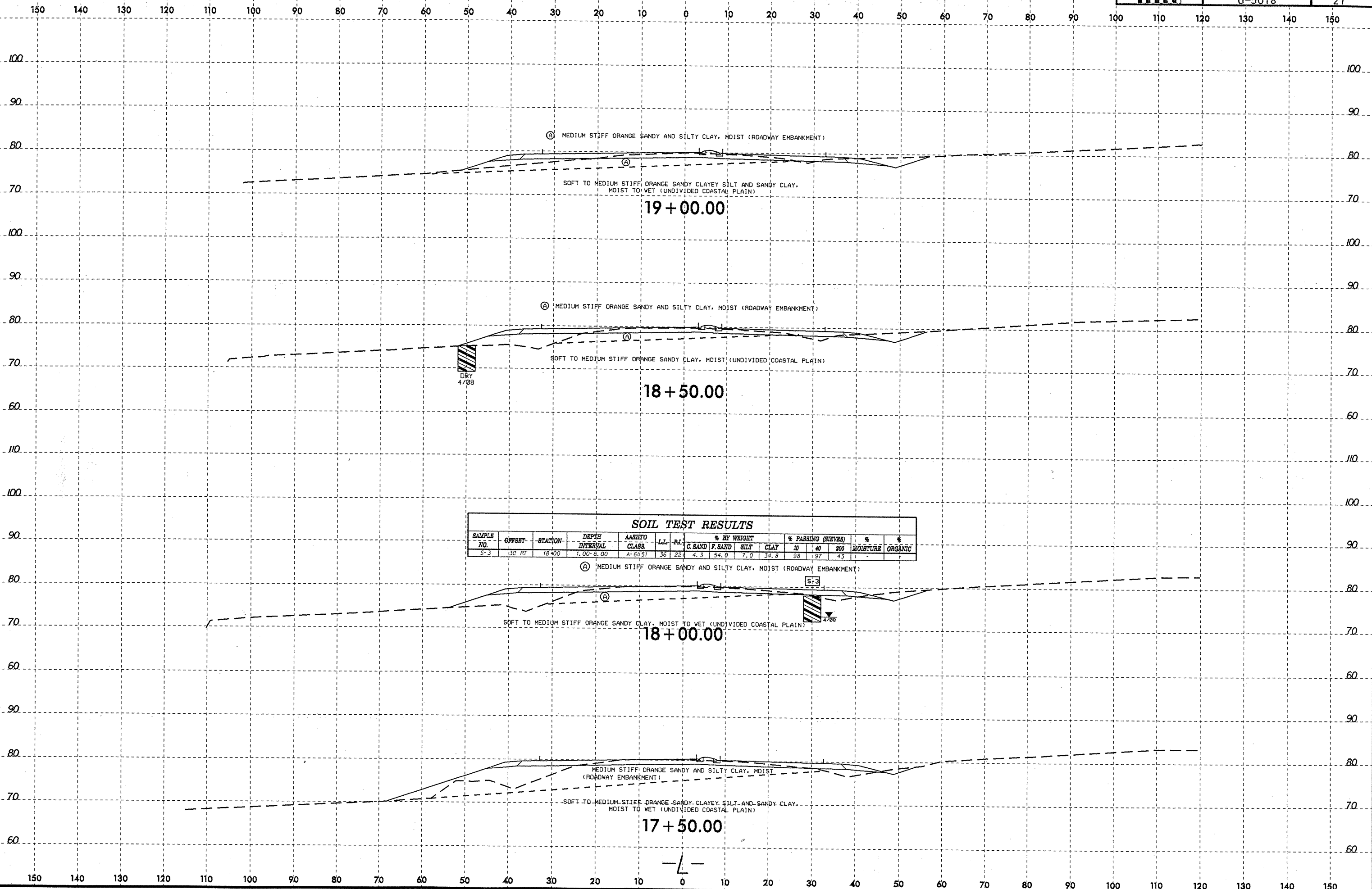


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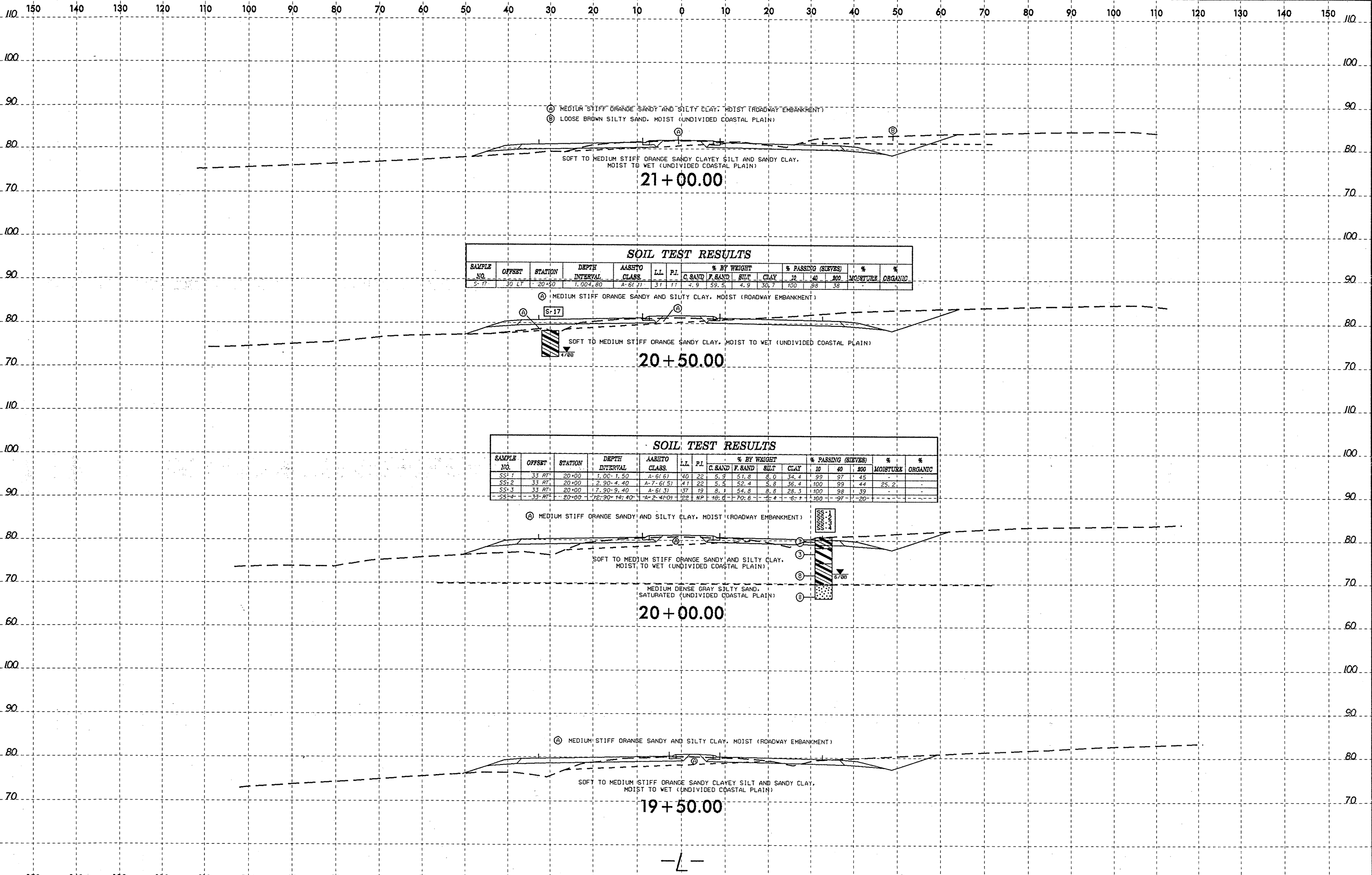


SOIL TEST RESULTS

SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS	LL	PL	% BY WEIGHT			% PASSING (SIZES)			% MOISTURE	% ORGANIC
							C. BAND	F. BAND	SILT CLAY	10	40	200		
S-3	30 FT	18+00	1.00-8.00	A-6(5)	36	22	4.3	54.0	7.0	34.8	98	97	43	

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SOIL TEST RESULTS

SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AAEMTO CLASS	LL	PI	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C. SAND	F. SAND	SILT	CLAY	#10	#40	#200		
S-17	30 LT	20+50	1.00-1.80	A-6(1)	31	11	4.9	59.5	4.9	30.7	100	98	38	-	-

SOIL TEST RESULTS

SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AAEMTO CLASS	LL	PI	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C. SAND	F. SAND	SILT	CLAY	#10	#40	#200		
SS-1	33 RT	20+00	1.00-1.50	A-6(6)	40	22	5.9	51.8	8.0	34.4	99	97	45	-	-
SS-2	33 RT	20+00	2.90-4.40	A-7(6.5)	41	22	5.5	52.4	5.8	36.4	100	99	44	25.2	-
SS-3	33 RT	20+00	7.90-9.40	A-6(3)	37	19	8.1	54.8	8.8	28.3	100	98	39	-	-
SS-4	33 RT	20+00	12.90-14.40	A-2(4.0)	22	NP	16.0	70.8	4.4	8.7	100	97	20	-	-

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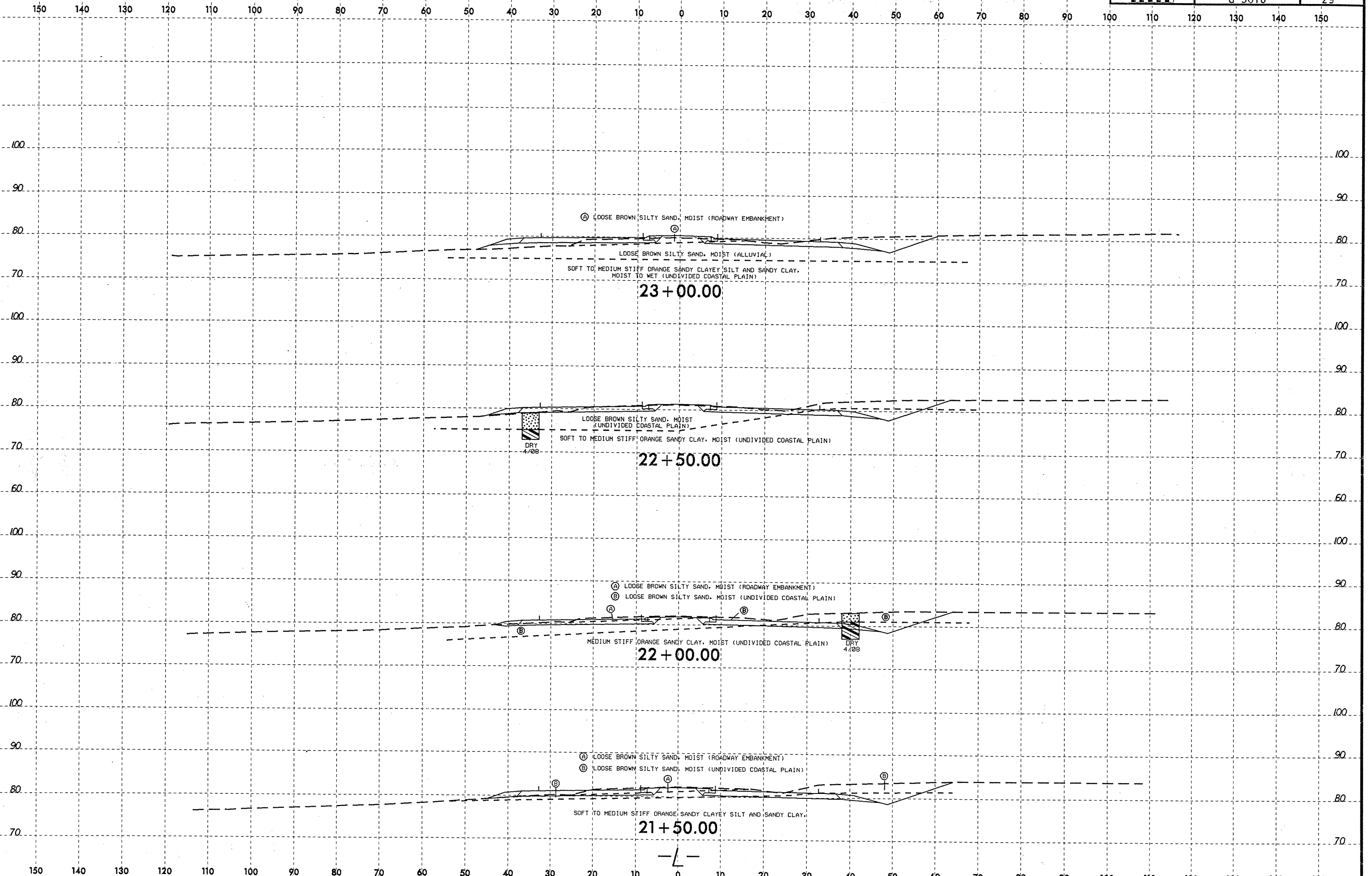
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PROJ. REFERENCE NO.
U-5018

SHEET NO.
29



(A) LOOSE BROWN SILTY SAND, MOIST (ROADWAY EMBANKMENT)

LOOSE BROWN SILTY SAND, MOIST (ALLUVIAL)

SOFT TO MEDIUM STIFF ORANGE SANDY CLAYEY SILT AND SANDY CLAY, MOIST TO WET (UNDIVIDED COASTAL PLAIN)

23 + 00.00

(A) LOOSE BROWN SILTY SAND, MOIST (UNDIVIDED COASTAL PLAIN)

SOFT TO MEDIUM STIFF ORANGE SANDY CLAY, MOIST (UNDIVIDED COASTAL PLAIN)

22 + 50.00

(A) LOOSE BROWN SILTY SAND, MOIST (ROADWAY EMBANKMENT)

(B) LOOSE BROWN SILTY SAND, MOIST (UNDIVIDED COASTAL PLAIN)

MEDIUM STIFF ORANGE SANDY CLAY, MOIST (UNDIVIDED COASTAL PLAIN)

22 + 00.00

(A) LOOSE BROWN SILTY SAND, MOIST (ROADWAY EMBANKMENT)

(B) LOOSE BROWN SILTY SAND, MOIST (UNDIVIDED COASTAL PLAIN)

SOFT TO MEDIUM STIFF ORANGE SANDY CLAYEY SILT AND SANDY CLAY,

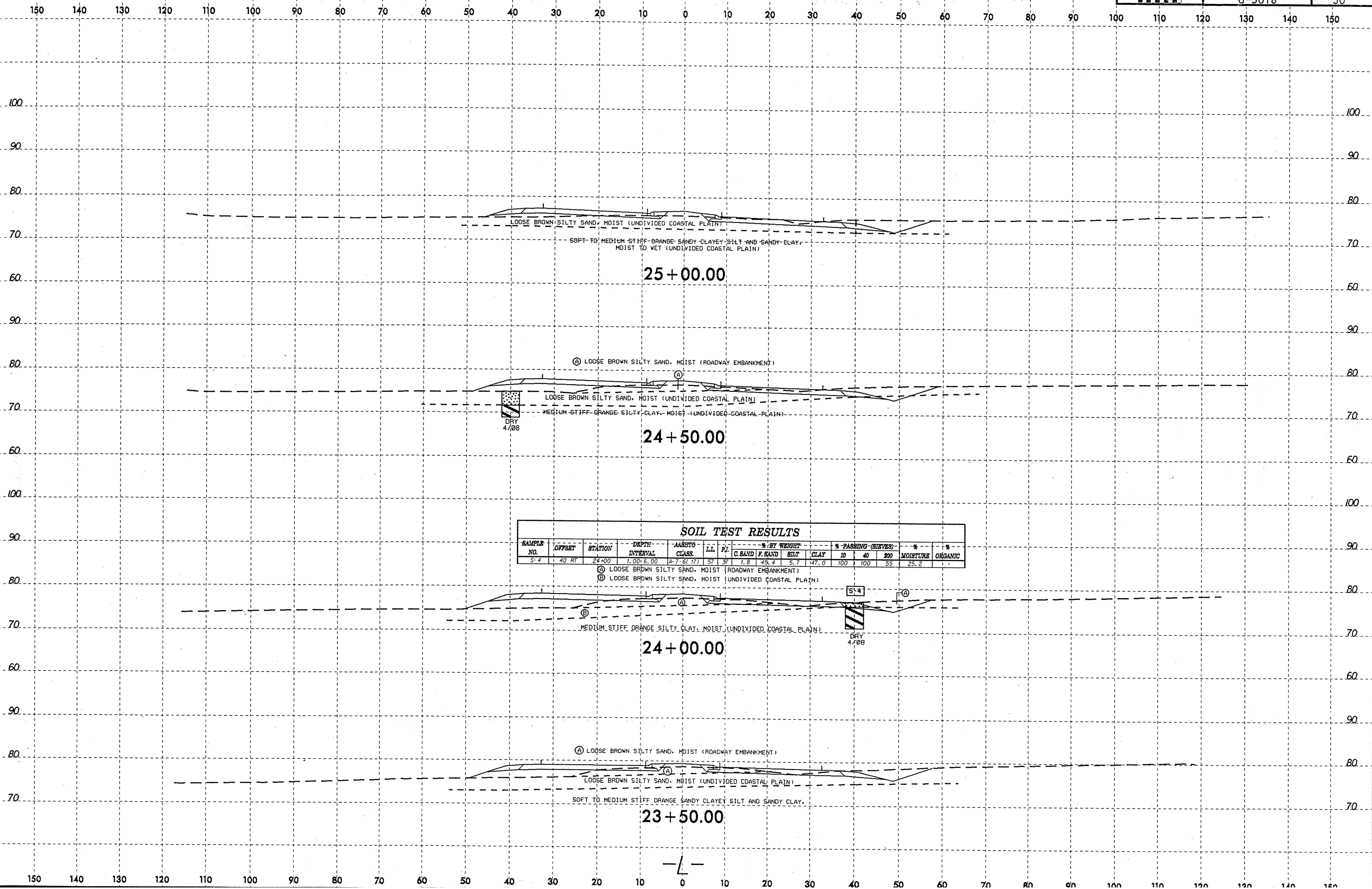
21 + 50.00

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8/23/99



PROJ. REFERENCE NO. U-5018 SHEET NO. 30



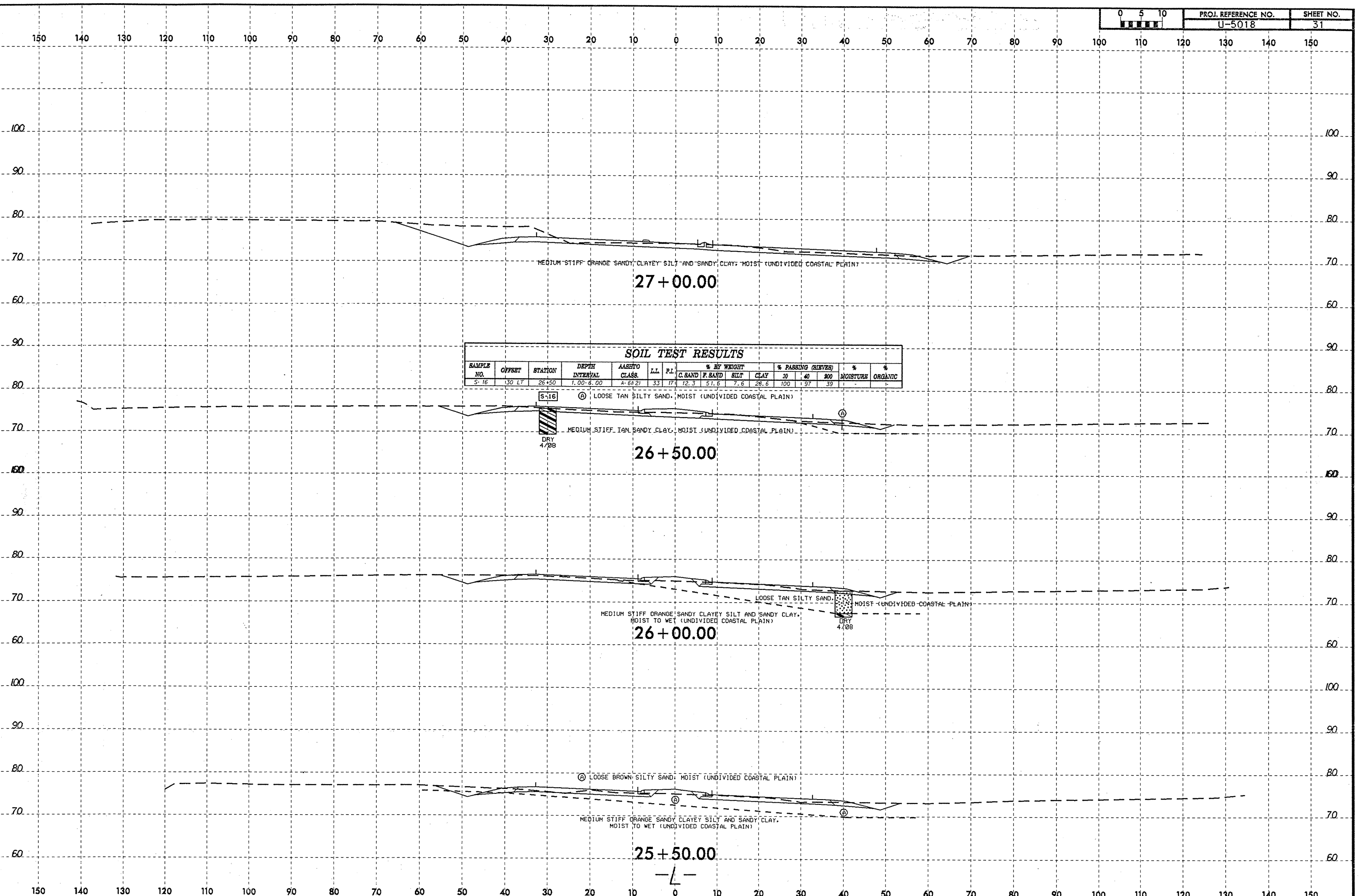
SOIL TEST RESULTS

SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	ASHTO CLASS	LL	PL	% BY WEIGHT			% PASSING (SIEVES)			% MOISTURE	% ORGANIC	
							C. SAND	F. SAND	SILT	CLAY	10	40			200
S-4	40 RT	24+00	1.00-6.00	A-7-6(1)	57	37	1.8	45.4	5.7	47.0	100	100	55	25.2	-

SOIL TEST RESULTS (continued text from table)

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8/23/99
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 VERONICA GARDNER
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SOIL TEST RESULTS

SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	ASHTO CLASS.	LL	PI	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C. SAND	F. SAND	SILT	CLAY	#10	#40	#200		
S-16	30 LT	26+50	1.00-6.00	A-60(2)	33	17	12.3	51.5	7.6	28.6	100	97	39		

27+00.00

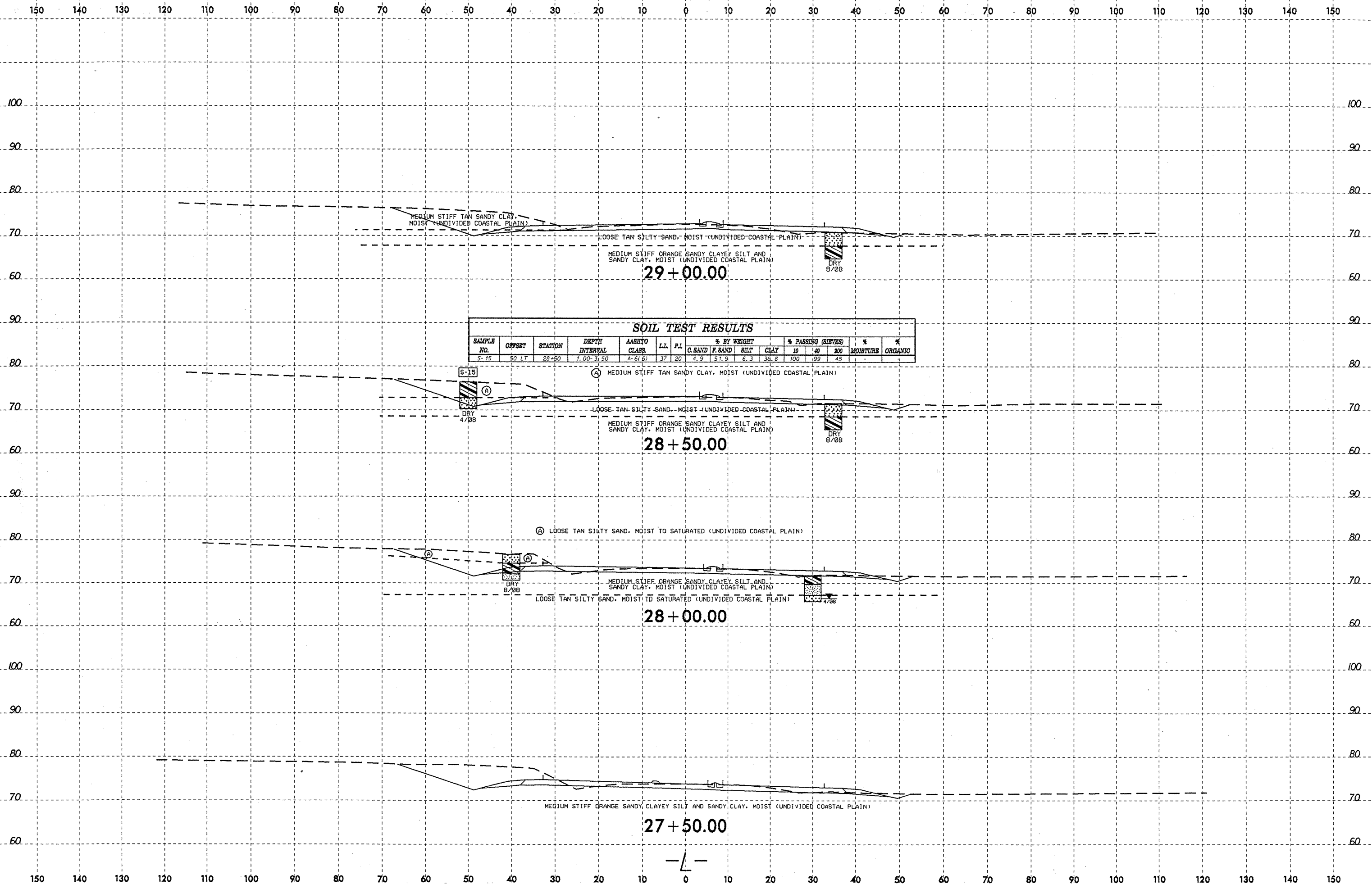
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26+00.00

25+50.00

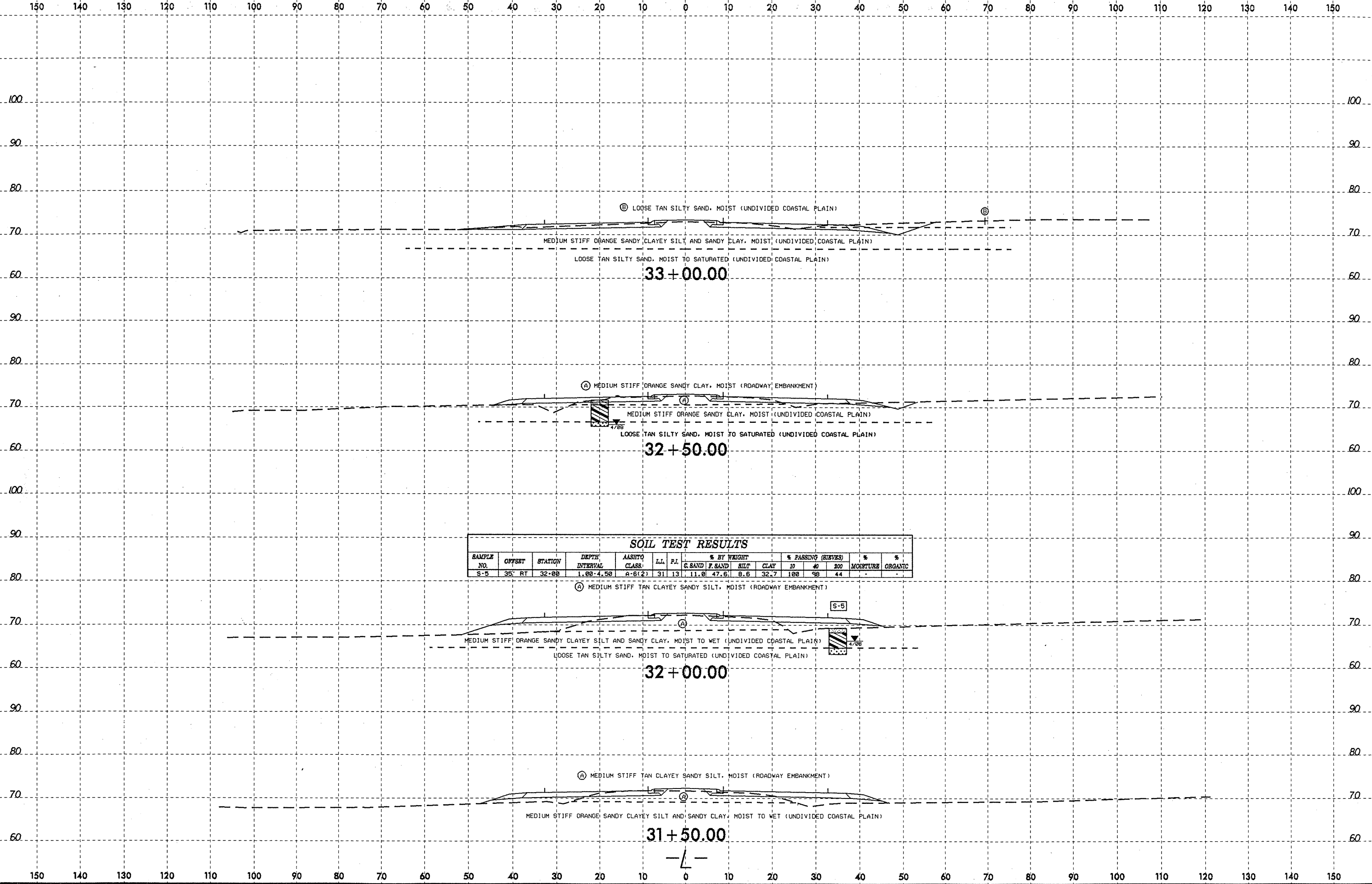
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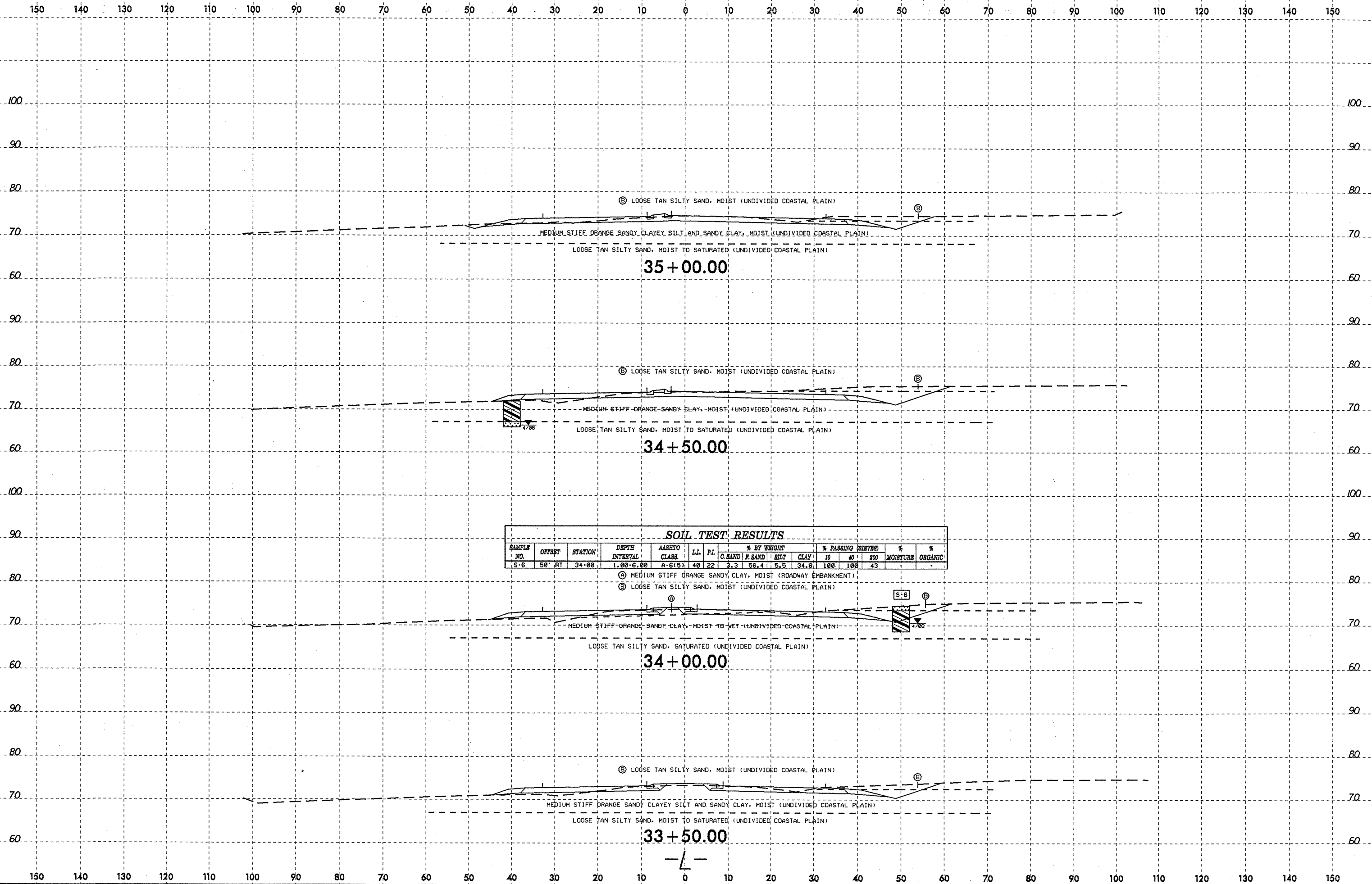
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 AT 06/24/08



SOIL TEST RESULTS

SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS	LL	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							G. SAND	F. SAND	SILT	CLAY	10	40	200		
S-5	35' RT	32+00	1.00-4.50	A-6(2)	31	13	11.0	47.6	8.6	32.7	100	98	44	-	-

8/23/99

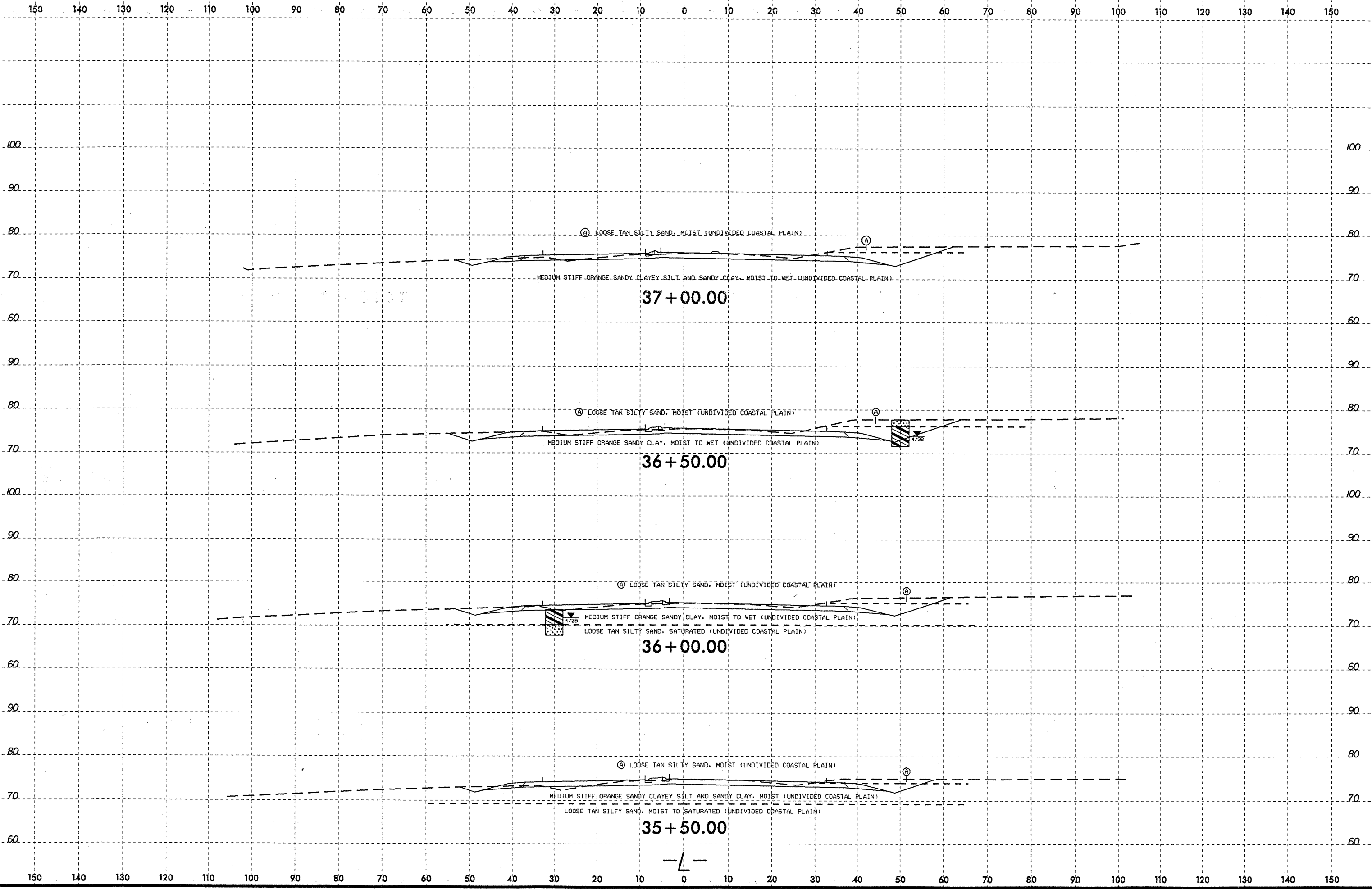


SOIL TEST RESULTS

SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	LL	PL	% BY WEIGHT				% PASSING (SIEVES)			MOISTURE %	ORGANIC %
							C. SAND	F. SAND	SILT	CLAY	10	40	200		
S-6	50' RT	34+00	1.00-6.00	A-6(5)	40	22	3.3	56.4	5.5	34.8	100	100	43		

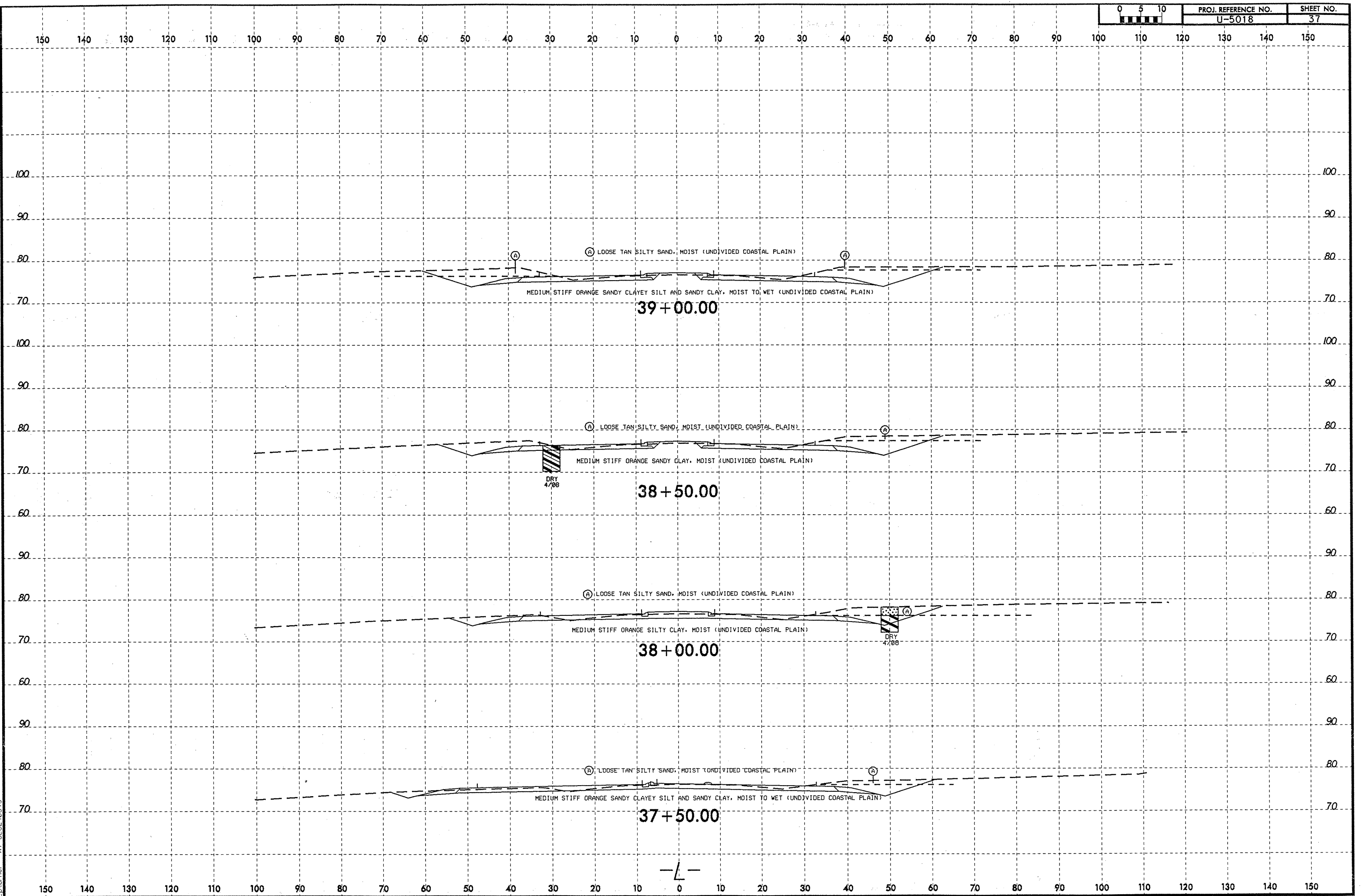
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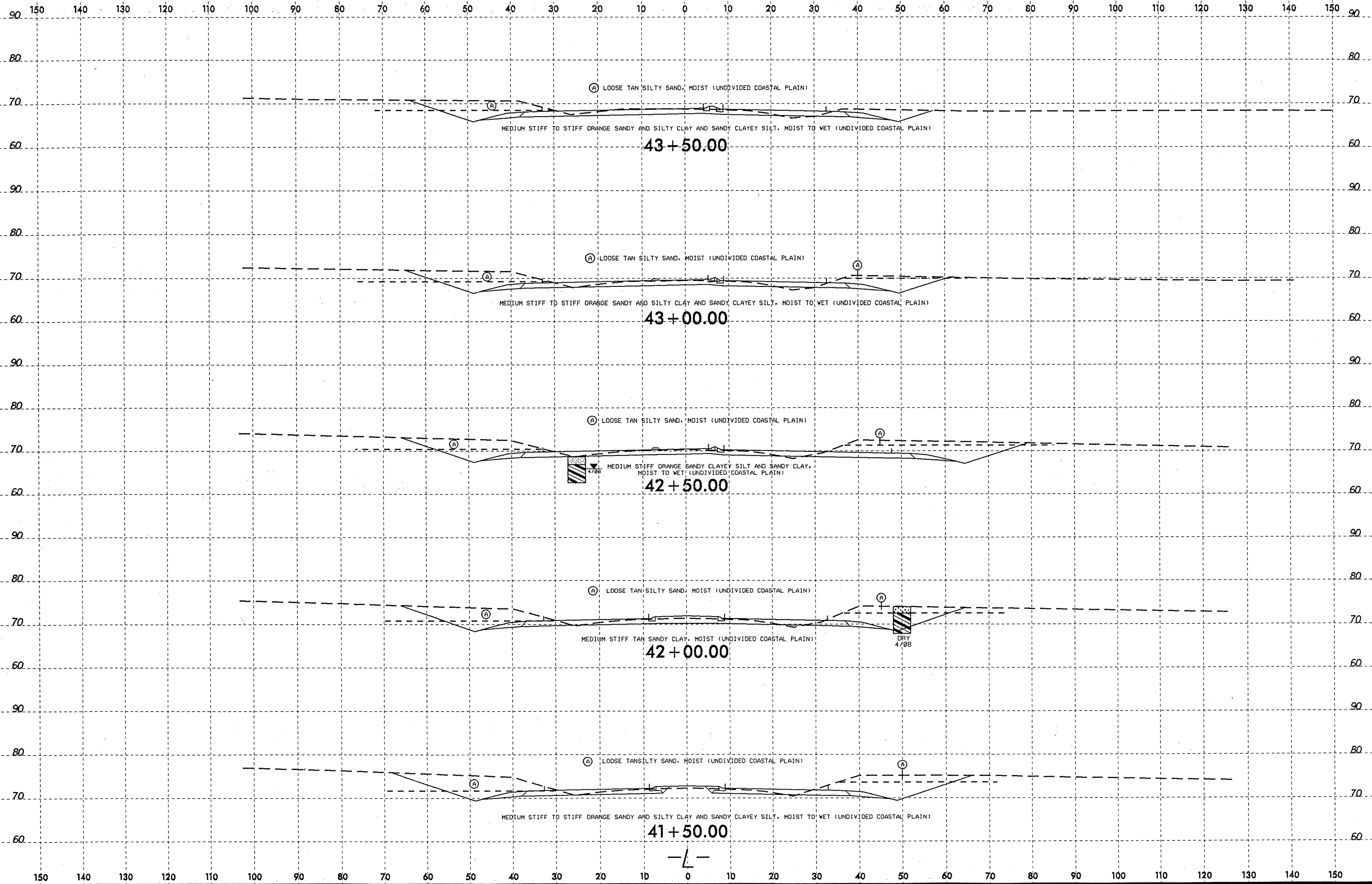
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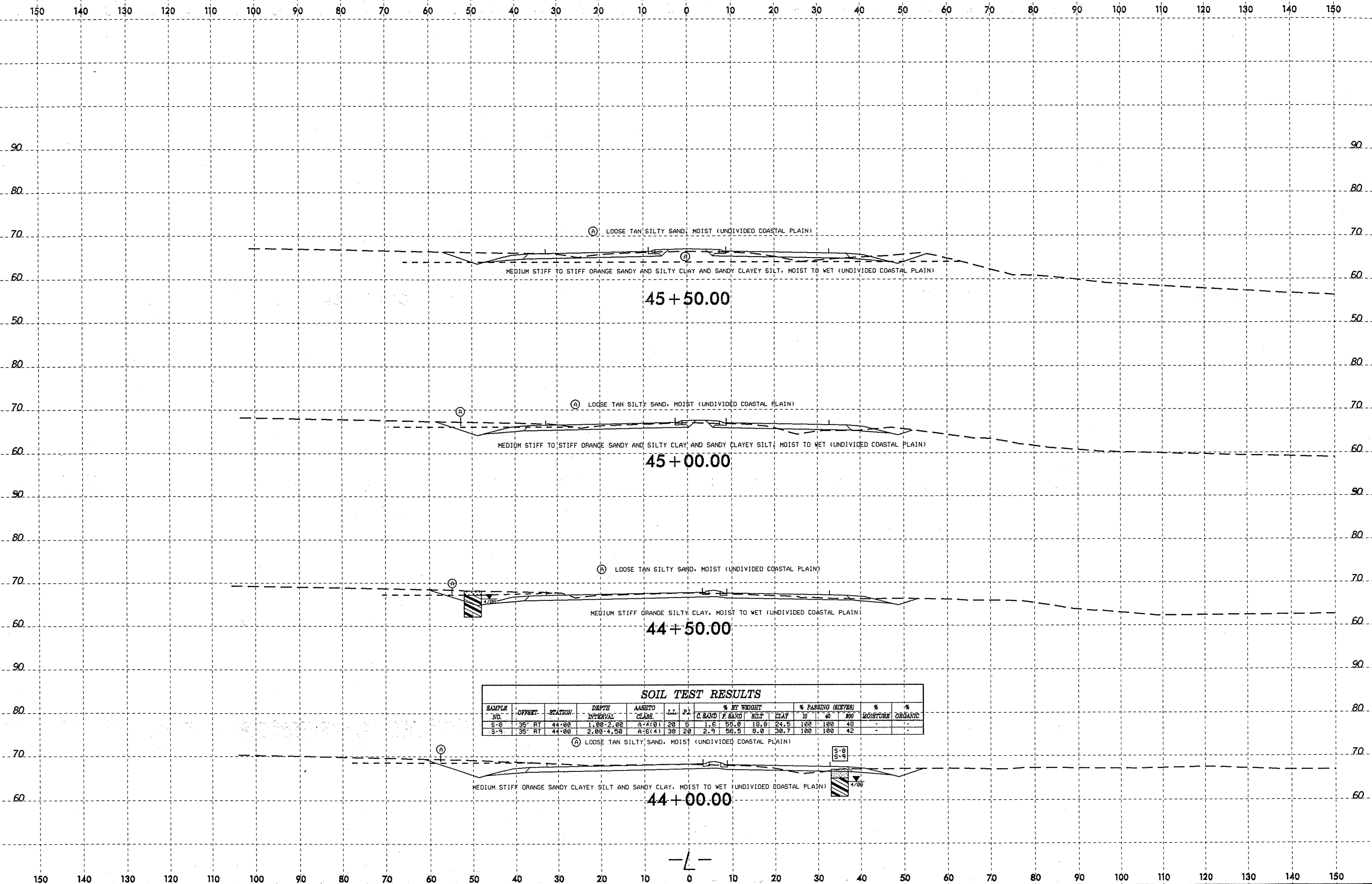
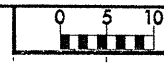
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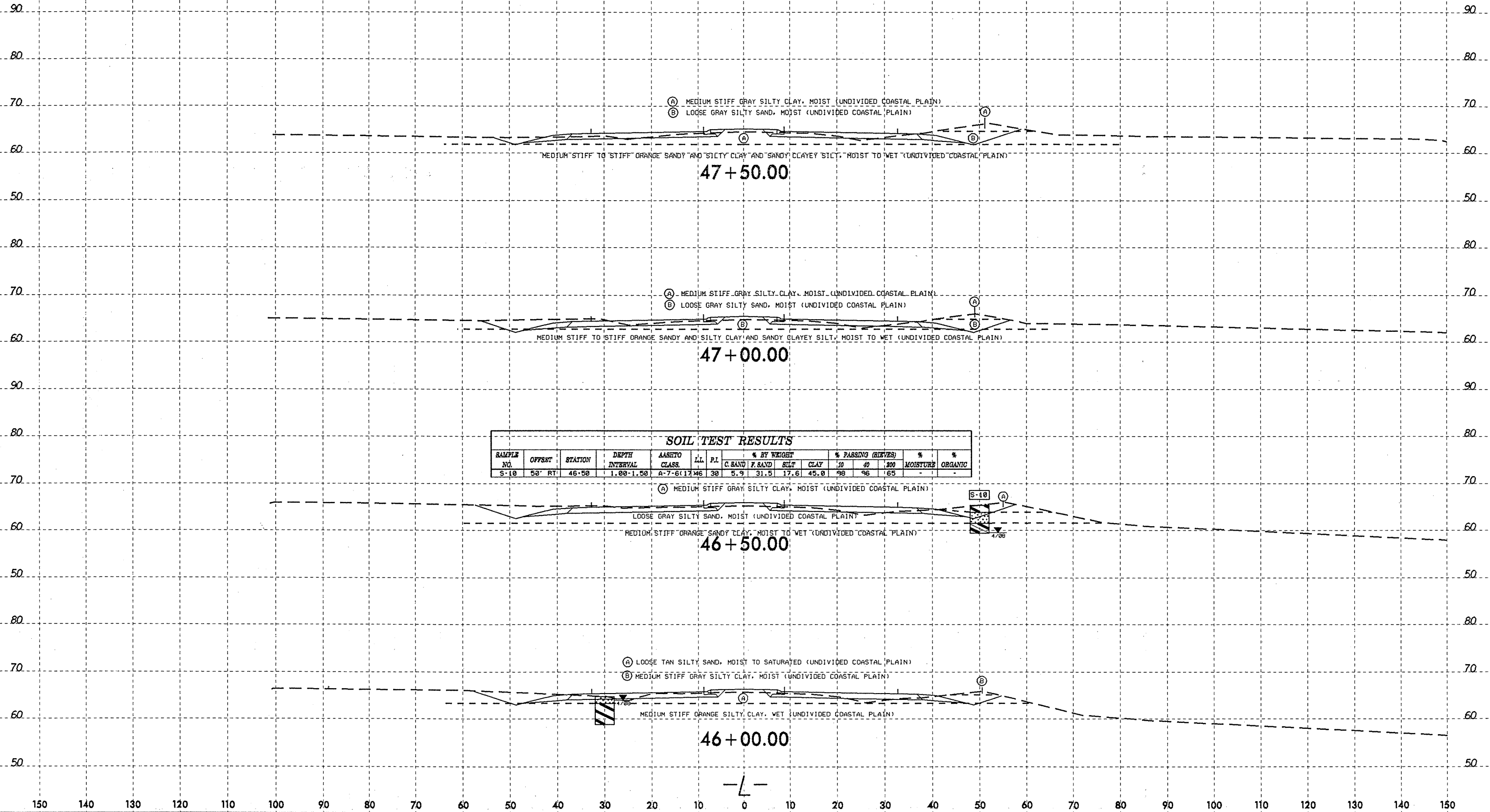
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SOIL TEST RESULTS													
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	LL	PI	% BY WEIGHT			% PASSING (SIEVES)		% MOISTURE	% ORGANIC
							C. SAND	F. SAND	SILT	CLAY	10		
S-8	35' RT	44+00	1.00-2.00	A-4(0)	20	5	1.6	55.0	18.8	24.5	100	100	48
S-9	35' RT	44+00	2.00-4.50	A-6(4)	38	20	2.9	58.5	8.0	30.7	100	100	42

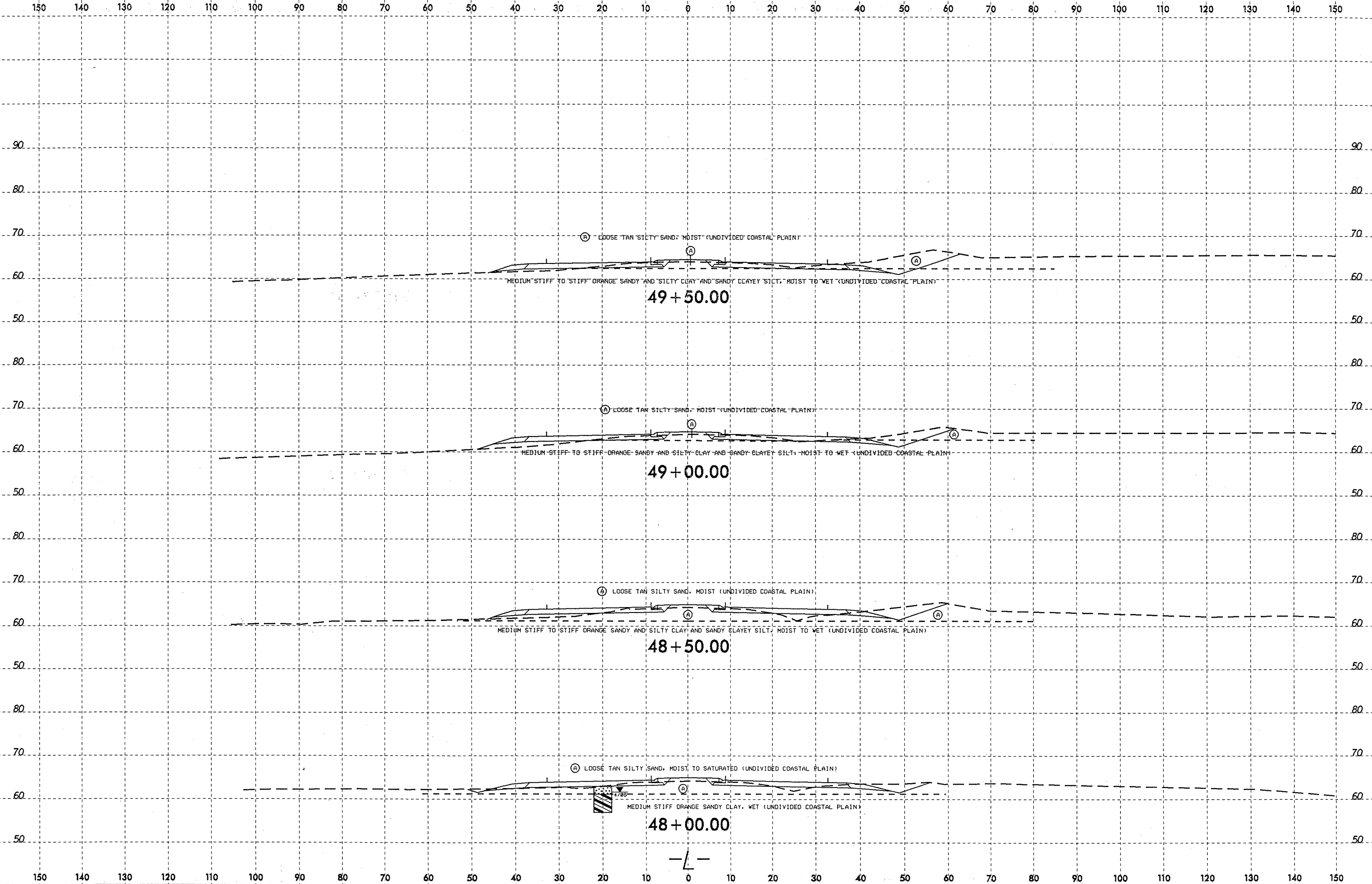
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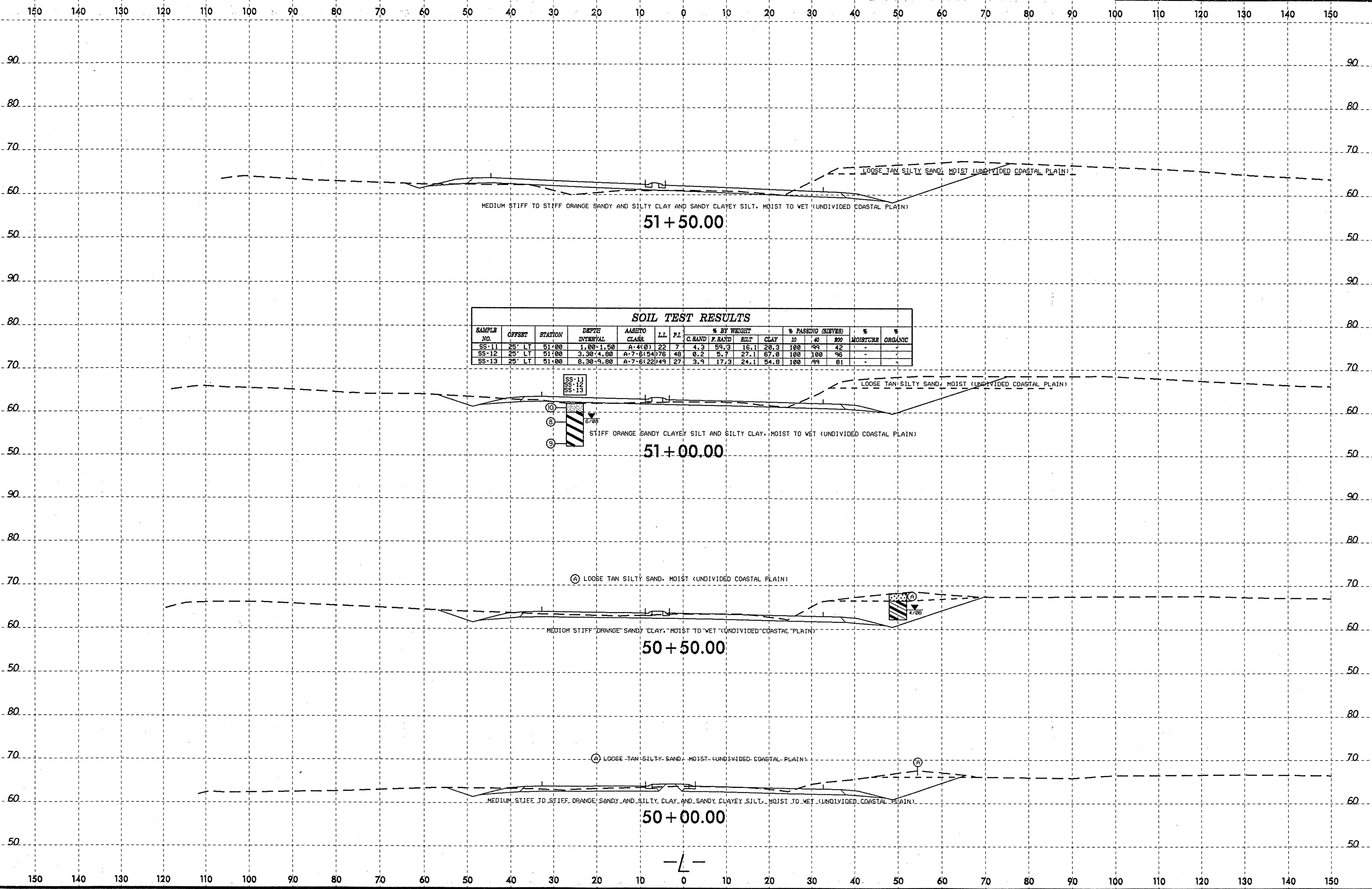
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SOIL TEST RESULTS

SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	LL	PL	% BY WEIGHT				% PASSING (SIEVES)			%	%
							C. SAND	F. SAND	SILT	CLAY	10	40	200		
SS-11	25' LT	51+00	1.00-1.50	A-4(0)	22	7	4.3	59.3	16.1	20.3	100	99	42	-	-
SS-12	25' LT	51+00	3.30-4.80	A-7-6(54)	76	48	0.2	5.7	27.1	67.0	100	100	96	-	-
SS-13	25' LT	51+00	8.30-9.80	A-7-6(22)	49	27	3.9	17.3	24.1	54.8	100	99	81	-	-

51+50.00

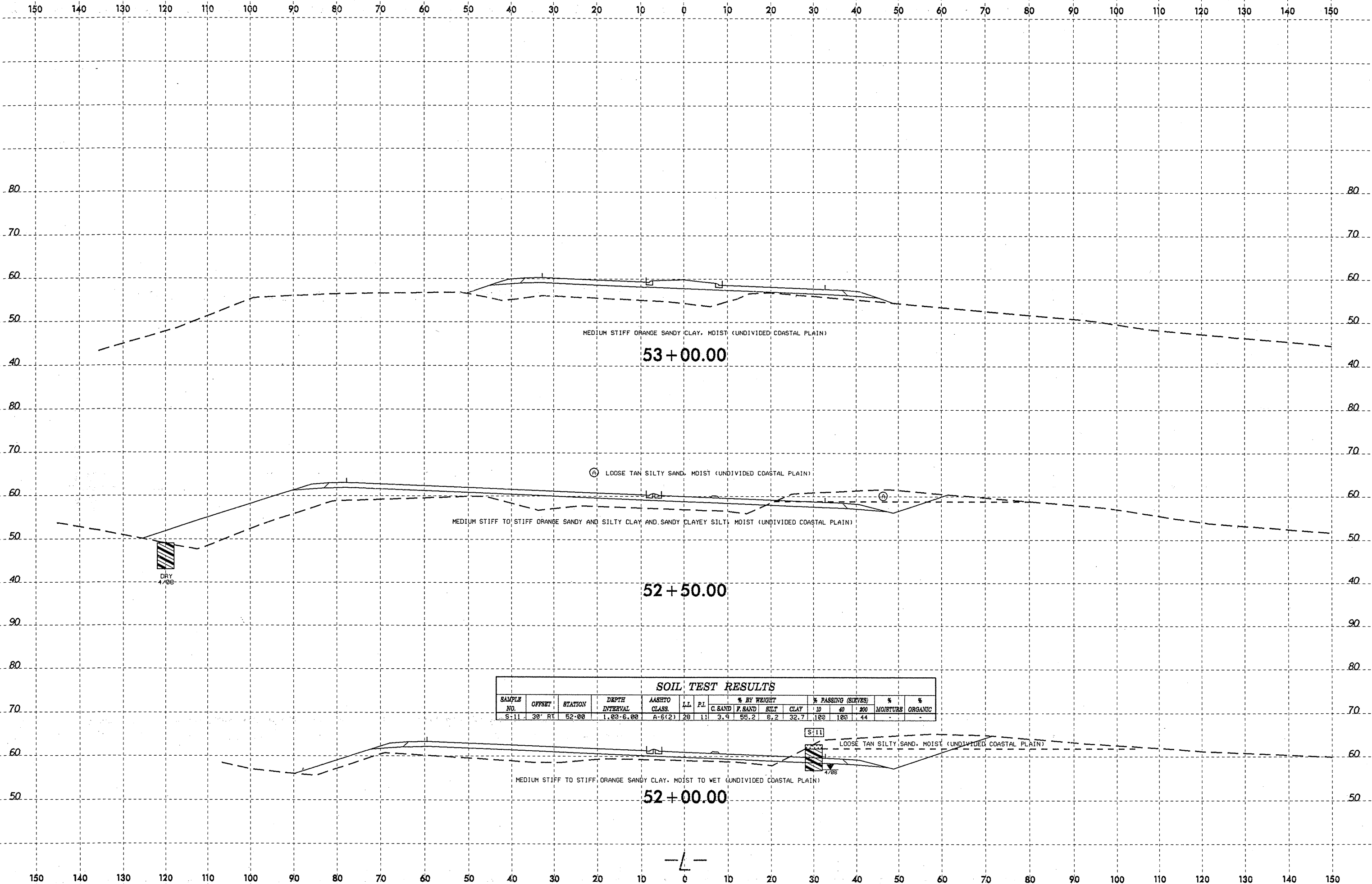
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50+00.00

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SOIL TEST RESULTS

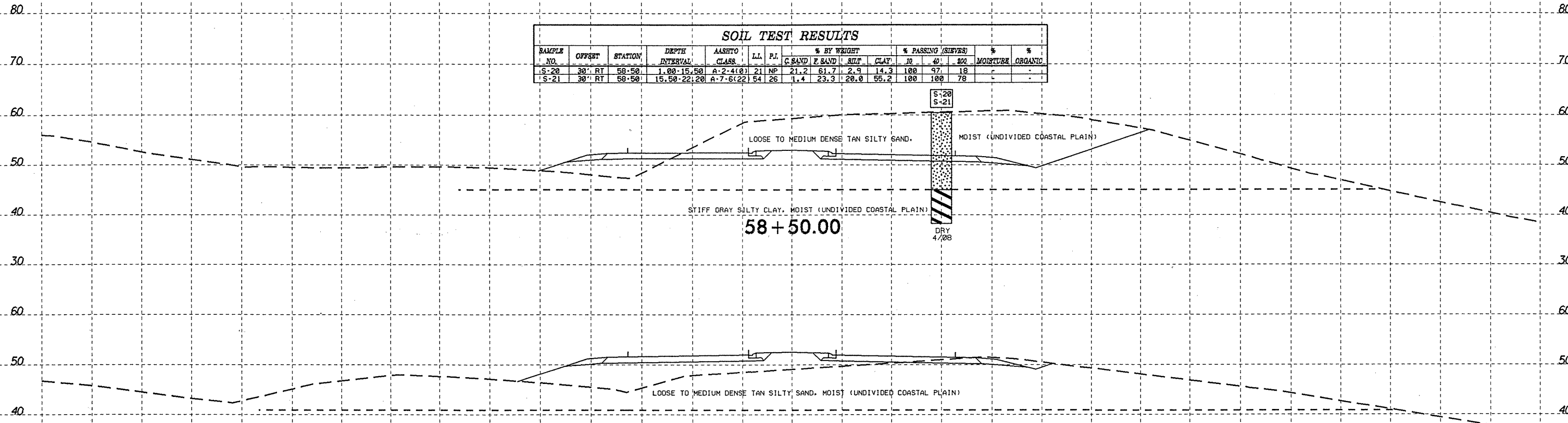
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							C. SAND	F. SAND	SILT	CLAY	10	200		
S-11	30' RT	52+00	1.00-6.00	A-6(2)	28	11	3.9	55.2	8.2	32.7	100	100	44	-

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150 140 130 120 110 100 90 80 70 60 50 40 30 20 10 0 10 20 30 40 50 60 70 80 90 100 110 120 130 140 150

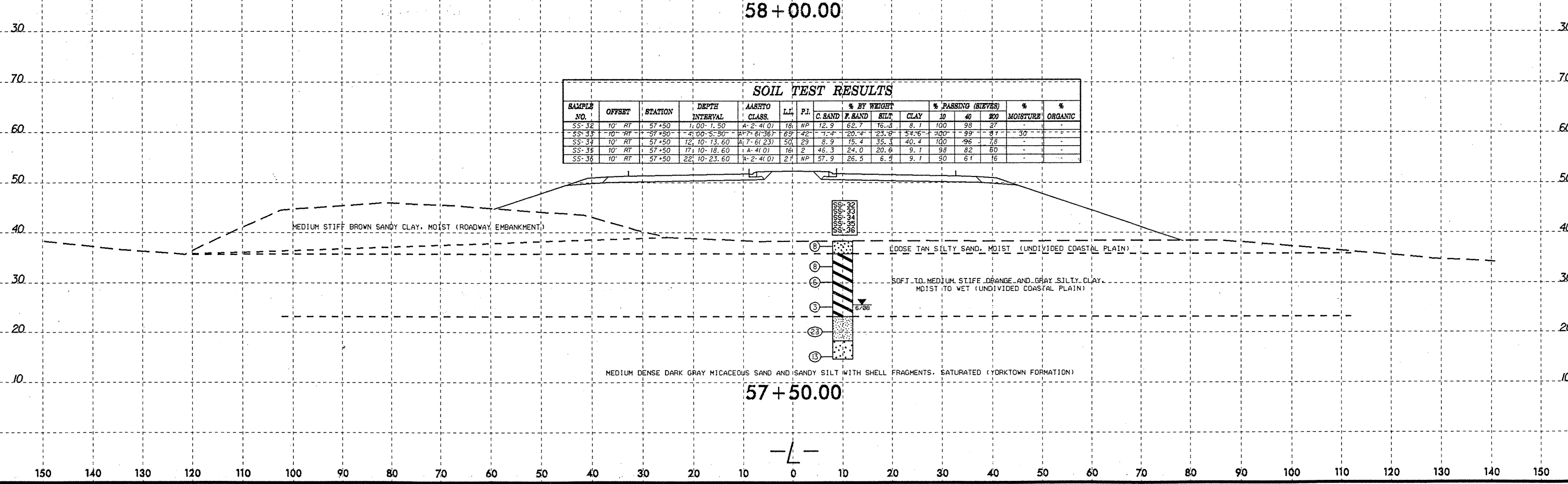
SOIL TEST RESULTS

SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS	LL	P.I.	% BY WEIGHT				% PASSING (SIEVES)			MOISTURE	ORGANIC
							C. SAND	F. SAND	SILT	CLAY	10	40	200		
S-20	30' RT	58+50	1.00-15.50	A-2-4(0)	21	NP	21.2	61.7	2.9	14.3	100	97	18	-	-
S-21	30' RT	58+50	15.50-22.20	A-7-6(22)	54	26	1.4	23.3	20.0	55.2	100	100	78	-	-



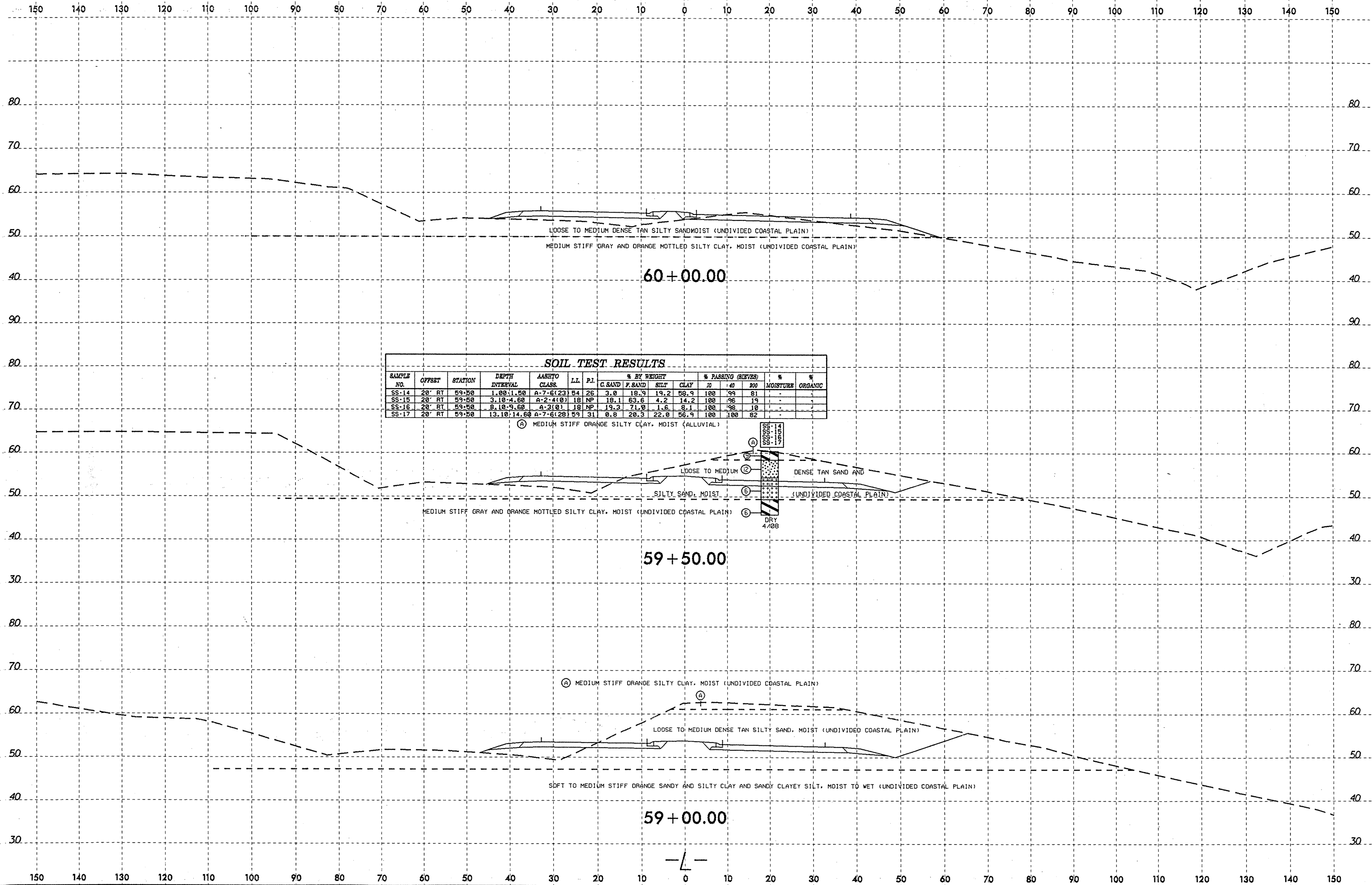
SOIL TEST RESULTS

SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS	LL	P.I.	% BY WEIGHT				% PASSING (SIEVES)			MOISTURE	ORGANIC
							C. SAND	F. SAND	SILT	CLAY	10	40	200		
SS-32	10' RT	57+50	1.00-1.50	A-2-4(0)	18	NP	12.9	62.7	16.3	8.1	100	98	27	-	-
SS-33	10' RT	57+50	4.00-5.50	A-7-6(36)	65	42	1.4	20.4	23.0	54.6	100	99	81	30	-
SS-34	10' RT	57+50	12.10-13.60	A-7-6(23)	50	29	8.9	15.4	35.3	40.4	100	96	78	-	-
SS-35	10' RT	57+50	17.10-18.60	A-4(0)	16	2	46.3	24.0	20.8	9.1	98	82	50	-	-
SS-36	10' RT	57+50	22.10-23.60	A-2-4(0)	21	NP	57.9	26.5	6.3	9.1	90	61	16	-	-



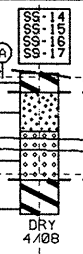
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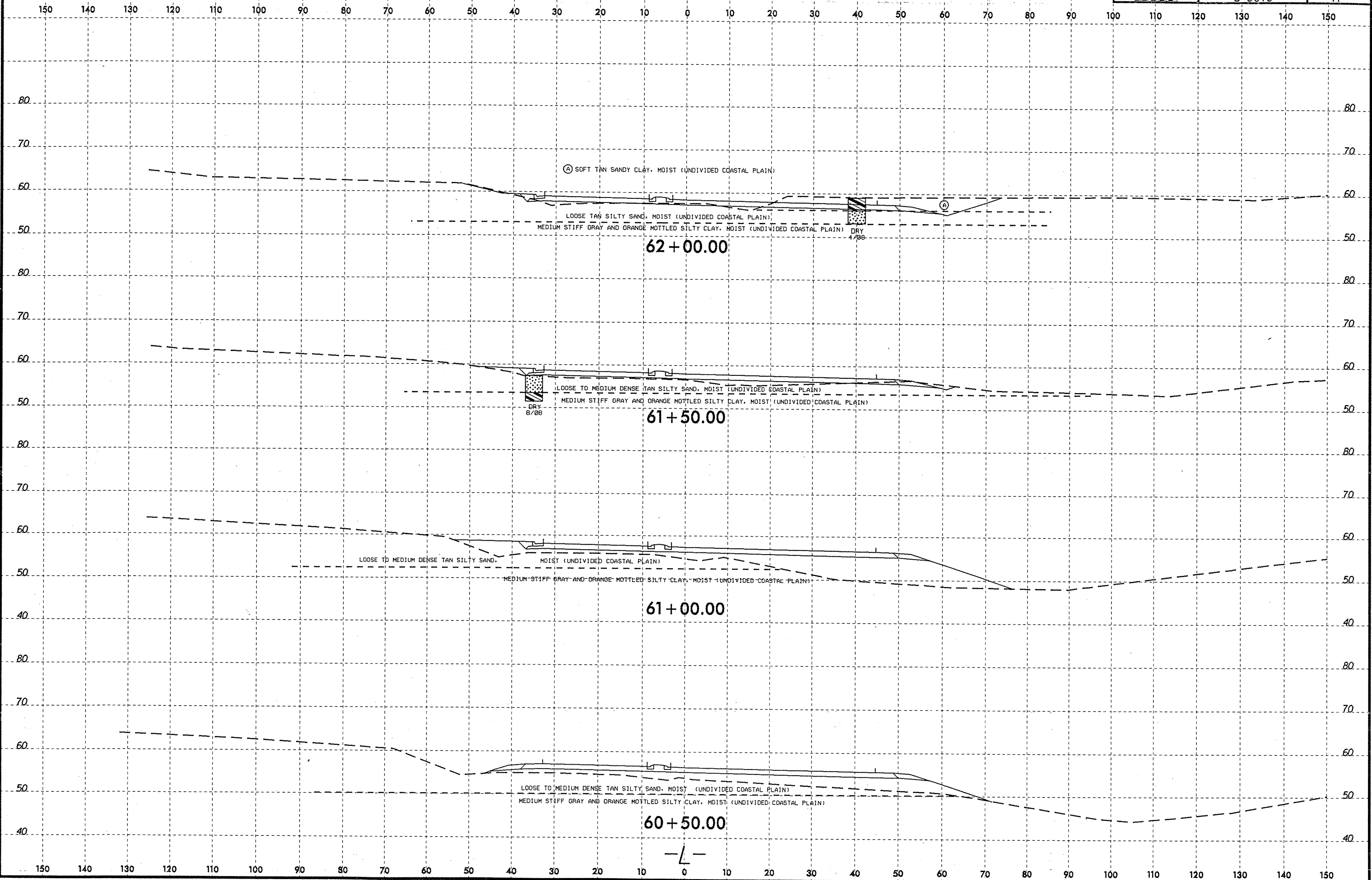
SOIL TEST RESULTS

SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	ASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE ORGANIC	
							C. SAND	F. SAND	SILT	CLAY	#10	#40	#200	MOISTURE	ORGANIC
SS-14	20'	RT 59+50	1.00-1.50	A-7-6(23)	54	26	3.0	18.9	19.2	58.9	100	99	81	-	-
SS-15	20'	RT 59+50	3.10-4.60	A-2-4(0)	18	NP	18.1	63.6	4.2	14.2	100	96	19	-	-
SS-16	20'	RT 59+50	8.10-9.60	A-3(0)	18	NP	19.3	71.0	1.6	8.1	100	98	10	-	-
SS-17	20'	RT 59+50	13.10-14.60	A-7-6(28)	59	31	0.8	20.3	22.0	56.9	100	100	82	-	-



8/23/99

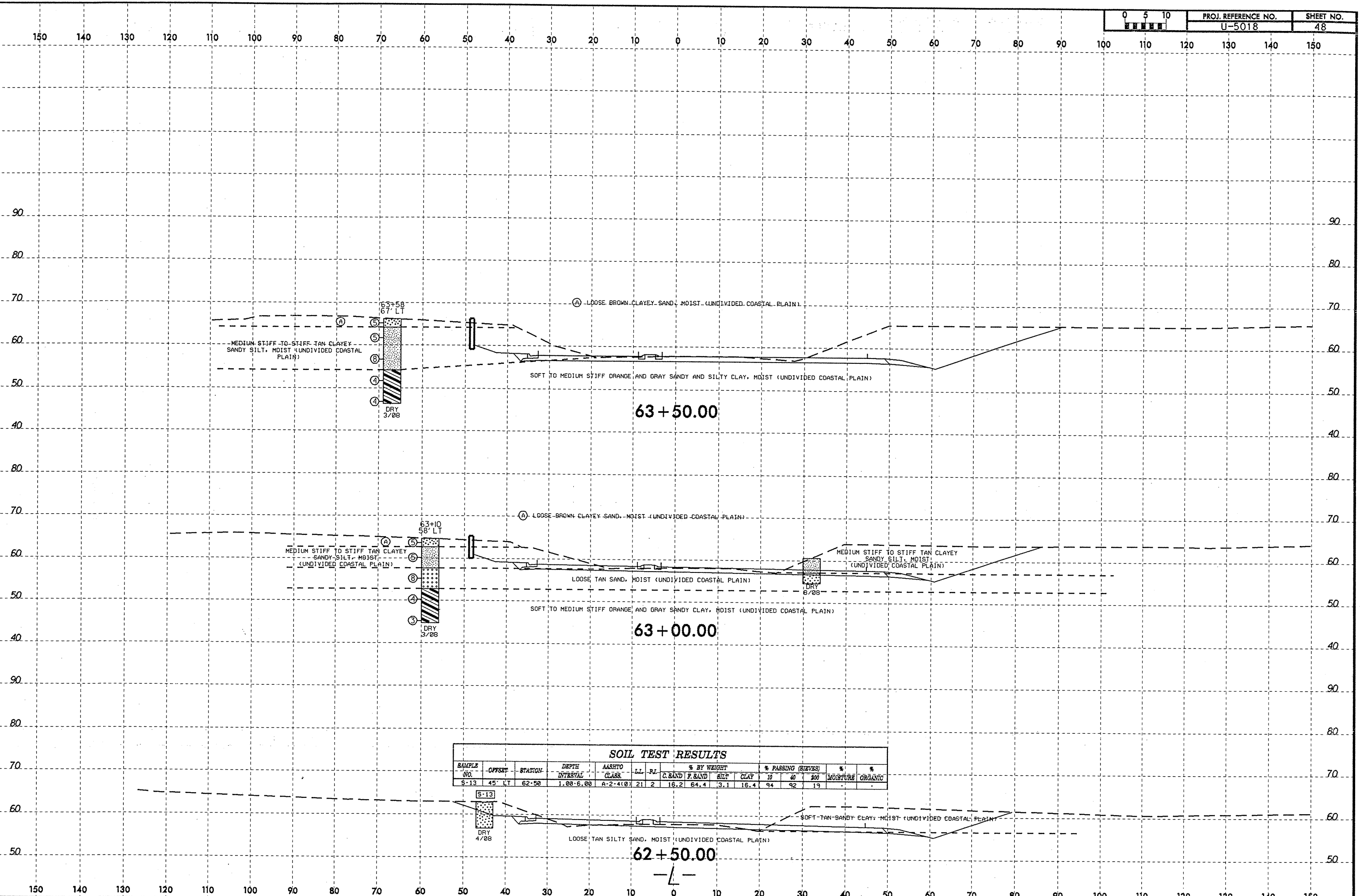
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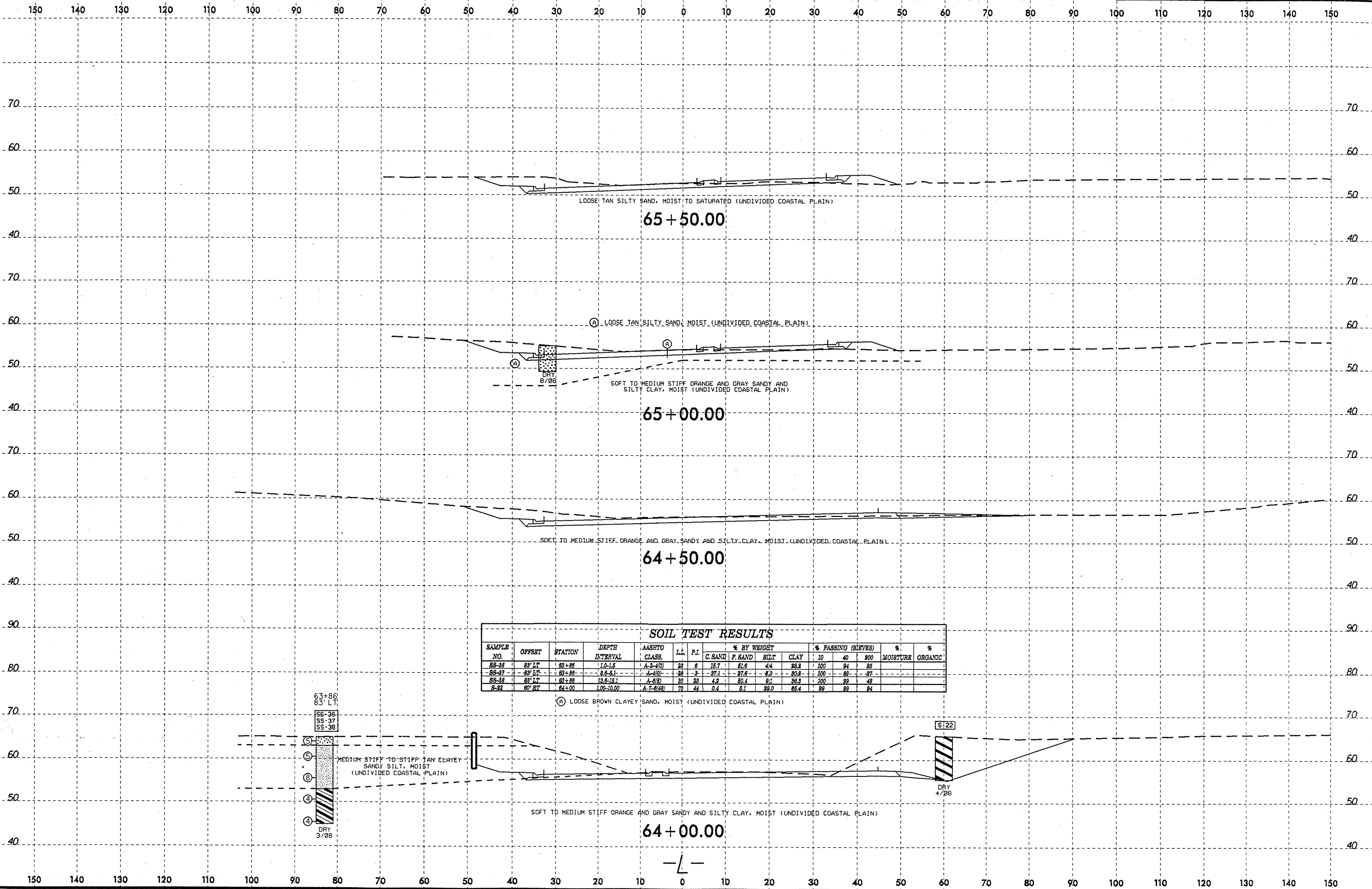
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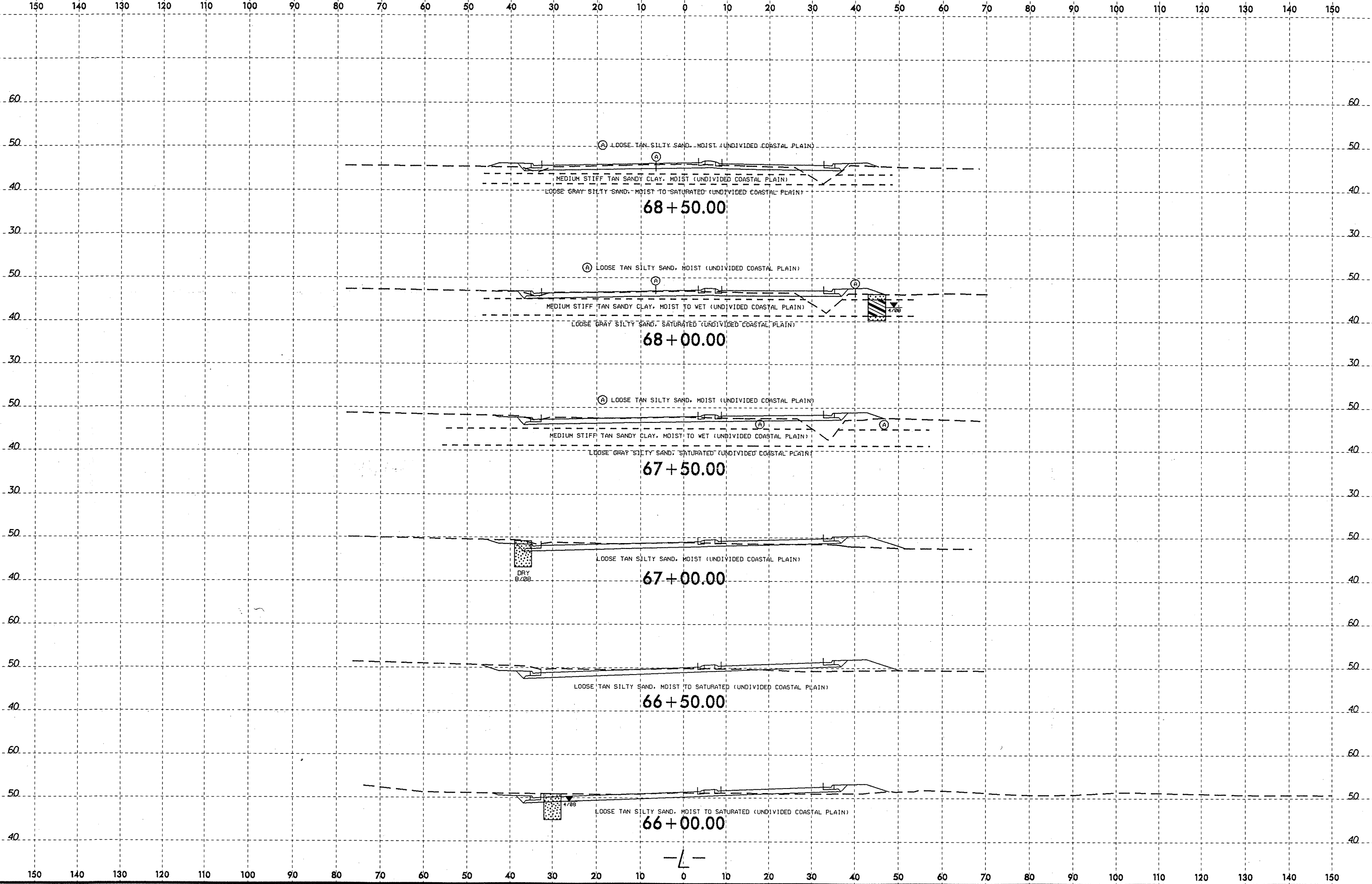


SOIL TEST RESULTS

SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE ORGANIC	
							C. SAND	F. SAND	SILT	CLAY	10	40	200	%	%
SS-36	83' LT	63+88	1.0-1.5	A-2-4(0)	28	8	15.7	61.6	4.4	28.2	100	94	85		
SS-37	83' LT	63+88	3.6-5.1	A-4(0)	28	8	37.1	27.6	4.9	20.2	100	89	37		
SS-38	83' LT	63+88	13.6-15.1	A-6(0)	20	28	4.2	50.4	9.1	38.2	100	99	48		
S-22	60' RT	64+00	1.00-10.00	A-7-6(48)	70	44	0.4	6.1	28.0	65.4	89	89	84		

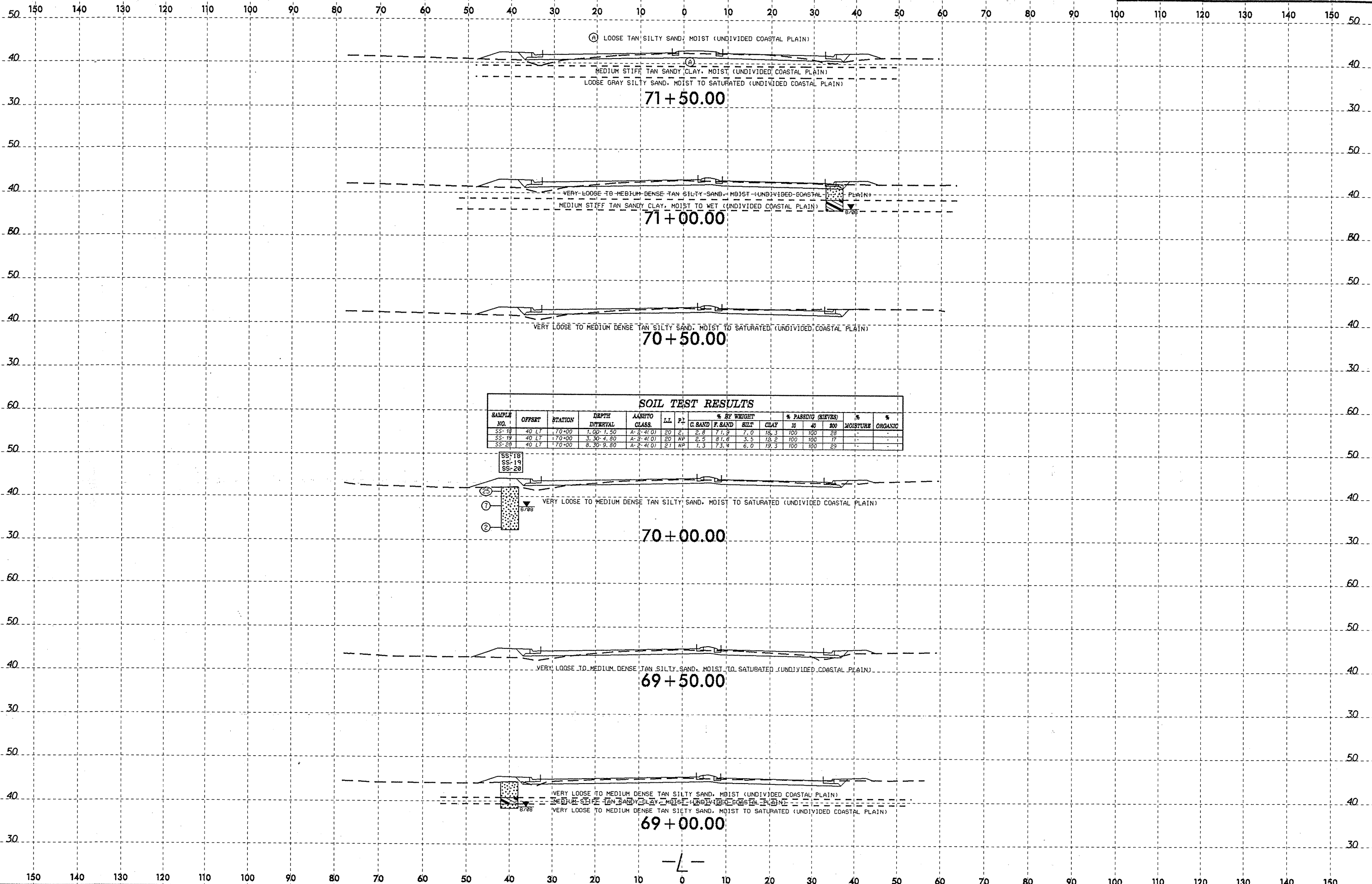
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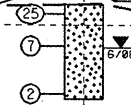
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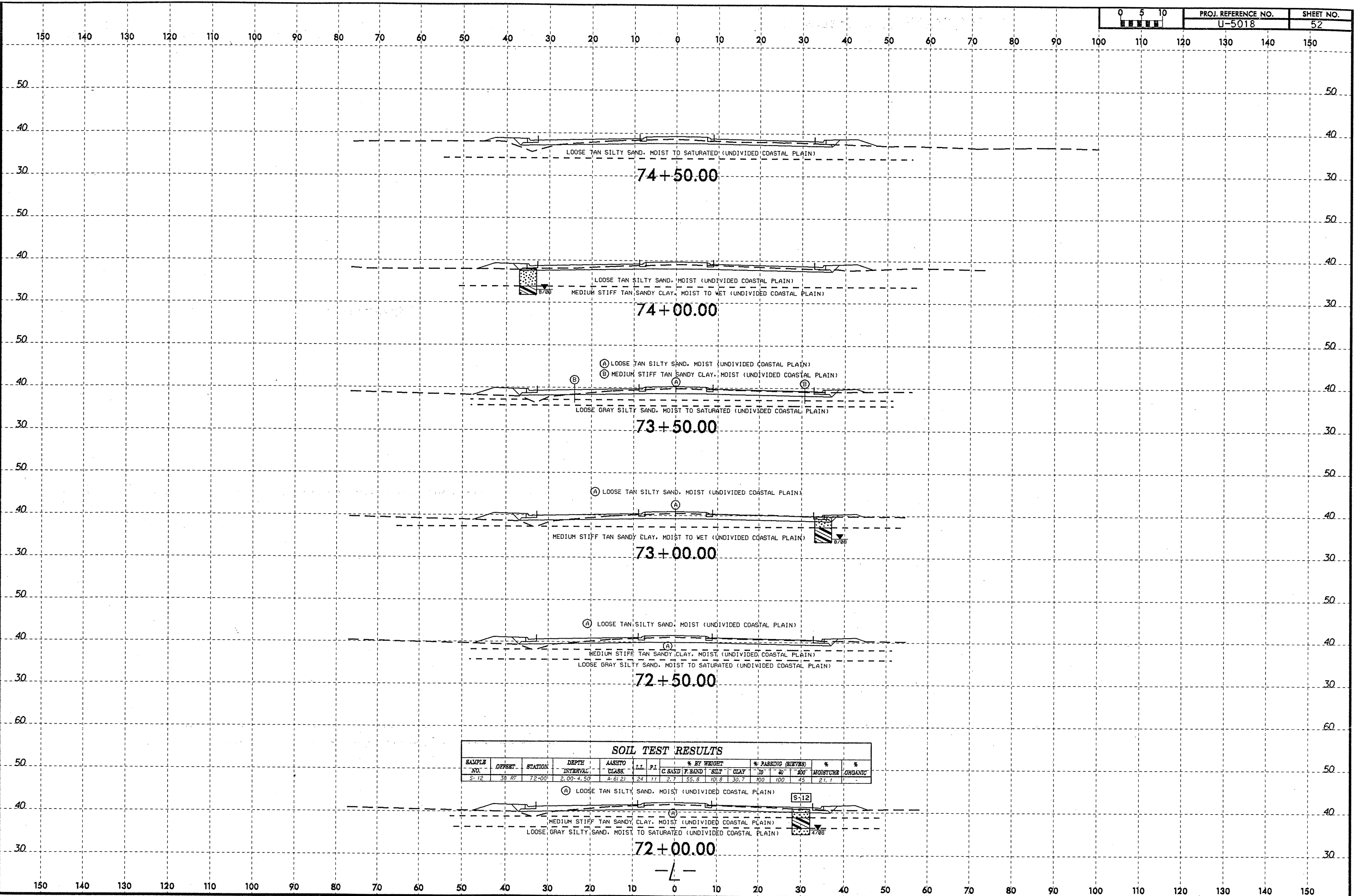
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SOIL TEST RESULTS															
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	LABORATORY CLASS.	LL	PL	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							G. SAND	F. SAND	SILT	CLAY	#10	#40	#200		
SS-18	40 LT	70+00	1.00-1.50	A-2-4(0)	20	21	2.8	71.9	7.0	18.3	100	100	28	-	-
SS-19	40 LT	70+00	3.30-4.80	A-2-4(0)	20	19	2.5	81.8	3.5	13.2	100	100	17	-	-
SS-20	40 LT	70+00	8.30-9.80	A-2-4(0)	21	19	1.3	73.4	6.0	19.3	100	100	29	-	-

SS-18
SS-19
SS-20



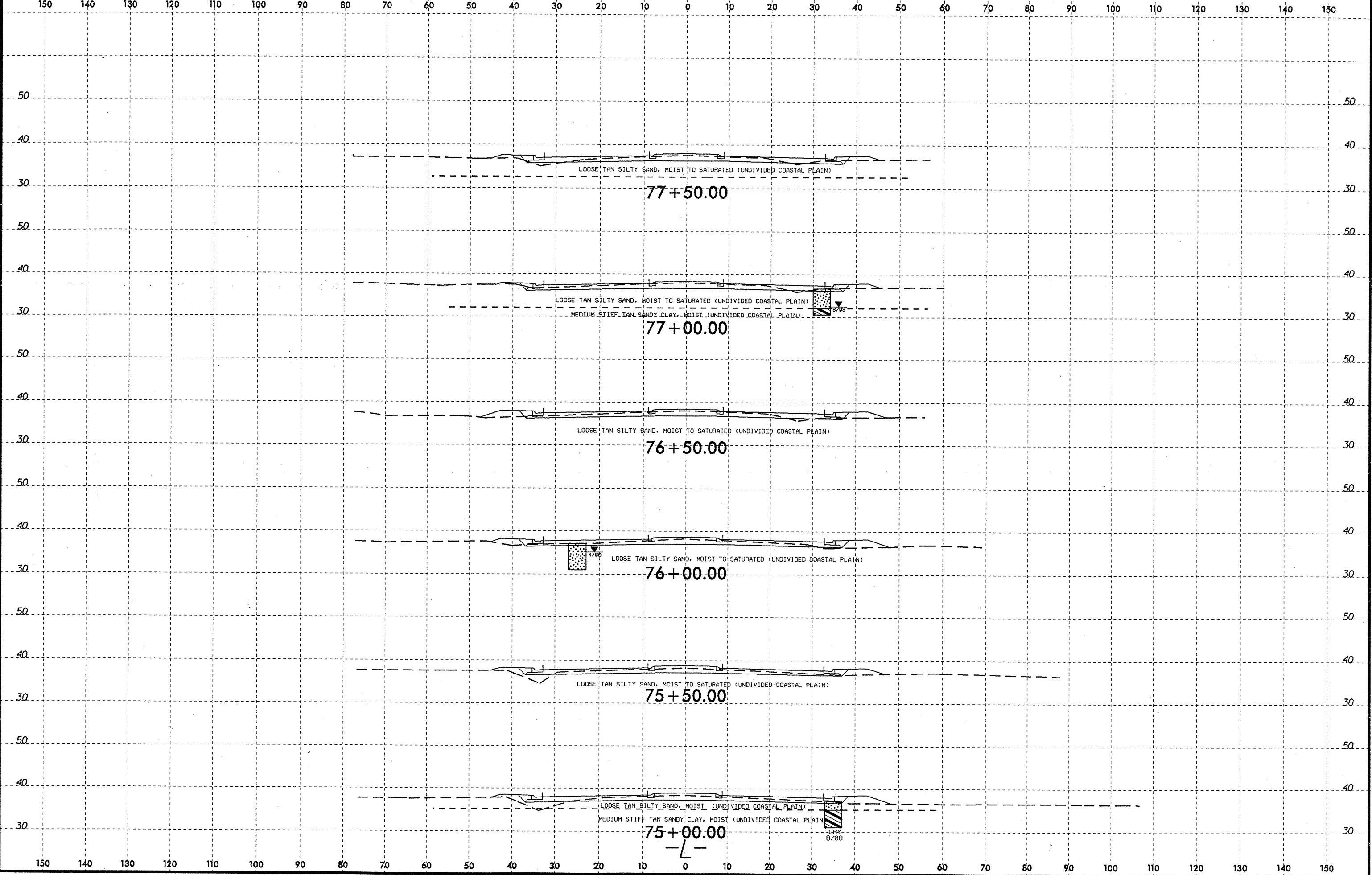
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SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	ASTM CLASS.	LL	PI	% BY WEIGHT			% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C SAND	F SAND	SILT	CLAY	#10	#40		
S-12	30 FT	72+00	2.00-4.50	A-6(2)	24	17	2.7	55.8	10.8	30.7	100	100	45	21.1

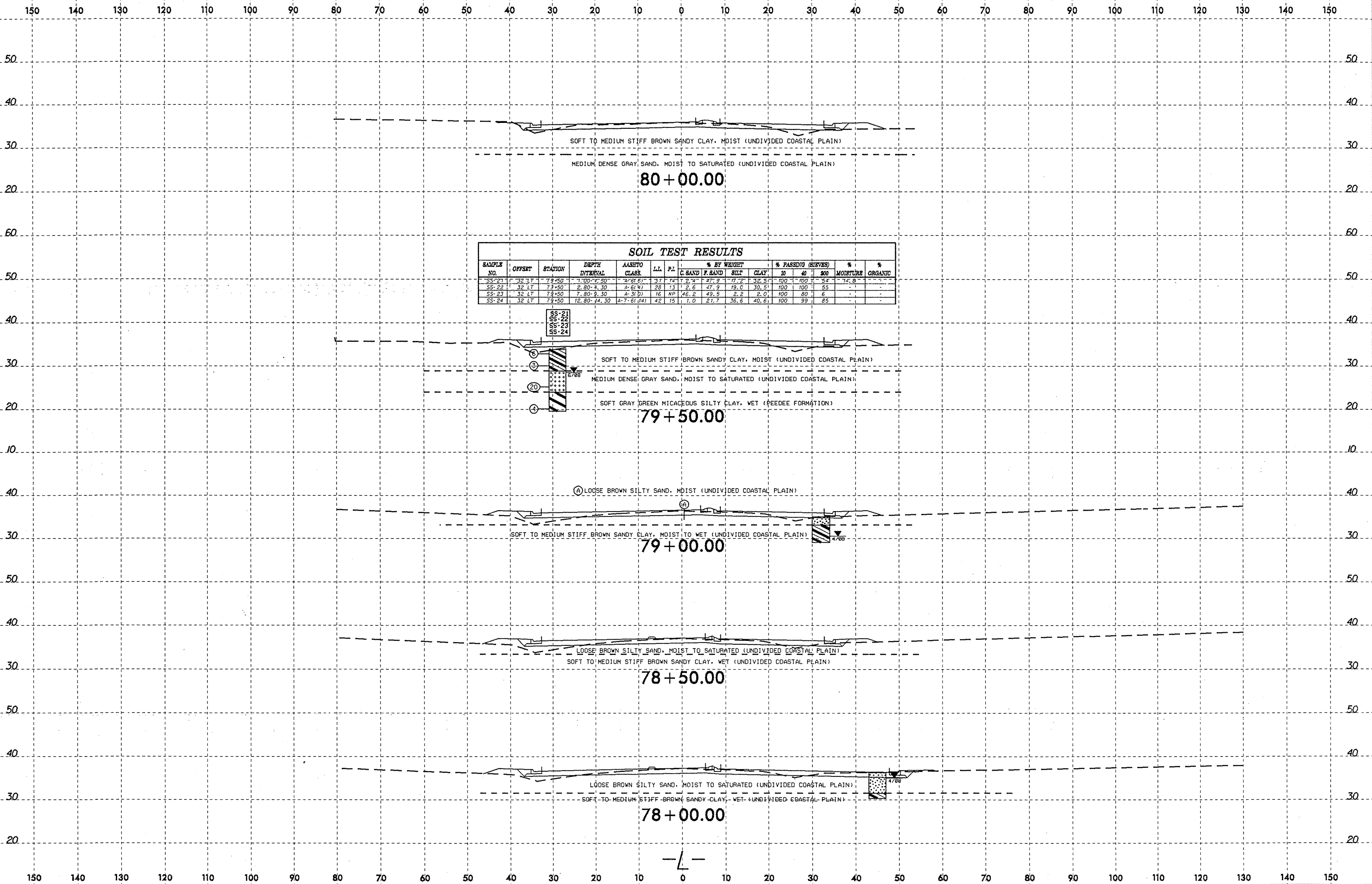
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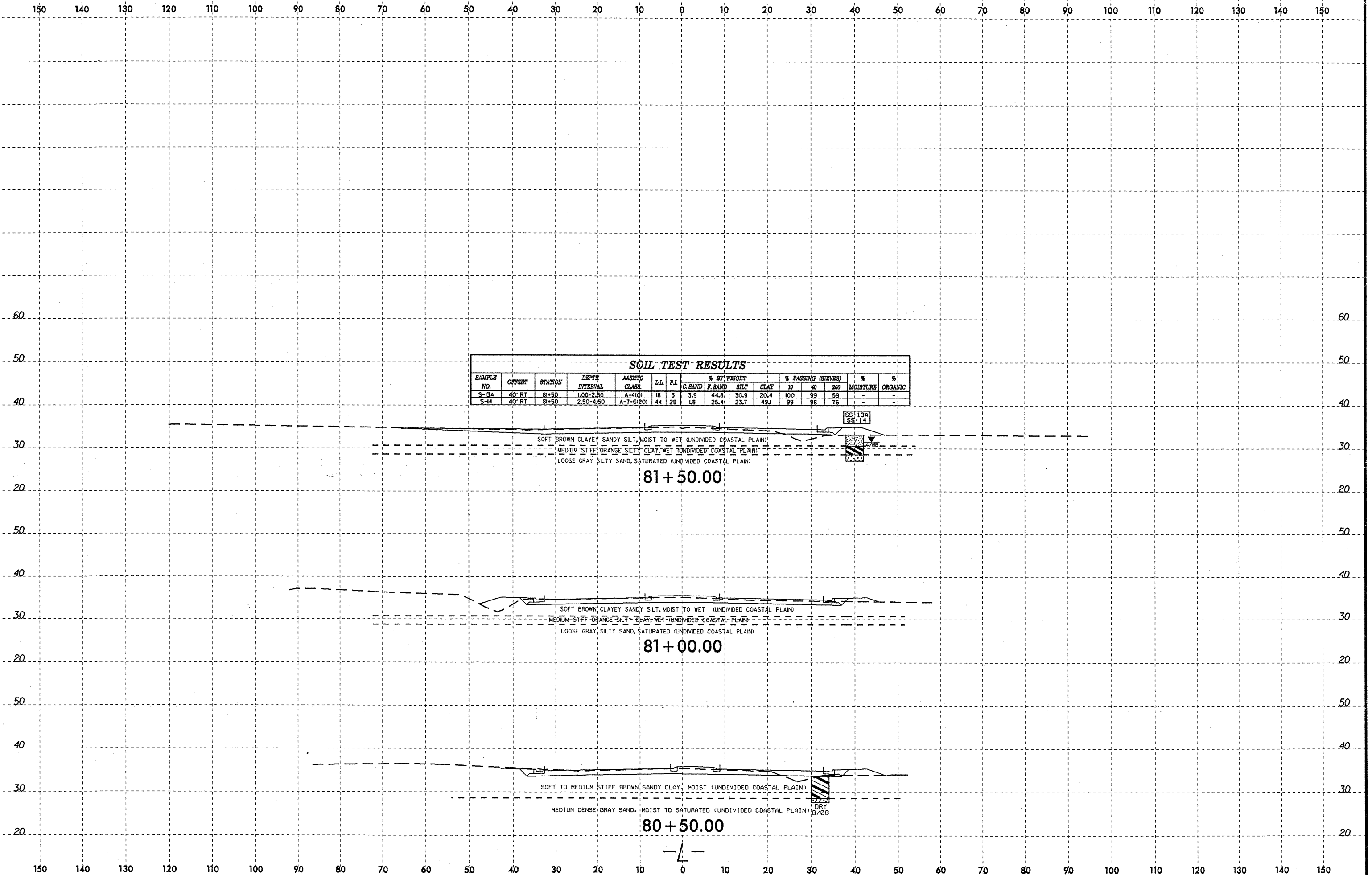
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SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	ASSETO CLASS	LL	PL	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C. SAND	F. SAND	SILT	CLAY	10	40	200		
SS-21	32 LT	79+50	1.00-1.50	A-2(6)	31	14	27.9	17.2	32.3	100	100	54	74.8	-	
SS-22	32 LT	79+50	2.80-3.30	A-3(D)	28	13	47.9	19.0	30.5	100	100	55	-	-	
SS-23	32 LT	79+50	7.80-9.30	A-3(D)	16	NP	46.2	49.5	2.2	2.0	100	80	5	-	
SS-24	32 LT	79+50	12.80-14.30	A-7-G(14)	42	15	1.0	21.7	36.6	40.6	100	99	85	-	

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SOIL TEST RESULTS

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							C. SAND	F. SAND	SILT	CLAY	10	40	200		
S-13A	40' RT	81+50	1.00-2.50	A-4(0)	18	3	3.9	44.8	30.9	20.4	100	99	59	-	-
S-14	40' RT	81+50	2.50-4.50	A-7-6(20)	44	28	1.8	25.4	23.7	49.1	99	98	76	-	-

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CONTRACT: ID: U-5018

NOTE: SEE SHEET 3 FOR PLAN SHEET LAYOUT AT TIME OF INVESTIGATION

CONTENTS

LINE	STATION	PLAN	PROFILE
-L-	12+25 TO 145+84	4-14	15-24

CROSS SECTIONS

LINE	STATION	SHEET NO.
-L-	13+50 TO 53+00	25-44
-L-	57+50 TO 81+50	45-55

STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 DIVISION OF HIGHWAYS
 GEOTECHNICAL ENGINEERING UNIT

ROADWAY
SUBSURFACE INVESTIGATION

PROJ. REFERENCE NO. 41431.1.1 (U-5018) F.A. PROJ. STP-0043 (8)
 COUNTY PITT
 PROJECT DESCRIPTION NC 43 FROM US 264 TO NC 11
(MEMORIAL DRIVE)

RECOMMENDATIONS

STATE	STATE PROJECT REFERENCE NO.	SHEET NO.	TOTAL SHEETS
N.C.	U-5018	1	55
STATE PROJ. NO.	F.A. PROJ. NO.	DESCRIPTION	
41431.1.1 (U-5018)	STP-0043 (8)	P.E. RW & UTIL.	

CAUTION NOTICE

THE SUBSURFACE INFORMATION AND THE SUBSURFACE INVESTIGATION ON WHICH IT IS BASED WERE MADE FOR THE PURPOSE OF STUDY, PLANNING, AND DESIGN, AND NOT FOR CONSTRUCTION OR PAY PURPOSES. THE VARIOUS FIELD BORING LOGS, ROCK CORES, AND SOIL TEST DATA AVAILABLE MAY BE REVIEWED OR INSPECTED IN RALEIGH BY CONTACTING THE N.C. DEPARTMENT OF TRANSPORTATION, GEOTECHNICAL ENGINEERING UNIT AT (919) 250-4088. NEITHER THE SUBSURFACE PLANS AND REPORTS, NOR THE FIELD BORING LOGS, ROCK CORES, OR SOIL TEST DATA ARE PART OF THE CONTRACT.

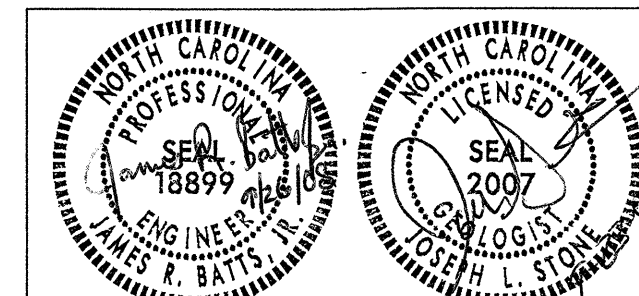
GENERAL SOIL AND ROCK STRATA DESCRIPTIONS AND INDICATED BOUNDARIES ARE BASED ON A GEOTECHNICAL INTERPRETATION OF ALL AVAILABLE SUBSURFACE DATA AND MAY NOT NECESSARILY REFLECT THE ACTUAL SUBSURFACE CONDITIONS BETWEEN BORINGS OR BETWEEN SAMPLED STRATA WITHIN THE BOREHOLE. THE LABORATORY SAMPLE DATA AND THE IN SITU (IN-PLACE) TEST DATA CAN BE RELIED ON ONLY TO THE DEGREE OF RELIABILITY INHERENT IN THE STANDARD TEST METHOD. THE OBSERVED WATER LEVELS OR SOIL MOISTURE CONDITIONS INDICATED IN THE SUBSURFACE INVESTIGATIONS ARE AS RECORDED AT THE TIME OF THE INVESTIGATION. THESE WATER LEVELS OR SOIL MOISTURE CONDITIONS MAY VARY CONSIDERABLY WITH TIME ACCORDING TO CLIMATIC CONDITIONS INCLUDING TEMPERATURES, PRECIPITATION, AND WIND, AS WELL AS OTHER NON-CLIMATIC FACTORS.

THE BIDDER OR CONTRACTOR IS CAUTIONED THAT DETAILS SHOWN ON THE SUBSURFACE PLANS ARE PRELIMINARY ONLY AND IN MANY CASES THE FINAL DESIGN DETAILS ARE DIFFERENT. FOR BIDDING AND CONSTRUCTION PURPOSES, REFER TO THE CONSTRUCTION PLANS AND DOCUMENTS FOR FINAL DESIGN INFORMATION ON THIS PROJECT. THE DEPARTMENT DOES NOT WARRANT OR GUARANTEE THE SUFFICIENCY OR ACCURACY OF THE INVESTIGATION MADE, NOR THE INTERPRETATIONS MADE, OR OPINION OF THE DEPARTMENT AS TO THE TYPE OF MATERIALS AND CONDITIONS TO BE ENCOUNTERED. THE BIDDER OR CONTRACTOR IS CAUTIONED TO MAKE SUCH INDEPENDENT SUBSURFACE INVESTIGATIONS AS HE DEEMS NECESSARY TO SATISFY HIMSELF AS TO CONDITIONS TO BE ENCOUNTERED ON THIS PROJECT. THE CONTRACTOR SHALL HAVE NO CLAIM FOR ADDITIONAL COMPENSATION OR FOR AN EXTENSION OF TIME FOR ANY REASON RESULTING FROM THE ACTUAL CONDITIONS ENCOUNTERED AT THE SITE DIFFERING FROM THOSE INDICATED IN THE SUBSURFACE INFORMATION.

PERSONNEL

- TCB
- JRS
- RES
- S&ME

INVESTIGATED BY TC BOTTOMS
 CHECKED BY DN ARGENBRIGHT
 SUBMITTED BY DN ARGENBRIGHT
 DATE SEPTEMBER, 2008



DRAWN BY: T.C. BOTTOMS, C.P. TURNER

NOTE - THE INFORMATION CONTAINED HEREIN IS NOT IMPLIED OR GUARANTEED BY THE N.C. DEPARTMENT OF TRANSPORTATION AS BEING ACCURATE NOR IS IT CONSIDERED TO BE PART OF THE PLANS, SPECIFICATIONS, OR CONTRACT FOR THE PROJECT.





NOTE - BY HAVING REQUESTED THIS INFORMATION THE CONTRACTOR SPECIFICALLY WAIVES ANY CLAIMS FOR INCREASED COMPENSATION OR EXTENSION OF TIME BASED ON DIFFERENCES BETWEEN THE CONDITIONS INDICATED HEREIN AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

**NORTH CAROLINA DEPARTMENT OF TRANSPORTATION
DIVISION OF HIGHWAYS
GEOTECHNICAL ENGINEERING UNIT**

PROJECT REFERENCE NO. U-5018	SHEET NO. 2
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SUBSURFACE INVESTIGATION

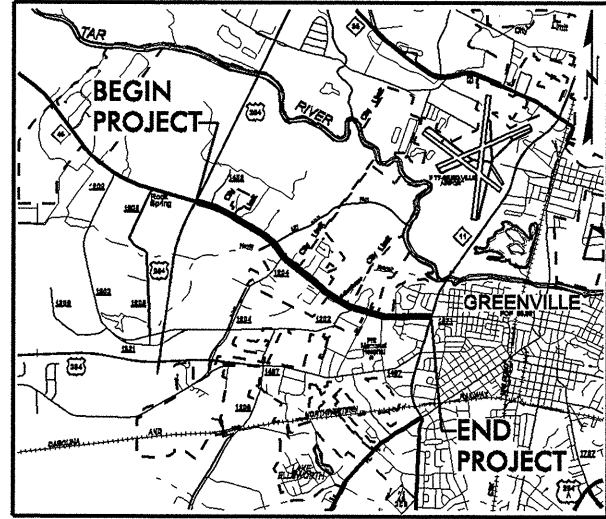
SOIL AND ROCK LEGEND, TERMS, SYMBOLS, AND ABBREVIATIONS

SOIL DESCRIPTION		GRADATION		ROCK DESCRIPTION		TERMS AND DEFINITIONS					
SOIL IS CONSIDERED TO BE THE UNCONSOLIDATED, SEMI-CONSOLIDATED, OR WEATHERED EARTH MATERIALS THAT CAN BE PENETRATED WITH A CONTINUOUS FLIGHT POWER AUGER, AND YIELD LESS THAN 100 BLOWS PER FOOT ACCORDING TO STANDARD PENETRATION TEST (AASHTO T286, ASTM D-1586). SOIL CLASSIFICATION IS BASED ON THE AASHTO SYSTEM. BASIC DESCRIPTIONS GENERALLY SHALL INCLUDE: CONSISTENCY, COLOR, TEXTURE, MOISTURE, AASHTO CLASSIFICATION, AND OTHER PERTINENT FACTORS SUCH AS MINERALOGICAL COMPOSITION, ANGULARITY, STRUCTURE, PLASTICITY, ETC. EXAMPLE: <i>VERY STIFF, GRAY, SILTY CLAY, MOST WITH INTERBEDDED FINE SAND LAYERS, HIGH PLASTIC, A-7-6</i>		WELL GRADED - INDICATES A GOOD REPRESENTATION OF PARTICLE SIZES FROM FINE TO COARSE. UNIFORM - INDICATES THAT SOIL PARTICLES ARE ALL APPROXIMATELY THE SAME SIZE. (ALSO POORLY GRADED) POORLY GRADED GAP-GRADED - INDICATES A MIXTURE OF UNIFORM PARTICLES OF TWO OR MORE SIZES. THE ANGULARITY OR ROUNDNESS OF SOIL GRAINS IS DESIGNATED BY THE TERMS: ANGULAR, SUBANGULAR, SUBROUNDED, OR ROUNDED.		HARD ROCK IS NON-COASTAL PLAIN MATERIAL THAT IF TESTED, WOULD YIELD SPT REFUSAL. AN INFERRED ROCK LINE INDICATES THE LEVEL AT WHICH NON-COASTAL PLAIN MATERIAL WOULD YIELD SPT REFUSAL. SPT REFUSAL IS PENETRATION BY A SPLIT SPOON SAMPLER EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS. IN NON-COASTAL PLAIN MATERIAL, THE TRANSITION BETWEEN SOIL AND ROCK IS OFTEN REPRESENTED BY A ZONE OF WEATHERED ROCK. ROCK MATERIALS ARE TYPICALLY DIVIDED AS FOLLOWS:  WEATHERED ROCK (WR)  CRYSTALLINE ROCK (CR)  NON-CRYSTALLINE ROCK (NCR)  COASTAL PLAIN SEDIMENTARY ROCK (CP)		ALLUVIUM (ALLUV.) - SOILS THAT HAVE BEEN TRANSPORTED BY WATER. AQUIFER - A WATER BEARING FORMATION OR STRATA. ARENACEOUS - APPLIED TO ROCKS THAT HAVE BEEN DERIVED FROM SAND OR THAT CONTAIN SAND. ARGILLACEOUS - APPLIED TO ALL ROCKS OR SUBSTANCES COMPOSED OF CLAY MINERALS, OR HAVING A NOTABLE PROPORTION OF CLAY IN THEIR COMPOSITION, AS SHALE, SLATE, ETC. ARTESIAN - GROUND WATER THAT IS UNDER SUFFICIENT PRESSURE TO RISE ABOVE THE LEVEL AT WHICH IT IS ENCOUNTERED, BUT WHICH DOES NOT NECESSARILY RISE TO OR ABOVE THE GROUND SURFACE. CALCAREOUS (CALC.) - SOILS THAT CONTAIN APPRECIABLE AMOUNTS OF CALCIUM CARBONATE. COLLUVIUM - ROCK FRAGMENTS MIXED WITH SOIL DEPOSITED BY GRAVITY ON SLOPE OR AT BOTTOM OF SLOPE. CORE RECOVERY (REC.) - TOTAL LENGTH OF ALL MATERIAL RECOVERED IN THE CORE BARREL DIVIDED BY TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE. DIKE - A TABULAR BODY OF IGNEOUS ROCK THAT CUTS ACROSS THE STRUCTURE OF ADJACENT ROCKS OR CUTS MASSIVE ROCK. DIP - THE ANGLE AT WHICH A STRATUM OR ANY PLANAR FEATURE IS INCLINED FROM THE HORIZONTAL. DIP DIRECTION (DIP AZIMUTH) - THE DIRECTION OR BEARING OF THE HORIZONTAL TRACE OF THE LINE OF DIP, MEASURED CLOCKWISE FROM NORTH. FAULT - A FRACTURE OR FRACTURE ZONE ALONG WHICH THERE HAS BEEN DISPLACEMENT OF THE SIDES RELATIVE TO ONE ANOTHER PARALLEL TO THE FRACTURE. FISSILE - A PROPERTY OF SPLITTING ALONG CLOSELY SPACED PARALLEL PLANES. FLOAT - ROCK FRAGMENTS ON SURFACE NEAR THEIR ORIGINAL POSITION AND DISLODGED FROM PARENT MATERIAL. FLOOD PLAIN (FP) - LAND BORDERING A STREAM, BUILT OF SEDIMENTS DEPOSITED BY THE STREAM. FORMATION (FM) - A MAPPABLE GEOLOGIC UNIT THAT CAN BE RECOGNIZED AND TRACED IN THE FIELD. JOINT - FRACTURE IN ROCK ALONG WHICH NO APPRECIABLE MOVEMENT HAS OCCURRED. LEDGE - A SHELF-LIKE RIDGE OR PROJECTION OF ROCK WHOSE THICKNESS IS SMALL COMPARED TO ITS LATERAL EXTENT. LENS - A BODY OF SOIL OR ROCK THAT THINS OUT IN ONE OR MORE DIRECTIONS. MOTTLED (MOT.) - IRREGULARLY MARKED WITH SPOTS OF DIFFERENT COLORS, MOTTLING IN SOILS USUALLY INDICATES POOR AERATION AND LACK OF GOOD DRAINAGE. PERCHED WATER - WATER MAINTAINED ABOVE THE NORMAL GROUND WATER LEVEL BY THE PRESENCE OF AN INTERVENING IMPERVIOUS STRATUM. RESIDUAL (RES.) SOIL - SOIL FORMED IN PLACE BY THE WEATHERING OF ROCK. ROCK QUALITY DESIGNATION (ROD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE. SAPROLITE (SAP.) - RESIDUAL SOIL THAT RETAINS THE RELIC STRUCTURE OR FABRIC OF THE PARENT ROCK. SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, THAT HAS BEEN REPLACED PARALLEL TO THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS. SLICKENSIDE - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT OR SLIP PLANE. STANDARD PENETRATION TEST (PENETRATION RESISTANCE) (SPT) - NUMBER OF BLOWS (N OR BPF) OF A 140 LB. HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF 1 FOOT INTO SOIL WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT REFUSAL IS PENETRATION EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS. STRATA CORE RECOVERY (SREC.) - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY TOTAL LENGTH OF STRATUM AND EXPRESSED AS A PERCENTAGE. STRATA ROCK QUALITY DESIGNATION (SRQD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS WITHIN A STRATUM EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF STRATA AND EXPRESSED AS A PERCENTAGE. TOPSOIL (TS.) - SURFACE SOILS USUALLY CONTAINING ORGANIC MATTER.					
SOIL LEGEND AND AASHTO CLASSIFICATION		MINERALOGICAL COMPOSITION		WEATHERING							
GENERAL CLASS. GRANULAR MATERIALS (<= 35% PASSING #200) SILT-CLAY MATERIALS (> 35% PASSING #200) ORGANIC MATERIALS		MINERAL NAMES SUCH AS QUARTZ, FELDSPAR, MICA, TALC, KAOLIN, ETC. ARE USED IN DESCRIPTIONS WHENEVER THEY ARE CONSIDERED OF SIGNIFICANCE.		FRESH ROCK FRESH, CRYSTALS BRIGHT, FEW JOINTS MAY SHOW SLIGHT STAINING. ROCK RINGS UNDER HAMMER IF CRYSTALLINE. VERY SLIGHT (V SLJ) ROCK GENERALLY FRESH, JOINTS STAINED, SOME JOINTS MAY SHOW THIN CLAY COATINGS IF OPEN, CRYSTALS ON A BROKEN SPECIMEN FACE SHINE BRIGHTLY. ROCK RINGS UNDER HAMMER BLOWS IF OF A CRYSTALLINE NATURE. SLIGHT (SLJ) ROCK GENERALLY FRESH, JOINTS STAINED AND DISCOLORATION EXTENDS INTO ROCK UP TO 1 INCH. OPEN JOINTS MAY CONTAIN CLAY. IN GRANITOID ROCKS SOME OCCASIONAL FELDSPAR CRYSTALS ARE DULL AND DISCOLORED. CRYSTALLINE ROCKS RING UNDER HAMMER BLOWS. MODERATE (MOD) SIGNIFICANT PORTIONS OF ROCK SHOW DISCOLORATION AND WEATHERING EFFECTS. IN GRANITOID ROCKS, MOST FELDSPARS ARE DULL AND DISCOLORED, SOME SHOW CLAY. ROCK HAS DULL SOUND UNDER HAMMER BLOWS AND SHOWS SIGNIFICANT LOSS OF STRENGTH AS COMPARED WITH FRESH ROCK. MODERATELY SEVERE (MOD, SEV) ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. IN GRANITOID ROCKS, ALL FELDSPARS DULL AND DISCOLORED AND A MAJORITY SHOW KAOLINIZATION. ROCK SHOWS SEVERE LOSS OF STRENGTH AND CAN BE EXCAVATED WITH A GEOLOGIST'S PICK. ROCK GIVES "CLUNK" SOUND WHEN STRUCK. <i>IF TESTED, WOULD YIELD SPT REFUSAL</i> SEVERE (SEV) ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. ROCK FABRIC CLEAR AND EVIDENT BUT REDUCED IN STRENGTH TO STRONG SOIL. IN GRANITOID ROCKS ALL FELDSPARS ARE KAOLINIZED TO SOME EXTENT. SOME FRAGMENTS OF STRONG ROCK USUALLY REMAIN. <i>IF TESTED, WOULD YIELD SPT REFUSAL</i> VERY SEVERE (V SEV) ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. ROCK FABRIC ELEMENTS ARE DISCERNIBLE BUT THE MASS IS EFFECTIVELY REDUCED TO SOIL STATUS, WITH ONLY FRAGMENTS OF STRONG ROCK REMAINING. SAPROLITE IS AN EXAMPLE OF ROCK WEATHERED TO A DEGREE SUCH THAT ONLY MINOR VESTIGES OF THE ORIGINAL ROCK FABRIC REMAIN. <i>IF TESTED, YIELDS SPT N VALUES < 100 BPF</i> COMPLETE ROCK REDUCED TO SOIL. ROCK FABRIC NOT DISCERNIBLE, OR DISCERNIBLE ONLY IN SMALL AND SCATTERED CONCENTRATIONS. QUARTZ MAY BE PRESENT AS DIKES OR STRINGERS. SAPROLITE IS ALSO AN EXAMPLE.		SLIGHTLY COMPRESSIBLE LIQUID LIMIT LESS THAN 31 MODERATELY COMPRESSIBLE LIQUID LIMIT EQUAL TO 31-50 HIGHLY COMPRESSIBLE LIQUID LIMIT GREATER THAN 50		WEATHERING			
COMPRESSION		PERCENTAGE OF MATERIAL		GROUND WATER		MISCELLANEOUS SYMBOLS					
TRACE OF ORGANIC MATTER 2 - 3% LITTLE ORGANIC MATTER 3 - 5% MODERATELY ORGANIC 5 - 10% HIGHLY ORGANIC >10%		ORGANIC MATERIAL GRANULAR SOILS SILT - CLAY SOILS OTHER MATERIAL TRACE OF ORGANIC MATTER 2 - 3% 3 - 5% TRACE 1 - 10% LITTLE ORGANIC MATTER 3 - 5% 5 - 12% LITTLE 10 - 20% MODERATELY ORGANIC 5 - 10% 12 - 20% SOME 20 - 35% HIGHLY ORGANIC >10% >20% HIGHLY 35% AND ABOVE		WATER LEVEL IN BORE HOLE IMMEDIATELY AFTER DRILLING STATIC WATER LEVEL AFTER 24 HOURS PERCHED WATER, SATURATED ZONE, OR WATER BEARING STRATA SPRING OR SEEP		ROADWAY EMBANKMENT (RE) WITH SOIL DESCRIPTION SOIL SYMBOL ARTIFICIAL FILL (AF) OTHER THAN ROADWAY EMBANKMENT INFERRED SOIL BOUNDARY INFERRED ROCK LINE ALLUVIAL SOIL BOUNDARY DIP & DIP DIRECTION OF ROCK STRUCTURES SOUNDING ROD SPT TEST BORING AUGER BORING CORE BORING MONITORING WELL PIEZOMETER INSTALLATION SLOPE INDICATOR INSTALLATION SPT N-VALUE SPT REFUSAL S - BULK SAMPLE SS - SPLIT SPOON SAMPLE ST - SHELBY TUBE SAMPLE RS - ROCK SAMPLE RT - RECOMPACTED TRIAXIAL SAMPLE CBR - CALIFORNIA BEARING RATIO SAMPLE					
TEXTURE OR GRAIN SIZE		ABBREVIATIONS		ROCK HARDNESS		EQUIPMENT USED ON SUBJECT PROJECT					
U.S. STD. SIEVE SIZE OPENING (MM) 4 10 40 60 200 270 4.75 2.00 0.42 0.25 0.075 0.053		AR - AUGER REFUSAL BT - BORING TERMINATED CL - CLAY CPT - CONE PENETRATION TEST CSE - COARSE DMT - DILATOMETER TEST DPT - DYNAMIC PENETRATION TEST e - VOID RATIO F - FINE FOSS - FOSSILIFEROUS FRAC. - FRACTURED, FRACTURES FRAGS. - FRAGMENTS HI - HIGHLY MED. - MEDIUM MICA - MICACEOUS MOD. - MODERATELY NP - NON PLASTIC ORG. - ORGANIC PMT - PRESSUREMETER TEST SAP. - SAPROLITIC SD. - SAND, SANDY SL. - SILT, SILTY SLI. - SLIGHTLY TCR - TRICONE REFUSAL w - MOISTURE CONTENT v - VERY VST - VANE SHEAR TEST WEA. - WEATHERED w - UNIT WEIGHT w _c - DRY UNIT WEIGHT		VERY HARD CANNOT BE SCRATCHED BY KNIFE OR SHARP PICK. BREAKING OF HAND SPECIMENS REQUIRES SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK. HARD CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY. HARD HAMMER BLOWS REQUIRED TO DETACH HAND SPECIMEN. MODERATELY HARD CAN BE SCRATCHED BY KNIFE OR PICK. GOUGES OR GROOVES TO 0.25 INCHES DEEP CAN BE EXCAVATED BY HARD BLOW OF A GEOLOGIST'S PICK. HAND SPECIMENS CAN BE DETACHED BY MODERATE BLOWS. MEDIUM HARD CAN BE GROOVED OR GOUGED 0.05 INCHES DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT. CAN BE EXCAVATED IN SMALL CHIPS TO PEICES 1 INCH MAXIMUM SIZE BY HARD BLOWS OF THE POINT OF A GEOLOGIST'S PICK. SOFT CAN BE GROVED OR GOUGED READILY BY KNIFE OR PICK. CAN BE EXCAVATED IN FRAGMENTS FROM CHIPS TO SEVERAL INCHES IN SIZE BY MODERATE BLOWS OF A PICK POINT. SMALL, THIN PIECES CAN BE BROKEN BY FINGER PRESSURE. VERY SOFT CAN BE CARVED WITH KNIFE. CAN BE EXCAVATED READILY WITH POINT OF PICK. PIECES 1 INCH OR MORE IN THICKNESS CAN BE BROKEN BY FINGER PRESSURE. CAN BE SCRATCHED READILY BY FINGER NAIL.		DRILL UNITS: <input type="checkbox"/> MOBILE B- <input type="checkbox"/> BK-51 <input type="checkbox"/> CME-45B <input type="checkbox"/> CME-550 <input type="checkbox"/> PORTABLE HOIST <input checked="" type="checkbox"/> DIETRICH D-50 ADVANCING TOOLS: <input type="checkbox"/> CLAY BITS <input checked="" type="checkbox"/> 6" CONTINUOUS FLIGHT AUGER <input type="checkbox"/> 8" HOLLOW AUGERS <input type="checkbox"/> HARD FACED FINGER BITS <input type="checkbox"/> TUNG-CARBIDE INSERTS <input checked="" type="checkbox"/> CASING w/ ADVANCER <input checked="" type="checkbox"/> TRICONE 2 1/2" STEEL TEETH <input type="checkbox"/> TRICONE " TUNG-CARB. <input type="checkbox"/> CORE BIT HAMMER TYPE: <input checked="" type="checkbox"/> AUTOMATIC <input type="checkbox"/> MANUAL CORE SIZE: <input type="checkbox"/> B- <input type="checkbox"/> N- <input type="checkbox"/> H- HAND TOOLS: <input type="checkbox"/> POST HOLE DIGGER <input checked="" type="checkbox"/> HAND AUGER <input type="checkbox"/> SOUNDING ROD <input type="checkbox"/> VANE SHEAR TEST					
SOIL MOISTURE - CORRELATION OF TERMS		FRACTURE SPACING		BEDDING		INDURATION					
SOIL MOISTURE SCALE (ATTERBERG LIMITS) FIELD MOISTURE DESCRIPTION GUIDE FOR FIELD MOISTURE DESCRIPTION LL - LIQUID LIMIT PL - PLASTIC LIMIT OM - OPTIMUM MOISTURE SL - SHRINKAGE LIMIT - SATURATED - (SAT) USUALLY LIQUID; VERY WET, USUALLY FROM BELOW THE GROUND WATER TABLE - WET - (W) SEMISOLID; REQUIRES DRYING TO ATTAIN OPTIMUM MOISTURE - MOIST - (M) SOLID; AT OR NEAR OPTIMUM MOISTURE - DRY - (D) REQUIRES ADDITIONAL WATER TO ATTAIN OPTIMUM MOISTURE		TERM SPACING VERY WIDE MORE THAN 10 FEET WIDE 3 TO 10 FEET MODERATELY CLOSE 1 TO 3 FEET CLOSE 0.16 TO 1 FEET VERY CLOSE LESS THAN 0.16 FEET		TERM THICKNESS VERY THICKLY BEDDED > 4 FEET THICKLY BEDDED 1.5 - 4 FEET THINLY BEDDED 0.16 - 1.5 FEET VERY THINLY BEDDED 0.03 - 0.16 FEET THICKLY LAMINATED 0.008 - 0.03 FEET THINLY LAMINATED < 0.008 FEET		FOR SEDIMENTARY ROCKS, INDURATION IS THE HARDENING OF THE MATERIAL BY CEMENTING, HEAT, PRESSURE, ETC. FRIABLE RUBBING WITH FINGER FREES NUMEROUS GRAINS; GENTLE BLOW BY HAMMER DISINTEGRATES SAMPLE. MODERATELY INDURATED GRAINS CAN BE SEPARATED FROM SAMPLE WITH STEEL PROBE; BREAKS EASILY WHEN HIT WITH HAMMER. INDURATED GRAINS ARE DIFFICULT TO SEPARATE WITH STEEL PROBE; DIFFICULT TO BREAK WITH HAMMER. EXTREMELY INDURATED SHARP HAMMER BLOWS REQUIRED TO BREAK SAMPLE; SAMPLE BREAKS ACROSS GRAINS.					
PLASTICITY		BENCH MARK:		NOTES:		ELEVATION: FT.					
NONPLASTIC LOW PLASTICITY MED. PLASTICITY HIGH PLASTICITY PLASTICITY INDEX (PI) DRY STRENGTH VERY LOW SLIGHT MEDIUM HIGH 0-5 6-15 16-25 26 OR MORE		<input type="checkbox"/> _____		<input type="checkbox"/> _____		<input type="checkbox"/> _____					
DESCRIPTIONS MAY INCLUDE COLOR OR COLOR COMBINATIONS (TAN, RED, YELLOW-BROWN, BLUE-GRAY). MODIFIERS SUCH AS LIGHT, DARK, STREAKED, ETC. ARE USED TO DESCRIBE APPEARANCE.											

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 cp\turner AT 6EG240345

CONTRACT: C201904 **TIP PROJECT: U-5018**

See Sheet 1-A For Index of Sheets
See Sheet 1-B For Conventional Symbols



VICINITY MAP

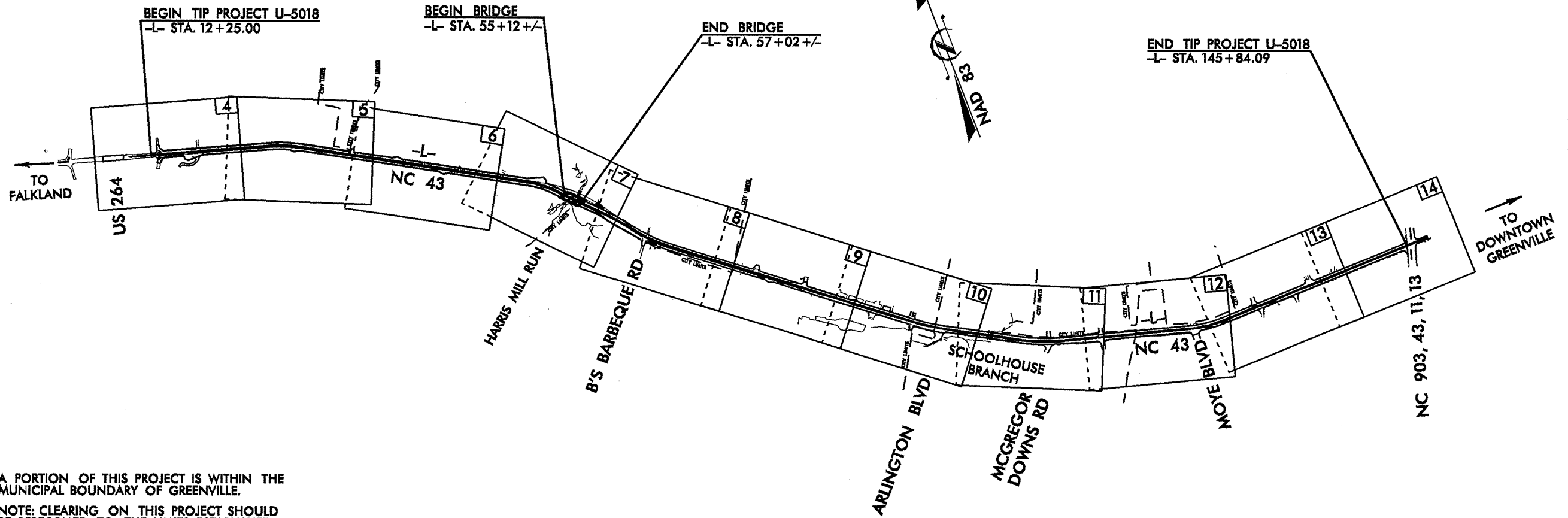
STATE OF NORTH CAROLINA
DIVISION OF HIGHWAYS

PITT COUNTY

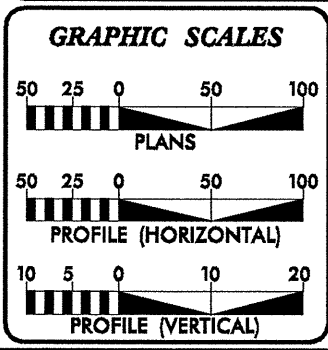
LOCATION: NC 43 FROM US 264 TO NC 11

TYPE OF WORK: GRADING, PAVING, DRAINAGE, AND STRUCTURE

STATE	STATE PROJECT REFERENCE NO.	SUBST. NO.	TOTAL SHEETS
N.C.	U-5018	3	
WSR NO.	P.A. PROJ. NO.	DESCRIPTION	
41431.1.1	STP-0043(0)	P.E.	
41431.3.1	STP-0043(0)	CONST.	



A PORTION OF THIS PROJECT IS WITHIN THE MUNICIPAL BOUNDARY OF GREENVILLE.
NOTE: CLEARING ON THIS PROJECT SHOULD BE PERFORMED TO THE LIMITS ESTABLISHED BY METHOD III.



DESIGN DATA

ADT 2007 =	19,700
ADT 2029 =	40,600
DHV =	10 %
D =	50 %
T =	6 % *
V =	50 MPH
(* TTST 2 % + DUAL 4 %)	

PROJECT LENGTH

LENGTH ROADWAY TIP PROJECT U-5018	=	2.494 MILES
LENGTH STRUCTURE TIP PROJECT U-5018	=	0.036 MILES
TOTAL LENGTH TIP PROJECT U-5018	=	2.530 MILES

Prepared in the Office of:

MULKEY
ENGINEERS & CONSULTANTS
FOR
NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

2006 STANDARD SPECIFICATIONS

RIGHT OF WAY DATE: ???????	TIM JORDAN, PE PROJECT ENGINEER
LETTING DATE: AUGUST 19, 2008	JEFF RECK, PE HYDRAULICS ENGINEER

NCDOT CONTACT: JOHN ROUSE

HYDRAULICS ENGINEER

SIGNATURE: _____ P.E.

ROADWAY DESIGN ENGINEER

SIGNATURE: _____ P.E.

**DIVISION OF HIGHWAYS
STATE OF NORTH CAROLINA**

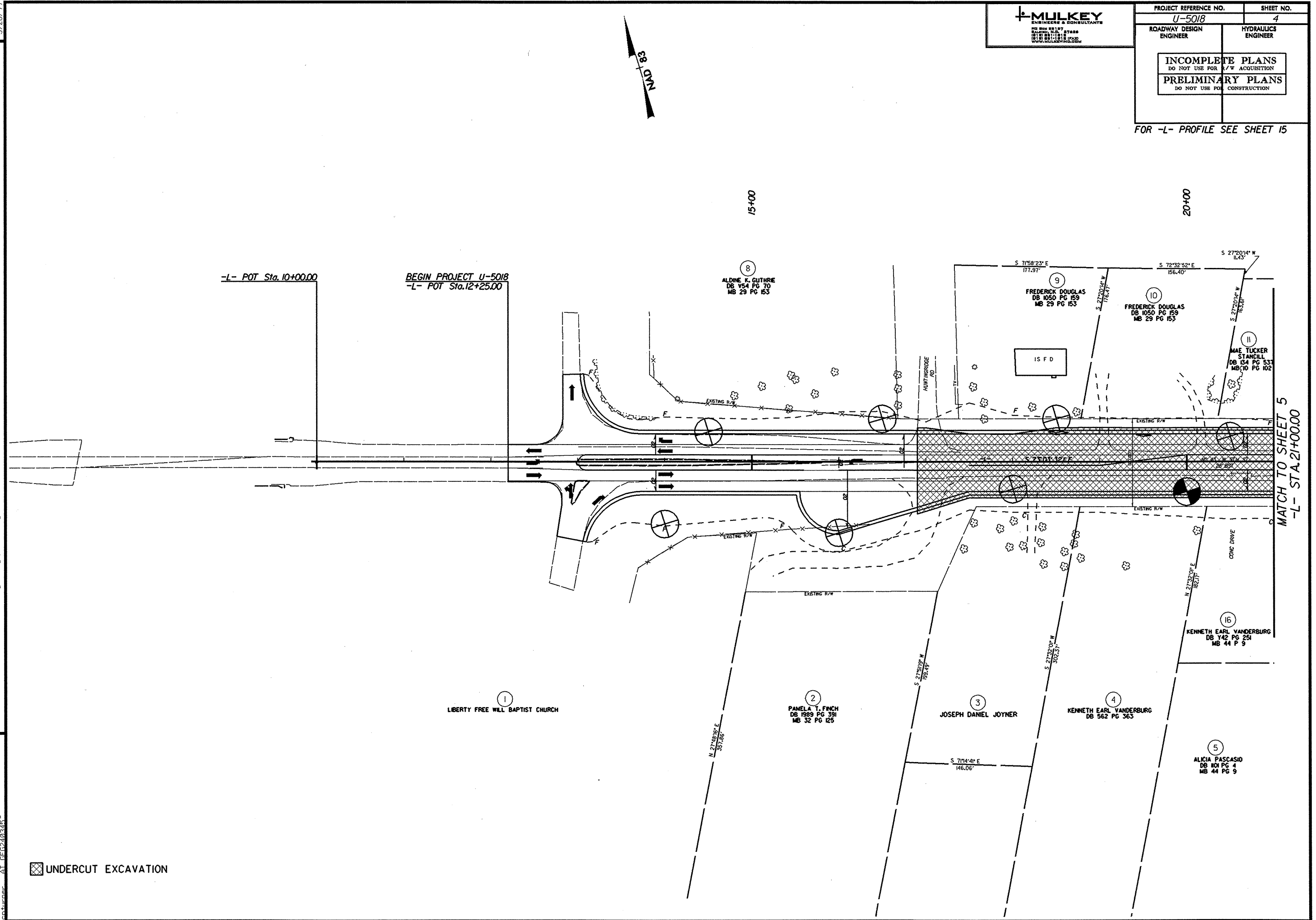
STATE HIGHWAY DESIGN ENGINEER

5/28/99
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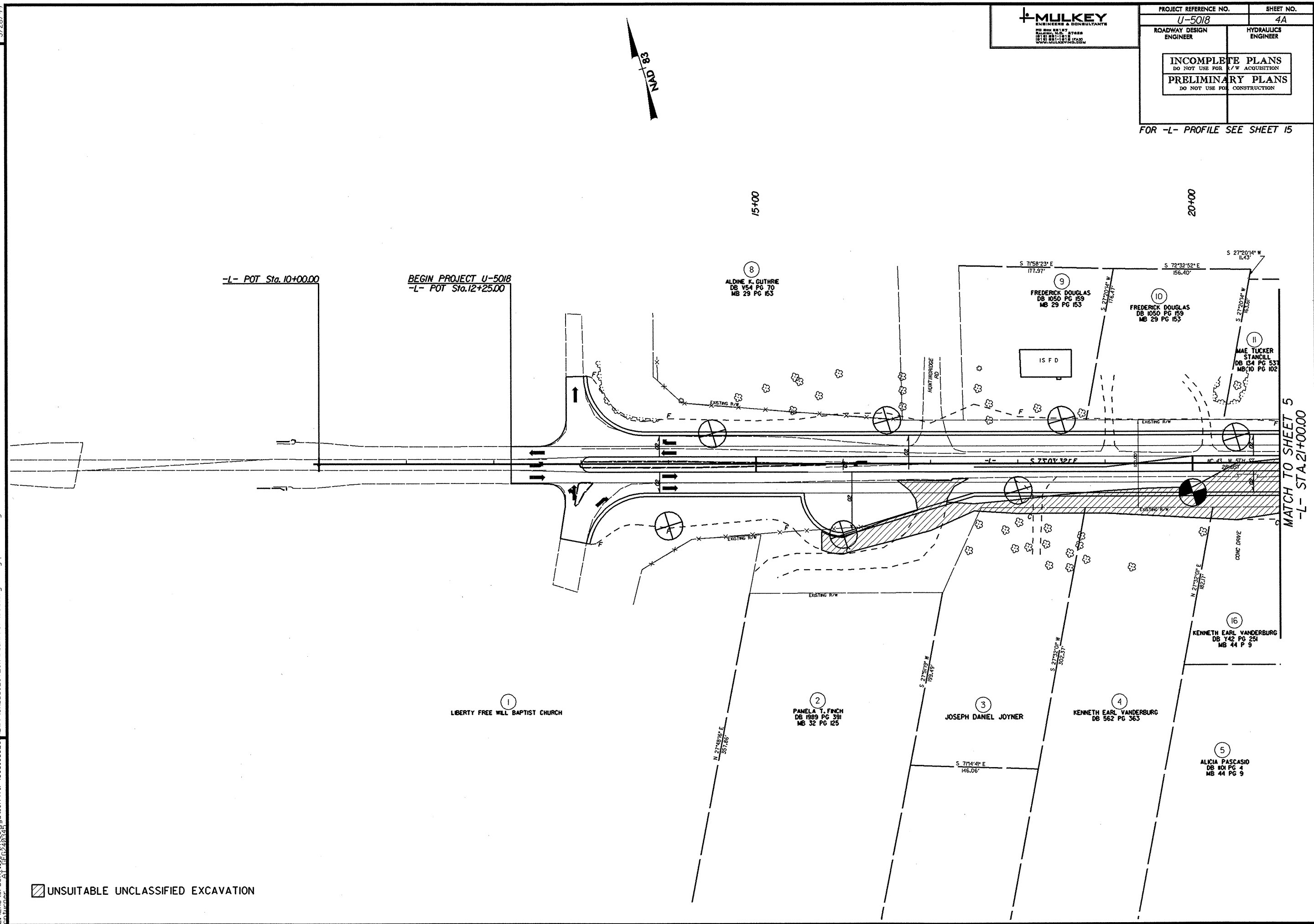
PROJECT REFERENCE NO. <i>U-5018</i>		SHEET NO. 4	
ROADWAY DESIGN ENGINEER		HYDRAULICS ENGINEER	
INCOMPLETE PLANS DO NOT USE FOR R/W ACQUISITION PRELIMINARY PLANS DO NOT USE FOR CONSTRUCTION			

FOR -L- PROFILE SEE SHEET 15



FOR -L- PROFILE SEE SHEET 15

5/28/99
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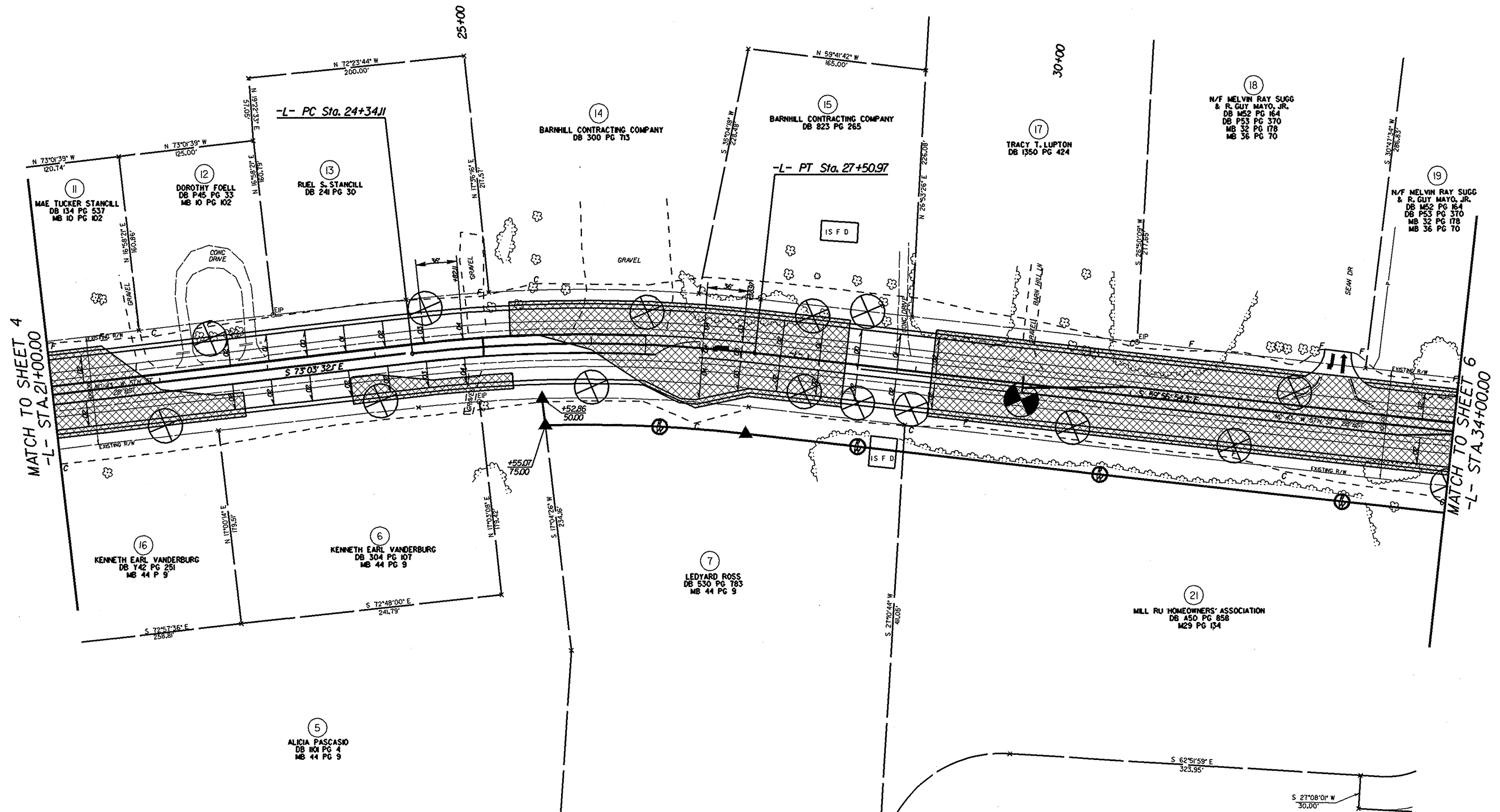
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MATCH TO SHEET 5
 -L- STA. 21+00.00

PROJECT REFERENCE NO. U-5018	SHEET NO. 5
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
INCOMPLETE PLANS DO NOT USE FOR R/W ACQUISITION PRELIMINARY PLANS DO NOT USE FOR CONSTRUCTION	

FOR -L- PROFILE SEE SHEET 15

-L-
PI Sta 25+93.23
 $\Delta = 13^{\circ}06'37.8"$ (RT)
D = 4'08"15.5"
L = 316.86'
T = 159.12'
R = 1,384.74'
SE = 04
RO = 144'



UNDERCUT EXCAVATION

REVISIONS

5/28/99

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 RDWY\CADD_GEO\TECH\Plan\ProF\U5018.GEO
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5/28/99

-L-
 PI Sta 25+93.23
 $\Delta = 13^{\circ}06'37.8''$ (RT)
 $D = 4'08''15.5''$
 $L = 316.86'$
 $T = 159.12'$
 $R = 1,384.74'$
 $SE = 04$
 $RO = 144'$

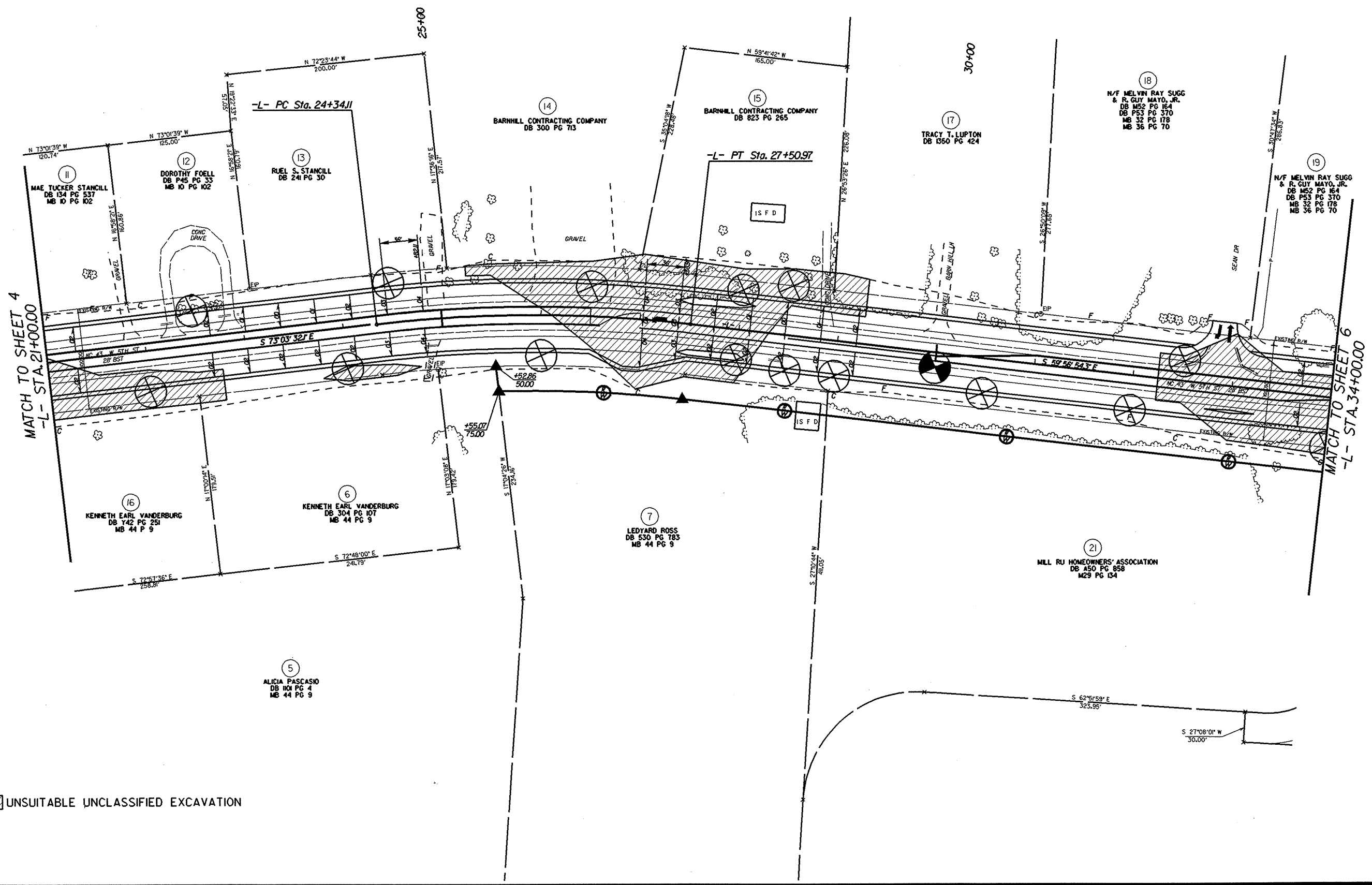


MULKEY
 ENGINEERS & CONSULTANTS
 1010 BELLEVILLE ROAD
 SUITE 100
 ST. LOUIS, MO 63103
 WWW.MULKEYENGINEERS.COM

PROJECT REFERENCE NO. U-5018		SHEET NO. 5A	
ROADWAY DESIGN ENGINEER		HYDRAULICS ENGINEER	
INCOMPLETE PLANS DO NOT USE FOR ACQUISITION PRELIMINARY PLANS DO NOT USE FOR CONSTRUCTION			
FOR -L- PROFILE SEE SHEET 15			

REVISIONS

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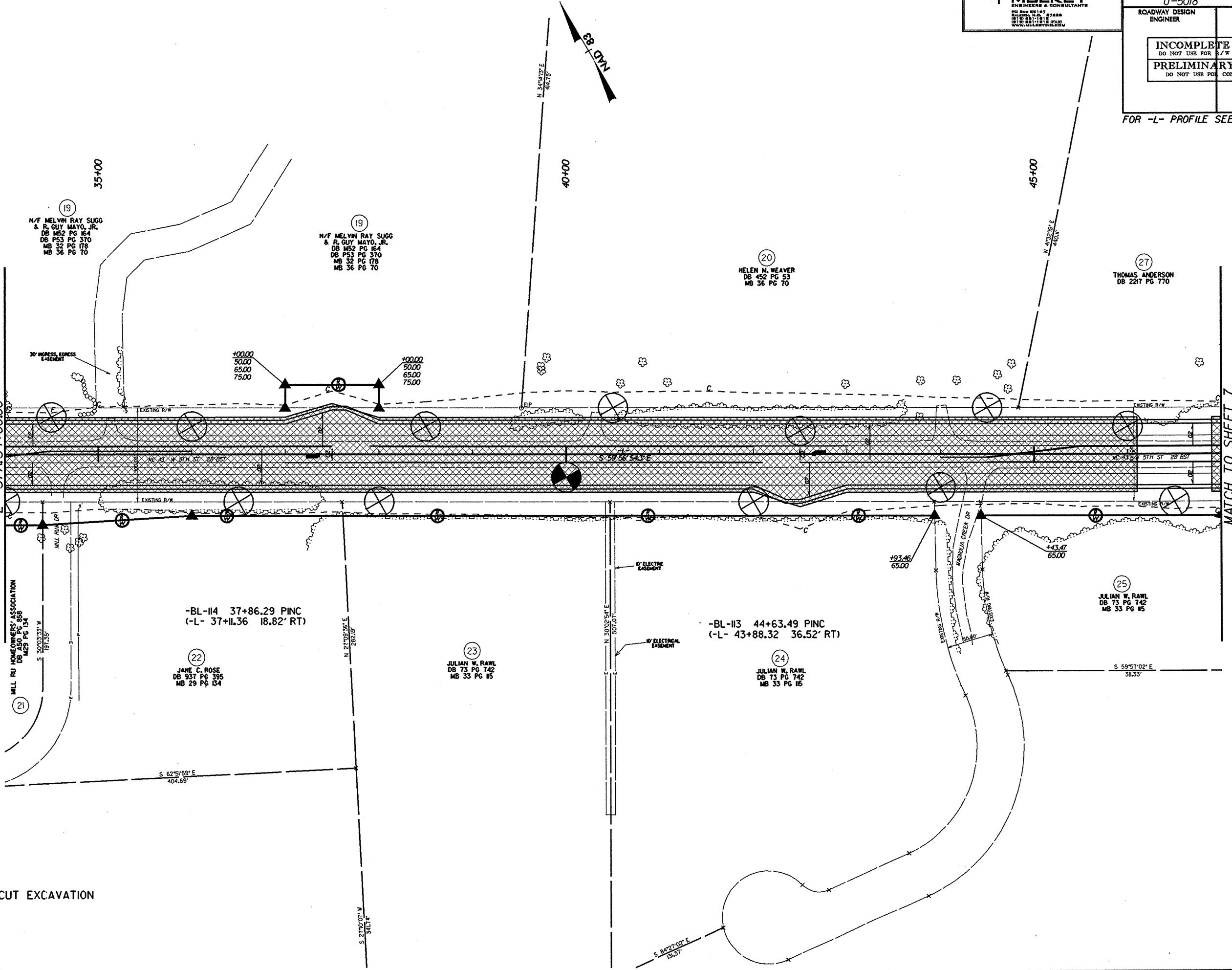


PROJECT REFERENCE NO. U-5018	SHEET NO. 6
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
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FOR -L- PROFILE SEE SHEET 16	

MATCH TO SHEET 5
-L- STA. 34+00.00

MATCH TO SHEET 7
-L- STA. 47+00.00

⊠ UNDERCUT EXCAVATION



REVISIONS

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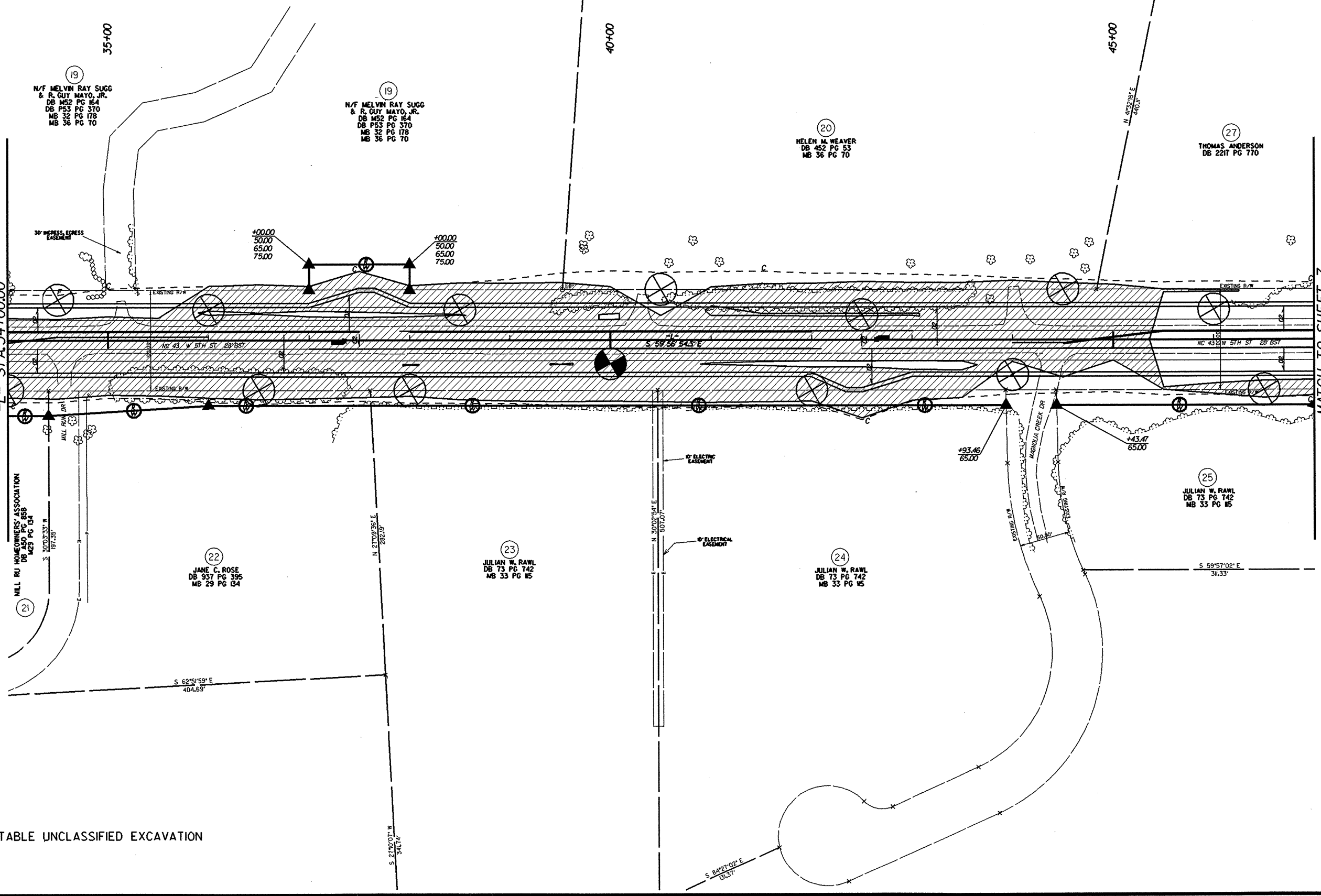


PROJECT REFERENCE NO. U-5018		SHEET NO. 6A	
ROADWAY DESIGN ENGINEER		HYDRAULICS ENGINEER	
INCOMPLETE PLANS DO NOT USE FOR ACQUISITION		PRELIMINARY PLANS DO NOT USE FOR CONSTRUCTION	

FOR -L- PROFILE SEE SHEET 16

MATCH TO SHEET 5
-L- STA. 34+00.00

MATCH TO SHEET 7
-L- STA. 47+00.00



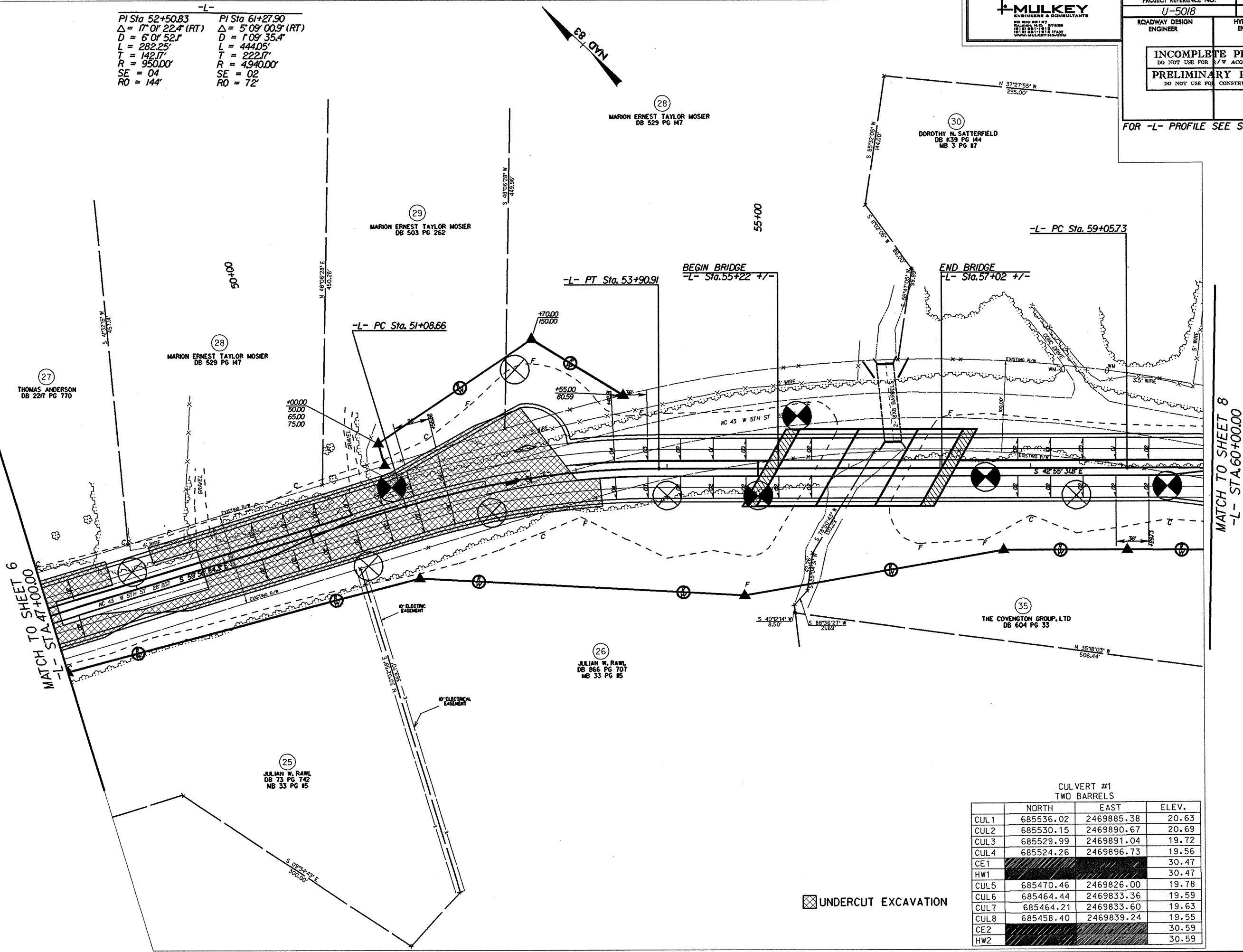
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 5/28/99 07:15
 5/28/99 07:15

-L-
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 $D = 6^{\circ} 01' 52.1''$ $D = 1^{\circ} 09' 35.4''$
 $L = 282.25'$ $L = 444.05'$
 $T = 142.17'$ $T = 222.17'$
 $R = 950.00'$ $R = 4940.00'$
 $SE = 04'$ $SE = 02'$
 $RO = 144'$ $RO = 72'$



PROJECT REFERENCE NO. U-5018	SHEET NO. 7
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
INCOMPLETE PLANS DO NOT USE FOR ACQUISITION PRELIMINARY PLANS DO NOT USE FOR CONSTRUCTION	
FOR -L- PROFILE SEE SHEET 16	



CULVERT #1
TWO BARRELS

	NORTH	EAST	ELEV.
CUL 1	685536.02	2469885.38	20.63
CUL 2	685530.15	2469890.67	20.69
CUL 3	685529.99	2469891.04	19.72
CUL 4	685524.26	2469896.73	19.56
CE 1			30.47
HW 1			30.47
CUL 5	685470.46	2469826.00	19.78
CUL 6	685464.44	2469833.36	19.59
CUL 7	685464.21	2469833.60	19.63
CUL 8	685458.40	2469839.24	19.55
CE 2			30.59
HW 2			30.59

☒ UNDERCUT EXCAVATION

MATCH TO SHEET 8
-L- STA. 60+00.00

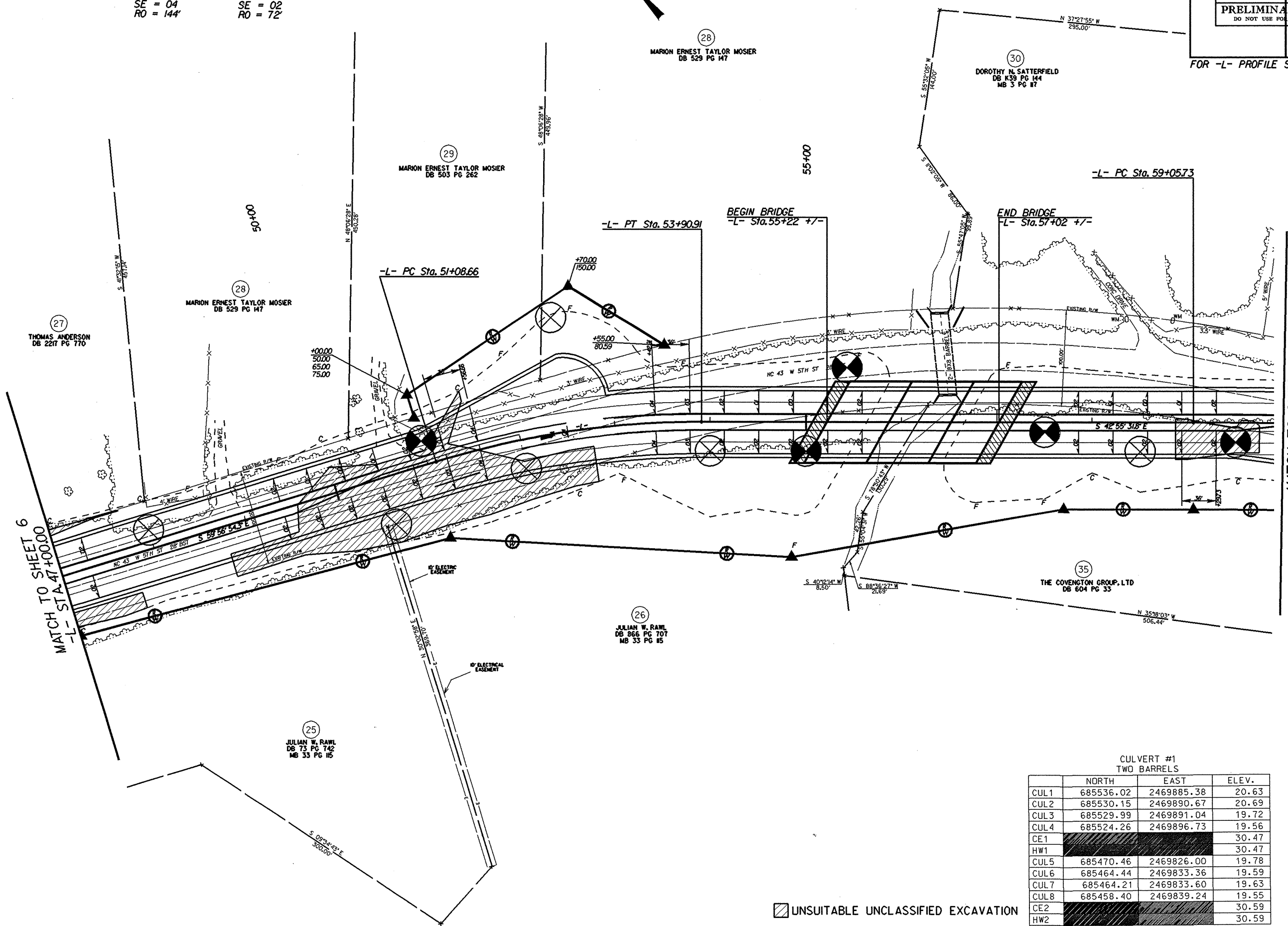
MATCH TO SHEET 6
-L- STA. 47+00.00

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-L-
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 $D = 6^{\circ} 01' 52.1"$ $D = 1^{\circ} 09' 35.4"$
 $L = 282.25'$ $L = 444.05'$
 $T = 142.17'$ $T = 222.17'$
 $R = 950.00'$ $R = 4940.00'$
 $SE = 04$ $SE = 02$
 $RO = 144$ $RO = 72$



PROJECT REFERENCE NO. U-5018	SHEET NO. 7A
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
INCOMPLETE PLANS DO NOT USE FOR A/W ACQUISITION PRELIMINARY PLANS DO NOT USE FOR CONSTRUCTION	
FOR -L- PROFILE SEE SHEET 16	



CULVERT #1
TWO BARRELS

	NORTH	EAST	ELEV.
CUL1	685536.02	2469885.38	20.63
CUL2	685530.15	2469890.67	20.69
CUL3	685529.99	2469891.04	19.72
CUL4	685524.26	2469896.73	19.56
CE1			30.47
HW1			30.47
CUL5	685470.46	2469826.00	19.78
CUL6	685464.44	2469833.36	19.59
CUL7	685464.21	2469833.60	19.63
CUL8	685458.40	2469839.24	19.55
CE2			30.59
HW2			30.59

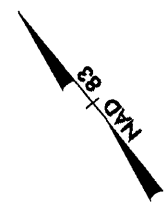
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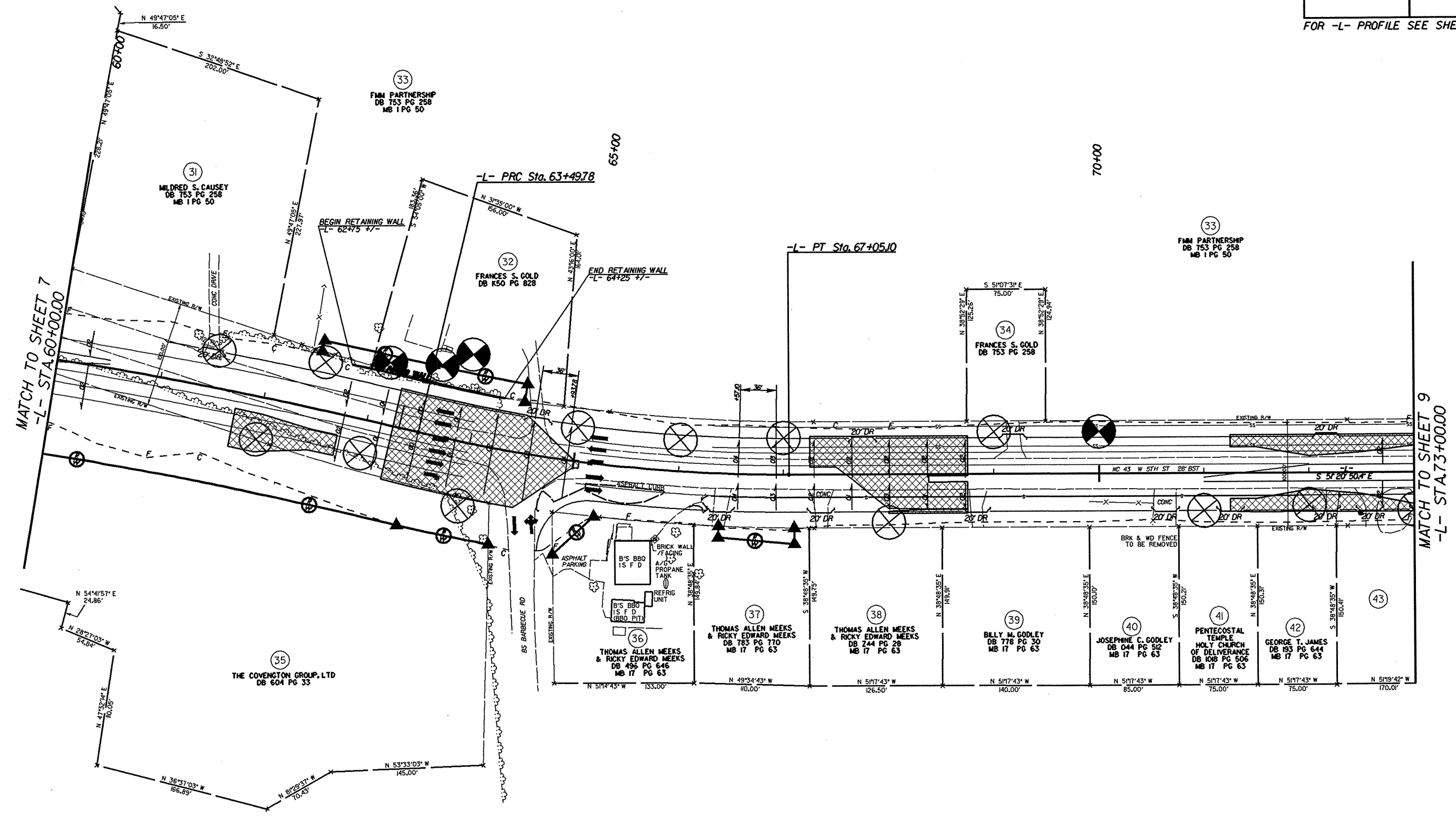
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PI Sta 61+27.90	PI Sta 65+28.27
$\Delta = 5^{\circ}09'00.9''$ (RT)	$\Delta = 13^{\circ}34'19.5''$ (LT)
D = 1'09'35.4"	D = 3'49'11.0"
L = 444.05'	L = 355.32'
T = 222.17'	T = 178.49'
R = 4,940.00'	R = 1,500.00'
SE = 02	SE = 04
RO = 72'	RO = 144'



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ENGINEERS & CONSULTANTS
NO. 1000 N. 10TH ST. SUITE 200
DENVER, CO 80202
(303) 733-1111
(303) 733-1122
WWW.MULKEYINC.COM

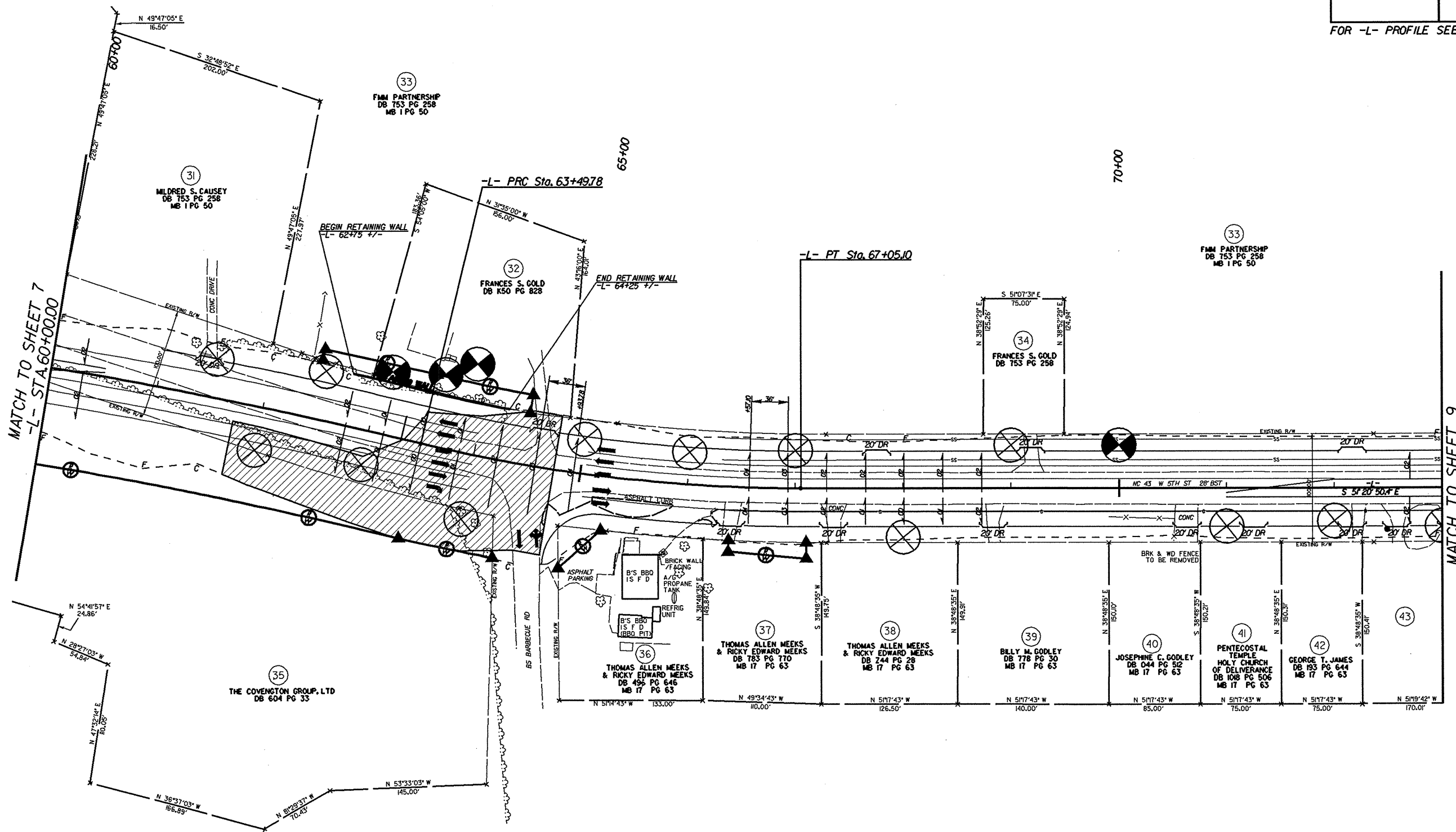
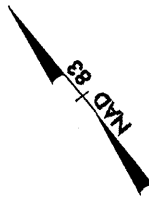
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ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
INCOMPLETE PLANS DO NOT USE FOR A/W ACQUISITION PRELIMINARY PLANS DO NOT USE FOR CONSTRUCTION	
FOR -L- PROFILE SEE SHEET 17	



UNDERCUT EXCAVATION

-L-

PI Sta 61+27.90	PI Sta 65+28.27
$\Delta = 5^{\circ}09'00.9''$ (RT)	$\Delta = 13^{\circ}34'19.5''$ (LT)
D = 1'09'35.4"	D = 3'49'11.0"
L = 444.05'	L = 355.32'
T = 222.17'	T = 178.49'
R = 4,940.00'	R = 1,500.00'
SE = 02	SE = 04
RO = 72	RO = 144



MATCH TO SHEET 7
-L- STA. 60+00.00

MATCH TO SHEET 9
-L- STA. 73+00.00

UNSUITABLE UNCLASSIFIED EXCAVATION

REVISIONS

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5/28/99

REVISIONS

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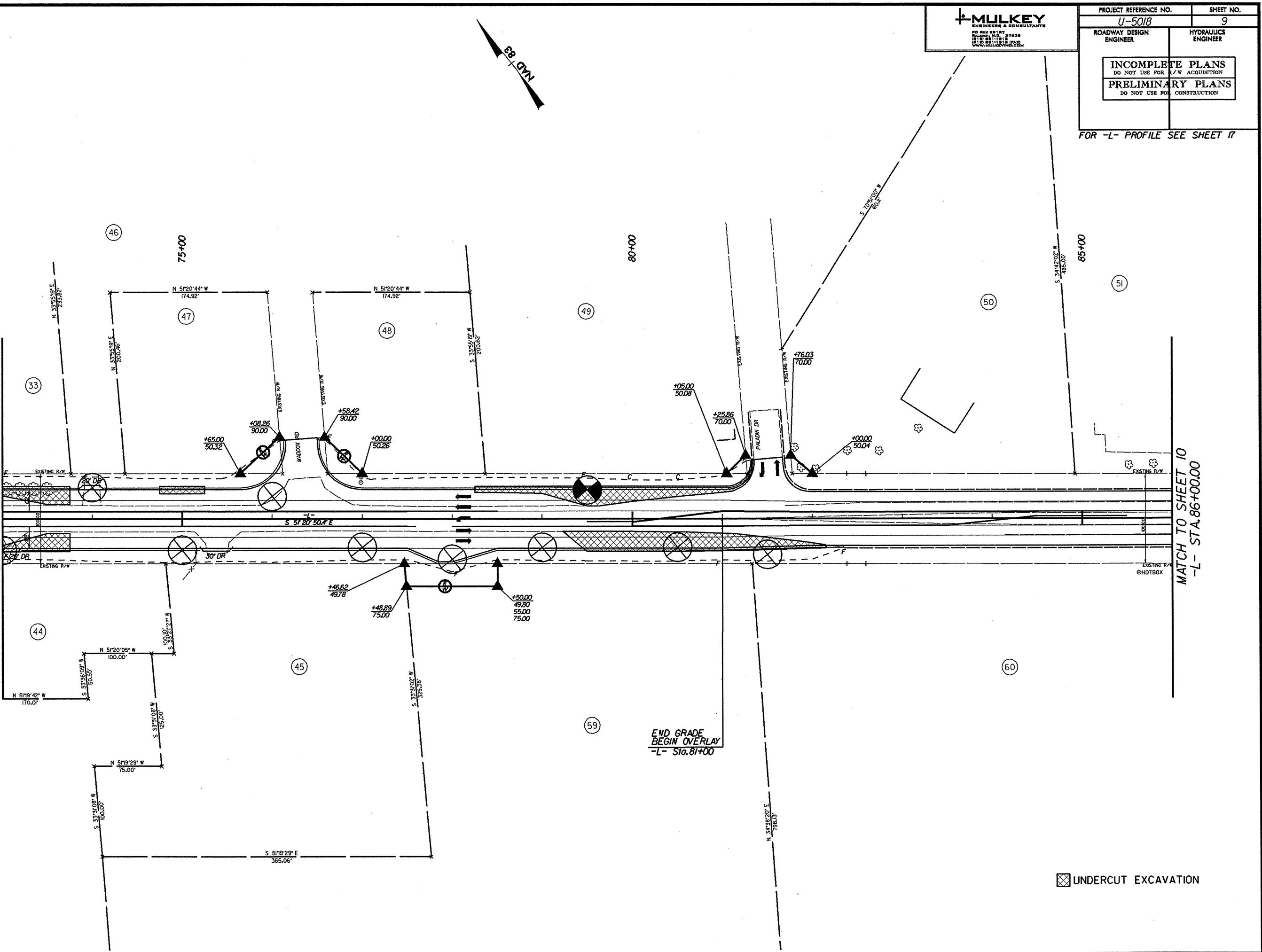


PROJECT REFERENCE NO. U-5018	SHEET NO. 9
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
INCOMPLETE PLANS DO NOT USE FOR R/W ACQUISITION PRELIMINARY PLANS DO NOT USE FOR CONSTRUCTION	

FOR -L- PROFILE SEE SHEET 17

MATCH TO SHEET 8
-L- STA.73+00.00

MATCH TO SHEET 10
-L- STA.86+00.00



END GRADE
BEGIN OVERLAY
-L- Sta.81+00

UNDERCUT EXCAVATION

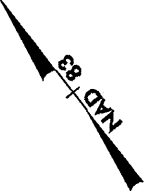
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MULKEY
ENGINEERS & CONSULTANTS
PO BOX 88187
DALLAS, TEXAS
75288-1818
TEL: 972-251-1818
WWW.MULKEYINC.COM

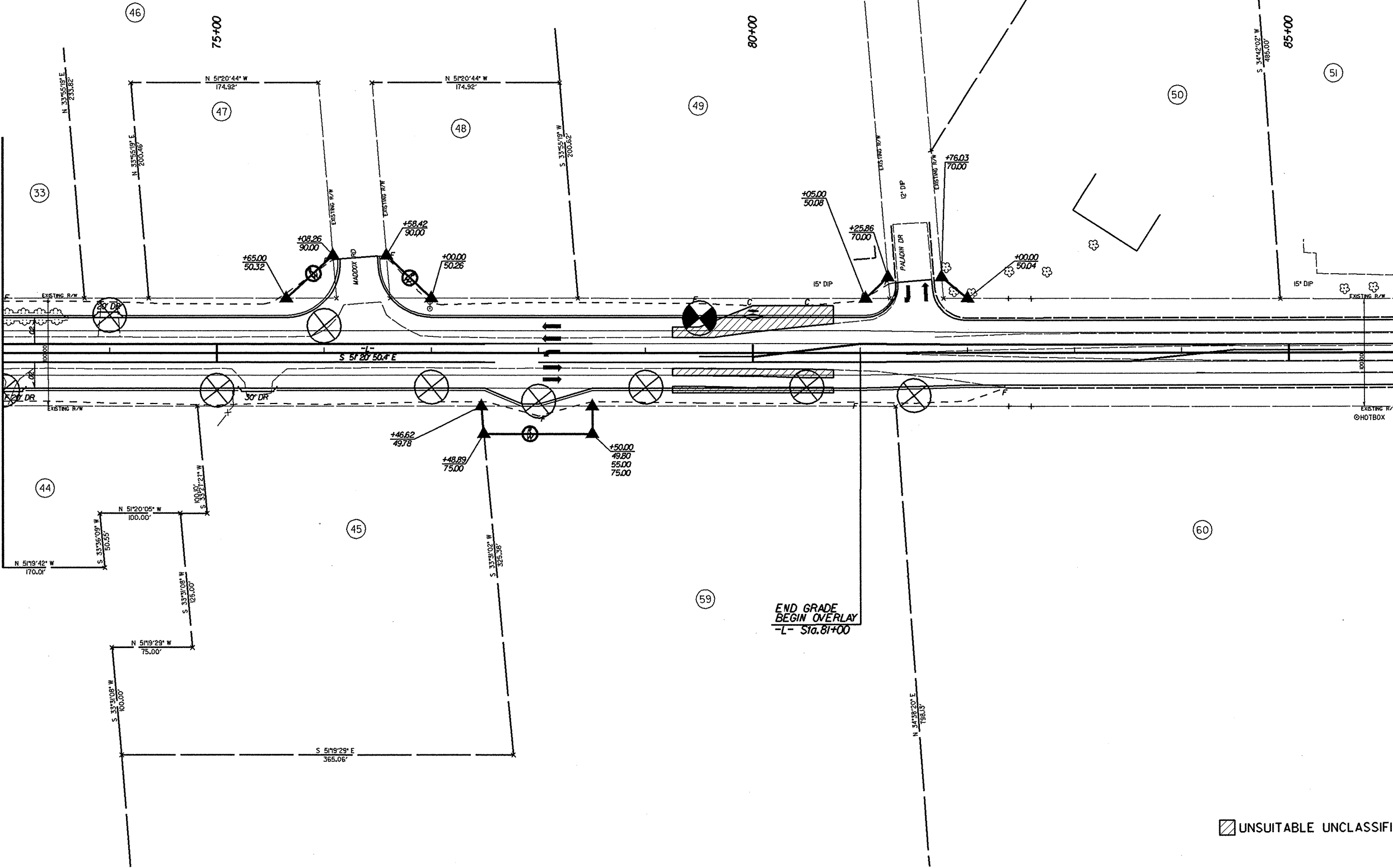
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ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
INCOMPLETE PLANS DO NOT USE FOR A/W ACQUISITION PRELIMINARY PLANS DO NOT USE FOR CONSTRUCTION	

FOR -L- PROFILE SEE SHEET 17



MATCH TO SHEET 8
-L- STA.73+00.00

MATCH TO SHEET 10
-L- STA.86+00.00



UNSUITABLE UNCLASSIFIED EXCAVATION

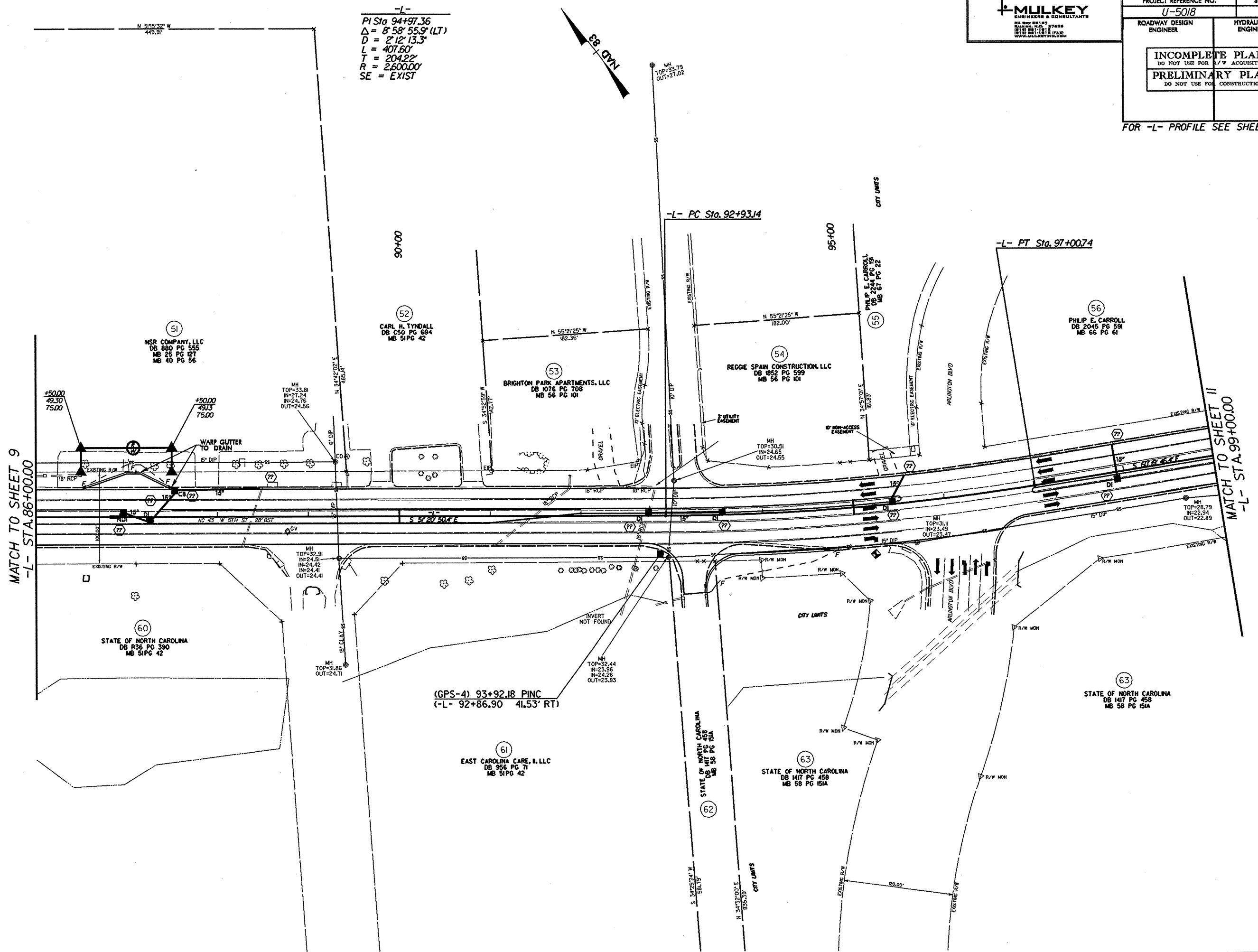
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REVISIONS

MULKEY
ENGINEERS & CONSULTANTS
10101 W. STATE ST. SUITE 1000
DALLAS, TEXAS 75243
TEL: 972.981.1111 FAX: 972.981.1112
WWW.MULKEYINC.COM

PROJECT REFERENCE NO. U-5018		SHEET NO. 10	
ROADWAY DESIGN ENGINEER		HYDRAULICS ENGINEER	
INCOMPLETE PLANS DO NOT USE FOR R/W ACQUISITION PRELIMINARY PLANS DO NOT USE FOR CONSTRUCTION			
FOR -L- PROFILE SEE SHEET 18			



-L-
PI Sta 94+97.36
 $\Delta = 8' 58'' 55.9'' (LT)$
 $D = 2' 12'' 13.3''$
 $L = 407.60'$
 $T = 204.22'$
 $R = 2,600.00'$
SE = EXIST

MATCH TO SHEET 9
-L- STA.86+00.00

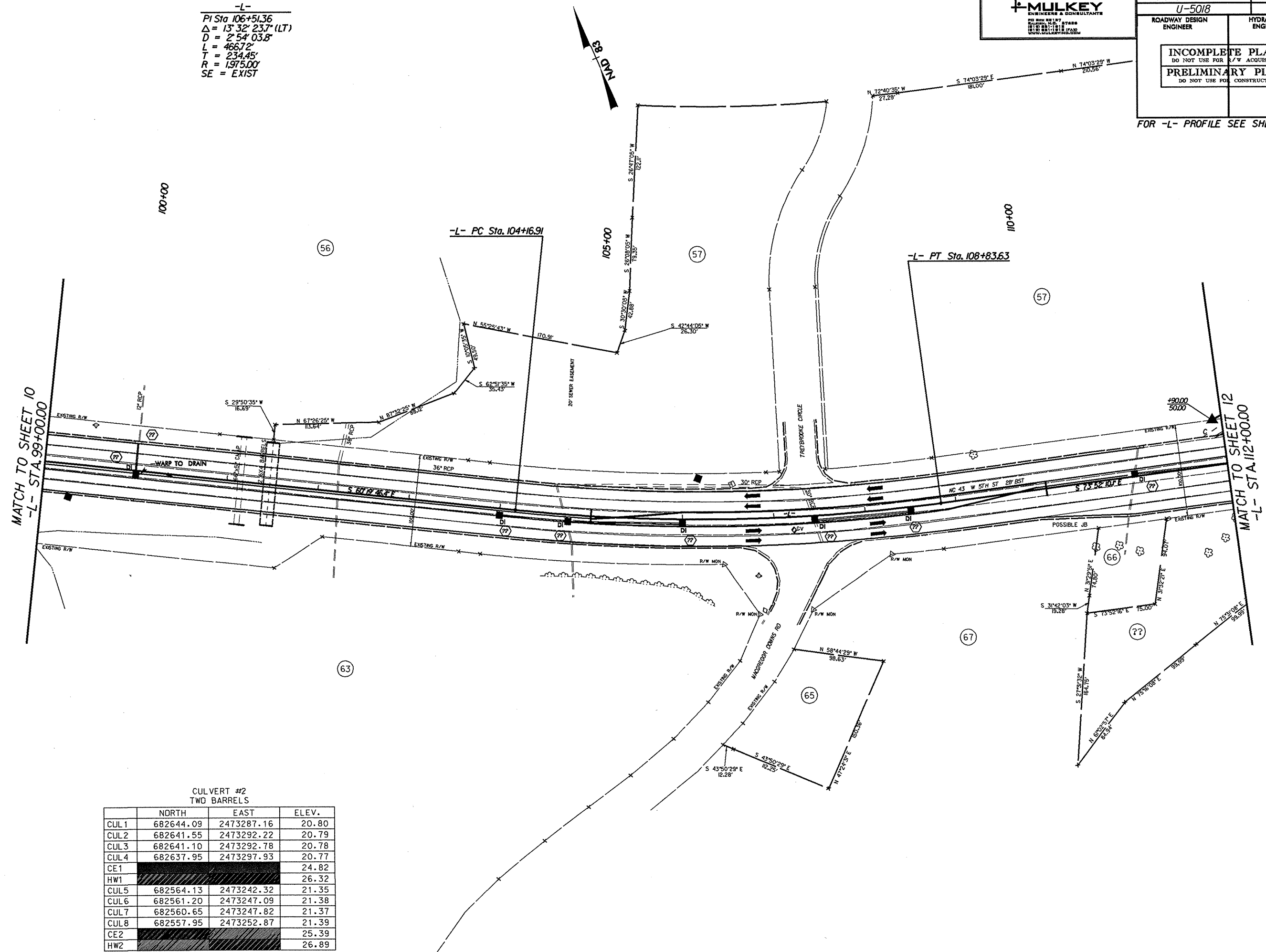
MATCH TO SHEET 11
-L- STA.99+00.00

(GPS-4) 93+92.18 P INC
(-L- 92+86.90 41.53' RT)



PROJECT REFERENCE NO. U-5018	SHEET NO. 11
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
INCOMPLETE PLANS DO NOT USE FOR A/CQUISITION PRELIMINARY PLANS DO NOT USE FOR CONSTRUCTION	
FOR -L- PROFILE SEE SHEET 18	

-L-
 PI Sta 106+51.36
 $\Delta = 13^{\circ} 32' 23.7" (LT)$
 $D = 2^{\circ} 54' 03.8"$
 $L = 466.72'$
 $T = 234.45'$
 $R = 1975.00'$
 SE = EXIST



MATCH TO SHEET 10
 -L- STA. 99+00.00

MATCH TO SHEET 12
 -L- STA. 112+00.00

CULVERT #2
 TWO BARRELS

	NORTH	EAST	ELEV.
CUL1	682644.09	2473287.16	20.80
CUL2	682641.55	2473292.22	20.79
CUL3	682641.10	2473292.78	20.78
CUL4	682637.95	2473297.93	20.77
CE1			24.82
HW1			26.32
CUL5	682564.13	2473242.32	21.35
CUL6	682561.20	2473247.09	21.38
CUL7	682560.65	2473247.82	21.37
CUL8	682557.95	2473252.87	21.39
CE2			25.39
HW2			26.89

REVISIONS

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 5/28/99
 RDWY\CADD_GEO\TECH\Plan\Prof\U5018_gcc_r.dwg
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5/28/99

8-SEP-2008 07:25 L:\VERO\green\11g... RDWY\CADD_GEO\TECHN\Plan\Prof\US018.gpc_rdl_psh12.dgn

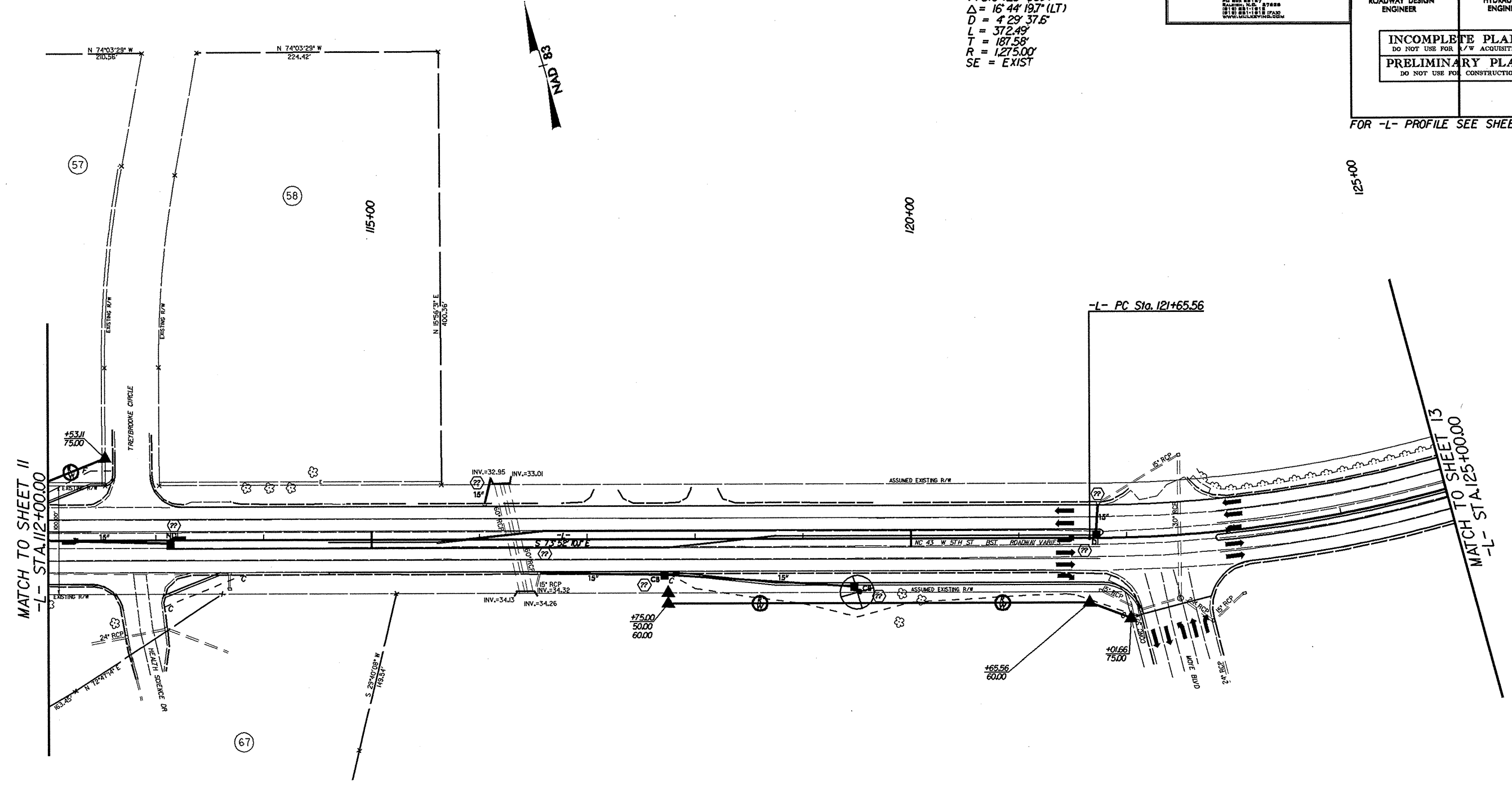
REVISIONS



PROJECT REFERENCE NO. U-5018	SHEET NO. 12
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
INCOMPLETE PLANS DO NOT USE FOR A/W ACQUISITION PRELIMINARY PLANS DO NOT USE FOR CONSTRUCTION	

FOR -L- PROFILE SEE SHEET 19

-L-
 PI Sta 123+53.14
 $\Delta = 16' 44" 19.7"$ (LT)
 $D = 429' 37.6"$
 $L = 372.49'$
 $T = 187.58'$
 $R = 1275.00'$
 SE = EXIST



125+00

120+00

57

58

115+00

N 15°56'27" E 400.38'



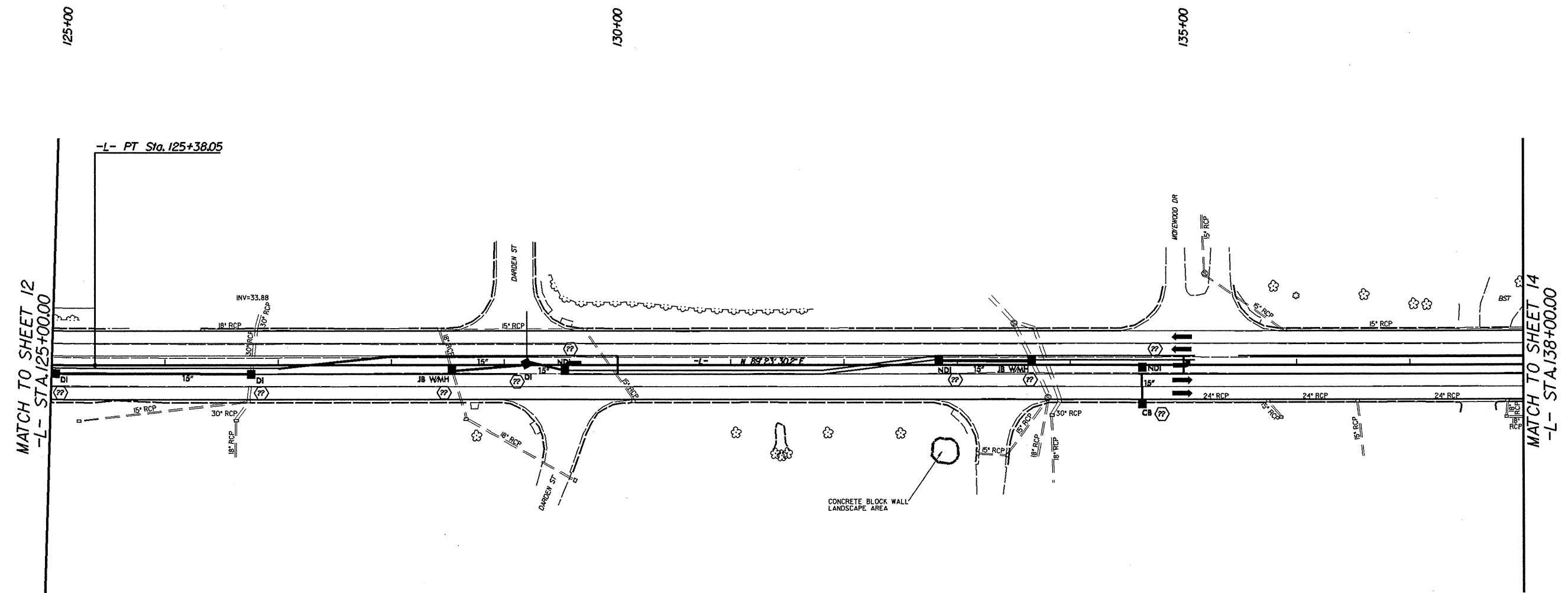
MATCH TO SHEET II
-L- STA. 112+00.00

-L- PC Sta. 121+65.56

MATCH TO SHEET III
-L- STA. 125+00.00

PROJECT REFERENCE NO. <i>U-5018</i>	SHEET NO. <i>13</i>
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
INCOMPLETE PLANS DO NOT USE FOR ACQUISITION	
PRELIMINARY PLANS DO NOT USE FOR CONSTRUCTION	

-L-
PI Sta. 123+53.14
 $\Delta = 16' 44" 19.7" (LT)$
D = 4' 29" 37.6"
L = 372.49'
T = 187.58'
R = 1,275.00'
SE = EXIST



MATCH TO SHEET 12
-L- STA. 125+00.00

MATCH TO SHEET 14
-L- STA. 138+00.00

-L- PT Sta. 125+38.05

REVISIONS

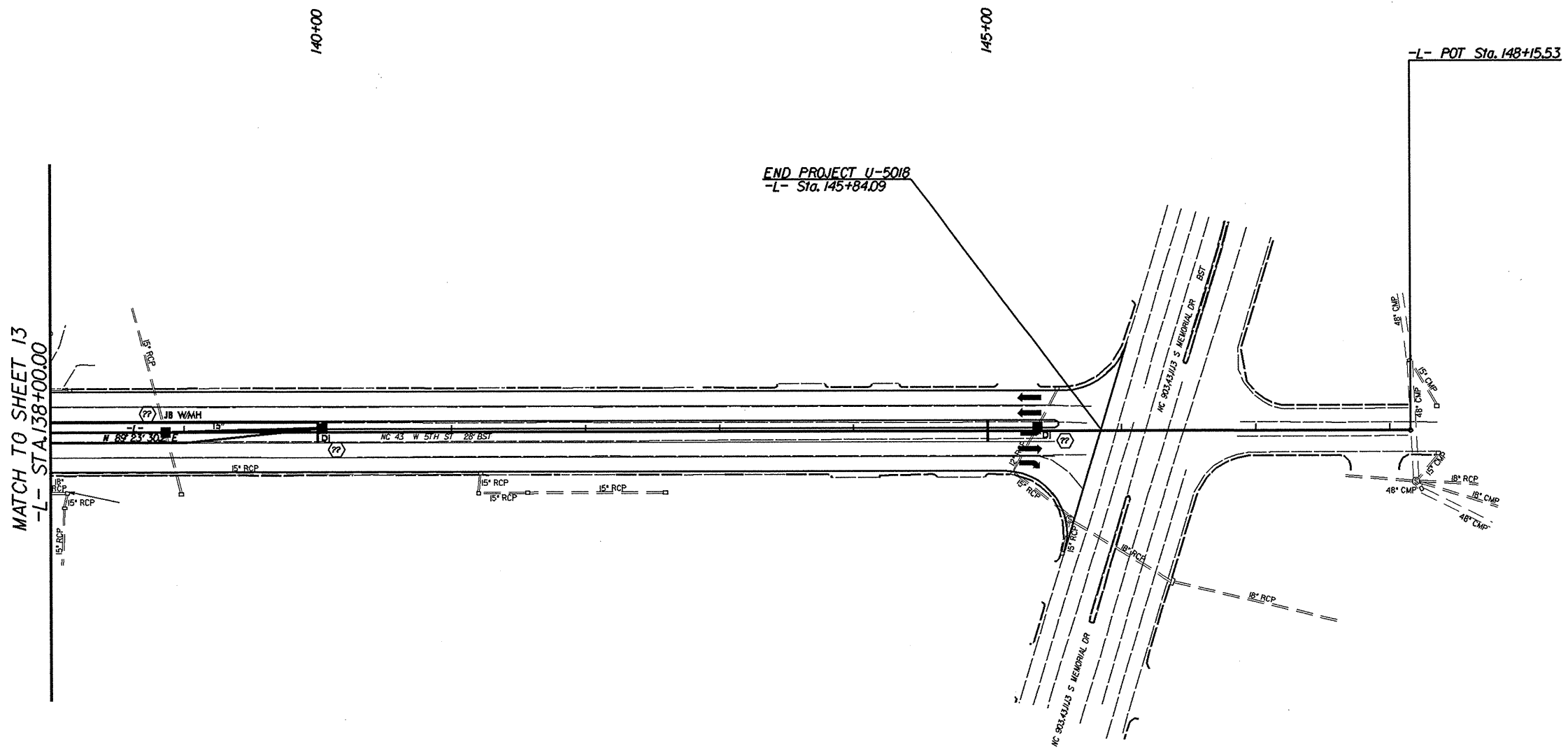
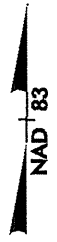
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5/28/99



PROJECT REFERENCE NO.		SHEET NO.	
U-5018		14	
ROADWAY DESIGN ENGINEER		HYDRAULICS ENGINEER	
INCOMPLETE PLANS DO NOT USE FOR R/W ACQUISITION PRELIMINARY PLANS DO NOT USE FOR CONSTRUCTION			

FOR -L- PROFILE SEE SHEET 20



REVISIONS

5/28/99

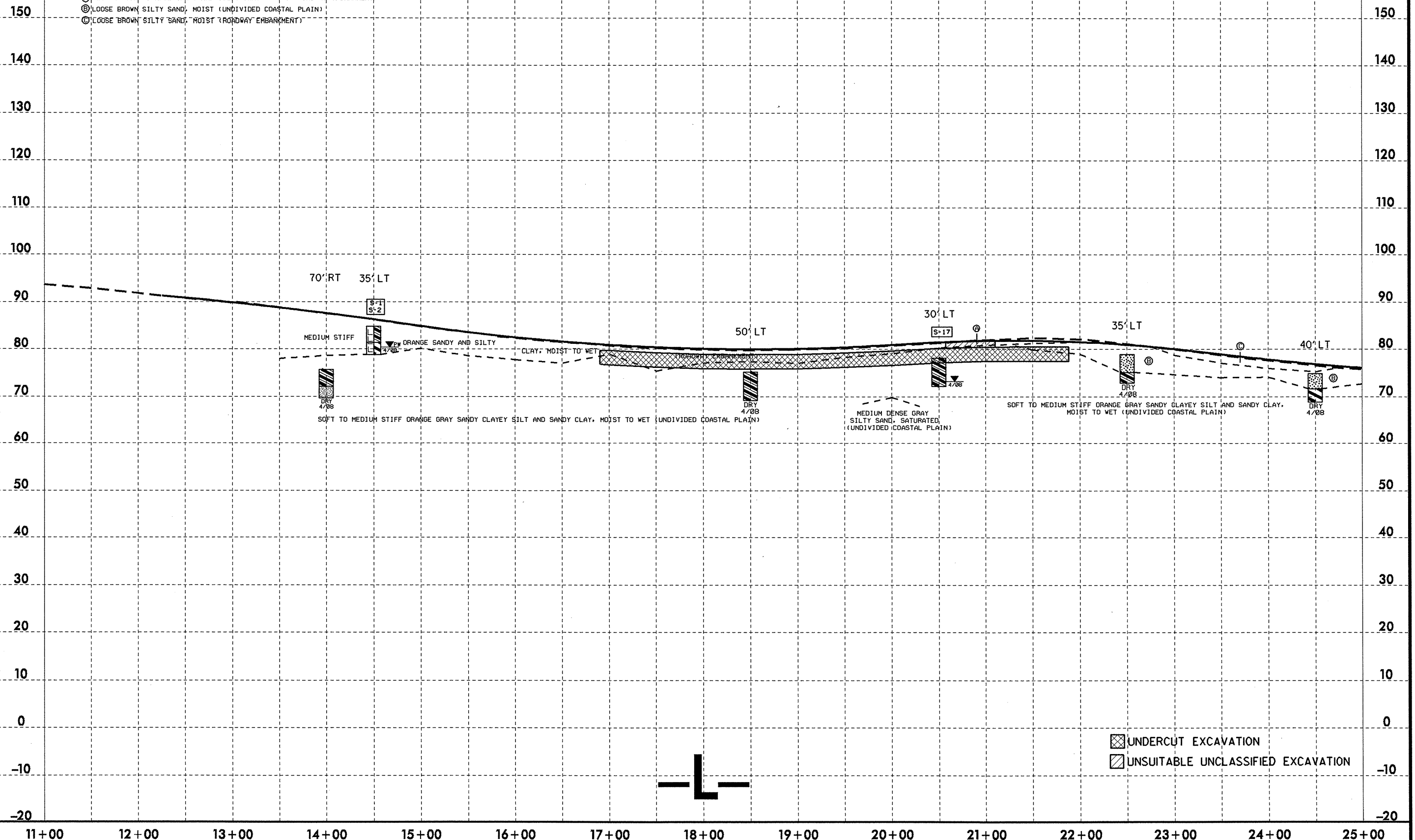
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 5/14/99

PROJECT REFERENCE NO.	SHEET NO.
U-5018	15
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
INCOMPLETE PLANS DO NOT USE FOR ACQUISITION	
PRELIMINARY PLANS DO NOT USE FOR CONSTRUCTION	

SOIL TEST RESULTS															
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	LL	PL	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							G. SAND	F. SAND	SILT	CLAY	20	40	200		
S-1	35' LT	14+50	1.00-3.50	A-6(1)	30	12	5.1	58.1	6.1	30.7	100	98	39	18.2	-
S-2	35' LT	14+50	3.50-6.00	A-7-6(3A)	64	43	7.2	19.0	24.7	49.1	100	96	77	-	-
S-17	30' LT	20+50	1.00-4.00	A-6(1)	37	11	4.9	59.5	4.9	30.7	100	98	38	-	-

- Ⓐ MEDIUM STIFF ORANGE SANDY AND SILTY CLAY, MOIST (ROADWAY EMBANKMENT)
- Ⓑ LOOSE BROWN SILTY SAND, MOIST (UNDIVIDED COASTAL PLAIN)
- Ⓒ LOOSE BROWN SILTY SAND, MOIST (ROADWAY EMBANKMENT)



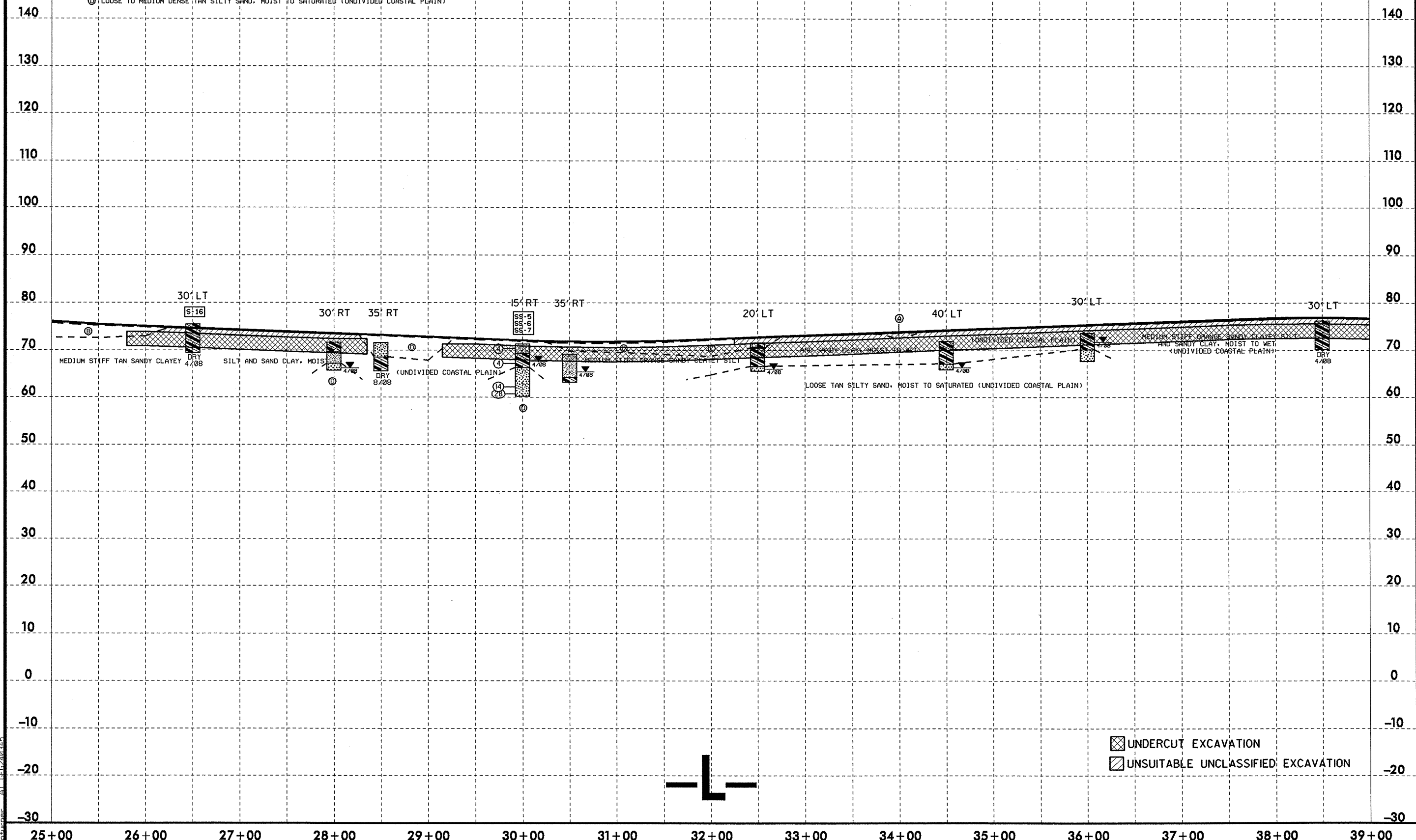
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 [Diagonal-hatched] UNSUITABLE UNCLASSIFIED EXCAVATION

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Author: AT

PROJECT REFERENCE NO.	SHEET NO.
U-5018	16
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
INCOMPLETE PLANS DO NOT USE FOR ACQUISITION	
PRELIMINARY PLANS DO NOT USE FOR CONSTRUCTION	

SOIL TEST RESULTS															
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS	LL	P.I.	% BY WEIGHT				% PASSING (SIEVES)			%	%
							C. SAND	F. SAND	SILT	CLAY	#10	#40	#200		
SS-5	15' RT	30+00	1.00-1.50	A-4(0)	19	5	3.8	54.8	21.1	20.2	100	99	46	-	-
SS-6	15' RT	30+00	3.10-4.60	A-6(5)	33	20	6.3	50.6	12.8	30.3	100	99	46	21.4	-
SS-7	15' RT	30+00	8.10-9.60	A-2-4(0)	23	5	23.1	56.6	2.1	18.2	100	97	22	-	-
S-16	30' LT	26+50	1.00-6.00	A-6(2)	33	17	12.3	51.5	7.6	28.6	100	97	39	-	-

- Ⓐ MEDIUM STIFF ORANGE SANDY CLAY, MOIST (ROADWAY EMBANKMENT)
- Ⓑ LOOSE BROWN SILTY SAND, MOIST (UNDIVIDED COASTAL PLAIN)
- Ⓒ MEDIUM STIFF TAN CLAYEY SANDY SILT, MOIST (ROADWAY EMBANKMENT)
- Ⓓ LOOSE TO MEDIUM DENSE TAN SILTY SAND, MOIST TO SATURATED (UNDIVIDED COASTAL PLAIN)

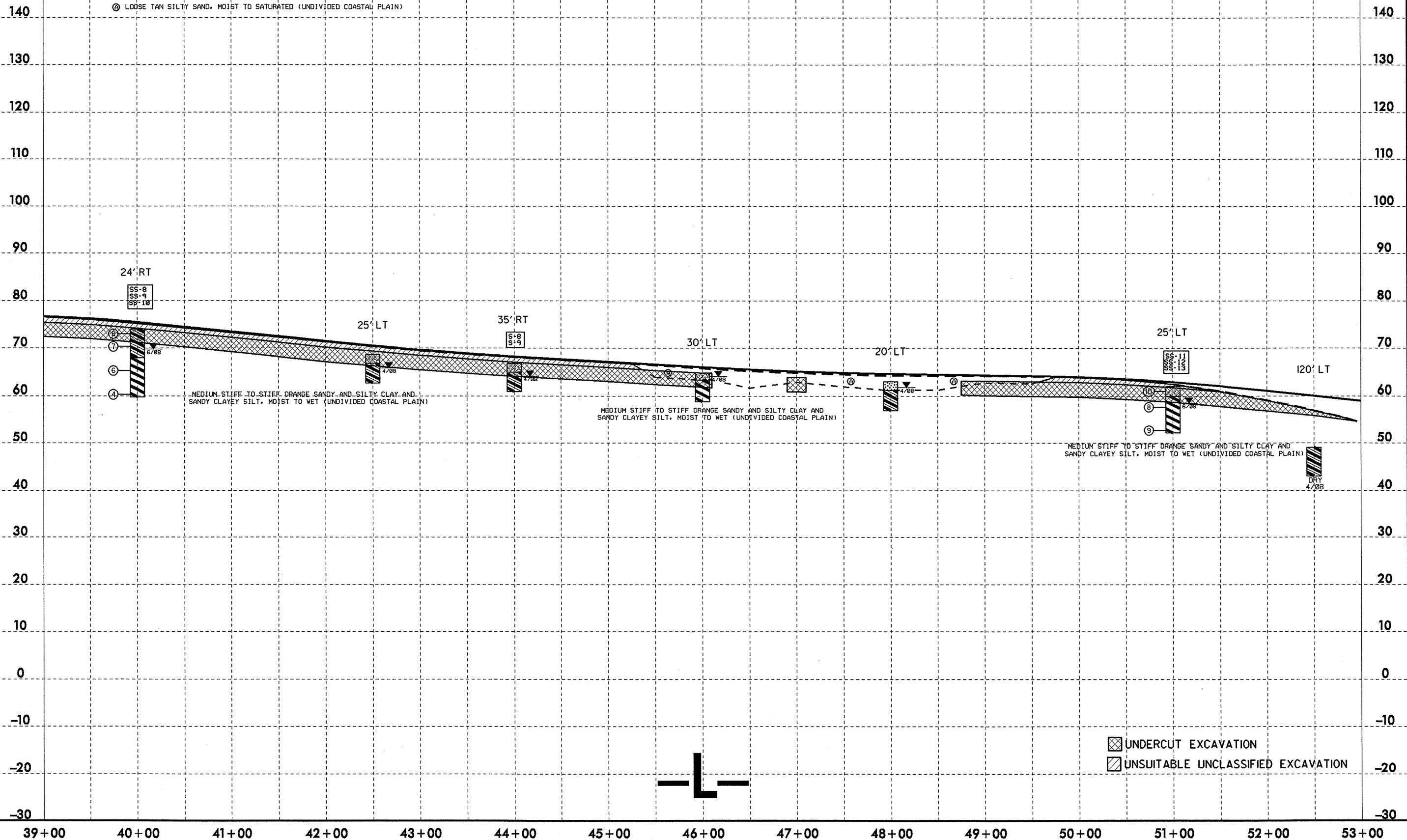


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PROJECT REFERENCE NO.	SHEET NO.
U-5018	17
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
INCOMPLETE PLANS DO NOT USE FOR R/W ACQUISITION	
PRELIMINARY PLANS DO NOT USE FOR CONSTRUCTION	

SOIL TEST RESULTS															
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS	LL	PL	% BY WEIGHT				% PASSING (SIEVES)			MOISTURE %	ORGANIC %
							C-SAND	F-SAND	SILT	CLAY	10	40	200		
SS-8	24' RT	40+00	1.00-1.50	A-6(1)	29	14	2.4	64.9	10.4	22.2	100	99	36	-	-
SS-9	24' RT	40+00	2.70-4.20	A-6(3)	35	18	0.4	61.1	8.2	30.3	100	100	40	26.5	-
SS-10	24' RT	40+00	7.70-9.20	A-7-6(6)	87	60	1.0	5.1	23.2	70.8	100	100	96	-	-
SS-11	25' LT	51+00	1.00-1.50	A-4(0)	22	7	4.3	59.3	16.1	20.3	100	99	42	-	-
SS-12	25' LT	51+00	3.30-4.80	A-7-6(5)	76	48	0.2	5.7	27.1	67.0	100	100	96	-	-
SS-13	25' LT	51+00	8.30-9.80	A-7-6(2)	49	27	3.9	17.3	24.1	54.8	100	99	81	-	-
S-8	35' RT	44+00	1.00-2.00	A-4(0)	20	5	1.6	55.0	18.8	24.5	100	100	48	-	-
S-9	35' RT	44+00	2.00-4.50	A-6(4)	38	20	2.9	58.5	8.0	30.7	100	100	42	-	-

Ⓢ LOOSE TAN SILTY SAND, MOIST TO SATURATED (UNDIVIDED COASTAL PLAIN)



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 User: jfj
 Job: U5018

⊠ UNDERCUT EXCAVATION
 ▨ UNSUITABLE UNCLASSIFIED EXCAVATION

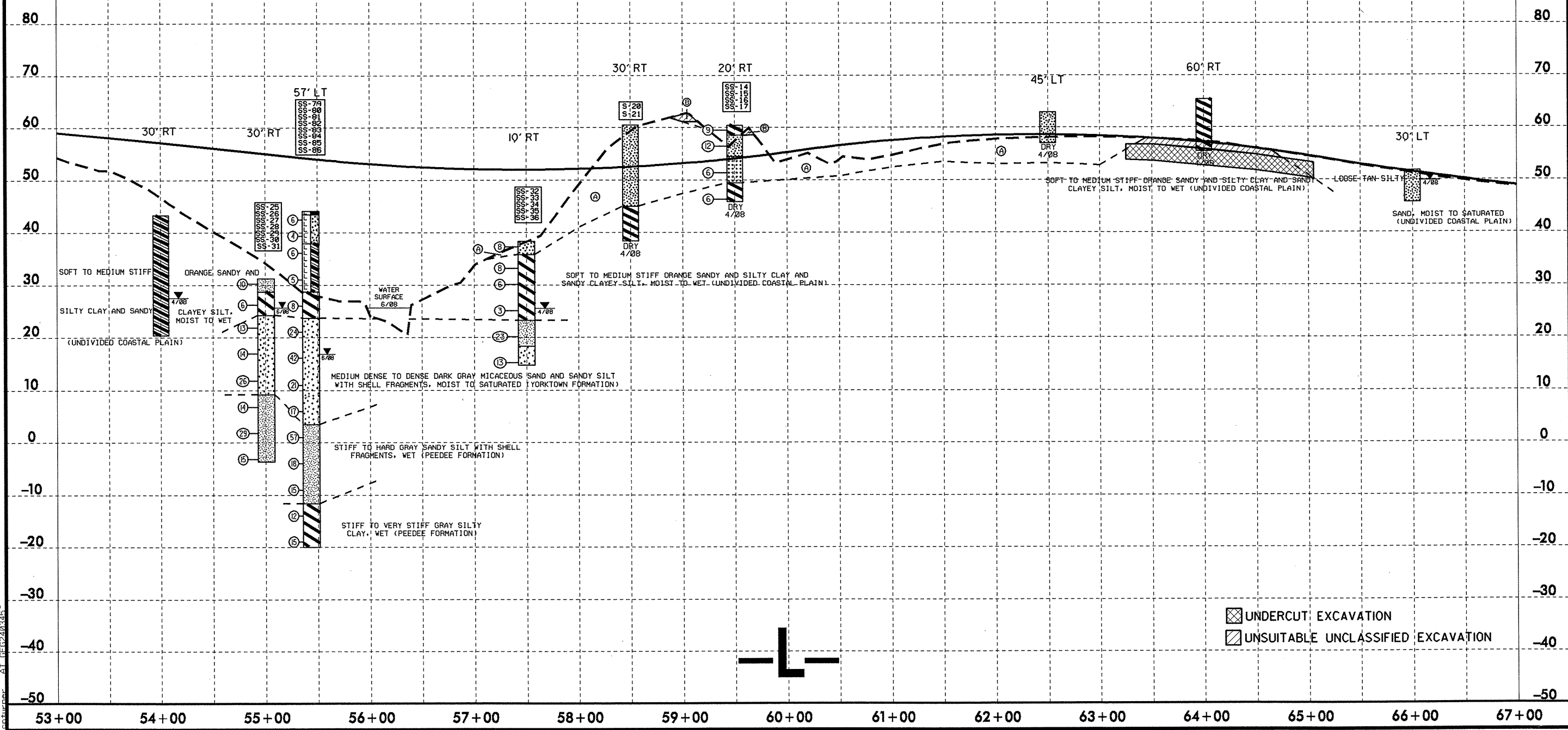
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PROJECT REFERENCE NO.		SHEET NO.	
U-5018		18	
ROADWAY DESIGN ENGINEER		HYDRAULICS ENGINEER	
INCOMPLETE PLANS DO NOT USE FOR ACQUISITION PRELIMINARY PLANS DO NOT USE FOR CONSTRUCTION			

SOIL TEST RESULTS

SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS	LL	P.I.	% BY WEIGHT			% PASSING (SIEVES)			MOISTURE	ORGANIC
							C. SAND	F. SAND	SILT	CLAY	10	40		
SS-14	20 RT	59+50	1.00-1.50	A-7-6(23)	54	26	3.0	18.9	19.2	58.9	100	99	81	-
SS-15	20 RT	59+50	3.10-4.60	A-2-4(0)	18	NP	18.1	63.6	4.2	14.2	100	96	19	-
SS-16	20 RT	59+50	8.10-9.60	A-3(0)	18	NP	19.3	71.0	1.6	8.1	100	98	10	-
SS-17	20 RT	59+50	13.10-14.60	A-7-6(28)	59	31	0.8	20.3	22.0	56.9	100	100	82	-
SS-25	30 RT	55+00	1.00-1.50	A-4(0)	23	5	4.4	54.2	19.1	22.2	100	99	46	-
SS-26	30 RT	55+00	4.00-5.50	A-7-6(32)	60	34	2.2	15.0	28.2	54.6	100	98	86	-
SS-27	30 RT	55+00	8.40-9.90	A-2-4(0)	18	4	50.6	24.2	16.2	9.1	98	78	26	-
SS-28	30 RT	55+00	13.40-14.90	A-2-4(0)	18	NP	65.0	23.5	4.4	7.1	94	61	12	-
SS-29	30 RT	55+00	18.40-19.90	A-2-4(0)	27	4	0.4	72.0	15.5	12.1	100	100	35	-
SS-30	30 RT	55+00	23.40-24.90	A-4(2)	31	7	2.6	64.5	14.7	18.2	100	98	52	34
SS-31	30 RT	55+00	28.40-29.90	A-4(1)	29	5	0.2	70.2	17.5	12.1	100	100	59	-
SS-79	57 LT	55+43	1.00-2.00	A-2-4(0)	18	2	6.9	63.5	11.2	18.3	98	95	93	-
SS-80	57 LT	55+43	7.10-8.60	A-6(4)	35	18	7.7	51.9	9.8	30.5	100	98	43	-
SS-81	57 LT	55+43	17.10-18.60	A-7-6(23)	53	26	3.5	18.5	21.0	57.0	100	98	61	-
SS-82	57 LT	55+43	22.10-23.60	A-2-4(0)	25	8	52.7	28.5	4.5	14.3	97	52	16	-
SS-83	57 LT	55+43	32.10-33.60	A-2-4(0)	29	3	1.8	75.2	6.7	16.3	99	98	33	-
SS-84	57 LT	55+43	42.10-43.60	A-4(0)	26	NP	0.8	82.6	10.5	6.1	98	97	37	-
SS-85	57 LT	55+43	47.10-48.60	A-4(4)	33	6	1.4	60.7	17.5	20.4	100	99	70	-
SS-86	57 LT	55+43	57.10-58.60	A-7-6(47)	43	25	2.6	45.2	17.5	34.6	100	99	73	-
SS-32	10 RT	57+50	1.00-1.50	A-2-4(0)	18	NP	12.9	62.7	16.3	8.1	100	98	27	-
SS-33	10 RT	57+50	4.00-5.50	A-7-6(36)	65	42	1.4	20.4	23.6	54.6	100	99	81	30
SS-34	10 RT	57+50	12.10-13.60	A-7-6(23)	50	29	8.9	15.4	35.3	40.4	100	96	78	-
SS-35	10 RT	57+50	17.10-18.60	A-4(0)	16	2	46.3	24.0	20.6	9.1	98	82	60	-
SS-36	10 RT	57+50	22.10-23.60	A-2-4(0)	21	NP	57.9	26.5	6.5	9.1	90	61	16	-
S-20	30 RT	58+50	1.00-15.50	A-2-4(0)	21	NP	21.2	61.7	2.9	14.3	100	97	18	-
S-21	30 RT	58+50	15.50-22.20	A-7-6(22)	54	26	1.4	23.3	20.0	55.2	100	100	78	34.3

- (A) LOOSE TO MEDIUM DENSE TAN SILTY SAND, MOIST (UNDIVIDED COASTAL PLAIN)
- (B) MEDIUM STIFF ORANGE SILTY CLAY, MOIST (UNDIVIDED COASTAL PLAIN)



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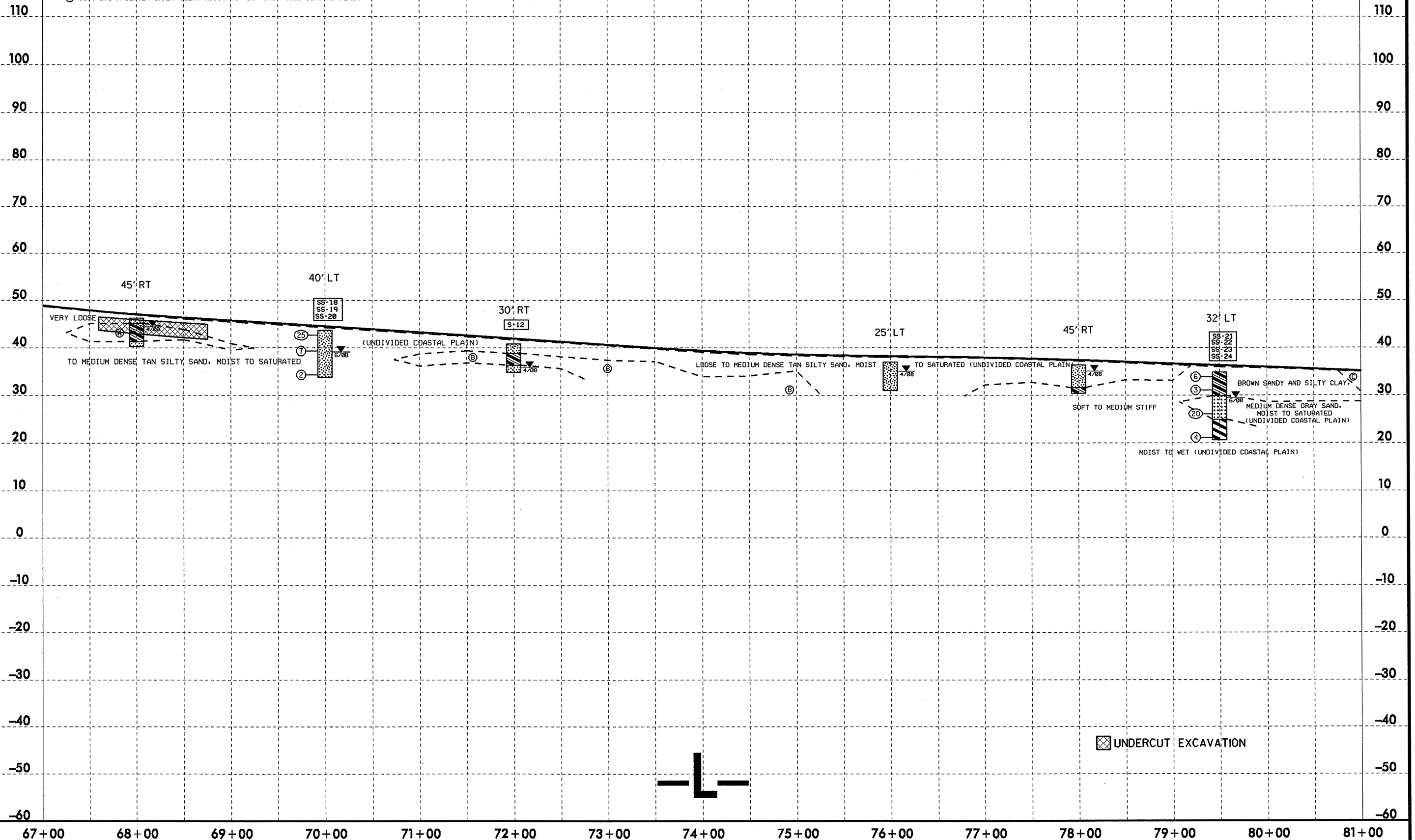
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PROJECT REFERENCE NO. U-5018		SHEET NO. 19	
ROADWAY DESIGN ENGINEER		HYDRAULICS ENGINEER	
INCOMPLETE PLANS DO NOT USE FOR ACQUISITION			
PRELIMINARY PLANS DO NOT USE FOR CONSTRUCTION			

SOIL TEST RESULTS

SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	ASTM CLASS	LL	PL	% BY WEIGHT				% PASSING (SIEVES)		% MOISTURE	% ORGANIC
							C. SAND	F. SAND	SILT	CLAY	#10	#200		
S-12	30 RT	72+00	2.00-4.50	A-1(6)2	24	17	2.7	55.8	10.8	30.7	100	100	45	27.1
SS-18	40 LT	70+00	1.00-1.50	A-2-4(0)	20	21	2.8	71.0	7.0	18.3	100	100	28	-
SS-19	40 LT	70+00	3.30-4.80	A-2-4(0)	20	NR	2.5	84.8	3.5	12.2	100	100	17	-
SS-20	40 LT	70+00	8.30-9.80	A-2-4(0)	21	NP	1.3	73.9	6.0	19.3	100	100	29	-
SS-21	32 LT	79+50	1.00-1.50	A-1(6)3	31	18	2.4	47.8	17.2	38.5	100	100	54	14.8
SS-22	32 LT	79+50	2.80-4.30	A-1(4)	28	15	2.6	47.9	19.0	30.5	100	100	55	-
SS-23	32 LT	79+50	7.80-9.30	A-1(0)	16	NP	46.2	49.5	2.2	2.0	80	80	6	-
SS-24	32 LT	79+50	12.80-14.30	A-1(6)14	42	15	1.0	21.7	36.6	40.6	100	99	85	-

- Ⓐ MEDIUM STIFF TAN SANDY CLAY, MOIST TO WET (UNDIVIDED COASTAL PLAIN)
- Ⓑ MEDIUM STIFF TAN SANDY CLAY, MOIST (UNDIVIDED COASTAL PLAIN)
- Ⓒ SOFT BROWN CLAYEY SANDY SILT, MOIST TO WET (UNDIVIDED COASTAL PLAIN)

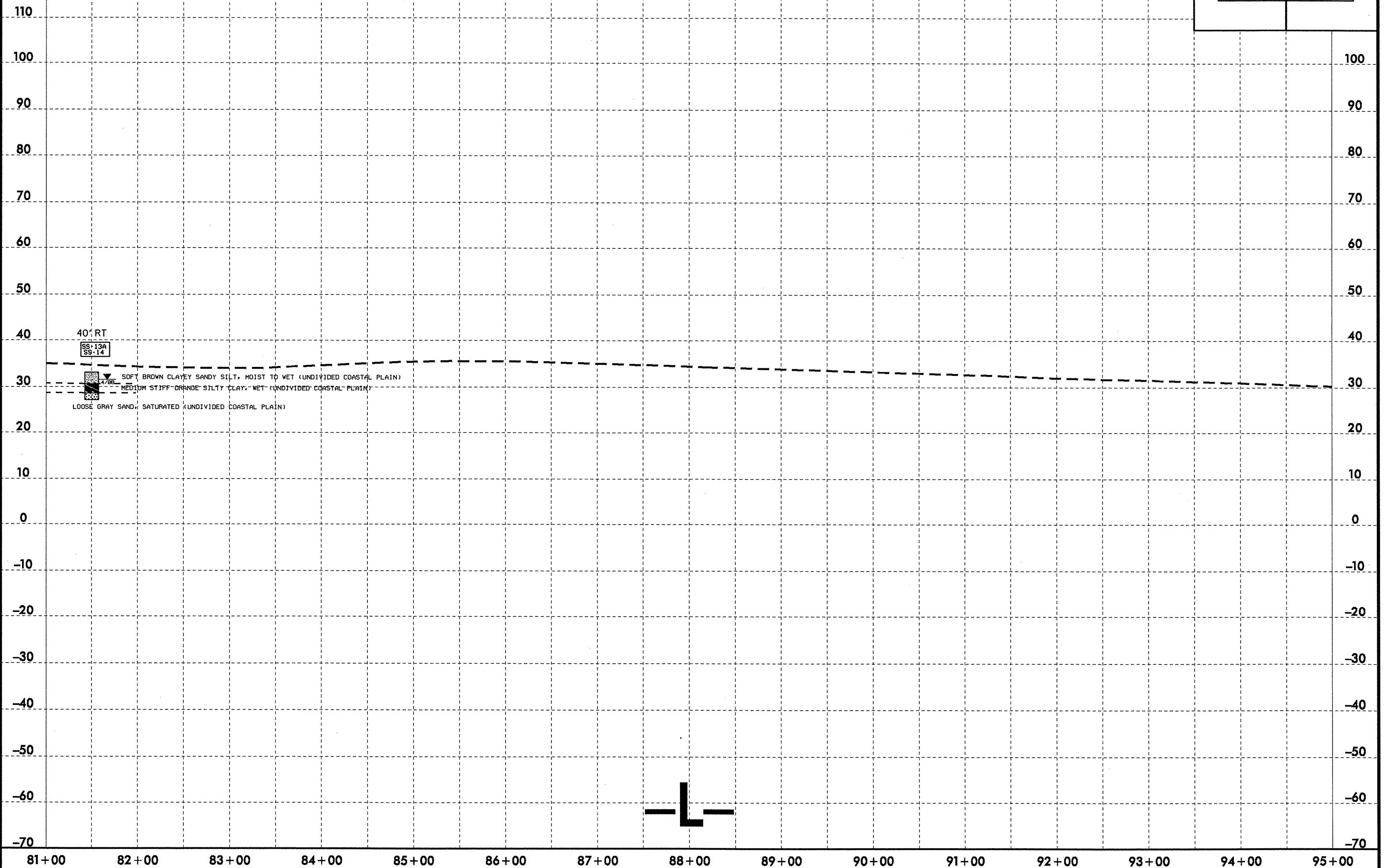


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PROJECT REFERENCE NO.	SHEET NO.
U-5018	20
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
INCOMPLETE PLANS DO NOT USE FOR ACQUISITION	
PRELIMINARY PLANS DO NOT USE FOR CONSTRUCTION	

SOIL TEST RESULTS															
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS	LL	PI	% BY WEIGHT				% PASSING (SIEVES)			%	%
							G.SAND	F.SAND	SILT	CLAY	10	40	100		
S-13A	40' RT	81+50	1.00-2.50	A-4(0)	8	3	3.9	44.8	30.9	20.4	100	99	59	-	-
S-14	40' RT	81+50	2.50-4.50	A-7-6(20)	44	28	1.8	25.4	23.7	49.1	99	98	76	-	-

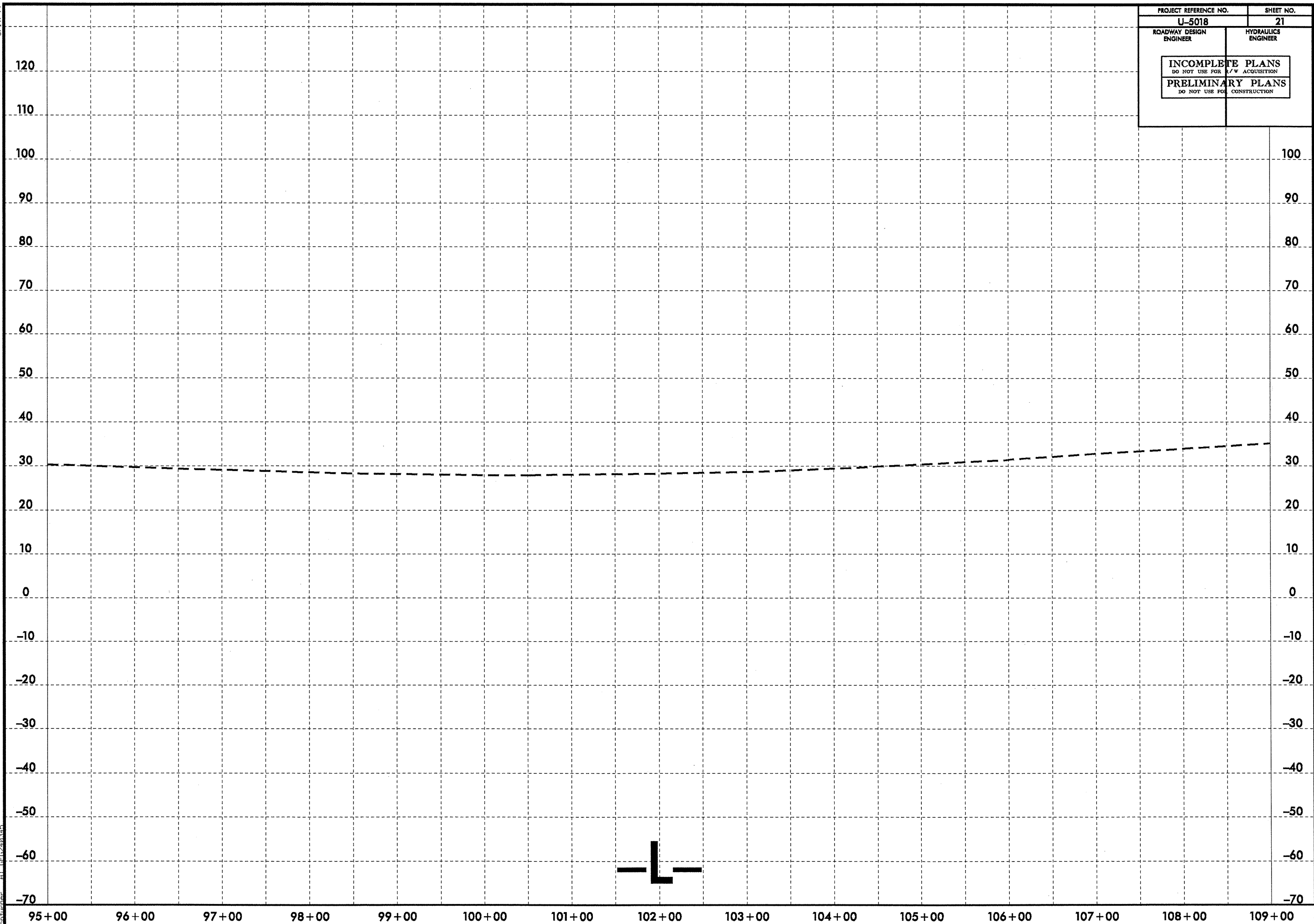


81+00 82+00 83+00 84+00 85+00 86+00 87+00 88+00 89+00 90+00 91+00 92+00 93+00 94+00 95+00

5/14/99

PROJECT REFERENCE NO. U-5018	SHEET NO. 21
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
INCOMPLETE PLANS DO NOT USE FOR ACQUISITION	
PRELIMINARY PLANS DO NOT USE FOR CONSTRUCTION	

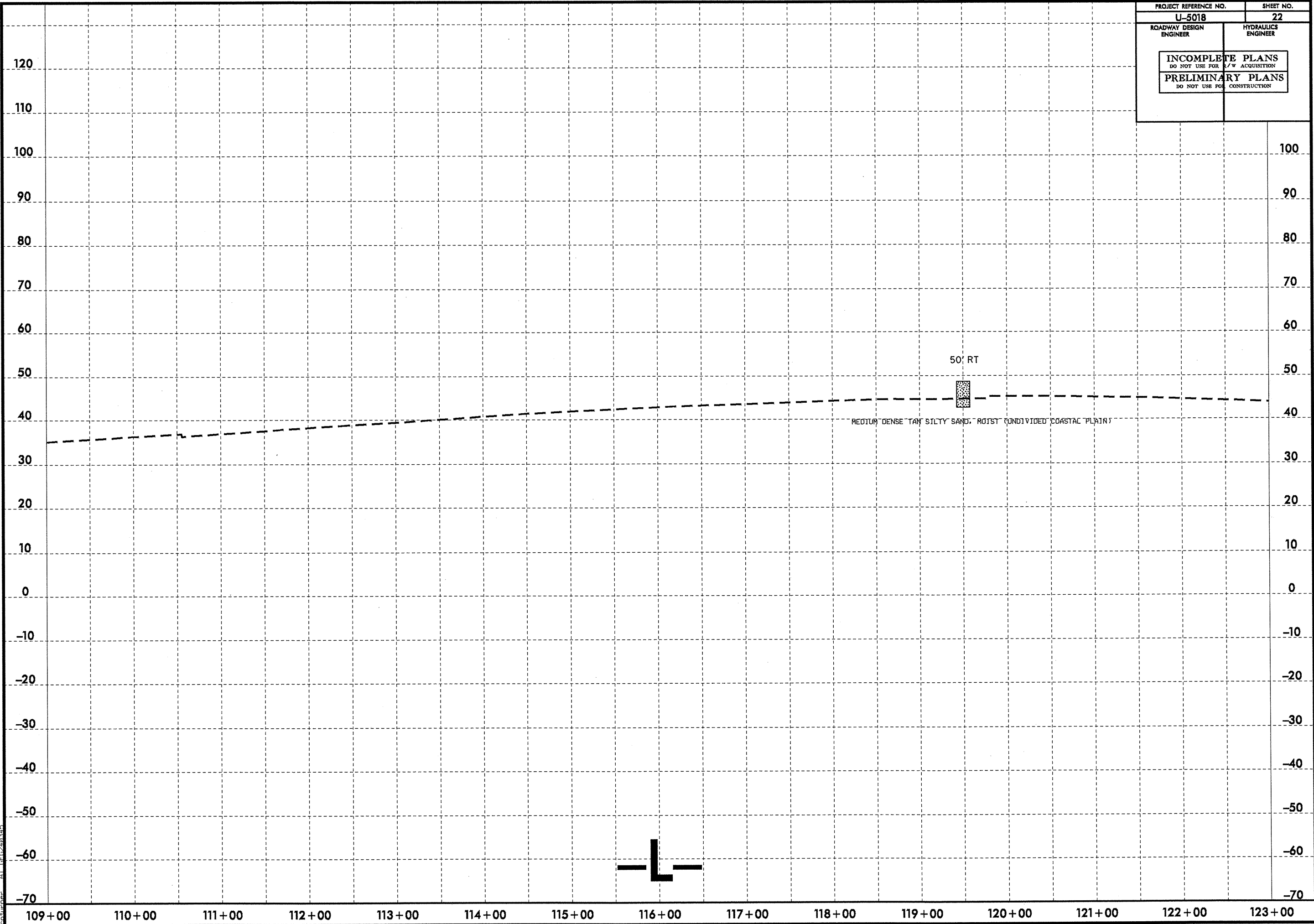
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AT: 06/24/2008



5/14/99

PROJECT REFERENCE NO.		SHEET NO.	
U-5018		22	
ROADWAY DESIGN ENGINEER		HYDRAULICS ENGINEER	
INCOMPLETE PLANS DO NOT USE FOR ACQUISITION		PRELIMINARY PLANS DO NOT USE FOR CONSTRUCTION	

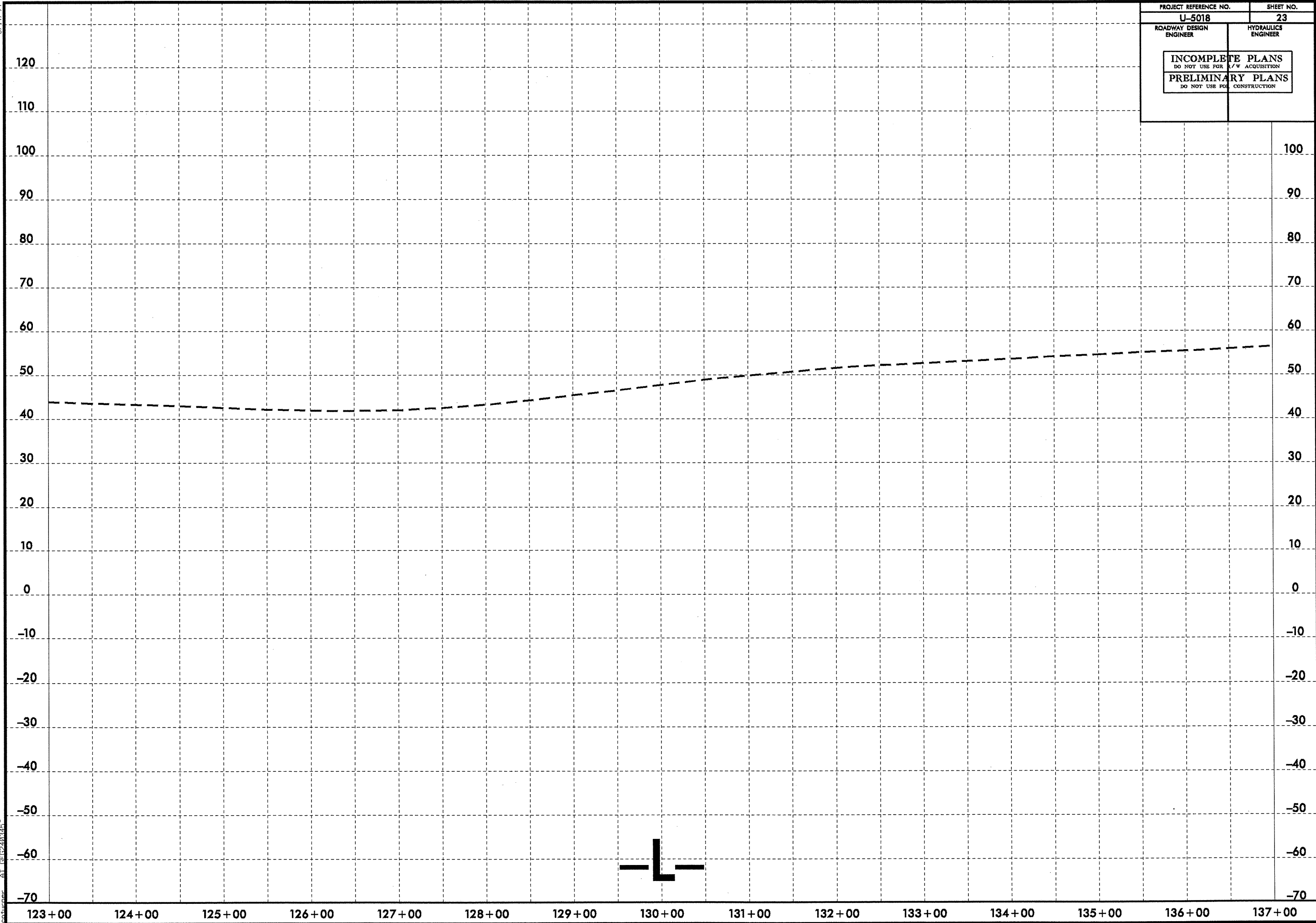
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100
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60
50
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-10
-20
-30
-40
-50
-60
-70

109+00 110+00 111+00 112+00 113+00 114+00 115+00 116+00 117+00 118+00 119+00 120+00 121+00 122+00 123+00

PROJECT REFERENCE NO.	SHEET NO.
U-5018	23
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
INCOMPLETE PLANS DO NOT USE FOR ACQUISITION	
PRELIMINARY PLANS DO NOT USE FOR CONSTRUCTION	

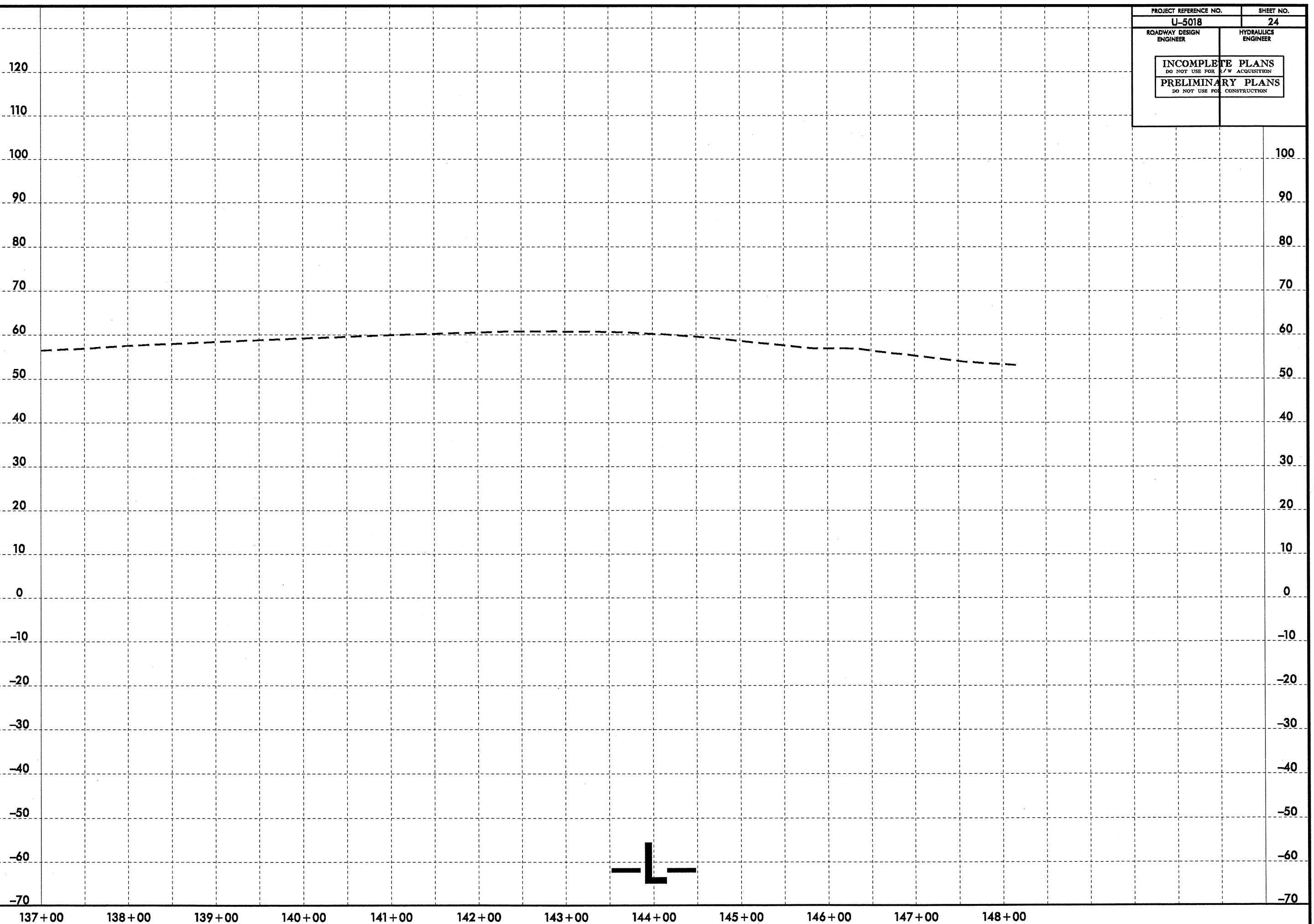


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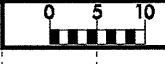
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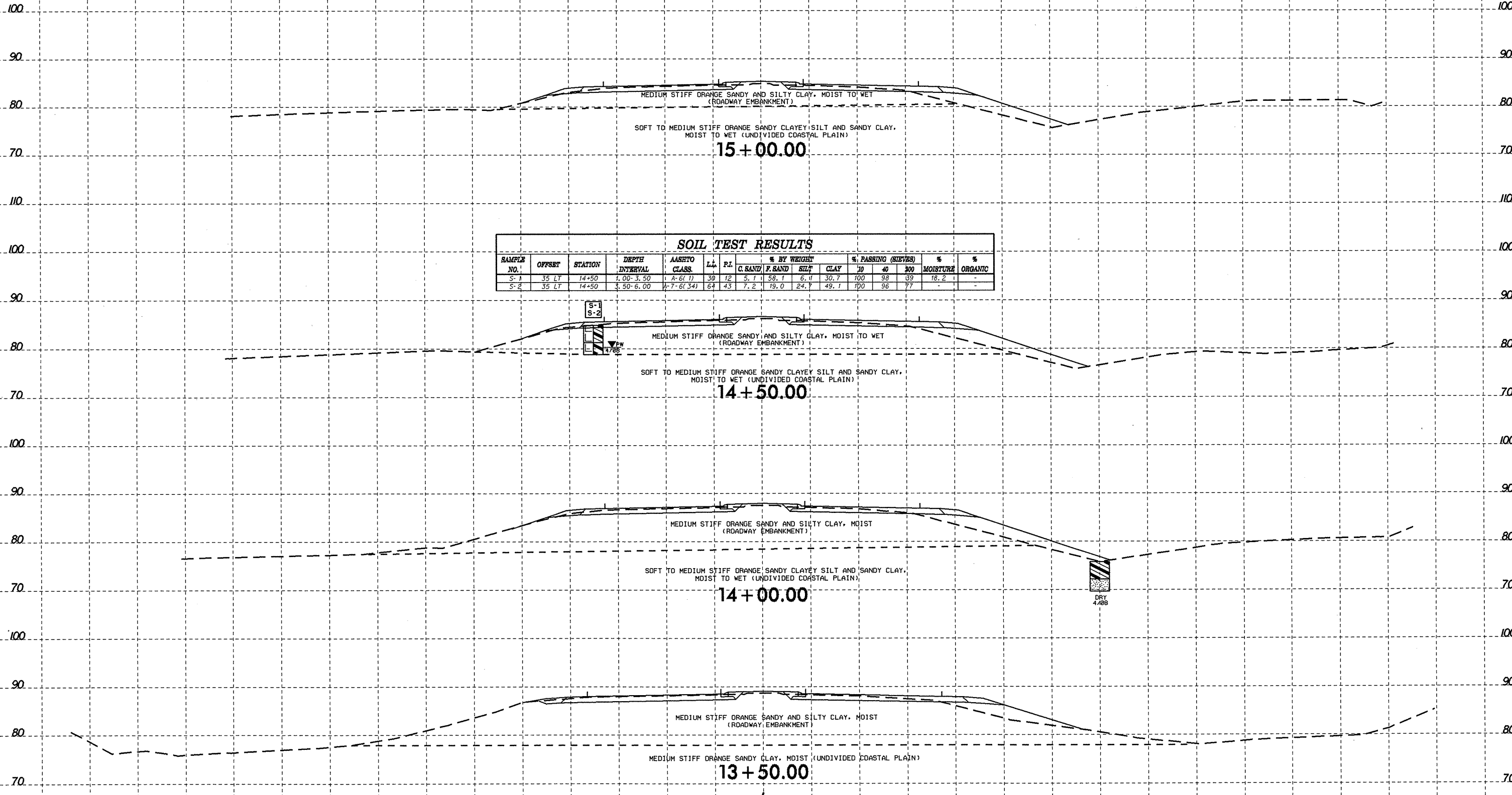
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ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
INCOMPLETE PLANS DO NOT USE FOR ACQUISITION	
PRELIMINARY PLANS DO NOT USE FOR CONSTRUCTION	



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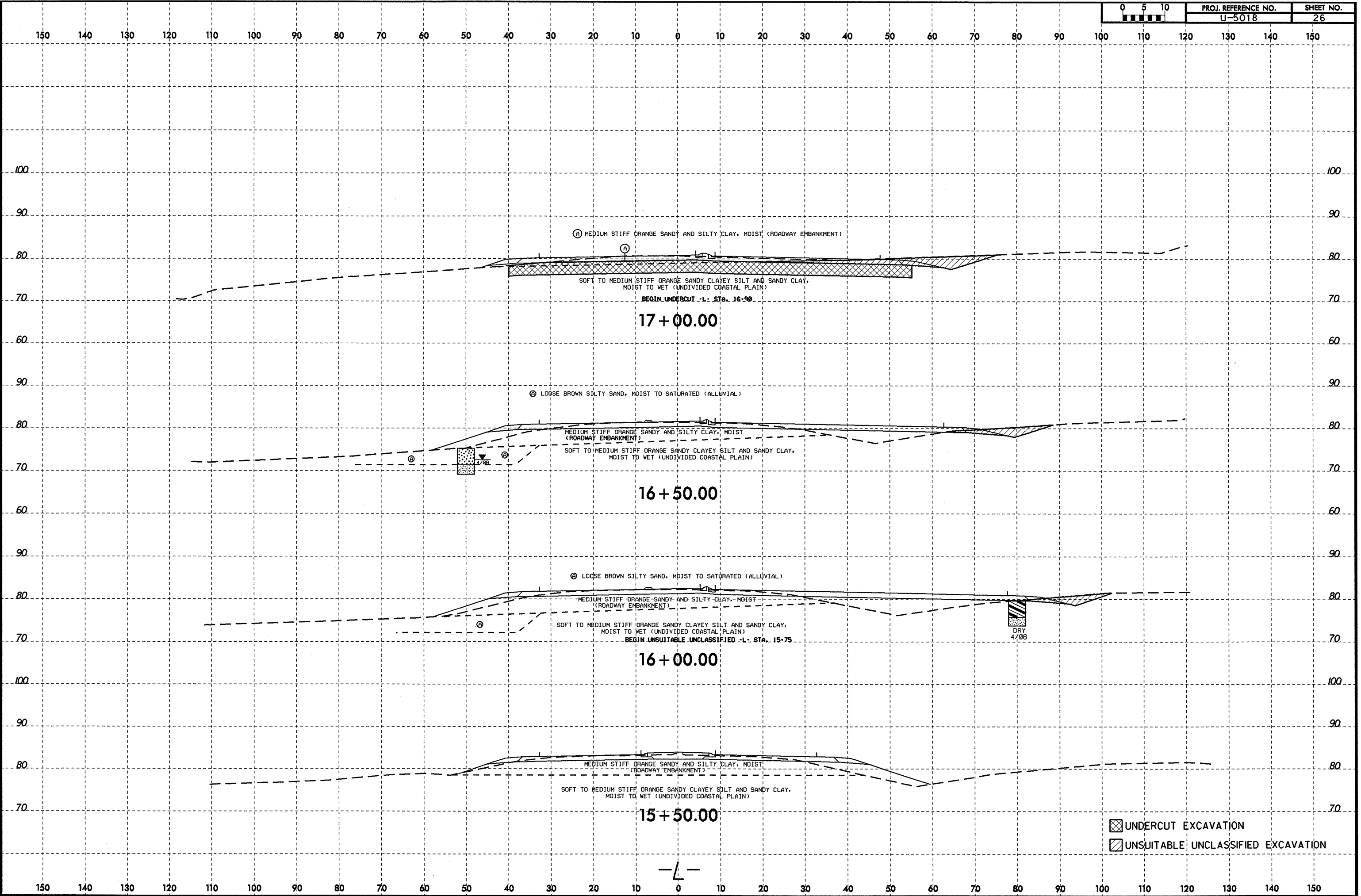


SOIL TEST RESULTS

SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASTHO CLASS.	LL	PI	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C. SAND	F. SAND	SILT	CLAY	#10	#40	#200		
S-1	35 LT	14+50	1.00-3.50	A-6(7)	30	12	5.7	58.1	6.1	30.7	100	98	89	18.2	-
S-2	35 LT	14+50	3.50-6.00	A-7-6(34)	64	43	7.2	19.0	24.7	49.1	100	96	77	-	-

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Author: AL BE 0240343



(A) MEDIUM STIFF ORANGE SANDY AND SILTY CLAY, MOIST (ROADWAY EMBANKMENT)

SOFT TO MEDIUM STIFF ORANGE SANDY CLAYEY SILT AND SANDY CLAY, MOIST TO WET (UNDIVIDED COASTAL PLAIN)

BEGIN UNDERCUT -L- STA. 16-90
17+00.00

(B) LOOSE BROWN SILTY SAND, MOIST TO SATURATED (ALLUVIAL)

MEDIUM STIFF ORANGE SANDY AND SILTY CLAY, MOIST (ROADWAY EMBANKMENT)
SOFT TO MEDIUM STIFF ORANGE SANDY CLAYEY SILT AND SANDY CLAY, MOIST TO WET (UNDIVIDED COASTAL PLAIN)

16+50.00

(B) LOOSE BROWN SILTY SAND, MOIST TO SATURATED (ALLUVIAL)

MEDIUM STIFF ORANGE SANDY AND SILTY CLAY, MOIST (ROADWAY EMBANKMENT)
SOFT TO MEDIUM STIFF ORANGE SANDY CLAYEY SILT AND SANDY CLAY, MOIST TO WET (UNDIVIDED COASTAL PLAIN)

BEGIN UNSUITABLE UNCLASSIFIED -L- STA. 15-75
16+00.00

DRY 4/08

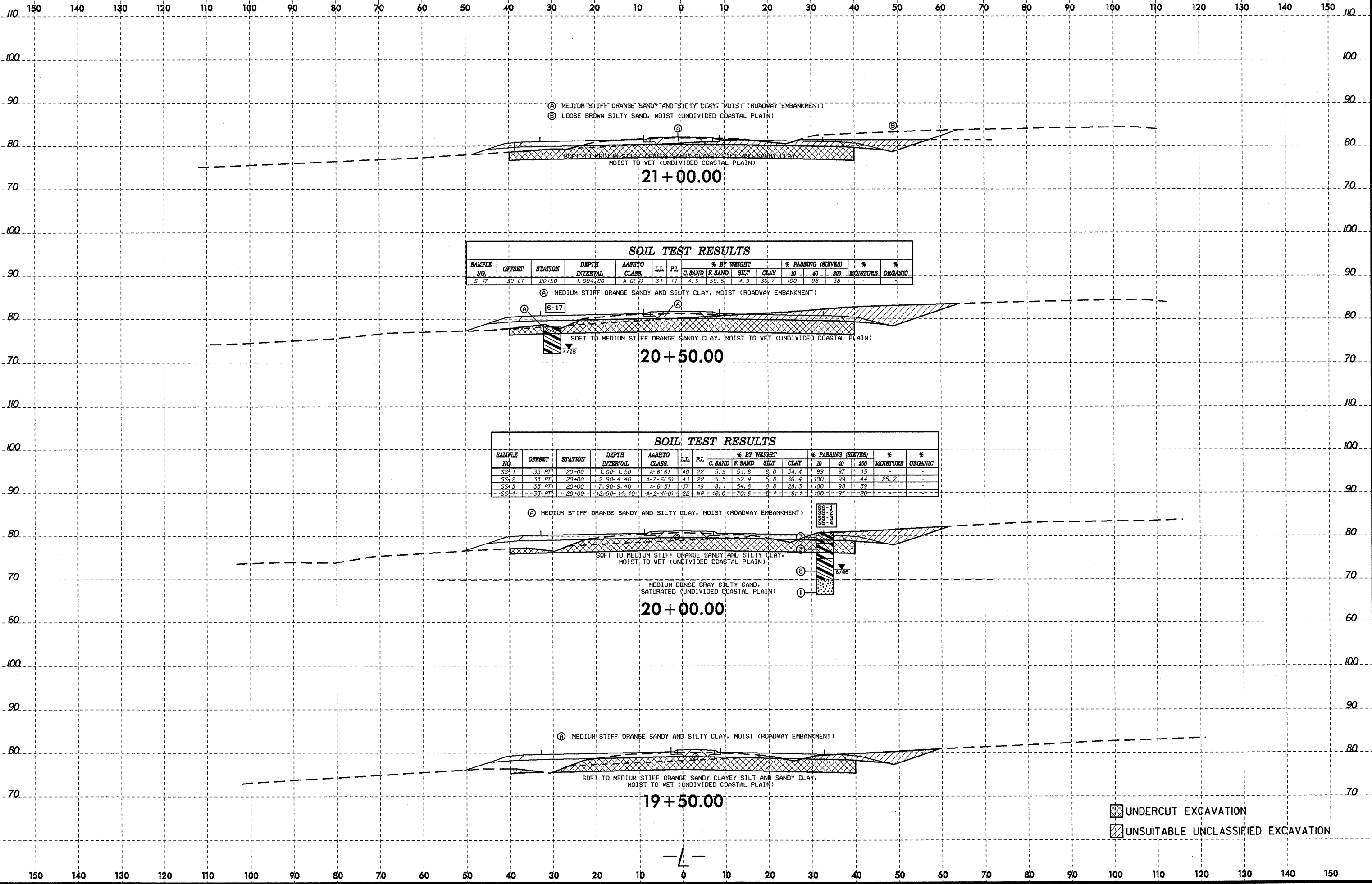
MEDIUM STIFF ORANGE SANDY AND SILTY CLAY, MOIST (ROADWAY EMBANKMENT)

SOFT TO MEDIUM STIFF ORANGE SANDY CLAYEY SILT AND SANDY CLAY, MOIST TO WET (UNDIVIDED COASTAL PLAIN)

15+50.00

UNDERCUT EXCAVATION
UNSUITABLE UNCLASSIFIED EXCAVATION





Ⓐ MEDIUM STIFF ORANGE SANDY AND SILTY CLAY, MOIST (ROADWAY EMBANKMENT)
 Ⓑ LOOSE BROWN SILTY SAND, MOIST (UNDIVIDED COASTAL PLAIN)

SOFT TO MEDIUM STIFF ORANGE SANDY CLAY, MOIST TO WET (UNDIVIDED COASTAL PLAIN)
 21+00.00

SOIL TEST RESULTS

SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			%	%
							C. SAND	F. SAND	SILT	CLAY	10	40	200		
S-17	30 LT	20+50	1.00-4.80	A-6(7)	37	11	4.9	59.5	4.9	30.7	100	98	38	-	-

Ⓐ MEDIUM STIFF ORANGE SANDY AND SILTY CLAY, MOIST (ROADWAY EMBANKMENT)

SOFT TO MEDIUM STIFF ORANGE SANDY CLAY, MOIST TO WET (UNDIVIDED COASTAL PLAIN)
 20+50.00

SOIL TEST RESULTS

SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			%	%
							C. SAND	F. SAND	SILT	CLAY	10	40	200		
SS-1	33 RT	20+00	1.00-1.50	A-6(6)	40	22	5.3	51.8	8.0	34.4	99	97	45	-	-
SS-2	33 RT	20+00	2.90-4.40	A-7-6(5)	41	22	5.5	52.4	5.8	36.4	100	99	44	25.2	-
SS-3	33 RT	20+00	7.90-9.40	A-6(3)	37	19	8.1	54.8	8.8	28.3	100	98	39	-	-
SS-4	33 RT	20+00	12.90-14.40	A-2-4(0)	22	NP	16.6	70.6	3.4	8.1	100	97	20	-	-

Ⓐ MEDIUM STIFF ORANGE SANDY AND SILTY CLAY, MOIST (ROADWAY EMBANKMENT)

SOFT TO MEDIUM STIFF ORANGE SANDY AND SILTY CLAY, MOIST TO WET (UNDIVIDED COASTAL PLAIN)

MEDIUM DENSE GRAY SILTY SAND, SATURATED (UNDIVIDED COASTAL PLAIN)

20+00.00

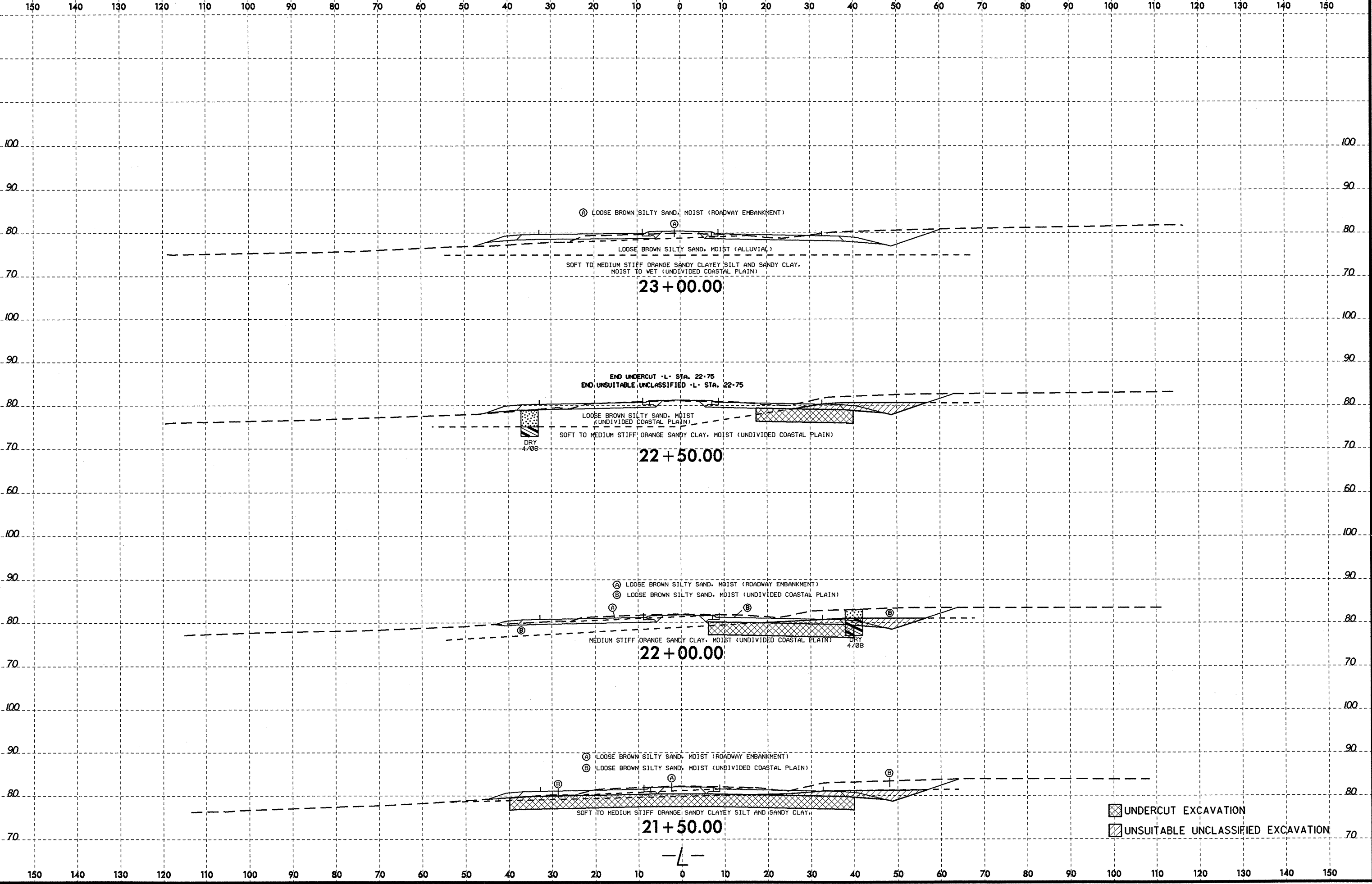
Ⓐ MEDIUM STIFF ORANGE SANDY AND SILTY CLAY, MOIST (ROADWAY EMBANKMENT)

SOFT TO MEDIUM STIFF ORANGE SANDY CLAYEY SILT AND SANDY CLAY, MOIST TO WET (UNDIVIDED COASTAL PLAIN)

19+50.00

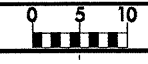
⊠ UNDERCUT EXCAVATION
 ⊞ UNSUITABLE UNCLASSIFIED EXCAVATION

8/23/99

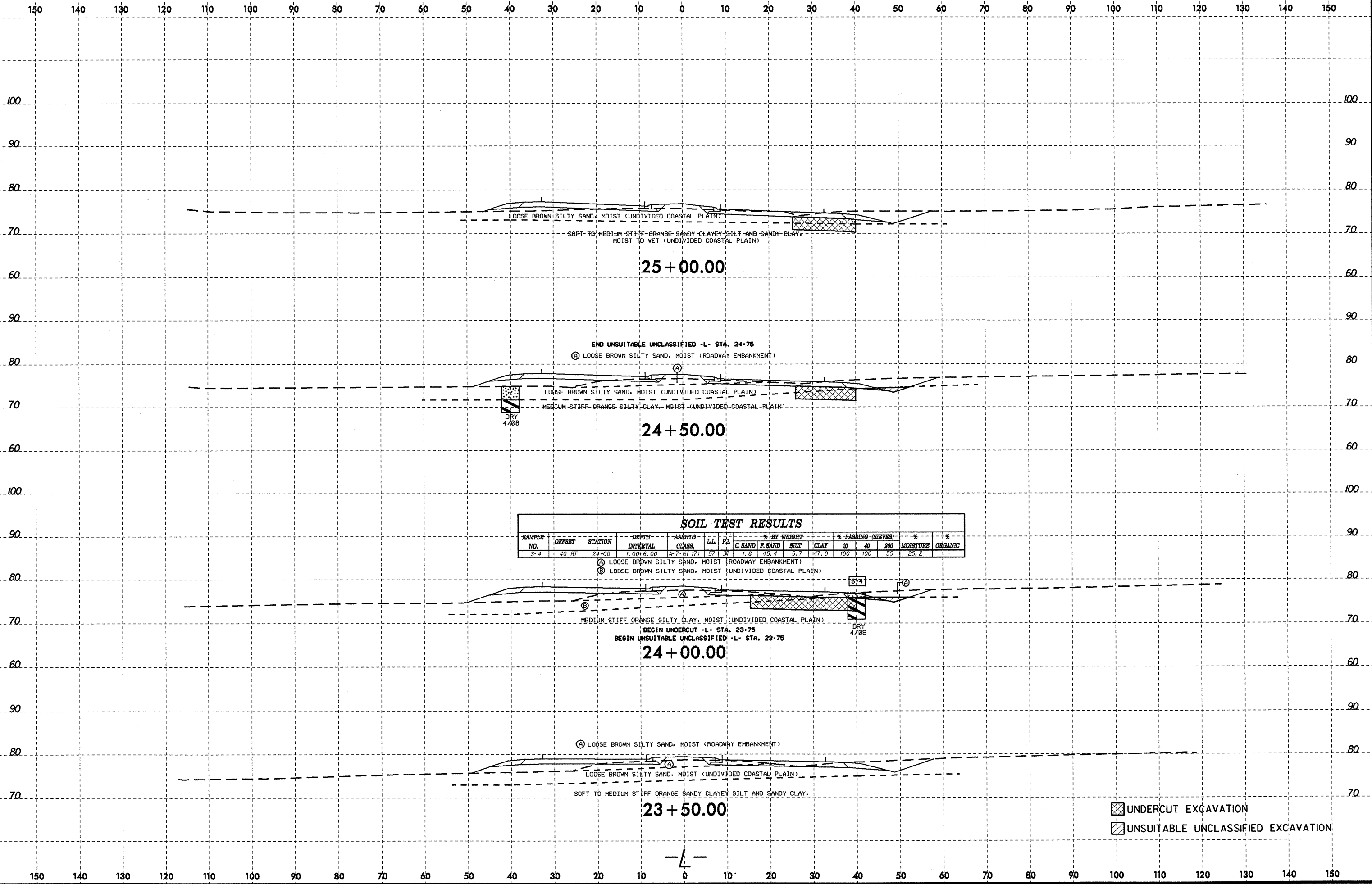


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SHEET NO. 30

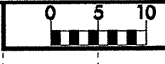


SOIL TEST RESULTS

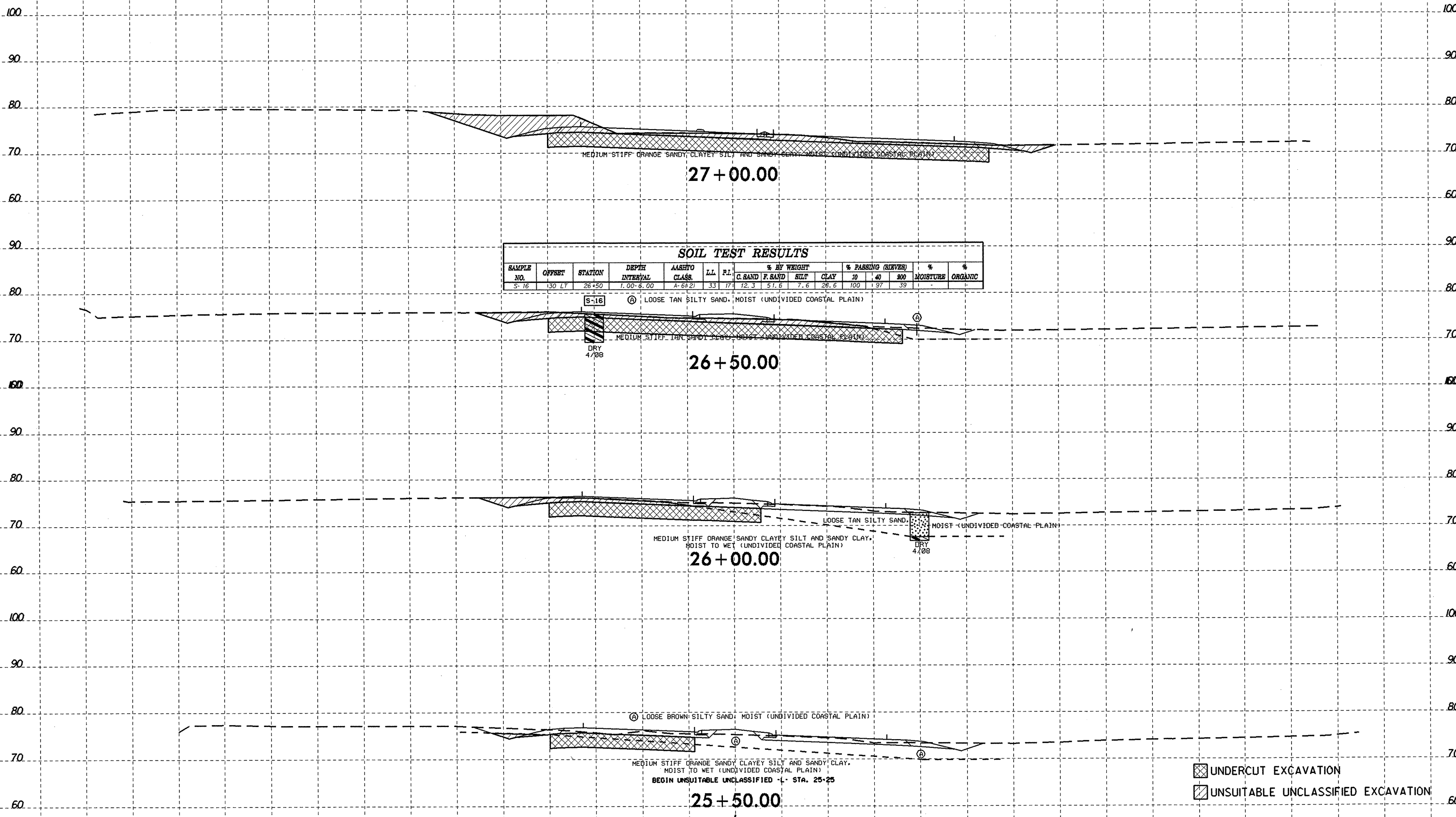
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	ASHTO CLASS.	LL	PL	% BY WEIGHT				% PASSING (SIZES)			% MOISTURE	% ORGANIC
							C. SAND	F. SAND	SILT	CLAY	#10	#40	#200		
S-4	40 FT	24+00	1.00' - 6.00'	A-7-6(17)	57	30	1.8	45.4	5.7	47.0	100	100	55	25.2	-

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27+00.00

MEDIUM STIFF ORANGE SANDY CLAYEY SILT AND SANDY CLAY, MOIST (UNDIVIDED COASTAL PLAIN)

SOIL TEST RESULTS

SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	LL	PL	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C. SAND	F. SAND	SILT	CLAY	20	40	200		
S-76	130 LT	26+50	1.00-5.00	A-6(2)	33	17	12.3	51.5	7.6	28.6	100	97	39		

S-16 (A) LOOSE TAN SILTY SAND, MOIST (UNDIVIDED COASTAL PLAIN)

26+50.00

DRY 4/08

MEDIUM STIFF TAN SANDY CLAY, MOIST (UNDIVIDED COASTAL PLAIN)

26+00.00

LOOSE TAN SILTY SAND, MOIST (UNDIVIDED COASTAL PLAIN)

MEDIUM STIFF ORANGE SANDY CLAYEY SILT AND SANDY CLAY, MOIST TO WET (UNDIVIDED COASTAL PLAIN)

DRY 4/08

(A) LOOSE BROWN SILTY SAND, MOIST (UNDIVIDED COASTAL PLAIN)

25+50.00

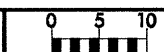
MEDIUM STIFF ORANGE SANDY CLAYEY SILT AND SANDY CLAY, MOIST TO WET (UNDIVIDED COASTAL PLAIN)

BEGIN UNSUITABLE UNCLASSIFIED - STA. 25+25

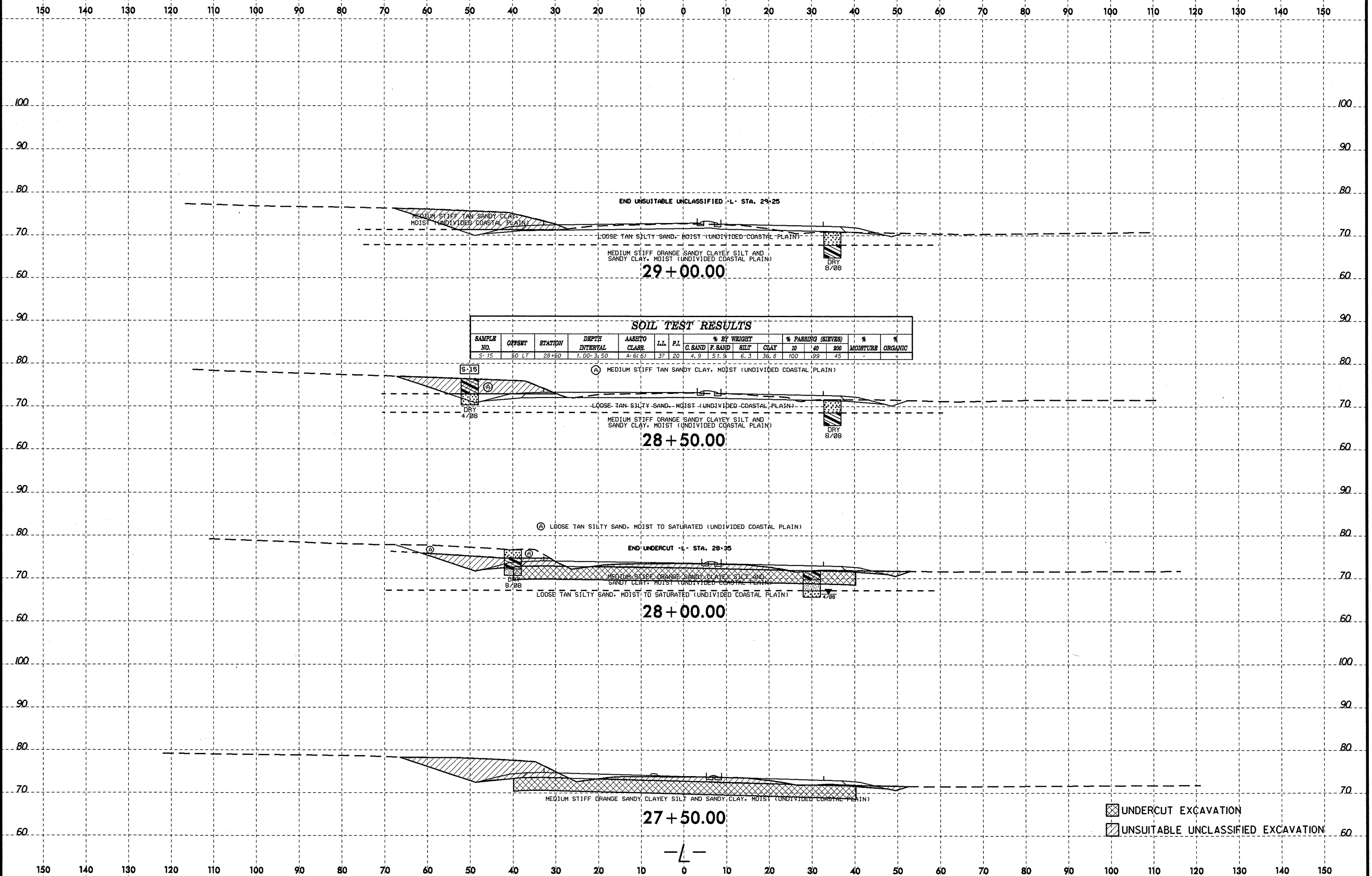
- UNDERCUT EXCAVATION
- UNSUITABLE UNCLASSIFIED EXCAVATION

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PROJ. REFERENCE NO. U-5018 SHEET NO. 32



END UNSUITABLE UNCLASSIFIED - L- STA. 29-25

MEDIUM STIFF TAN SANDY CLAY, MOIST (UNDIVIDED COASTAL PLAIN)
LOOSE TAN SILTY SAND, MOIST (UNDIVIDED COASTAL PLAIN)
MEDIUM STIFF ORANGE SANDY CLAYEY SILT AND SANDY CLAY, MOIST (UNDIVIDED COASTAL PLAIN)
29 + 00.00
DRY 8/08

SOIL TEST RESULTS

SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	LL	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C. SAND	F. SAND	SILT	CLAY	10	40	200		
S-15	50 LT	28+50	1.00-3.50	A-6(6)	37	20	4.9	51.9	6.3	36.8	100	99	45	-	-

(A) MEDIUM STIFF TAN SANDY CLAY, MOIST (UNDIVIDED COASTAL PLAIN)
LOOSE TAN SILTY SAND, MOIST (UNDIVIDED COASTAL PLAIN)
MEDIUM STIFF ORANGE SANDY CLAYEY SILT AND SANDY CLAY, MOIST (UNDIVIDED COASTAL PLAIN)
28 + 50.00
DRY 4/08
DRY 8/08

(A) LOOSE TAN SILTY SAND, MOIST TO SATURATED (UNDIVIDED COASTAL PLAIN)

END UNDERCUT - L- STA. 28-35

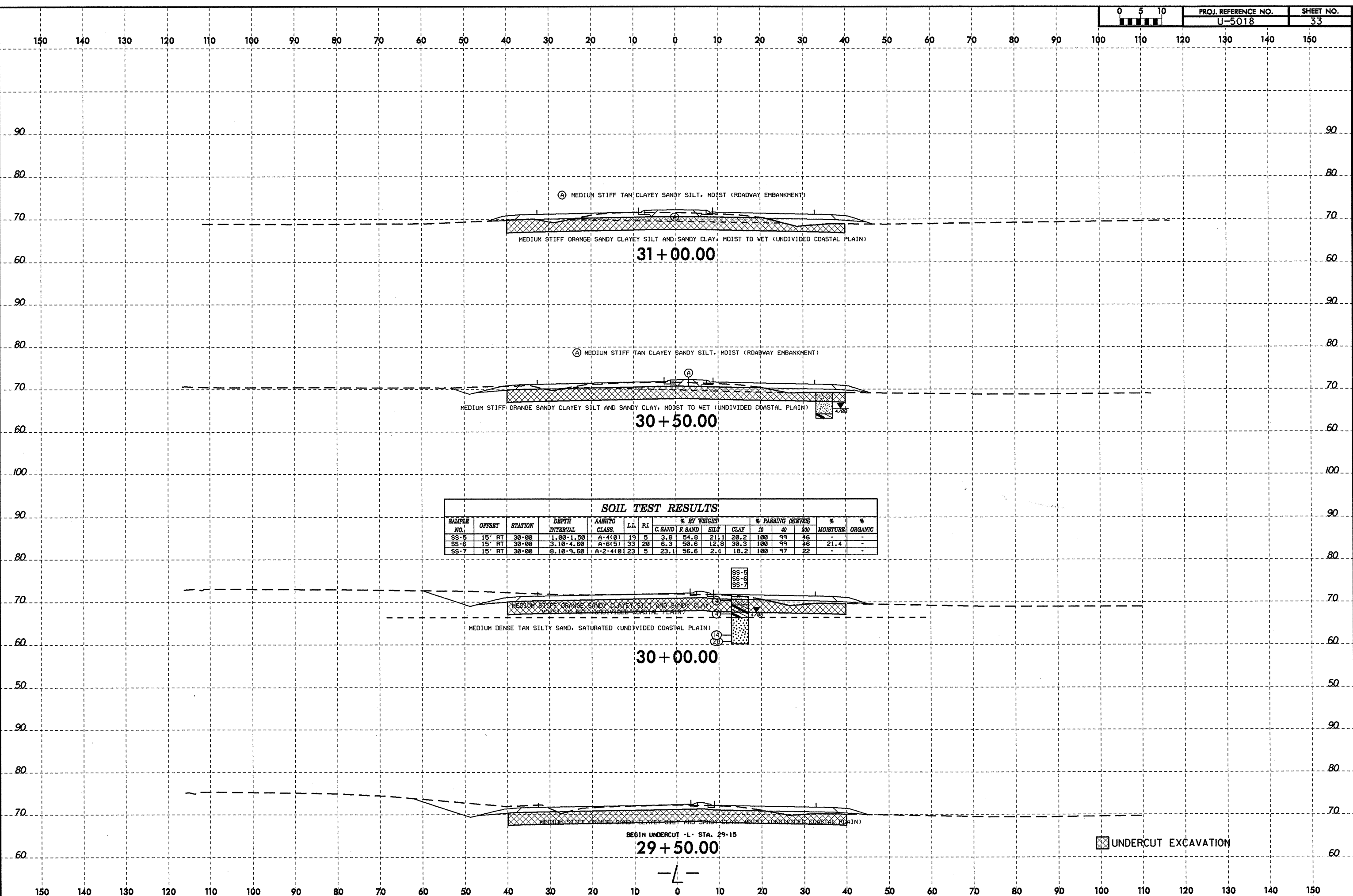
MEDIUM STIFF ORANGE SANDY CLAYEY SILT AND SANDY CLAY, MOIST (UNDIVIDED COASTAL PLAIN)
LOOSE TAN SILTY SAND, MOIST TO SATURATED (UNDIVIDED COASTAL PLAIN)
28 + 00.00
DRY 8/08
DRY 4/08

MEDIUM STIFF ORANGE SANDY CLAYEY SILT AND SANDY CLAY, MOIST (UNDIVIDED COASTAL PLAIN)
27 + 50.00

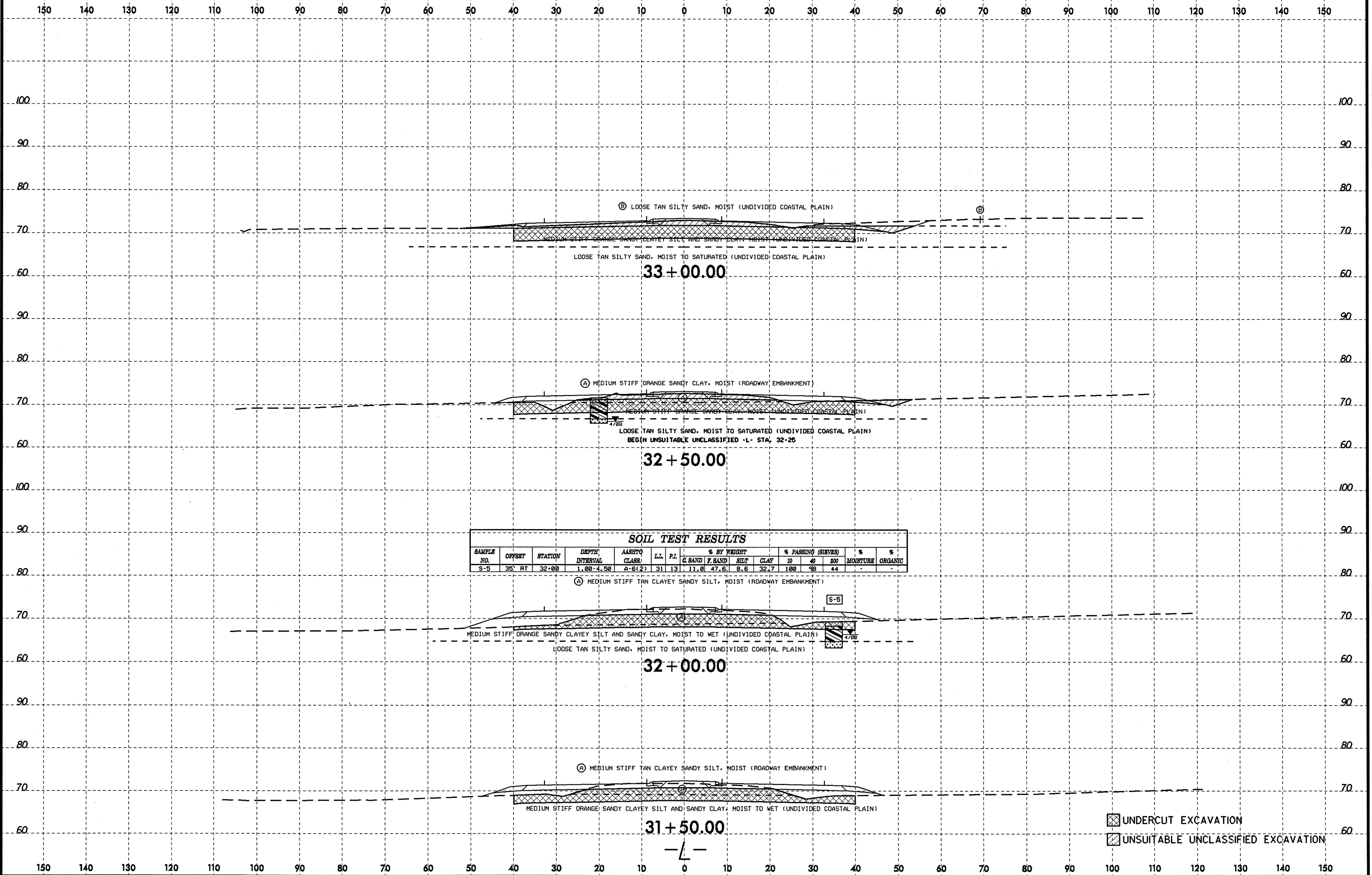
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ⓑ LOOSE TAN SILTY SAND, MOIST (UNDIVIDED COASTAL PLAIN)

MEDIUM STIFF ORANGE SANDY CLAYEY SILT AND SANDY CLAY, MOIST (UNDIVIDED COASTAL PLAIN)

LOOSE TAN SILTY SAND, MOIST TO SATURATED (UNDIVIDED COASTAL PLAIN)

35 + 00.00

90
80
70
60

ⓑ LOOSE TAN SILTY SAND, MOIST (UNDIVIDED COASTAL PLAIN)

MEDIUM STIFF ORANGE SANDY CLAY, MOIST (UNDIVIDED COASTAL PLAIN)

LOOSE TAN SILTY SAND, MOIST TO SATURATED (UNDIVIDED COASTAL PLAIN)

34 + 50.00

100
90
80
70

SOIL TEST RESULTS

SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	LL	PL	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C.SAND	F.SAND	SILT	CLAY	10	40	200		
S-6	50' RT	34+00	1.00-6.00	A-6(5)	40	22	2.3	56.4	5.5	34.8	100	100	43		

Ⓐ MEDIUM STIFF ORANGE SANDY CLAY, MOIST (ROADWAY EMBANKMENT)

ⓑ LOOSE TAN SILTY SAND, MOIST (UNDIVIDED COASTAL PLAIN)

MEDIUM STIFF ORANGE SANDY CLAY, MOIST TO SATURATED (UNDIVIDED COASTAL PLAIN)

LOOSE TAN SILTY SAND, SATURATED (UNDIVIDED COASTAL PLAIN)

34 + 00.00

90
80
70
60

ⓑ LOOSE TAN SILTY SAND, MOIST (UNDIVIDED COASTAL PLAIN)

MEDIUM STIFF ORANGE SANDY CLAYEY SILT AND SANDY CLAY, MOIST (UNDIVIDED COASTAL PLAIN)

LOOSE TAN SILTY SAND, MOIST TO SATURATED (UNDIVIDED COASTAL PLAIN)

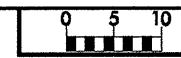
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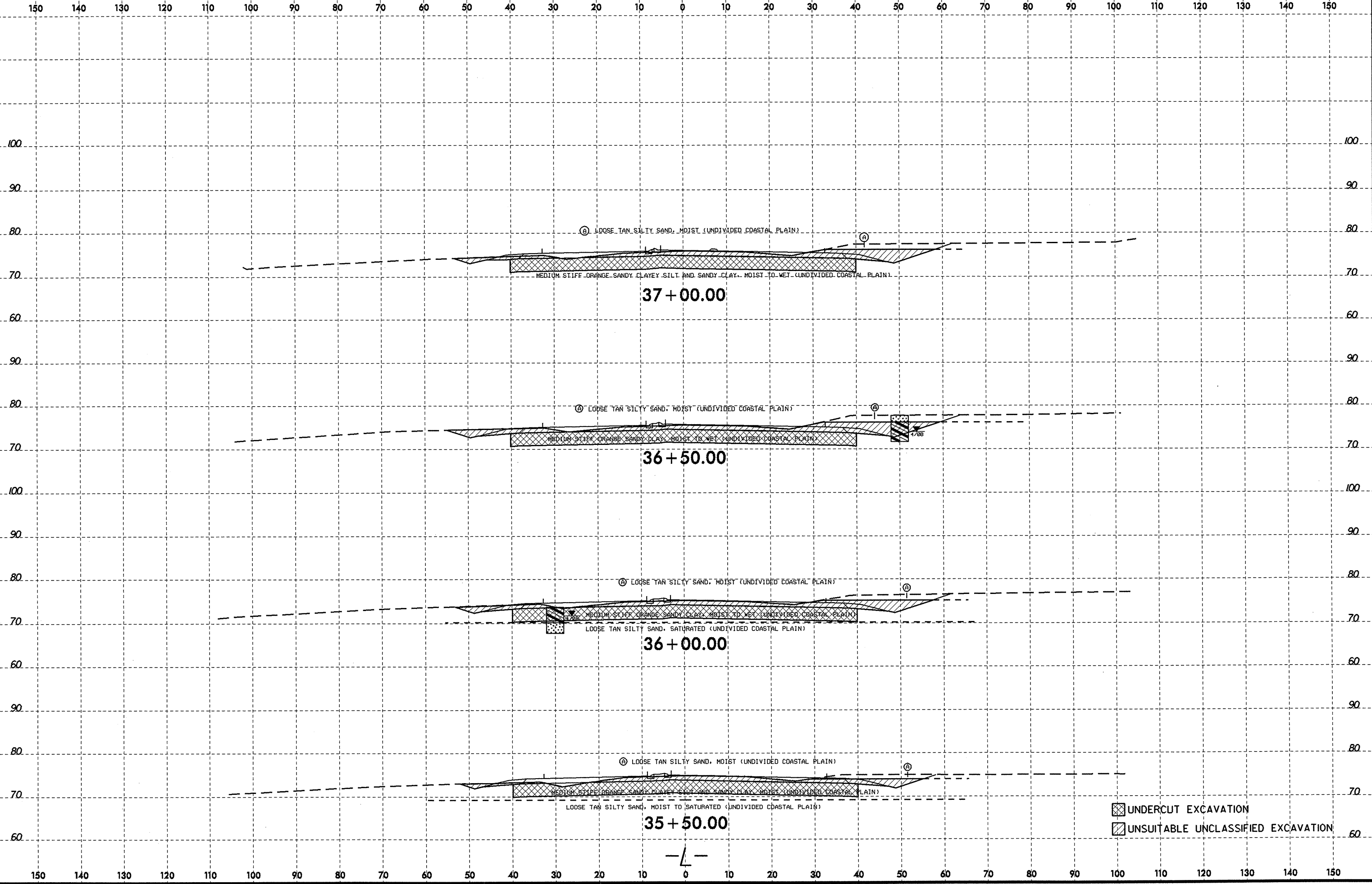
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⊞ UNSUITABLE UNCLASSIFIED EXCAVATION

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PROJ. REFERENCE NO.	SHEET NO.
U-5018	36



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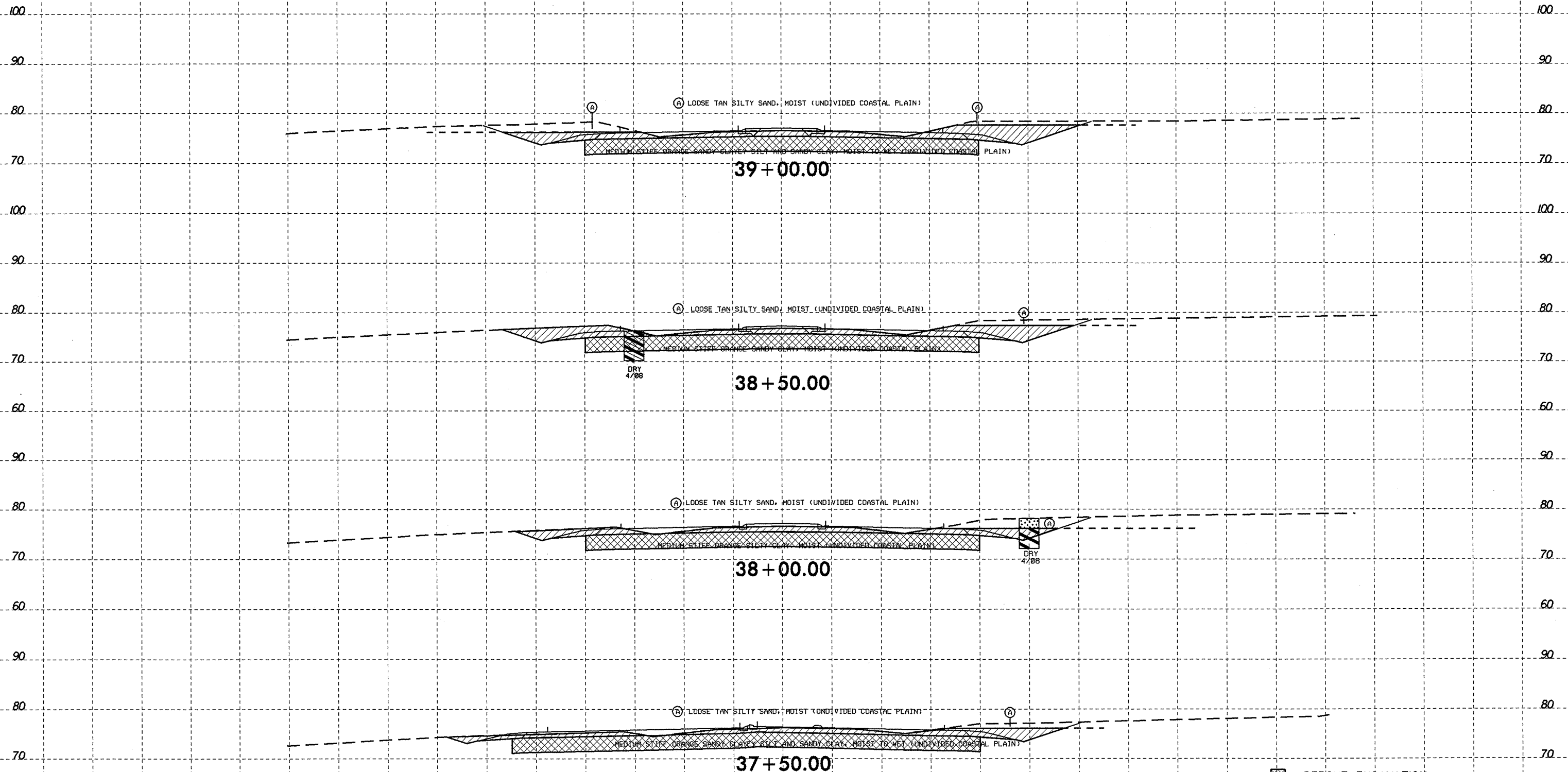
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
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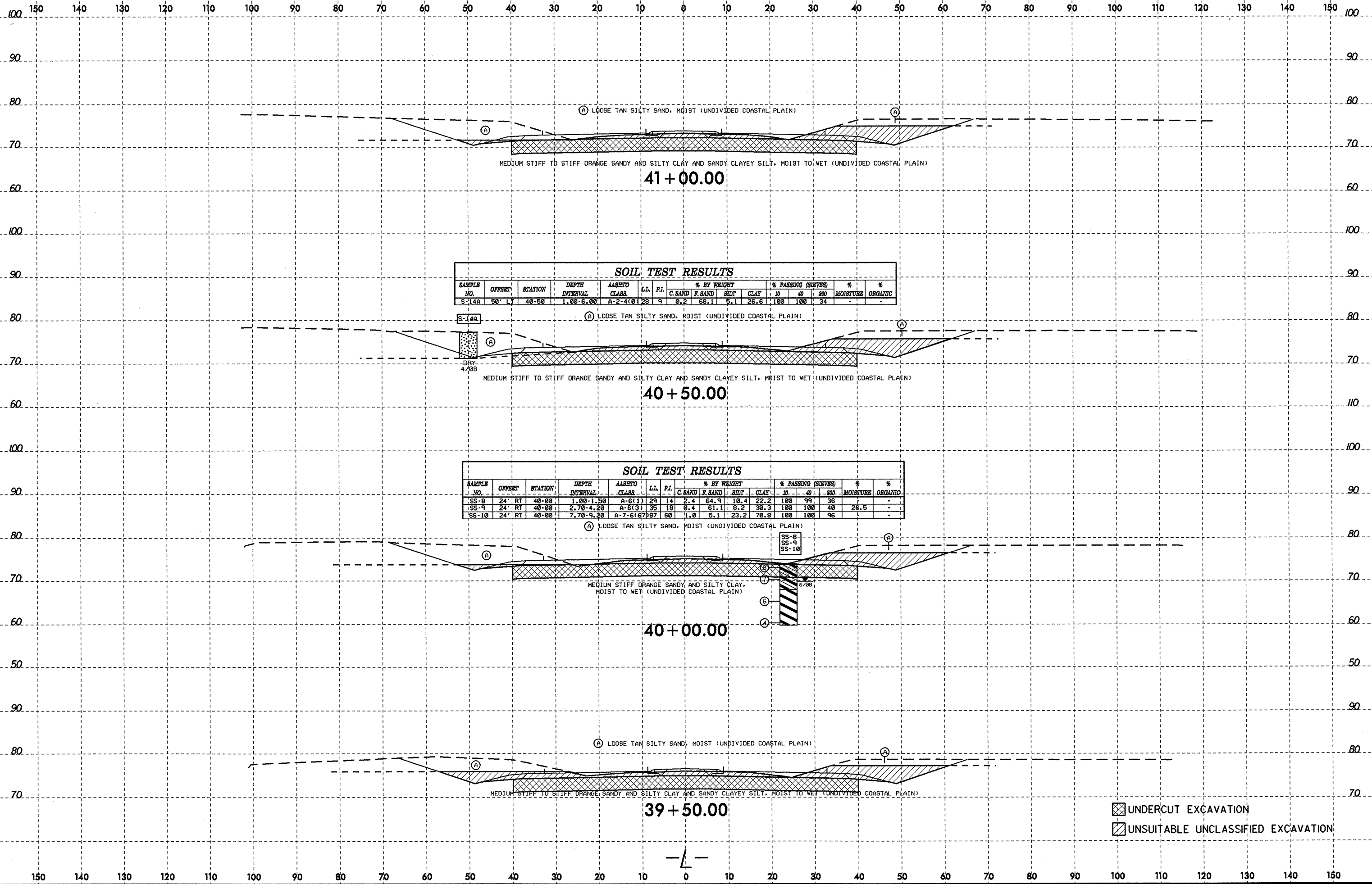


 UNDERCUT EXCAVATION
 UNSUITABLE UNCLASSIFIED EXCAVATION

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SOIL TEST RESULTS

SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	LL	PI	% BY WEIGHT				% PASSING (SIEVES)			%	%
							C. SAND	F. SAND	SILT	CLAY	#10	#40	#200		
S-14A	50' LT	40+50	1.00-6.00	A-2-4(0)	28	9	0.2	68.1	5.1	26.6	100	100	34	-	-

S-14A

-DRY
4/08

SOIL TEST RESULTS

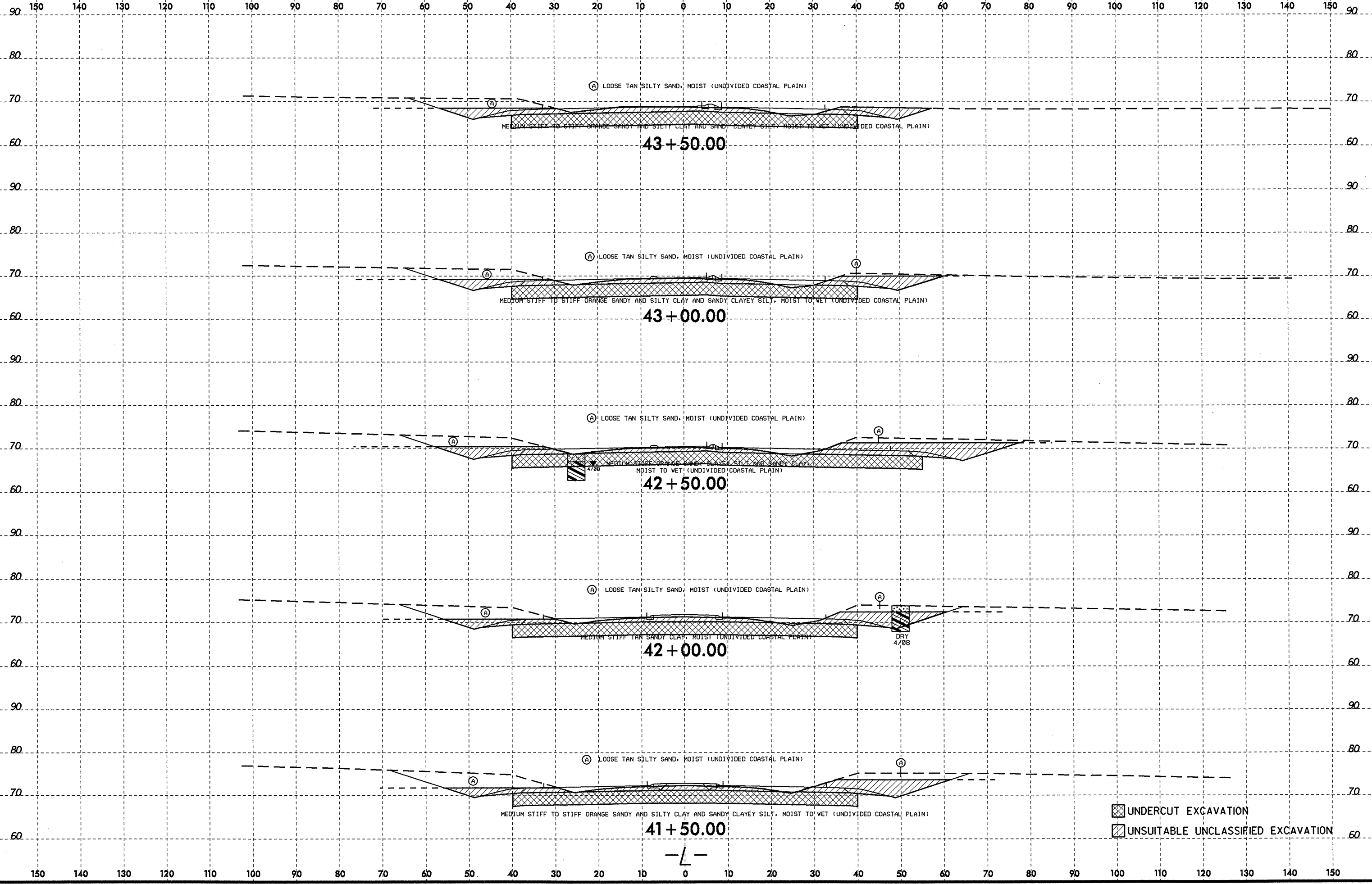
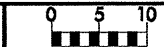
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	LL	PI	% BY WEIGHT				% PASSING (SIEVES)			%	%
							C. SAND	F. SAND	SILT	CLAY	#10	#40	#200		
SS-8	24' RT	40+00	1.00-1.50	A-6(1)	29	14	2.4	64.9	10.4	22.2	100	99	36	-	-
SS-9	24' RT	40+00	2.70-4.20	A-6(3)	35	18	0.4	61.1	8.2	30.3	100	100	40	26.5	-
SS-10	24' RT	40+00	7.70-9.20	A-7-6(67)	60	18	1.0	5.1	23.2	70.8	100	100	96	-	-

SS-8
SS-9
SS-10

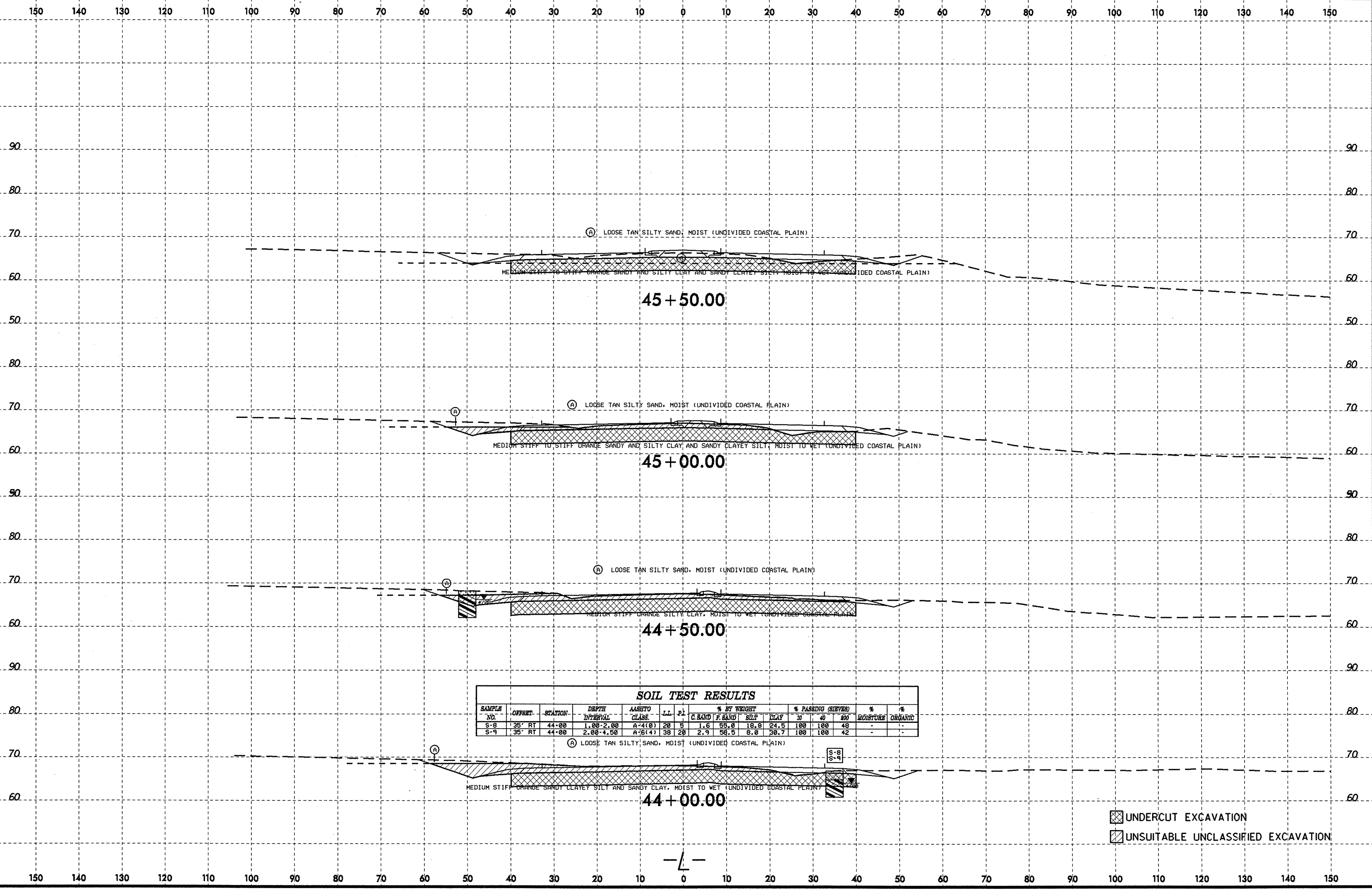
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 Author: BCG240248



SOIL TEST RESULTS

SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.L.	% BY WEIGHT				% PASSING (SIEVES)			MOISTURE	ORGANIC
							C. SAND	F. SAND	SILT	CLAY	30	40	800		
S-8	35' RT	44+00	1.00-2.00	A-4(0)	20	5	1.6	55.0	18.8	24.5	100	100	48	-	-
S-9	35' RT	44+00	2.00-4.50	A-6(4)	38	20	2.9	58.5	8.0	30.7	100	100	42	-	-

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UNDERCUT EXCAVATION
 UNSUITABLE UNCLASSIFIED EXCAVATION

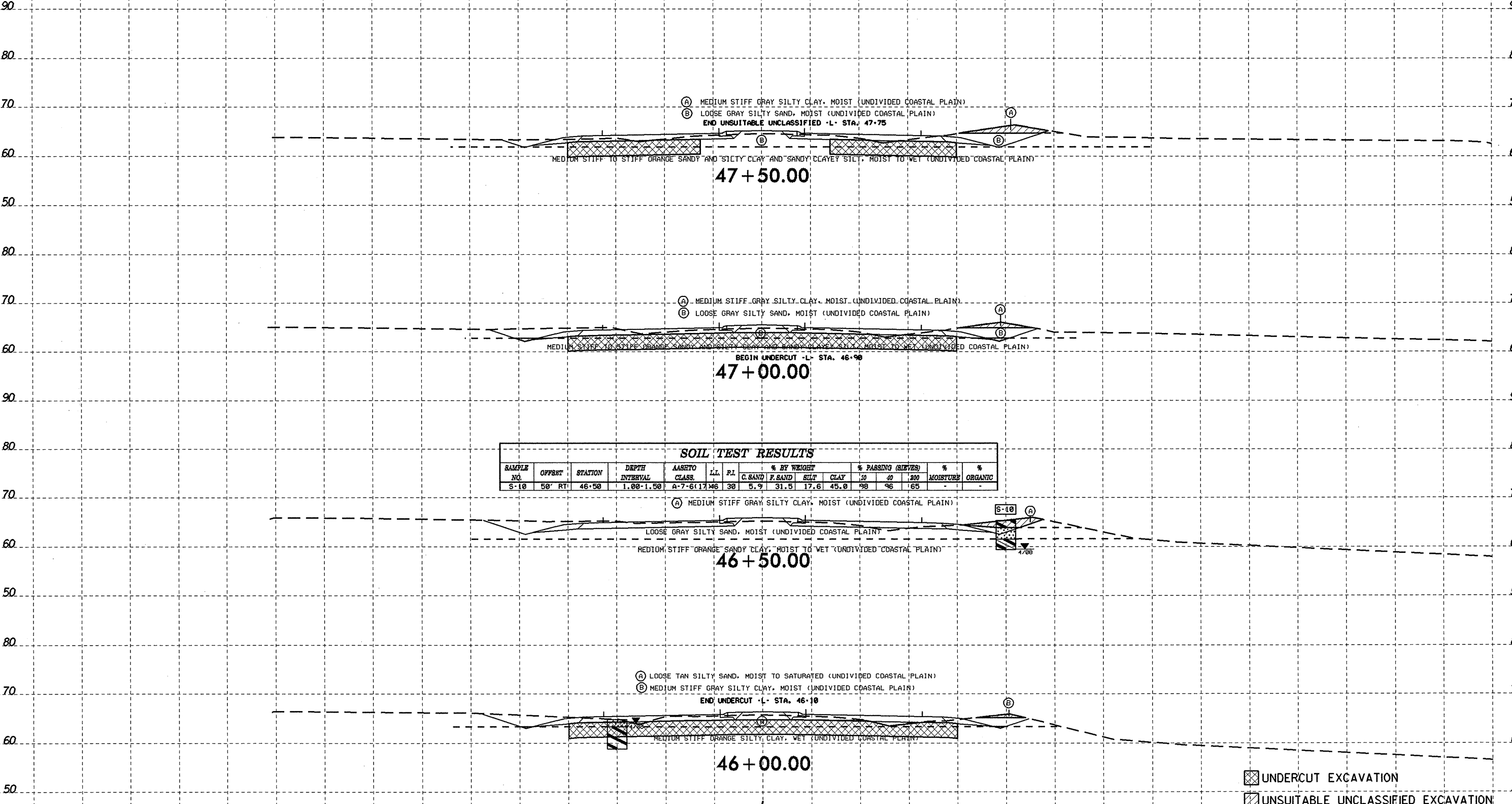
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U-5018

SHEET NO.
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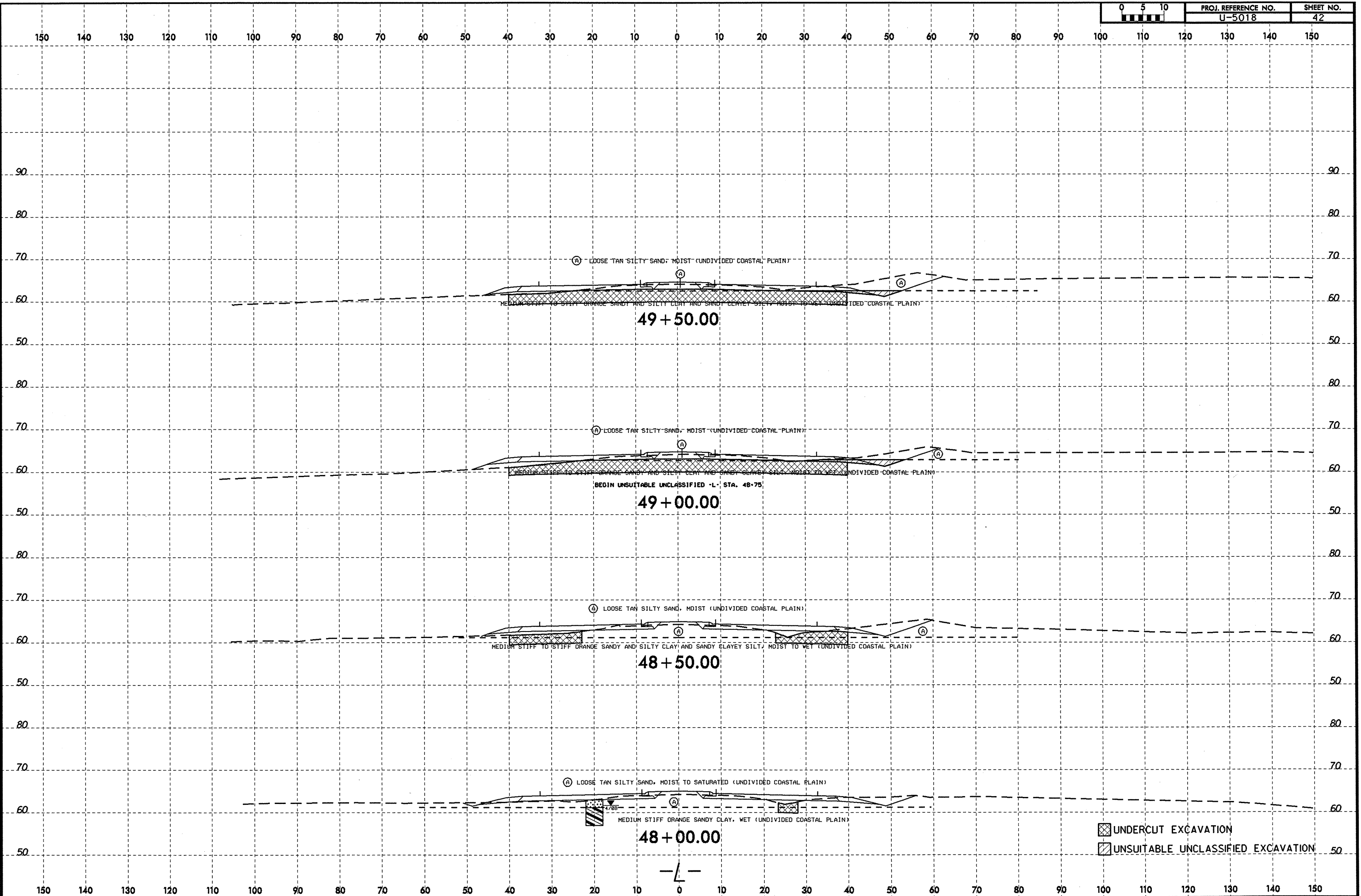
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UNDERCUT EXCAVATION
 UNSUITABLE UNCLASSIFIED EXCAVATION

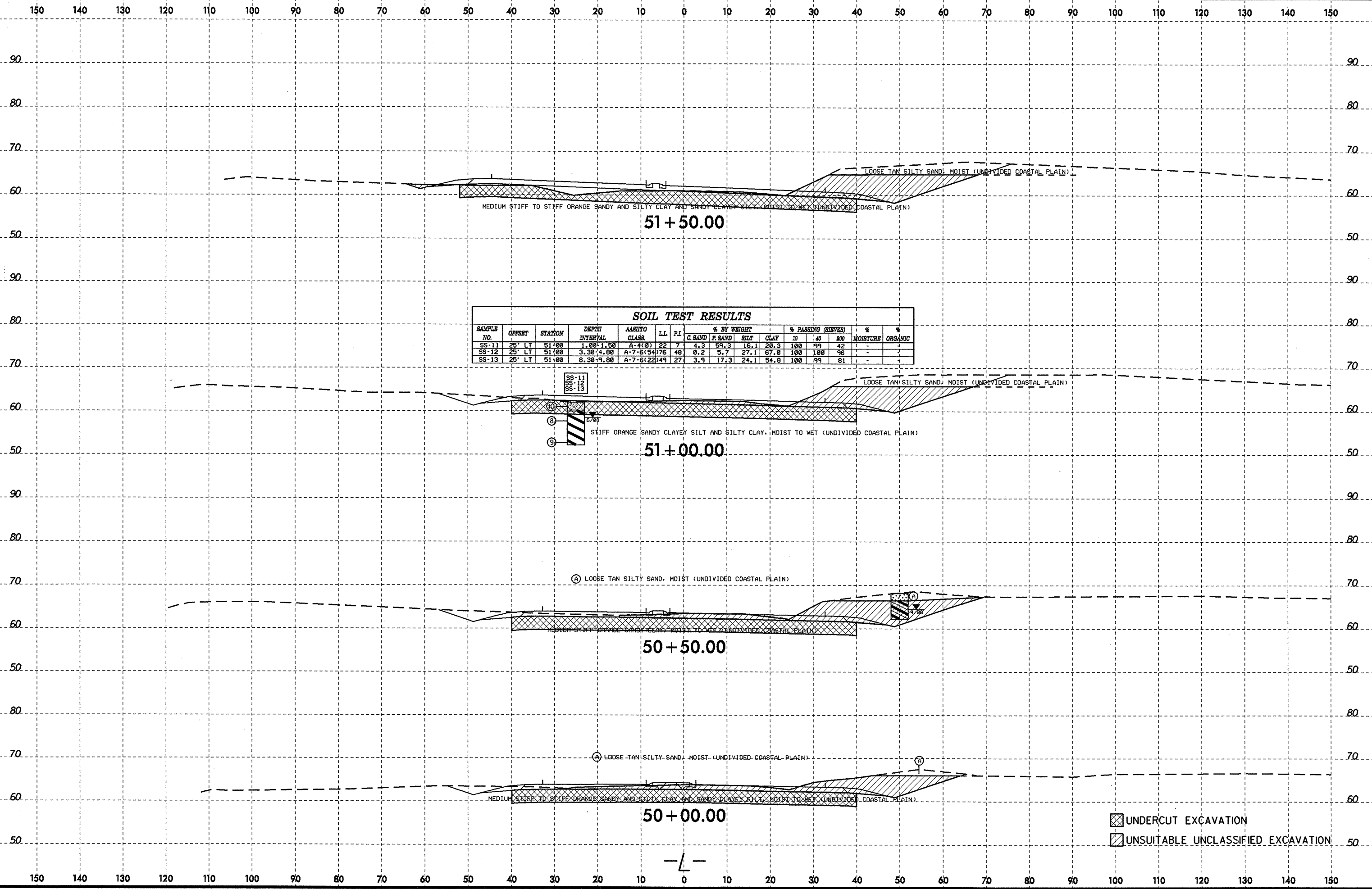
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cpturner AT 06240345



UNDERCUT EXCAVATION
 UNSUITABLE UNCLASSIFIED EXCAVATION

1/88

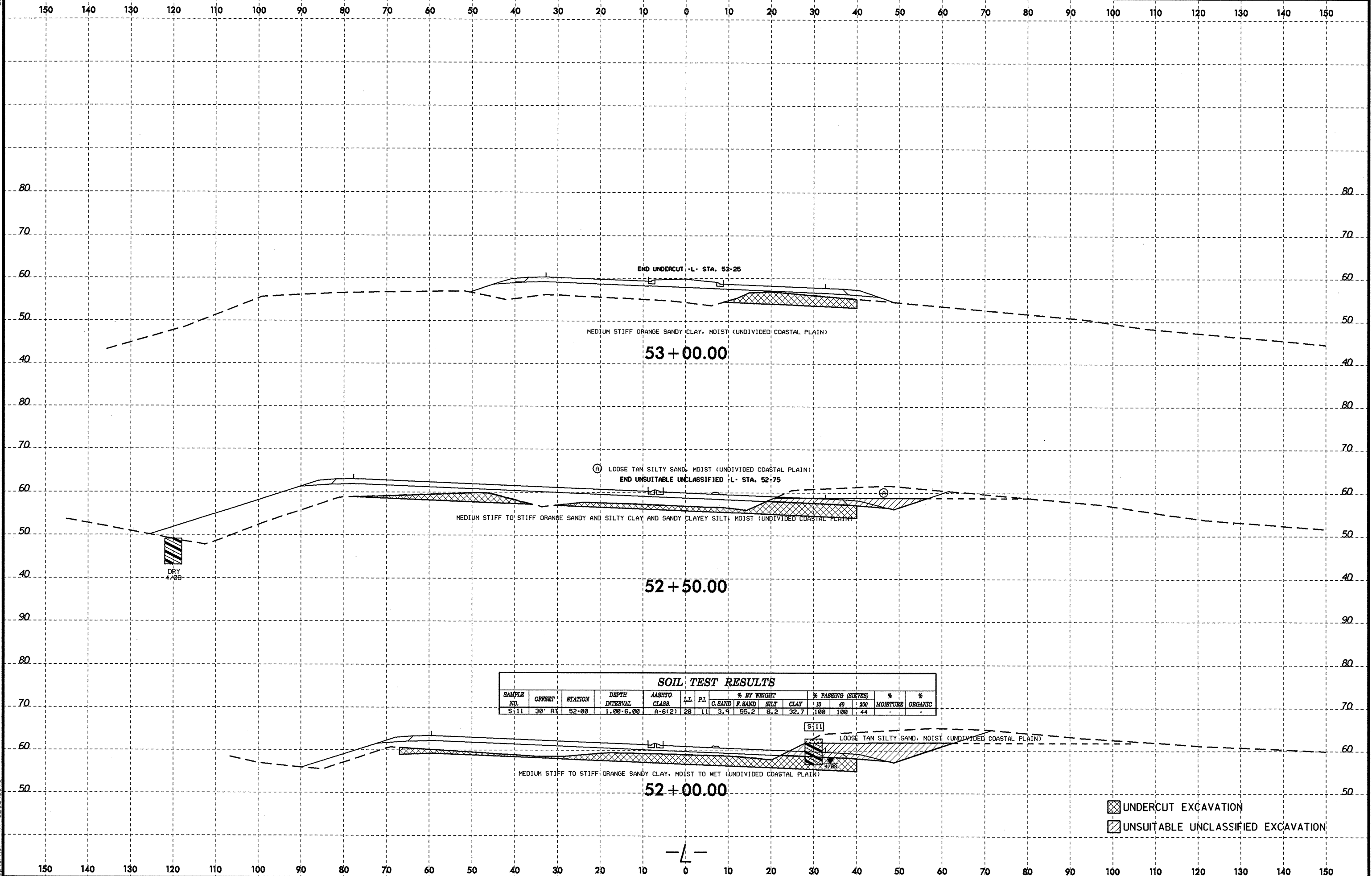


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UNDERCUT EXCAVATION
UNSUITABLE UNCLASSIFIED EXCAVATION

-L-

8/23/99



END UNDERCUT - L. STA. 53-25

MEDIUM STIFF ORANGE SANDY CLAY, MOIST (UNDIVIDED COASTAL PLAIN)

53 + 00.00

LOOSE TAN SILTY SAND, MOIST (UNDIVIDED COASTAL PLAIN)

END UNSUITABLE UNCLASSIFIED - L. STA. 52-75

MEDIUM STIFF TO STIFF ORANGE SANDY AND SILTY CLAY AND SANDY CLAYEY SILT, MOIST (UNDIVIDED COASTAL PLAIN)

52 + 50.00

DRY 4/08

SOIL TEST RESULTS															
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C. SAND	F. SAND	SILT	CLAY	#10	#40	#200		
S:11	30' RT	52-00	1.00-6.00	A-6(2)	28	11	3.9	55.2	8.2	32.7	100	100	44		

S:11

LOOSE TAN SILTY SAND, MOIST (UNDIVIDED COASTAL PLAIN)

MEDIUM STIFF TO STIFF ORANGE SANDY CLAY, MOIST TO WET (UNDIVIDED COASTAL PLAIN)

52 + 00.00

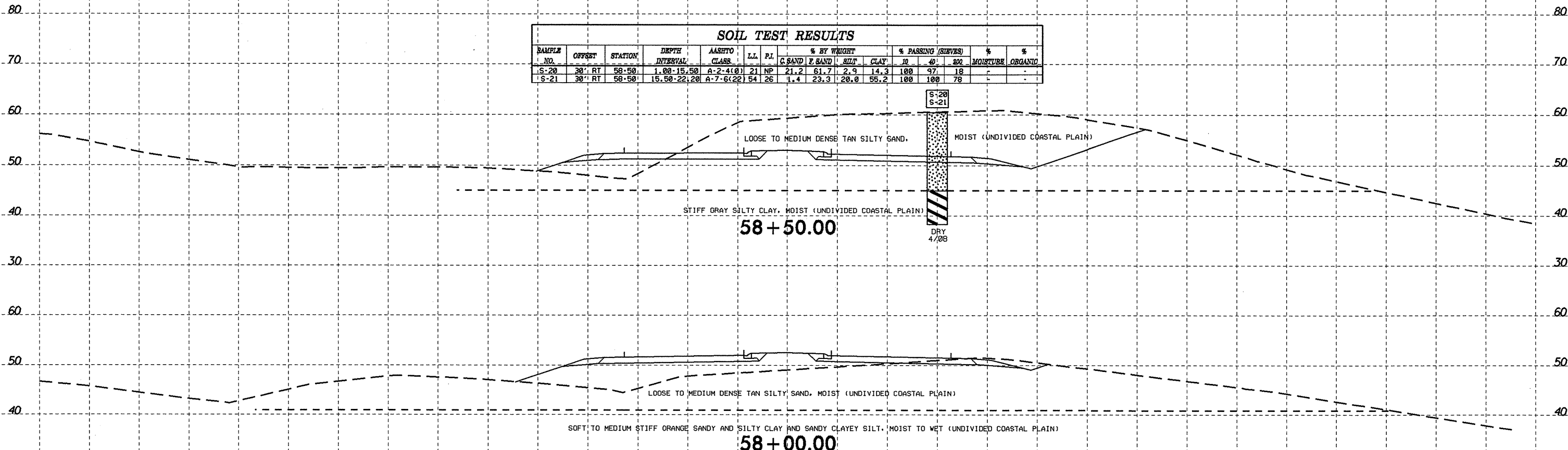
- UNDERCUT EXCAVATION
- UNSUITABLE UNCLASSIFIED EXCAVATION

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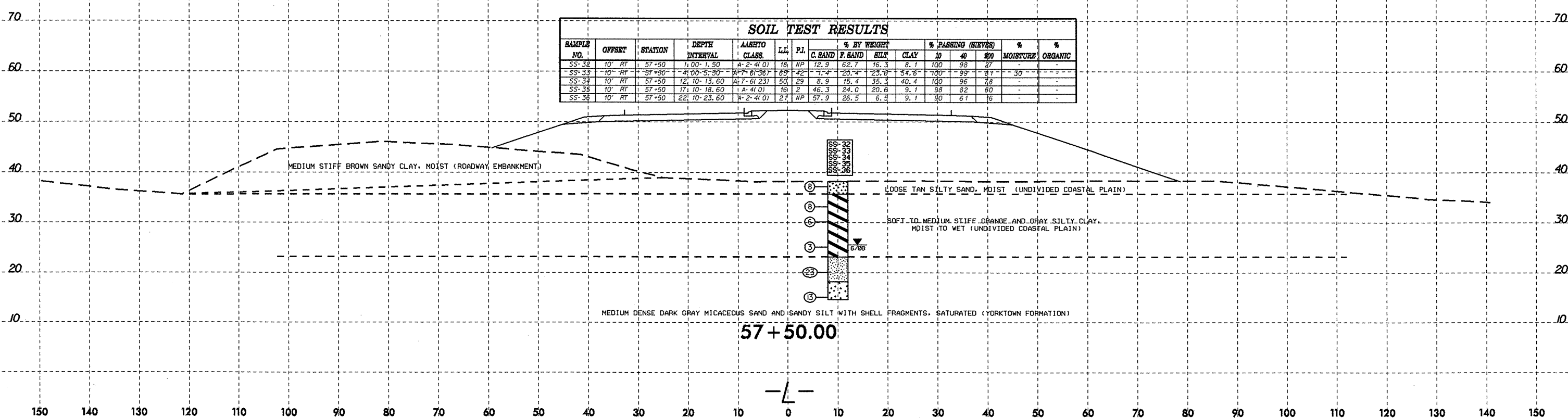
SOIL TEST RESULTS

SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS	LL	P.I.	% BY WEIGHT				% PASSING (SIEVES)			%	%
							C. SAND	F. SAND	SILT	CLAY	10	40	200		
S-20	30' RT	58+50	1.00-15.50	A-2-4(0)	21	NP	21.2	61.7	2.9	14.3	100	97	18	-	-
S-21	30' RT	58+50	15.50-22.20	A-7-6(22)	54	26	1.4	23.3	20.0	55.2	100	100	79	-	-

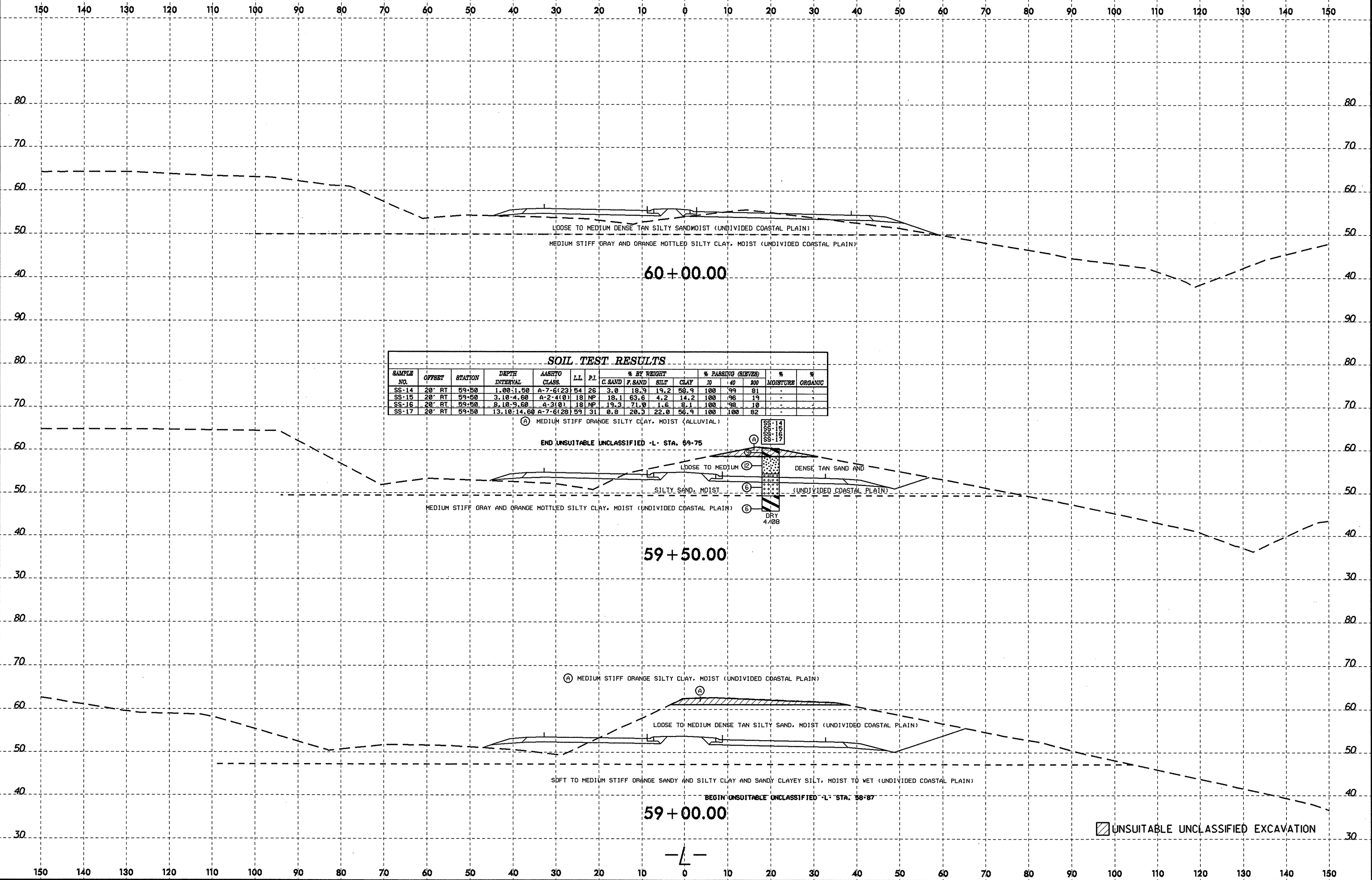


SOIL TEST RESULTS

SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS	LL	P.I.	% BY WEIGHT				% PASSING (SIEVES)			%	%
							C. SAND	F. SAND	SILT	CLAY	10	40	200		
SS-32	10' RT	57+50	1.00-1.50	A-2-4(0)	18	NP	12.9	62.7	16.3	8.1	100	98	27	-	-
SS-33	10' RT	57+50	4.00-5.80	A-7-6(36)	69	42	71.4	20.4	23.8	54.6	100	99	37	30	-
SS-34	10' RT	57+50	12.10-13.60	A-7-6(23)	50	29	8.9	15.4	35.3	40.4	100	96	78	-	-
SS-35	10' RT	57+50	17.10-18.60	A-4-4(0)	16	2	46.3	24.0	20.6	9.1	98	82	60	-	-
SS-36	10' RT	57+50	22.10-23.60	A-2-4(0)	21	NP	57.9	26.5	6.5	9.1	90	61	16	-	-

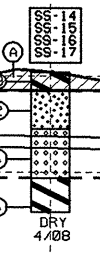


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SOIL TEST RESULTS

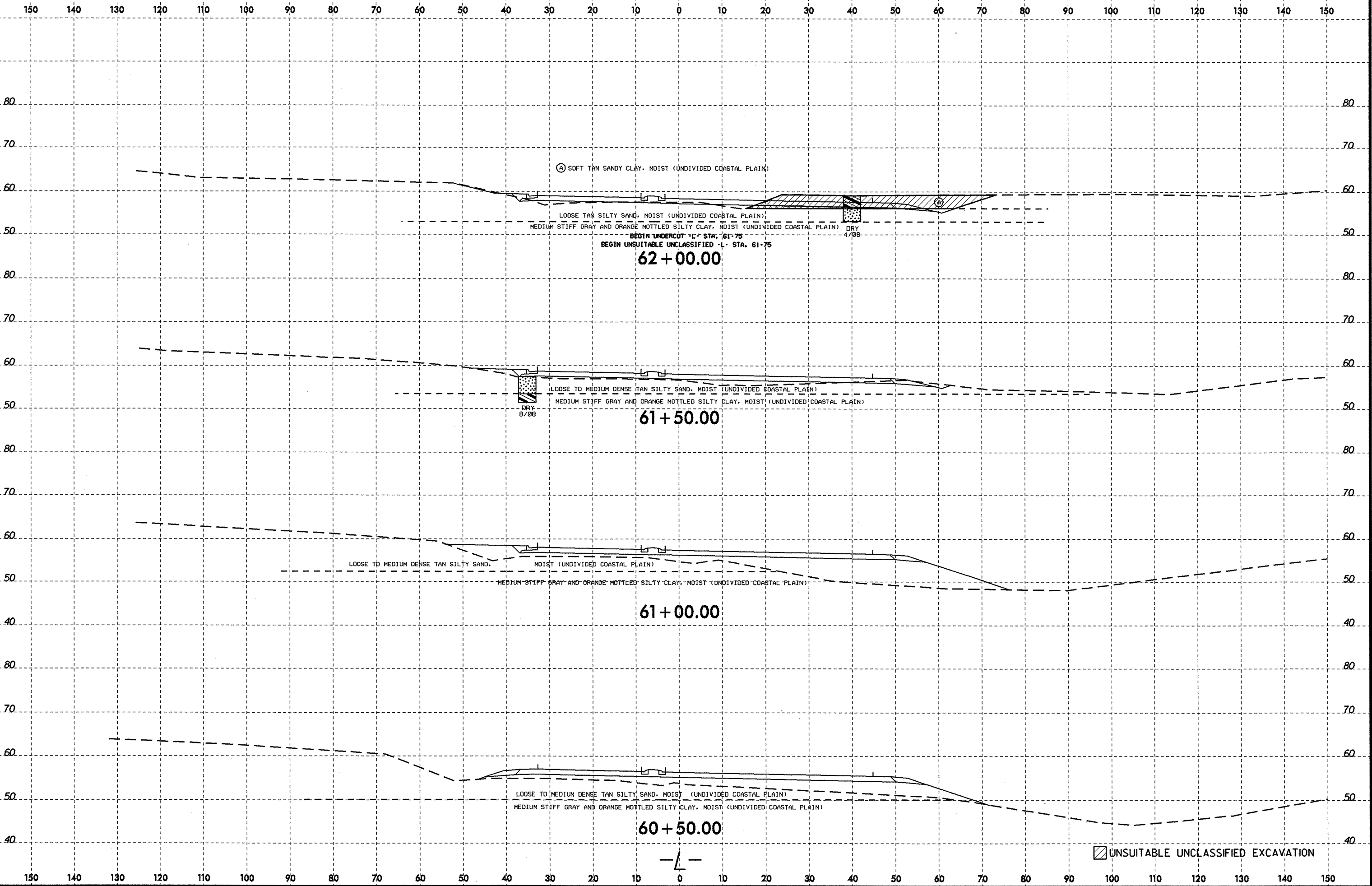
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	LL	PI	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C. SAND	F. SAND	SILT	CLAY	10	40	200		
SS-14	20'	RT 59+50	1.00-1.50	A-7-6(23)	54	26	3.0	18.9	19.2	58.9	100	99	81	-	-
SS-15	20'	RT 59+50	3.10-4.60	A-2-4(0)	18	NP	18.1	63.6	4.2	14.2	100	96	19	-	-
SS-16	20'	RT 59+50	8.10-9.60	A-3(0)	18	NP	19.3	71.0	1.6	8.1	100	98	10	-	-
SS-17	20'	RT 59+50	13.10-14.60	A-7-6(28)	59	31	0.8	20.3	22.0	56.9	100	100	82	-	-



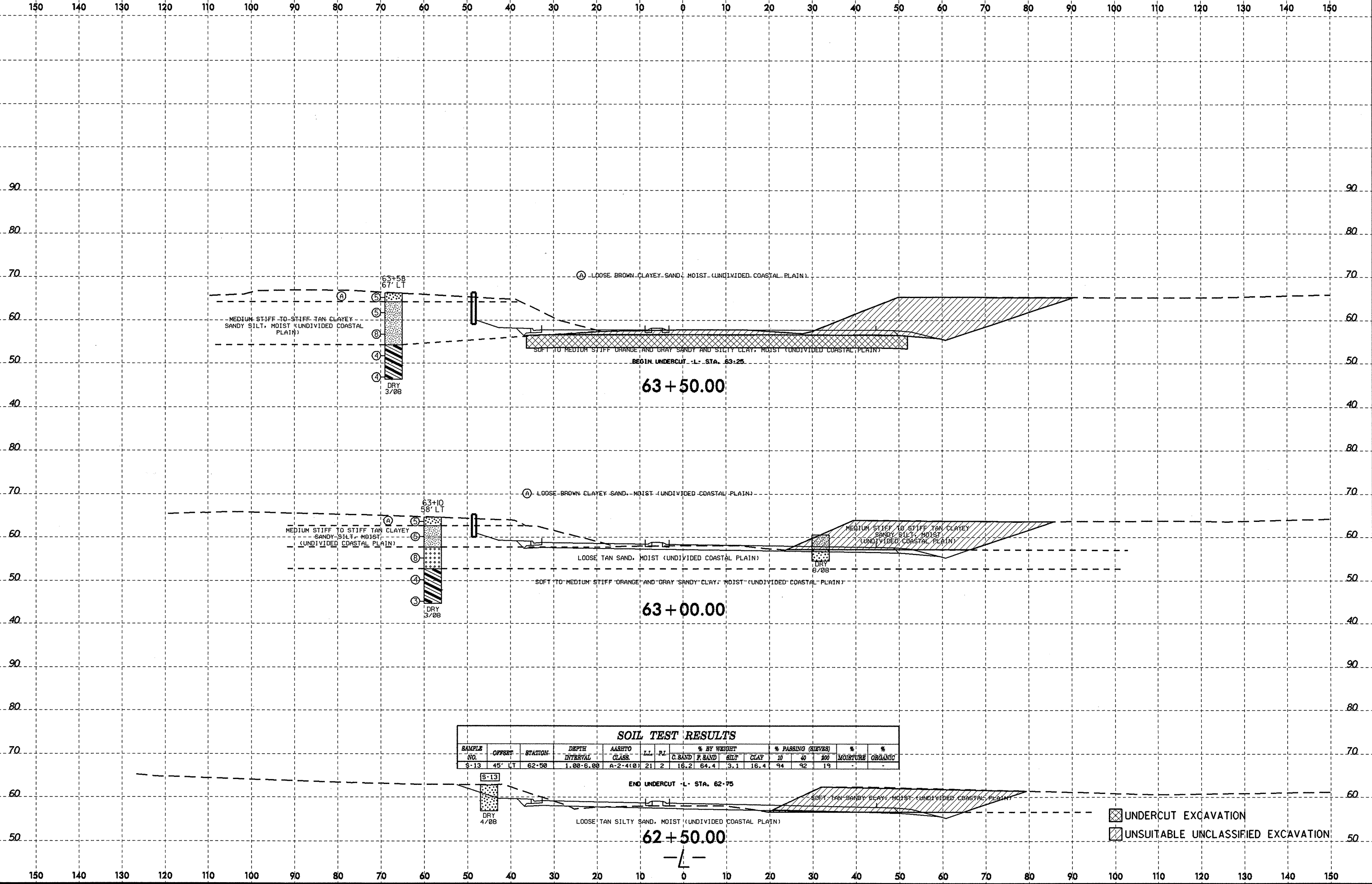
UNSUITABLE UNCLASSIFIED EXCAVATION

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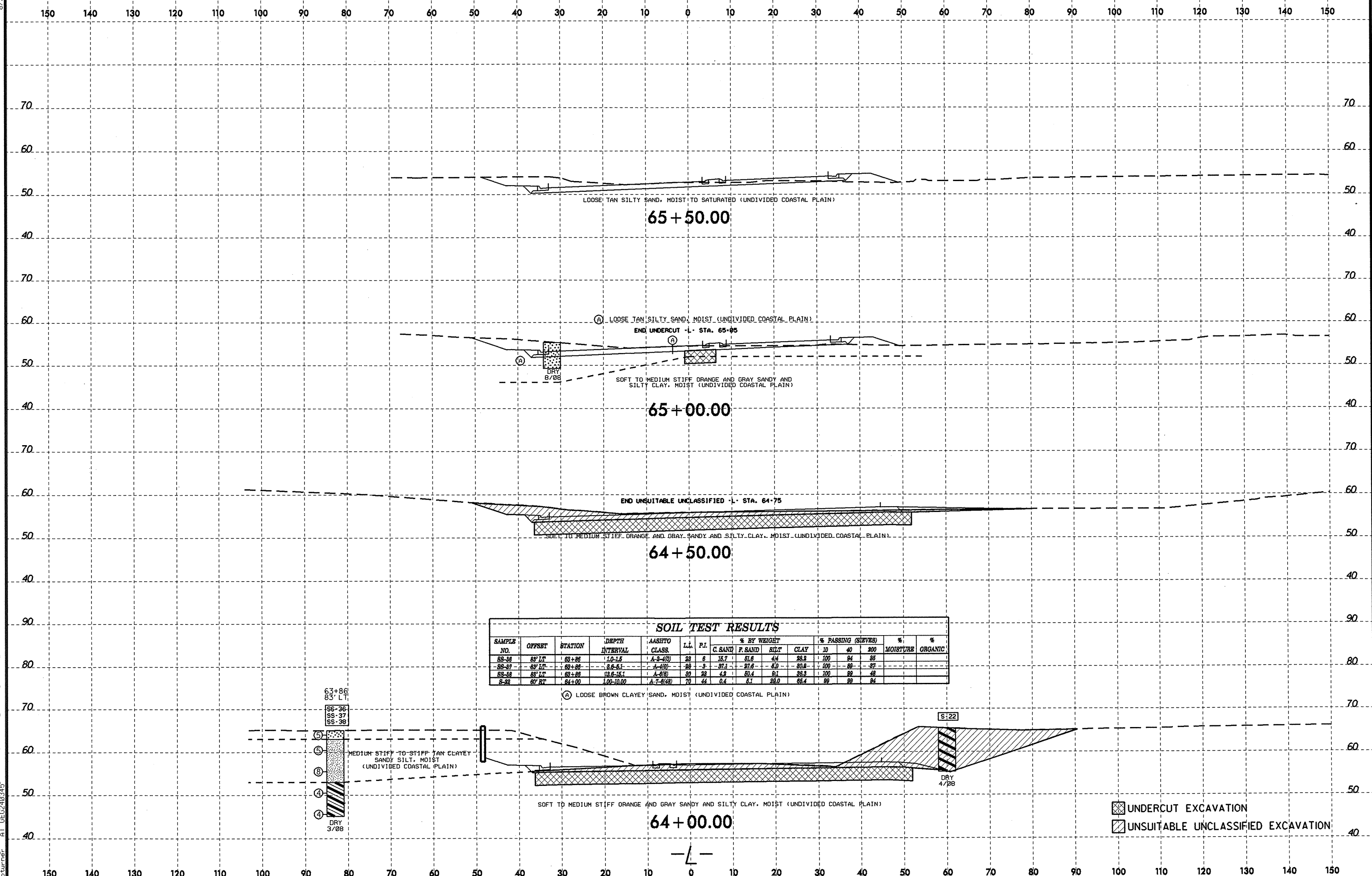
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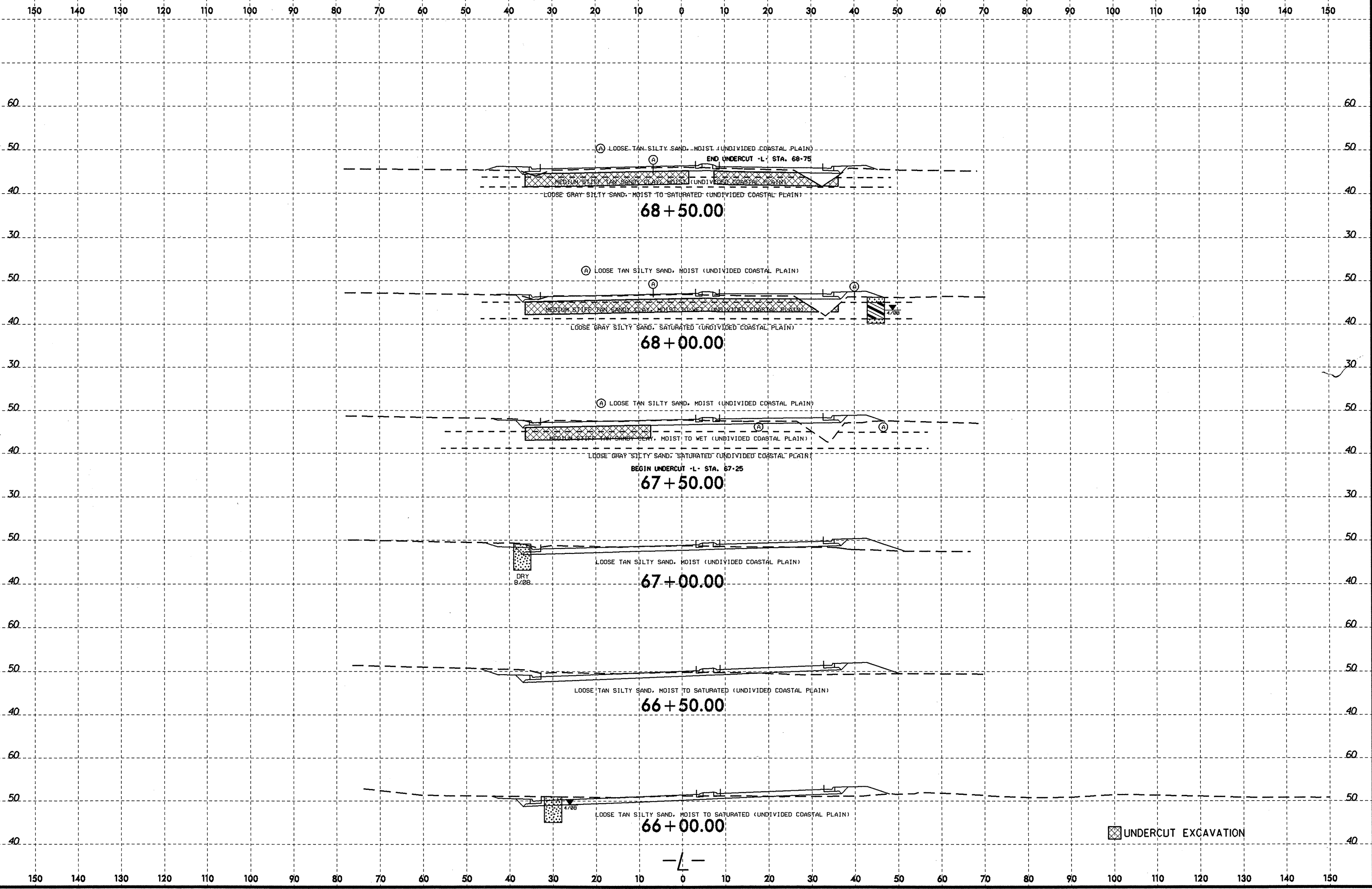


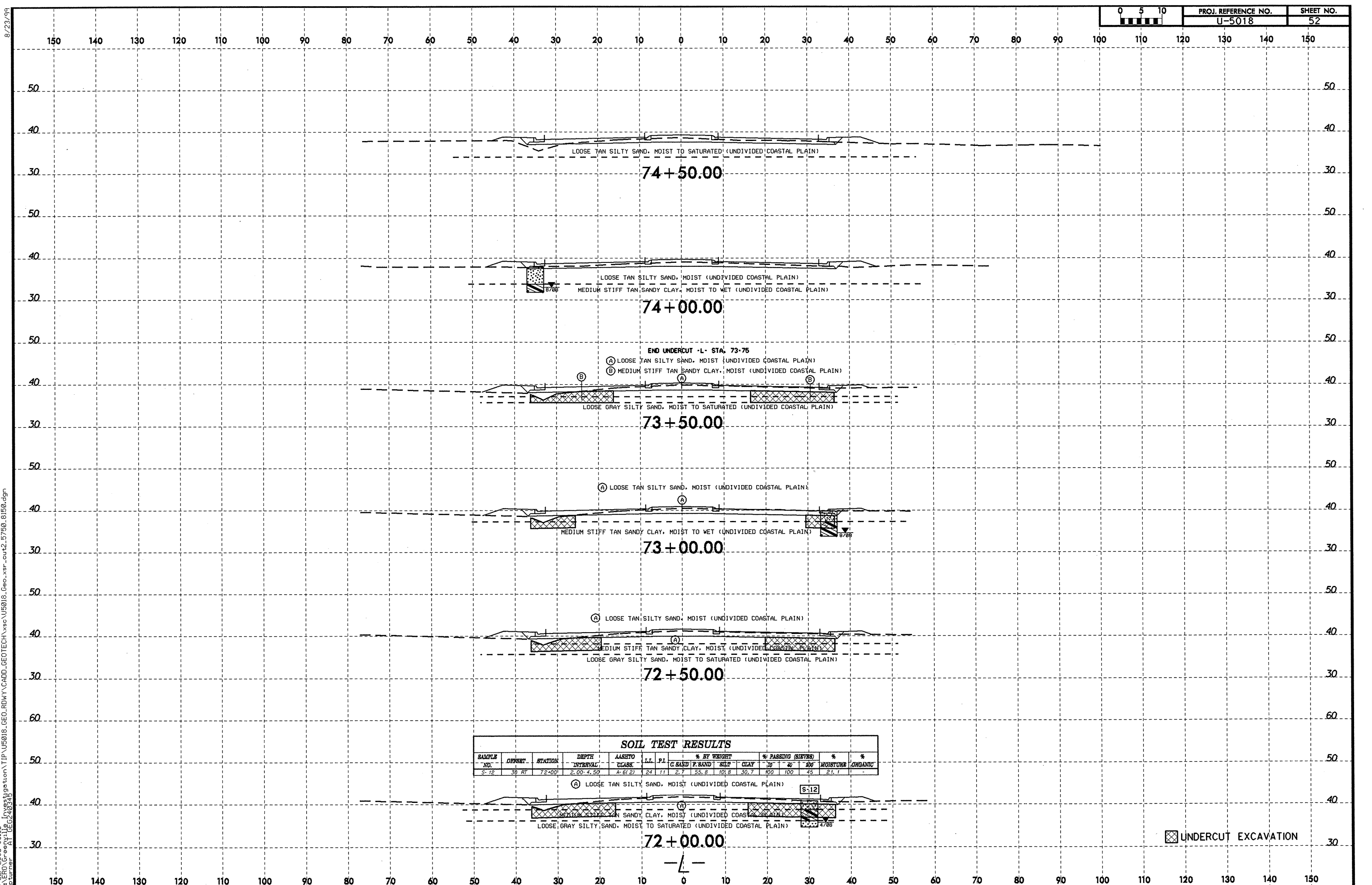
SOIL TEST RESULTS

SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	LL	PI	% BY WEIGHT				% PASSING (SIEVES)			%	%
							C. SAND	F. SAND	SILT	CLAY	10	40	200		
SS-36	83' LT	63+86	1.0-1.5	A-9 (U)	28	8	15.7	61.6	4.4	28.3	100	84	86		
SS-37	83' LT	63+86	3.0-5.1	A-10	28	8	27.1	27.6	5.0	30.8	100	86	87		
SS-38	83' LT	63+86	13.0-16.1	A-6 (U)	80	28	4.3	50.4	8.1	38.3	100	89	48		
S-22	60' RT	64+00	1.00-10.00	A-7-6 (US)	70	44	0.4	6.1	28.0	66.4	89	89	84		

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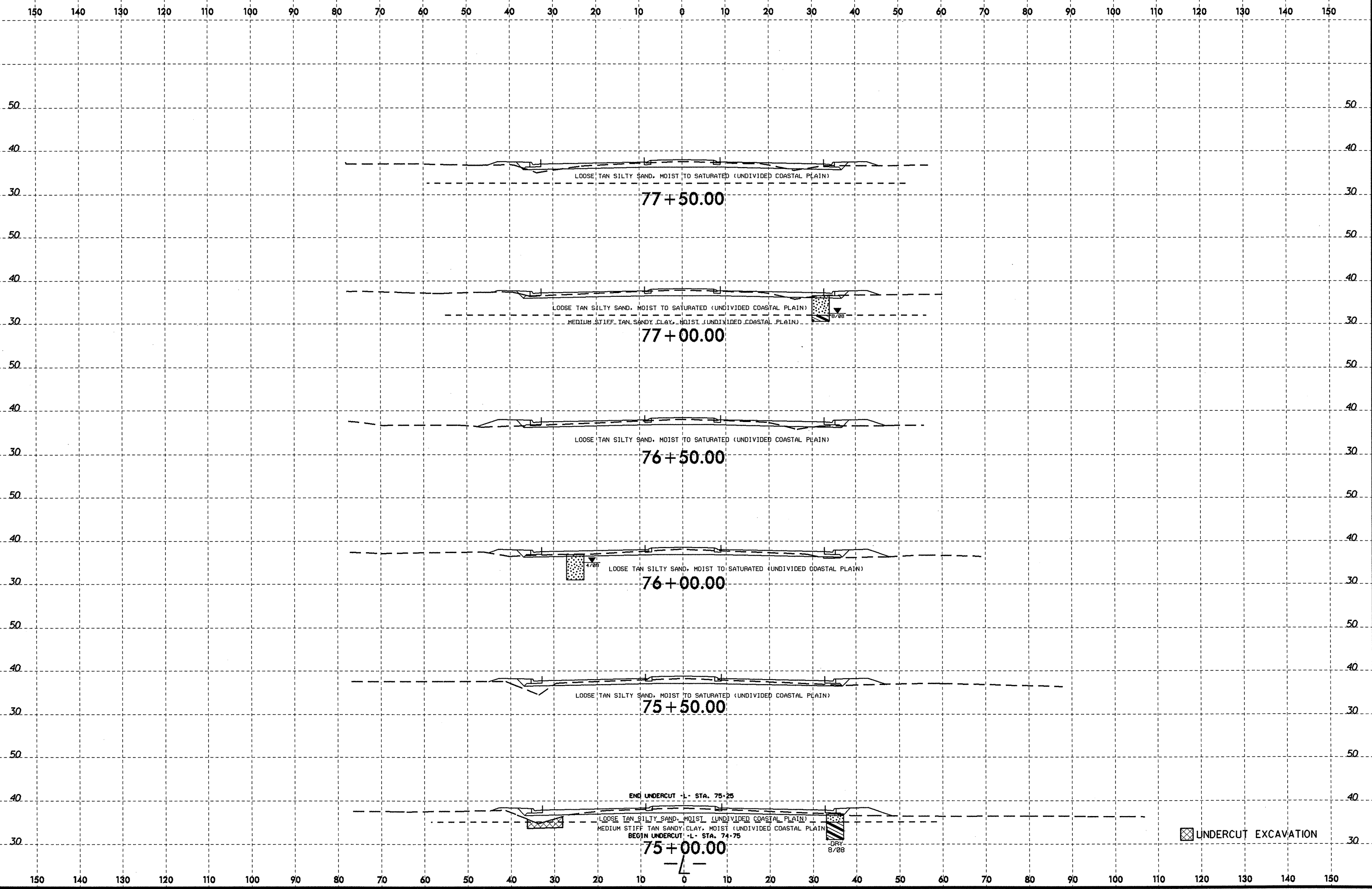


END UNDERCUT -L- STA. 73-75
 (A) LOOSE TAN SILTY SAND, MOIST (UNDIVIDED COASTAL PLAIN)
 (B) MEDIUM STIFF TAN SANDY CLAY, MOIST (UNDIVIDED COASTAL PLAIN)

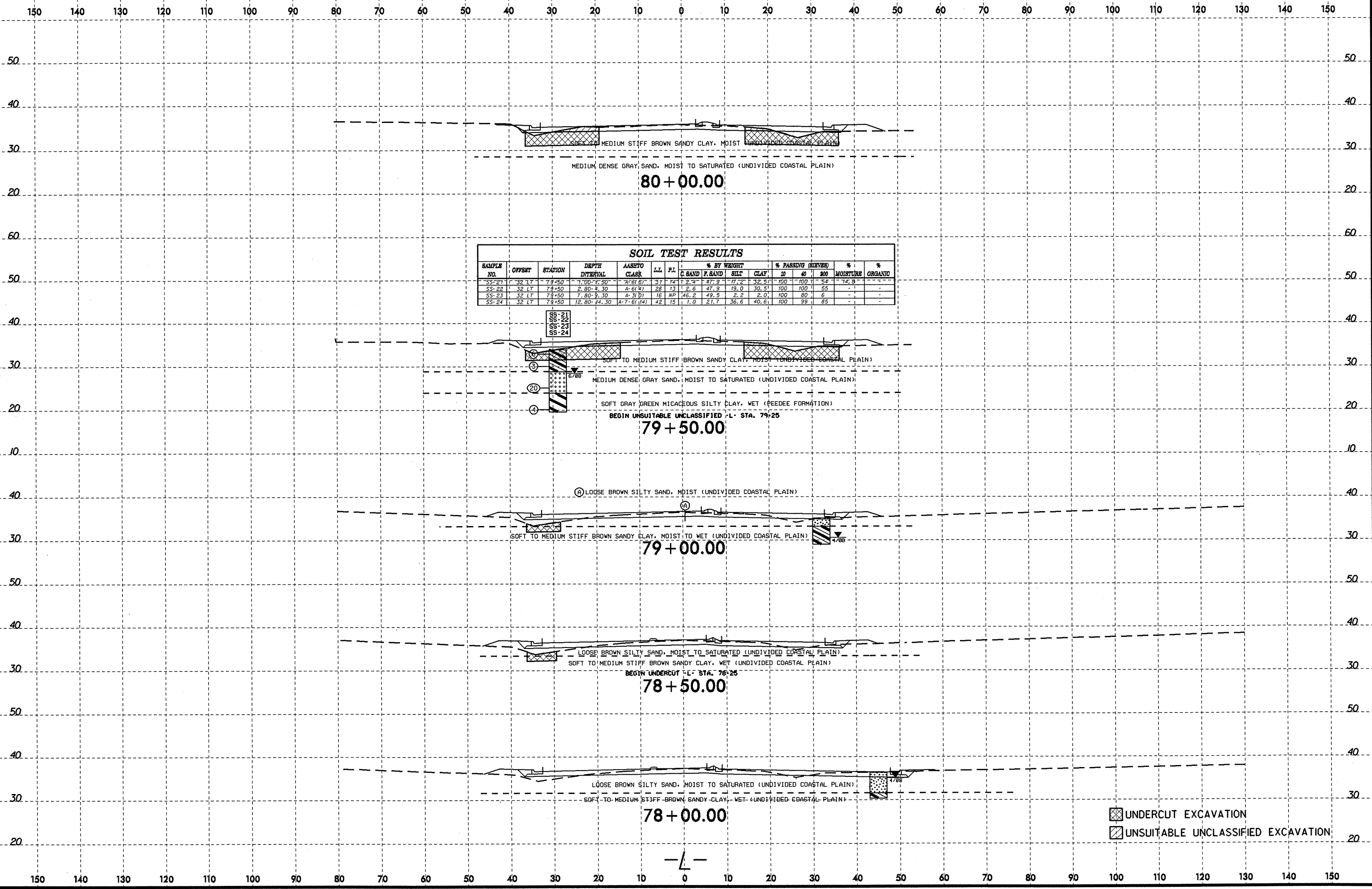
SOIL TEST RESULTS

SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	ASTM CLASS.	LL	PI	% BY WEIGHT			% PASSING (SIEVES)			MOISTURE	ORGANIC
							C SAND	F SAND	CLAY	10	40	200		
S-12	39 RT	72+00	2.00-4.50	A-6(2)	24	11	2.7	55.8	10.8	30.7	100	49	21.1	-

UNDERCUT EXCAVATION



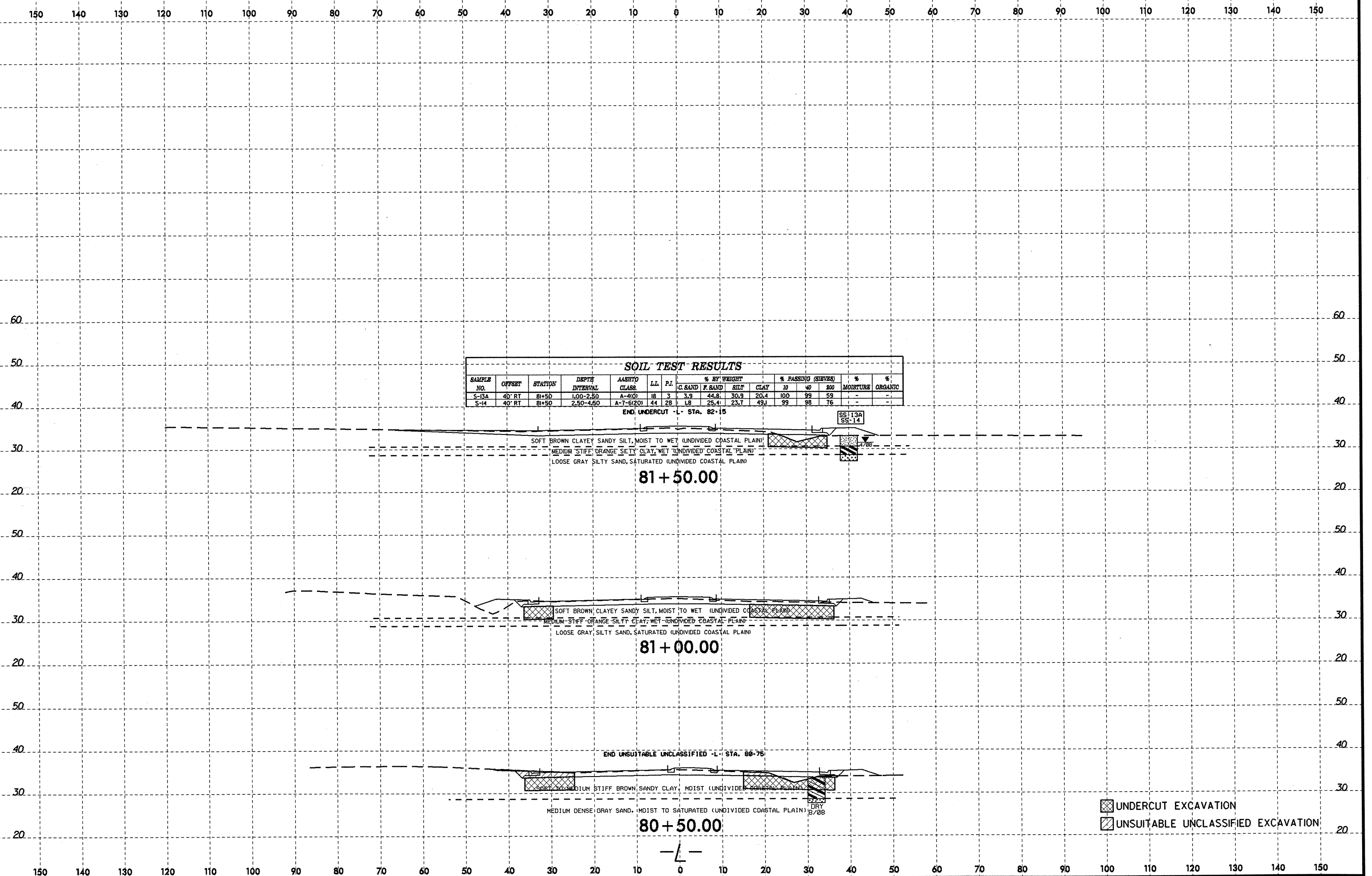
☒ UNDERCUT EXCAVATION



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SOIL TEST RESULTS															
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASTHO CLASS.	LL	PI	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C. SAND	F. SAND	SILT	CLAY	10	40	200		
S-13A	40' RT	81+50	1.00-2.50	A-4(1)	18	3	3.9	44.8	30.9	20.4	100	99	59	-	-
S-14	40' RT	81+50	2.50-4.50	A-7-6(20)	44	28	1.8	25.4	23.7	49.1	99	98	76	-	-



SOFT BROWN CLAYEY SANDY SILT, MOIST TO WET (UNDIVIDED COASTAL PLAIN)
 MEDIUM STIFF ORANGE SILTY CLAY, WET (UNDIVIDED COASTAL PLAIN)
 LOOSE GRAY SILTY SAND, SATURATED (UNDIVIDED COASTAL PLAIN)

81 + 50.00

SOFT BROWN CLAYEY SANDY SILT, MOIST TO WET (UNDIVIDED COASTAL PLAIN)
 MEDIUM STIFF ORANGE SILTY CLAY, WET (UNDIVIDED COASTAL PLAIN)
 LOOSE GRAY SILTY SAND, SATURATED (UNDIVIDED COASTAL PLAIN)

81 + 00.00

END UNSUITABLE UNCLASSIFIED - L - STA. 80-75

MEDIUM STIFF BROWN SANDY CLAY, MOIST (UNDIVIDED COASTAL PLAIN)
 MEDIUM DENSE GRAY SAND, MOIST TO SATURATED (UNDIVIDED COASTAL PLAIN)

80 + 50.00

UNDERCUT EXCAVATION
 UNSUITABLE UNCLASSIFIED EXCAVATION

- L -