CONTENTS: Title Sheet

NCDOT GeotechnicalUnit

Site Vicinity Map

Boring Location Plan

12.0' RT. of -L-

Test Boring Logs

Core Reports Core Photographs

Field Scour Report

Grain Size Curves

Site Photographs

Generalized Subsurface Profile

Through End Bent No. 1

Soiland Rock Classification Sheet

Generalized Subsurface Cross Section

Generalized Subsurface Cross Section

Generalized Subsurface Cross Section

Generalized Subsurface Cross Section

Through Interior Bent No. 1

Through Interior Bent No. 2

Summary of Laboratory Test Data

Summary of Rock Core Test Results

(Performed September II. 2009)

Through End Bent No. 2

Sheet I

Sheet 2

Sheet 3

Sheet 4

Sheet 5

Sheet 6

Sheet 7

Sheet 8

Sheet 9

Sheet 17

Sheet 18

Sheet 19

Sheets 20 - 22

Sheets 23 - 25

Sheets 10 - 16

STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION DIVISION OF HIGHWAYS GEOTECHNICAL ENGINEERING UNIT

STRUCTURE SUBSURFACE INVESTIGATION

STATE PROJECT_	33794.1.1	. I.D. NO	B-4610
F.A. PROJECT	BRZ-1518(2)		
COUNTYRANE		······································	
PROJECT DESCRIP	TION BRIDGE	No. 73	
OVER LAKE LUCAS C			

STATE	STATE	PROJECT REFERENCE	3 NO.	SHEET NO.	TOTAL SHEETS
N.C.		B-4610		1	25
STATE	PROJ. NO.	F. A. PROJ. NO.	T	DESCRIP	TION
337	794.1.1	BRZ-1518(2)		P.E.	
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CAUTION NOTICE

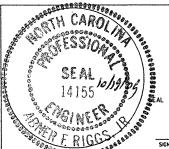
THE SUBSURFACE INFORMATION AND THE SUBSURFACE INVESTIGATION ON WHICH IT IS BASED WAS MADE FOR THE PURPOSE OF STUDY, PLANNING AND DESIGN, AND NOT FOR CONSTRUCTION OR PAY PURPOSES. THE VARIOUS FIELD BORING LOGS, ROCK CORES, AND SOIL TEST DATA AVAILABLE MAY BE REVIEWED OR INSPECTED IN RALEIGH BY CONTACTING THE N.C. DEPARTMENT OF TRANSPORTATION, GEOTECHNICAL UNIT @ (919) 250-4088. NEITHER THE SUBSURFACE PLANS AND REPORTS, NOR THE FIELD BORING LOGS, ROCK CORES, OR SOIL TEST DATA IS PART OF THE CONTRACT.

GENERAL SOIL AND ROCK STRATA DESCRIPTIONS AND INDICATED BOUNDARIES ARE BASED ON A GEOTECHNICAL INTERPRETATION OF ALL AVAILABLE SUBSURFACE DATA AND MAY NOT NECESSARILY REFLECT THE ACTUAL SUBSURFACE CONDITIONS BETWEEN BORINGS OR BETWEEN SAMPLED STRATA MITHIN THE BOREHOLE. THE LABORATORY SAMPLE DATA AND THE IN SITU (IN-PLACE) TEST DATA CAN BE RELIED ON ONLY TO THE DEGREE OF RELIABILITY INHERENT IN THE STANDARD TEST METHOD.

THE OBSERVED WATER LEVELS OR SOIL MOISTURE CONDITIONS INDICATED IN THE SUBSURFACE INVESTIGATIONS ARE AS RECORDED AT THE TIME OF THE INVESTIGATION. THESE WATER LEVELS OR SOIL MOISTURE CONDITIONS MAY VARY CONSIDERABLY WITH TIME ACCORDING TO CLIMATIC CONDITIONS INCLUDING TEMPERATURES, PRECIPITATION AND WIND, AS WELL AS OTHER NON-CLIMATIC FACTORS.

THE BIDDER OR CONTRACTOR IS CAUTIONED THAT DETAILS SHOWN ON THE SUBSURFACE PLANS ARE PRELIMINARY ONLY AND IN MANY CASES THE FINAL DESIGN DETAILS ARE DIFFERENT, FOR BIDDING AND CONSTRUCTION PURPOSES, REFER TO THE CONSTRUCTION PLANS AND DOCUMENTS FOR FINAL DESIGN INFORMATION ON THIS PROJECT. THE DEPARTMENT DOES NOT WARRANT OR GUARANTEE THE SUFFICIENCY OR ACCURACY OF THE INVESTIGATION MADE, NOR THE INTERPRETATIONS MADE OR OPINION OF THE DEPARTMENT AS TO THE TYPE OF MATERIALS AND CONDITIONS TO BE ENCOUNTERED. THE BIDDER OR CONTRACTOR IS CAUTIONED TO MAKE SUCH INDEPENDENT SUBSURFACE INVESTIGATIONS AS HE DEEMS NECESSARY TO SATISFY HIMSELF AS TO CONDITIONS TO BE ENCOUNTERED ON THIS PROJECT. THE CONTRACTOR SHALL HAVE NO CLAIM FOR ADDITIONAL COMPENSATION OR FOR AN EXTENSION OF TIME FOR ANY REASON RESULTING FROM THE ACTUAL CONDITIONS ENCOUNTERED AT THE SITE DIFFERING FROM THOSE INDICATED IN THE SUBSURFACE INFORMATION.

S&ME. INC. A. RIGGS INVESTIGATED BY N. BRADLEY A.F. RIGGS, JR. S&ME, INC. J. WHITE OCTOBER 2, 2009 S. LOW P. PHELPS T. PEREZ



NOTE - THE INFORMATION CONTAINED HEREIN IS NOT IMPLIED OR GUARANTEED BY THE N. C. DEPARTMENT OF TRANSPORTATION AS BEING ACCURATE NOR IT IS CONSIDERED TO BE PART OF THE PLANS, SPECIFICATIONS. OR CONTRACT FOR THE PROJECT.

NOTE - BY HAVING REQUESTED THIS INFORMATION THE CONTRACTOR SPECIFICALLY WAIVES ANY CLAIMS FOR INCREASED COMPENSATION OR EXTENSION OF TIME BASED ON DIFFERENCES BETWEEN THE CONDITIONS INDICATED HEREIN AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

DRAWN BY:	T. PEREZ

NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

DIVISION OF HIGHWAYS

GEOTECHNICAL ENGINEERING UNIT

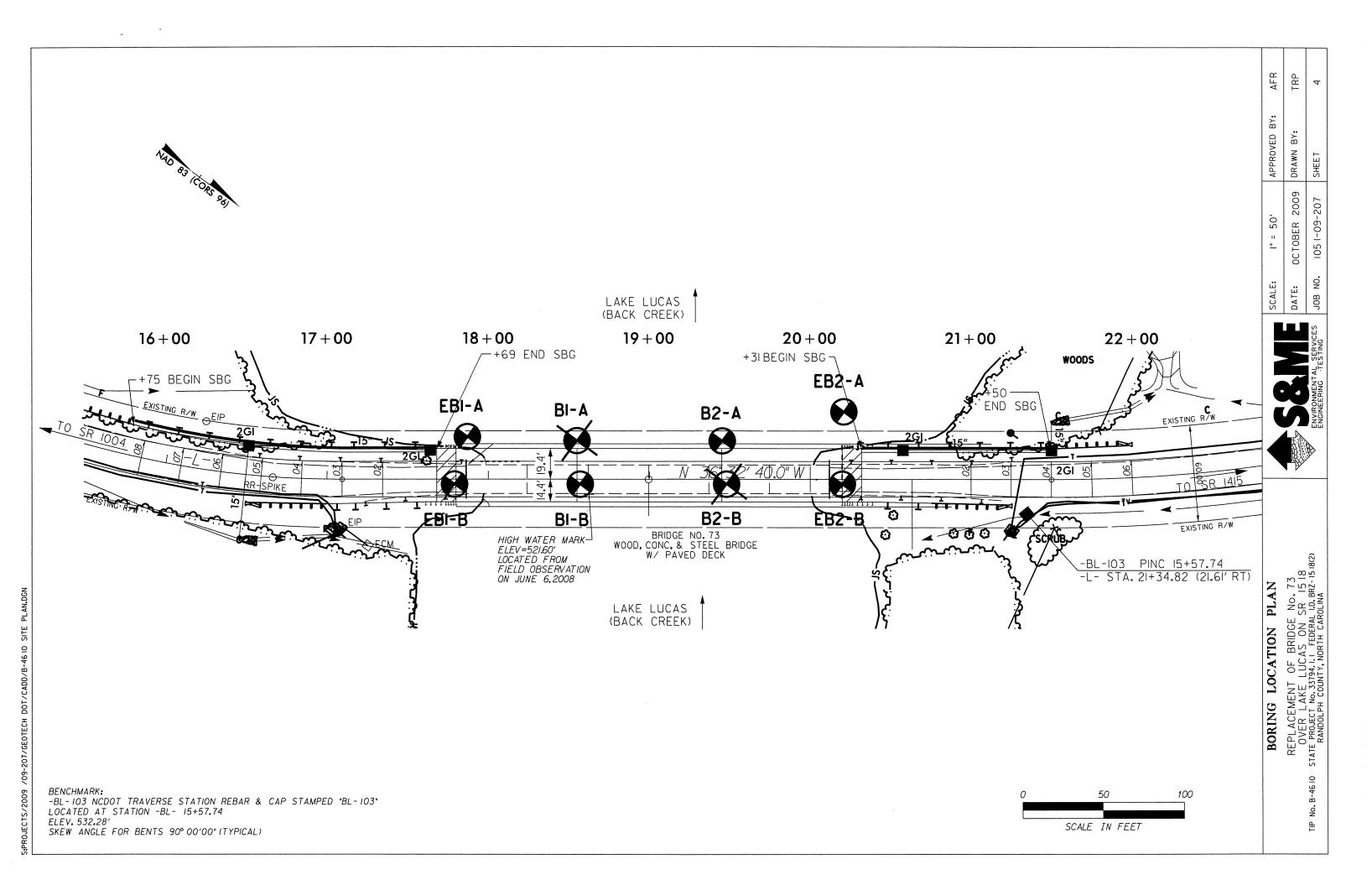
PROJECT REFERENCE NO. SHEET NO. 33794.1.1 2

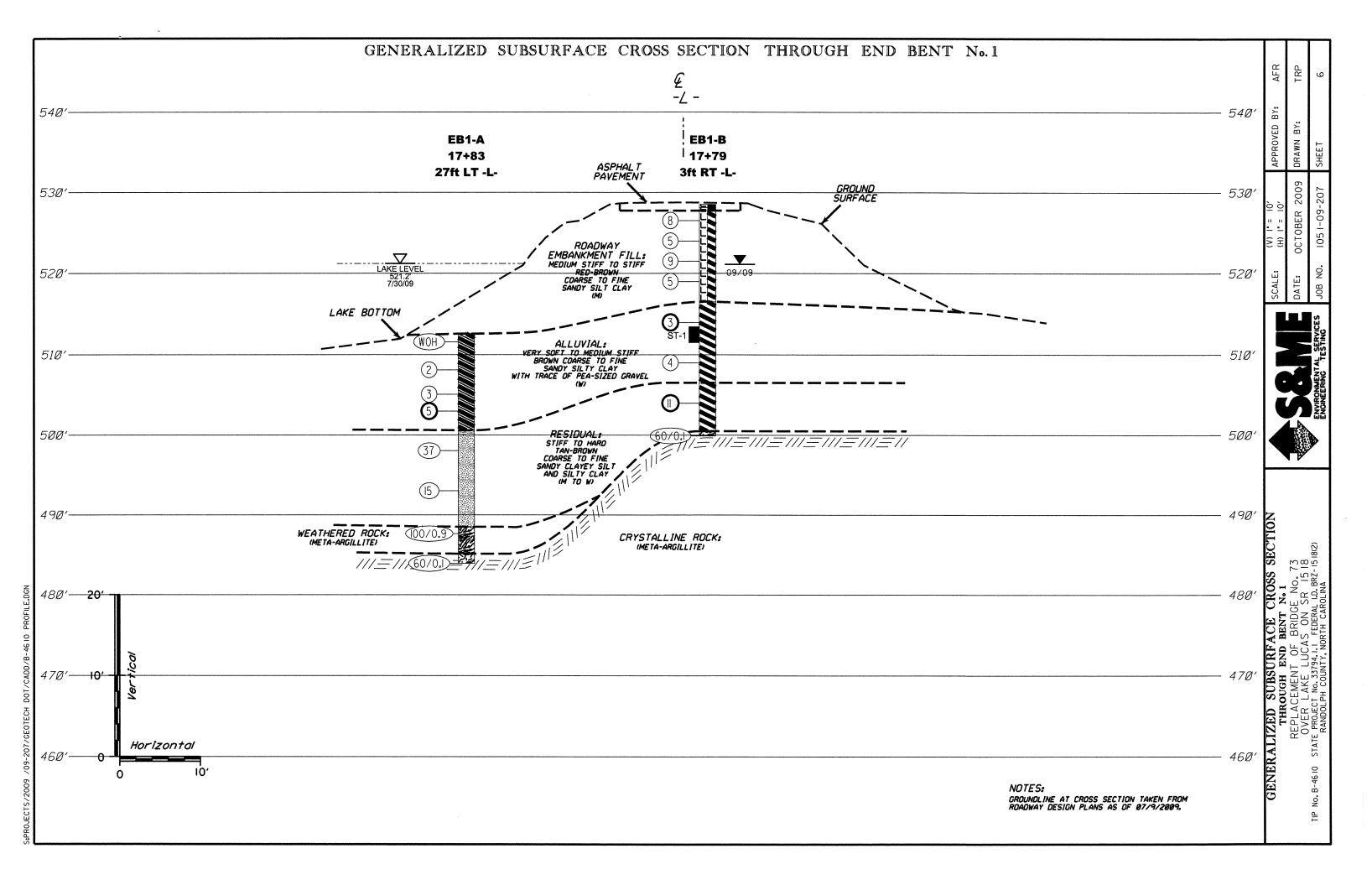
SUBSURFACE INVESTIGATION

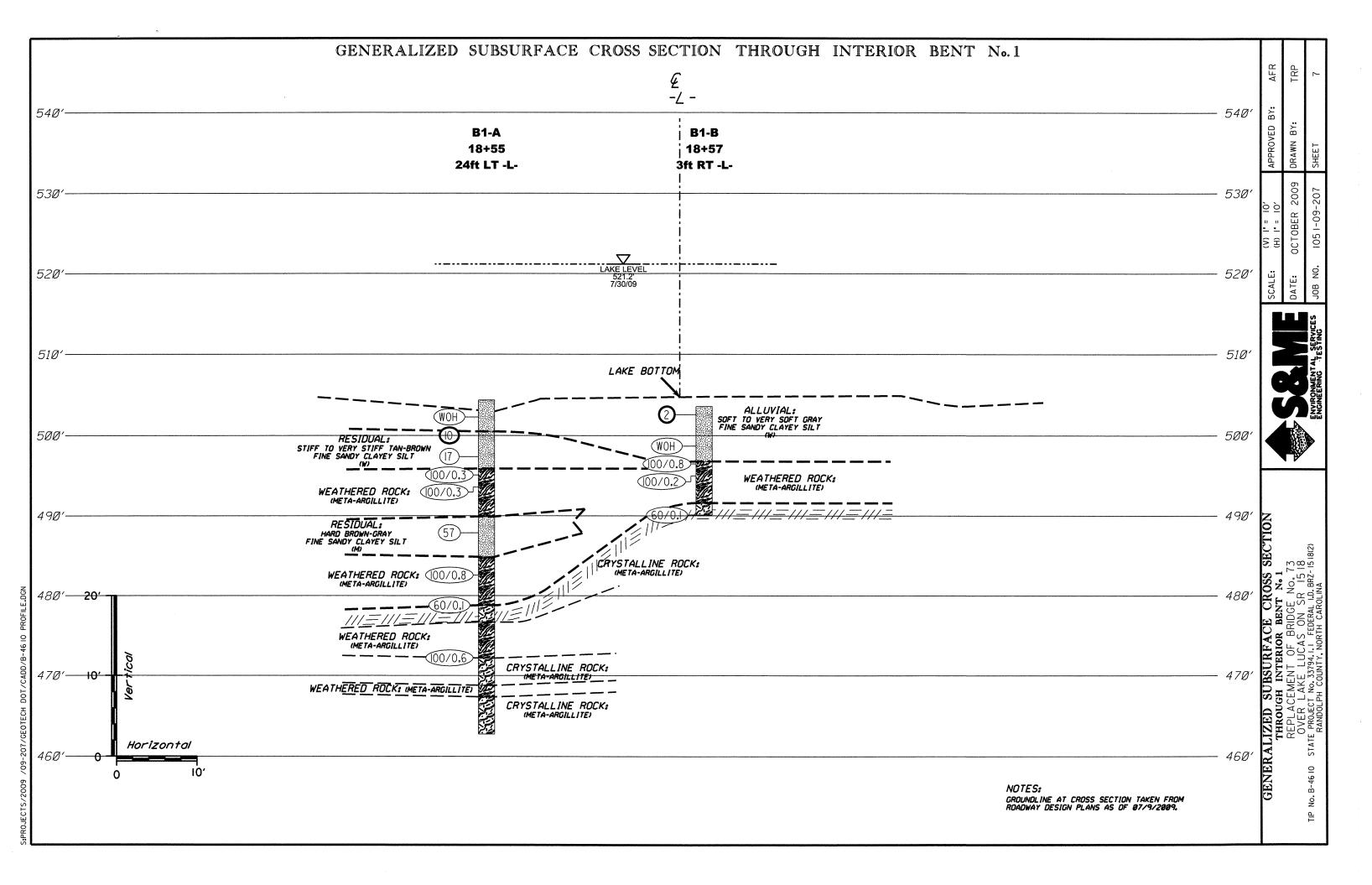
SOIL AND ROCK LEGEND, TERMS, SYMBOLS, AND ABBREVIATIONS

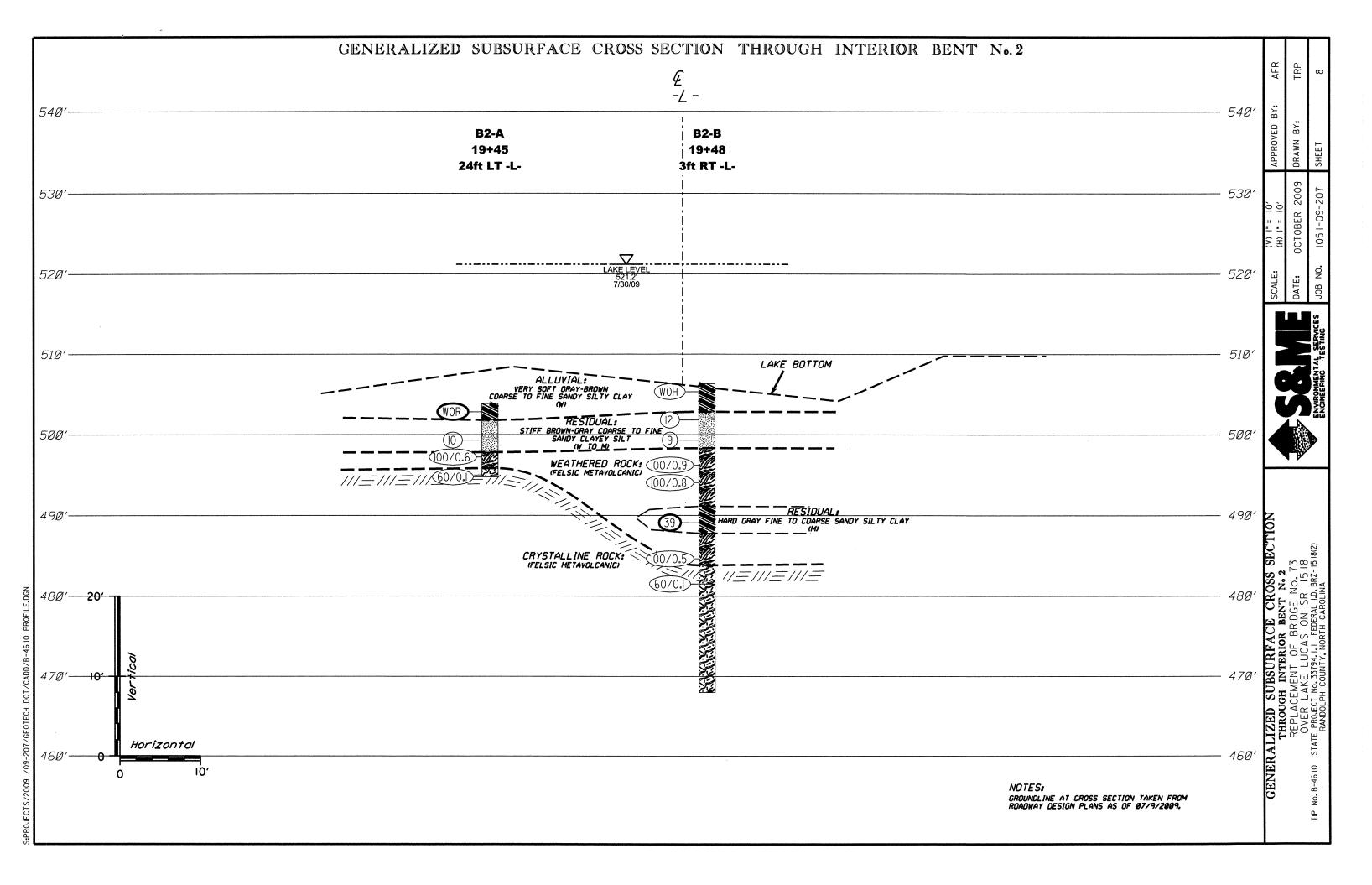
		T		_	· · · · · · · · · · · · · · · · · · ·			
SOIL DESCRIPTION			GRADATION				DESCRIPTION	TERMS AND DEFINITIONS
SOIL IS CONSIDERED TO BE THE UNCONSOLIDATED, SEMI-CONSOLIDATED OR WEATHERED EA	ADTH MATERIALS	WELL GRADED- INDICATES A GOOD RE	PRESENTATION OF PARTICLE SIZES FRO	IOM FINE TO COARSE			T WHEN TESTED, WOULD YIELD SPT REFUSAL, AN INFERRED	ALLUVIUM (ALLUV.) - SOILS WHICH HAVE BEEN TRANSPORTED BY WATER.
WHICH CAN BE PENETRATED WITH A CONTINUOUS FLIGHT POWER AUGER, AND WHICH YIELDS	S LESS THAN	POORLY GRADED)	TICLES ARE ALL APPROXIMATELY THE S	SAME SIZE. (ALSU	SPT REFLICAL IS PENE	FIRATION BY A SPILL SPOON	COASTAL PLAIN MATERIAL WOULD YIELD SPT REFUSAL.	
100 BLOWS PER FOOT ACCORDING TO STANDARD PENETRATION TEST (AASHTO T206, ASTM (D-1586). SOIL	GAP-GRADED- INDICATES A MIXTURE C	F UNIFORM PARTICLES OF TWO OR MO	DRE SIZES.	IN NON-COASTAL PLAT	IN MATERIAL. THE TRANSITI	ON BETWEEN SOIL AND ROCK IS OFTEN REPRESENTED BY A ZONE	AQUIFER - A WATER BEARING FORMATION OR STRATA.
CLASSIFICATION IS BASED ON THE AASHTO SYSTEM AND BASIC DESCRIPTIONS GENERALLY	SHALL INCLUDE:		ANGULARITY OF GRAINS	A STATE OF THE STA	OF WEATHERED ROCK.			ARENACEOUS - APPLIED TO ROCKS THAT HAVE BEEN DERIVED FROM SAND OR THAT CONTAIN SAND.
CONSISTENCY, COLOR, TEXTURE, MOISTURE, AASHTO CLASSIFICATION, AND OTHER PERTINENT	FACTORS SUCH		SOIL GRAINS ARE DESIGNATED BY THE	TERMS, ANGUL AR	ROCK MATERIALS ARE	TYPICALLY DIVIDED AS FOLD	OWS:	ARGILLACEOUS - APPLIED TO ALL ROCKS OR SUBSTANCES COMPOSED OF CLAY MINERALS,
AS MINERALOGICAL COMPOSITION, ANGULARITY, STRUCTURE, PLASTICITY, ETC. EXAMPLE:		SUBANGULAR, SUBROUNDED, OR ROUND		TETH 101 PROCESSA	WEATHERED E	NON-COASTAL P	LAIN MATERIAL THAT YIELDS SPT N VALUES > 100 BLOWS	OR HAVING A NOTABLE PROPORTION OF CLAY IN THEIR COMPOSITION, AS SHALE, SLATE, ETC.
VERY STIFF, GRAY SILTY CLAY, MOIST WITH INTERBEDDED FINE SAND LAYERS, HIGHLY PLASTIC, A-7-1					ROCK (WR)			ARTESIAN - GROUND WATER THAT IS UNDER SUFFICIENT PRESSURE TO RISE ABOVE THE LEVEL
SOIL LEGEND AND AASHTO CLASSIFICATION	N	MIN	ERALOGICAL COMPOSITIO	JN	CRYSTALLINE	FINE TO COARSE	E GRAIN IGNEOUS AND METAMORPHIC ROCK THAT	AT WHICH IS IS ENCOUNTERED, BUT WHICH DOES NOT NECESSARILY RISE TO OR ABOVE THE
GENERAL GRANULAR MATERIALS SILT-CLAY MATERIALS OR	RGANIC MATERIALS	MINERAL NAMES SUCH AS QUARTZ, FEI	DSPAR, MICA, TALC, KAOLIN, ETC. ARE U	ISED IN DESCRIPTIONS	ROCK (CR)		PT REFUSAL IF TESTED. ROCK TYPE INCLUDES GRANITE.	GROUND SURFACE.
CLASS. (.95% PASSING *200) (>85% PASSING *200)	HOHNIC PHIERIALS	WHENEVER THEY ARE CONSIDERED OF	SIGNIFICANCE.		<u> </u>	GNEISS, GABBRO,		CALCAREOUS (CALC.) - SOILS WHICH CONTAIN APPRECIABLE AMOUNTS OF CALCIUM CARBONATE.
GROUP A-1 A-3 A-2 A-4 A-5 A-6 A-7 A-1, A-	-2 A-4, A-5		COMPRESSIBILITY		NON-CRYSTALLINE		GRAIN METAMORPHIC AND NON-COASTAL PLAIN	COLLUVIUM - ROCK FRAGMENTS MIXED WITH SOIL DEPOSITED BY GRAVITY ON SLOPE OR AT BOTTOM
CLASS. A-1-6 A-1-6 A-2-4 A-2-5 A-2-6 A-2-7 A-7-5 A-3		SLIGHTLY COMPRESSIBLE		LESS THAN 30	ROCK (NCR)	INCLINES PHYLL	OCK THAT WOULD YEILD SPT REFUSAL IF TESTED. ROCK TYPE .ITE, SLATE, SANDSTONE, ETC.	OF SLOPE.
00000000000	· ·	MODERATELY COMPRESSIBLE			COASTAL PLAIN	COASTAL PLAIN	SEDIMENTS CEMENTED INTO ROCK, BUT MAY NOT YIELD	4
SYMBOL 80000800000		HIGHLY COMPRESSIBLE	LIQUID LIMIT	GREATER THAN 50	SEDIMENTARY ROCK	SPT REFUSAL, R	ROCK TYPE INCLUDES LIMESTONE, SANDSTONE, CEMENTED	CORE RECOVERY (REC.) - TOTAL LENGTH OF ALL MATERIAL RECOVERED IN THE CORE BARREL DIVIDED BY TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE.
% PASSING		Р	ERCENTAGE OF MATERIAL		(CP)	SHELL BEDS, ETC	C.	
2 PASSING GRANUL GRANUL	AR SILT- MUCK,	GRANI				WE	ATHERING	DIKE - A TABULAR BODY OF IGNEOUS ROCK THAT CUTS ACROSS THE STRUCTURE OF ADJACENT
* 40 30 MX50 MX51 MN SOILS	CLAY PEAT	ORGANIC MATERIAL SOI		OTHER MATERIAL	2004 505		A1170 1111 0101 011017 07111110 DOO: B1100 11100	ROCKS OR CUTS MASSIVE ROCK.
# 200 15 MX 25 MX 10 MX 35 MX 35 MX 35 MX 36 M	SOILS	TRACE OF ORGANIC MATTER 2 -		ACE 1 - 10%		F CRYSTALLINE.	OINTS MAY SHOW SLIGHT STAINING, ROCK RINGS UNDER	DIP - THE ANGLE AT WHICH A STRATUM OR ANY PLANAR FEATURE IS INCLINED FROM THE
		LITTLE ORGANIC MATTER 3 -	5% 5 - 12% LITI	TLE 10 - 20%	1			HORIZONTAL.
LIQUID LIMIT 40 MX41 MN 40 MX41 MN 40 MX 41 MN 40 MX 41 MN 50I	ILS WITH	MODERATELY ORGANIC 5 -					NED, SOME JOINTS MAY SHOW THIN CLAY COATINGS IF OPEN,	OJP DIRECTION (DIP AZIMUTH) - THE DIRECTION OR BEARING OF THE HORIZONTAL TRACE OF
	TTLE OR HIGHLY	HIGHLY ORGANIC >10	% >20% HIGH	HLY 35% AND ABOVE		STALLINE NATURE.	CE SHINE BRIGHTLY. ROCK RINGS UNDER HAMMER BLOWS IF	THE LINE OF DIP. MEASURED CLOCKWISE FROM NORTH.
	DERATE ORGANIC		GROUND WATER		1			FAULT - A FRACTURE OR FRACTURE ZONE ALONG WHICH THERE HAS BEEN DISPLACEMENT OF THE
USUAL TYPES STONE FRAGS.	OUNTS OF SOILS	WATER LEVEL	IN BORE HOLE IMMEDIATELY AFTER	ORILLING			NED AND DISCOLORATION EXTENDS INTO ROCK UP TO AY. IN GRANITOID ROCKS SOME OCCASIONAL FELDSPAR	SIDES RELATIVE TO ONE ANOTHER PARALLEL TO THE FRACTURE.
OF MAIDE CRAVEL AND FINE SILLY ON CERTE! SILLY	ATTER	WHIER CEVEL	IN BORE HOLE IMMEDIATELT AFTER	DRILLING.			CRYSTALLINE ROCKS RING UNDER HAMMER BLOWS.	FISSILE - A PROPERTY OF SPLITTING ALONG CLOSELY SPACED PARALLEL PLANES.
MATERIALS SAND SAND GRAVEL AND SAND SOILS SOILS MA	TI IEN	STATIC WATER	LEVEL AFTER 24 HOURS.					
CEN DATING							DISCOLORATION AND WEATHERING EFFECTS. IN RE DULL AND DISCOLORED, SOME SHOW CLAY, ROCK HAS	FLOAT - ROCK FRAGMENTS ON SURFACE NEAR THEIR ORIGINAL POSITION AND DISLODGED FROM
ACA EVELLENT TO COOD EATO TO POOP FHIR !		PERCHED WATER	R, SATURATED ZONE OR WATER BEARI	ING STRATA			ND SHOWS SIGNIFICANT LOSS OF STRENGTH AS COMPARED	PARENT MATERIAL.
SUBGRADE POOR	3	0000	2005		WITH FRES		LO STORM TOWN LOSS OF STREAM IN NO CONTARED	FLOOD PLAIN (F.P.) - LAND BORDERING A STREAM, BUILT OF SEDIMENTS DEPOSITED BY
P.I. OF A-7-5 SUBGROUP IS ≤ L.L 30 : P.I. OF A-7-6 SUBGROUP IS	IS > L.L 30	SPRING OR SEER	TAUE		1		D OR STAINED. IN GRANITOID ROCKS, ALL FELDSPARS DULL	THE STREAM.
CONSISTENCY OR DENSENESS			MISCELLANEOUS SYMBOLS	<u> </u>			DW KAOLINIZATION. ROCK SHOWS SEVERE LOSS OF STRENGTH	FORMATION (FM.) - A MAPPABLE GEOLOGIC UNIT THAT CAN BE RECOGNIZED AND TRACED IN
DANCE OF STANDAGO PANE	IGE OF UNCONFINED	·					OGIST'S PICK. ROCK GIVES 'CLUNK' SOUND WHEN STRUCK.	THE FIELD.
DOLLARY COLL TYPE COMPACINESS ON DENETRATION DESIGNATION OF THE COMP	PRESSIVE STRENGTH	ROADWAY EMBANKMENT	SPT DCP DPT DMT TEST BORIN VST PMT	NG SAMPLE		D. WOULD YIELD SPT REFUSAL		JOINT - FRACTURE IN ROCK ALONG WHICH NO APPRECIABLE MOVEMENT HAS OCCURRED.
CONSISTENCY (N-VALUE)	(TONS/FT2)	WITH SOIL DESCRIPTION	VST PMT	DESIGNATIONS	SEVENE ALL BOCK	E EVEEDT OHABIT DIECOLOR	ED OR STAINED. ROCK FABRIC CLEAR AND EVIDENT BUT REDUCE:	
VERY LOOSE 44		1	A 41050 0001110				ANITOID ROCKS ALL FELDSPARS ARE KAOLINIZED TO SOME	LEDGE - A SHELF-LIKE HIDGE OR PROJECTION OF ROCK WHOSE THICKNESS IS SMALL COMPARED TO
LOOSE 4 TO 10		SOIL SYMBOL	AUGER BORING	S- BULK SAMPLE		SOME FRAGMENTS OF STRONG		ITS LATERAL EXTENT.
GRANULAR MEDIUM DENSE 10 TO 20	N/A	ARTIFICIAL FILL OTHER	THAN _	SS- SPLIT SPOON		D. YIELDS SPT N VALUES > 1		LENS - A BODY OF SOIL OR ROCK THAT THINS OUT IN ONE OR MORE DIRECTIONS.
MATERIAL DENSE 30 TO 50		ROADWAY EMBANKMENTS		SAMPLE	MEDA CEMEDE VII BUCK	EVERT OUARTZ DISCOLORE	D OR STAINED, ROCK FABRIC ELEMENTS ARE DISCERNIBLE BUT	MOTTLED (MOT.) - IRREGULARLY MARKED WITH SPOTS OF DIFFERENT COLORS. MOTTLING IN
VERY DENSE >50		المصا	Y	ST- SHELBY TUBE			TO SOIL STATUS, WITH ONLY FRAGMENTS OF STRONG ROCK	SOILS USUALLY INDICATES POOR AERATION AND LACK OF GOOD DRAINAGE.
VERY SOFT <2	/0.2E	INFERRED SOIL BOUNDA	RIES MONTTORING WEL	CAMDLE			OF ROCK WEATHERED TO A DEGREE SUCH THAT ONLY MINOR	PERCHED WATER - WATER MAINTAINED ABOVE THE NORMAL GROUND WATER LEVEL BY THE PRESENCE OF AN
GENERALLY SOFT 2 TO 4	<0.25 0.25 TO 0.5	THE PROPERTY OF LINE	MONITORING WEL	RS- ROCK SAMPLE			RIC REMAIN. IF TESTED, YIELDS SPT N VALUES (100 BPF	INTERVENING IMPERVIOUS STRATUM.
SILT-CLAY MEDIUM STIFF 4 TO 8	0.5 TO 1	DIETE INFERRED ROCK LINE	∧ PIEZOMETER	NS NOCK SHAFEE	[RESIDUAL SOIL - SOIL FORMED IN PLACE BY THE WEATHERING OF ROCK.
MATERIAL STIFF 8 TO 15	1 TO 2	TTTTT ALLUVIAL SOIL BOUNDAR	RY INSTALLATION	RT- RECOMPACTED			NOT DISCERNIBLE, OR DISCERNIBLE ONLY IN SMALL AND MAY BE PRESENT AS DIKES OR STRINGERS. SAPROLITE IS	
(COHESIVE) VERY STIFF 15 TO 30	2 TO 4		SLOPE INDICATOR	R TRIAXIAL SAMPLE	ALSO AN E		MHT BE PRESENT HS DIKES ON STRINGERS. SHPROLITE IS	ROCK QUALITY DESIGNATION (R.Q.D.) - A MEASURE OF ROCK QUALITY DESCRIBED BY: TOTAL LENGTH OF
HARD >30	>4	25/025 DIP/DIP DIRECTION OF	INSTALLATION	CBR - CBR SAMPLE	7,000 777		LIADDNECC	ROCK SEGMENTS EQUAL TO OR CREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF CORE RUN AND
TEXTURE OR GRAIN SIZE		ROCK STRUCTURES				HULK	HARDNESS	EXPRESSED AS A PERCENTAGE.
TEXTORE ON CHIMN CIZE		·	SPT N-VALUE		VERY HARD CANNOT	BE SCRATCHED BY KNIFE OR	SHARP PICK, BREAKING OF HAND SPECIMENS REQUIRES	SAPROLITE (SAP.) - RESIDUAL SOIL WHICH RETAINS THE RELIC STRUCTURE OR FABRIC OF THE
U.S. STD. SIEVE SIZE 4 10 40 60 200 27	70	- SOUNDING ROD				HARD BLOWS OF THE GEOLO		PARENT ROCK.
OPENING (MM) 4.76 2.0 0.42 0.25 0.075 0.0	053		ADDDEVIATIONS		HARD CAN BE	SCRATCHED BY KNIFF OR PIC	CK ONLY WITH DIFFICULTY, HARD HAMMER BLOWS REQUIRED	SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND
COARSE FINE			ABBREVIATIONS			CH HAND SPECIMEN.	A GALL WITH DIVINGENT THAT THE DEGREE ALGORIED	RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, WHICH HAS BEEN EMPLACED PARALLEL
BOULDER COBBLE GRAVEL SAND SAND	SILT CLAY	AR - AUGER REFUSAL	HI HIGHLY	W - MOISTURE CONTENT	1			TO THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS
(BLDR.) (COB.) (GR.) (CSE. SD.) (F. SD.)	(SL.) (CL.)	BT - BORING TERMINATED	MED MEDIUM	V - VERY			CK. GOUGES OR GROOVES TO 0.25 INCHES DEEP CAN BE DLOGISTS PICK. HAND SPECIMENS CAN BE DETACHED	SLICKENSIDE - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT OR
	.05 0.005	CL CLAY	MICA MICACEOUS	VST - VANE SHEAR TEST			DEUGISTS PICK. HAND SPECIMENS CAN BE DETACHED	SLIP PLANE.
GRAIN MM 305 75 2.0 0.25 0.6 SIZE IN. 12' 3'	.05 0.005	CPT - CONE PENETRATION TEST	MOD MODERATELY	WEA WEATHERED	į.	RATE BLOWS.	ICHEC DEED BY EIOM DEECCHOE OF PANEE OD DIEW DOTHE	STANDARD PENETRATION TEST (PENETRATION RESISTANCE) (SPT) - NUMBER OF BLOWS (N OR B.P.F.) OF
i .		CSE COARSE	NP - NON PLASTIC	7 - UNIT WEIGHT			ICHES DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT.	A 140 LB. HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF 1 FOOT INTO SOIL WITH
SOIL MOISTURE - CORRELATION OF TERMS	S	DMT - DILATOMETER TEST	ORG ORGANIC	7d - DRY UNIT WEIGHT		F A GEOLOGISTS PICK.	TO PEICES 1 INCH MAXIMUM SIZE BY HARD BLOWS OF THE	A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER, SPT REFUSAL IS LESS THAN 0.1 FOOT PENETRATION
COT MOTOTION CON E STEED MOTOTION	MOISTURE DESCRIPTION	DPT - DYNAMIC PENETRATION TES		/ 0 2 5 #2.0//	1		BY KNIFE OR PICK. CAN BE EXCAVATED IN FRAGMENTS	WITH 60 BLOWS.
(ATTERBERG LIMITS) DESCRIPTION GOIDE FOR FIELD	הייסים יחב חבסרעור ווחא	e - VOID RATIO F FINE	SAP SAPROLITIC SD SAND, SANDY				SIZE BY MODERATE BLOWS OF A PICK POINT. SMALL, THIN	STRATA CORE RECOVERY (SREC.) - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY TOTAL LENGTH
	VEDV NET 115114	FOSS FOSSILIFEROUS	SL SILT, SILTY			CAN BE BROKEN BY FINGER P		OF STRATUM AND EXPRESSED AS A PERCENTAGE.
	VERY WET, USUALLY GROUND WATER TABLE	FRAC FRACTURED	SLI SLIGHTLY		1		EXCAVATED READILY WITH POINT OF PICK, PIECES 1 INCH	STRATA ROCK QUALITY DESIGNATION (S.R.O.D.) - A MEASURE OF ROCK QUALITY DESCRIBED BY:
	CHOCKE WHIER TABLE	FRAGS FRAGMENTS	TCR - TRICONE REFUSAL				KEN BY FINGER PRESSURE. CAN BE SCRATCHED READILY BY	TOTAL LENGTH OF ROCK SEGMENTS WITHIN A STRATUM EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE
BI ACTIC T		1			FINGERNA		C. C. CAROCA THEOGORE, CAN BE SCHATCHED REMDIET BY	TOTAL LENGTH OF STRATA AND EXPRESSED AS A PERCENTAGE.
DANCE J SEMISULIU; REUUIN		EOUIDME	NT USED ON SUBJECT P	DO IECT			DEDDING	TOPSOIL (T.S.) - SURFACE SOILS USUALLY CONTAINING ORGANIC MATTER.
ATTAIN UPTIMUM)	MOISTURE	EUUIPME	INT USED ON SUBJECT P	NUJEC I		E SPACING	BEDDING	
PLL + PLASTIC LIMIT		DRILL UNITS: AD	OVANCING TOOLS:	HAMMER TYPE:	TERM	SPACING	TERM THICKNESS	BENCH MARK: BL-103 NCDOT Traverse Station Rebar & Cap Stamped 'BL-103'
MOIOT AN COLID AT OR ME	EAR OPTIMUM MOISTURE	DIVILLE CHAILSE			VERY WIDE	MORE THAN 10 FEET	VERY THICKLY BEDDED > 4 FEET	Located at Station -BL- 15+57.74
OW T OF LINOW HOTS LOVE	THE OFTENOR MOTSTAKE	MOBILE B-	DRAG BITS	AUTOMATIC MANUAL	WIDE	3 TO 10 FEET	THICKLY BEDDED 1.5 - 4 FEET THINLY BEDDED 0.16 - 1.5 FEET	ELEVATION: 532.28'
SL _ SHRINKAGE LIMIT		T WORITE R -			MODERATELY CLOSE		VERY THINLY BEDDED 0.03 - 0.16 FEET	
REQUIRES ADDITIO	ONAL WATER TO	1 []	6 CONTINUOUS FLIGHT AUGER	CORE SIZE:	CLOSE	0.16 TO 1 FEET	THICKLY LAMINATED 0.008 - 0.03 FEET	NOTES:
- DRY - (D) ATTAIN OPTIMUM I	MOISTURE	BK-51	8 HOLLOW AUGERS		VERY CLOSE	LESS THAN 0.16 FEET	THINLY LAMINATED < 0.008 FEET	SPT LAB SAMPLE
DI ACTICITY				1		TNI	DURATION	1
PLASTICITY		_	HARD FACED FINGER BITS	N_WD4				
PLASTICITY INDEX (PI) DRY	STRENGTH		TUNGCARBIDE INSERTS	1	FUR SEDIMENTARY ROCKS	s, INDURATION IS THE HARDEN	NING OF THE MATERIAL BY CEMENTING, HEAT, PRESSURE, ETC.	ST - SHELBY TUBE
	ERY LOW				CDIADI C	RUBBING	WITH FINGER FREES NUMEROUS GRAINS:	
	SLIGHT	SILONIUN D-SE	CASING W/ ADVANCER		FRIABLE	GENTLE	BLOW BY HAMMER DISINTEGRATES SAMPLE.	
MED. PLASTICITY 16-25 M	MEDIUM		TRICONE 2-15/16 STEEL TEETH	HAND TOOLS:		CRATUS	CAN DE CEDADATED EDOM CAMPLE LUTU CTES DOCCE	
	HIGH			POST HOLE DIGGER	MODERATELY		CAN BE SEPARATED FROM SAMPLE WITH STEEL PROBE; EASILY WHEN HIT WITH HAMMER.	
COLOR	·····	152	TRICONE TUNGCARB.	HAND AUGER				
COLUN				SOUNDING ROD	INDURATED		ARE DIFFICULT TO SEPARATE WITH STEEL PROBE;	
DESCRIPTIONS MAY INCLUDE COLOR OR COLOR COMBINATIONS (TAN, RED, YEL-	-BRN, BLUE-GRAY)		CORE BIT			DIFFICU	ILT TO BREAK WITH HAMMER.	}
MODIFIERS SUCH AS LIGHT, DARK, STREAKED, ETC. ARE USED TO DESCRIBE API		OTHER	OTHER 3-1/4" H.S.A.	VANE SHEAR TEST	EXTREMELY IN	NOURATED SHARP	HAMMER BLOWS REQUIRED TO BREAK SAMPLE:	
The state of the s				OTHER	CATHERELT IF		BREAKS ACROSS GRAINS.	
					<u> </u>			

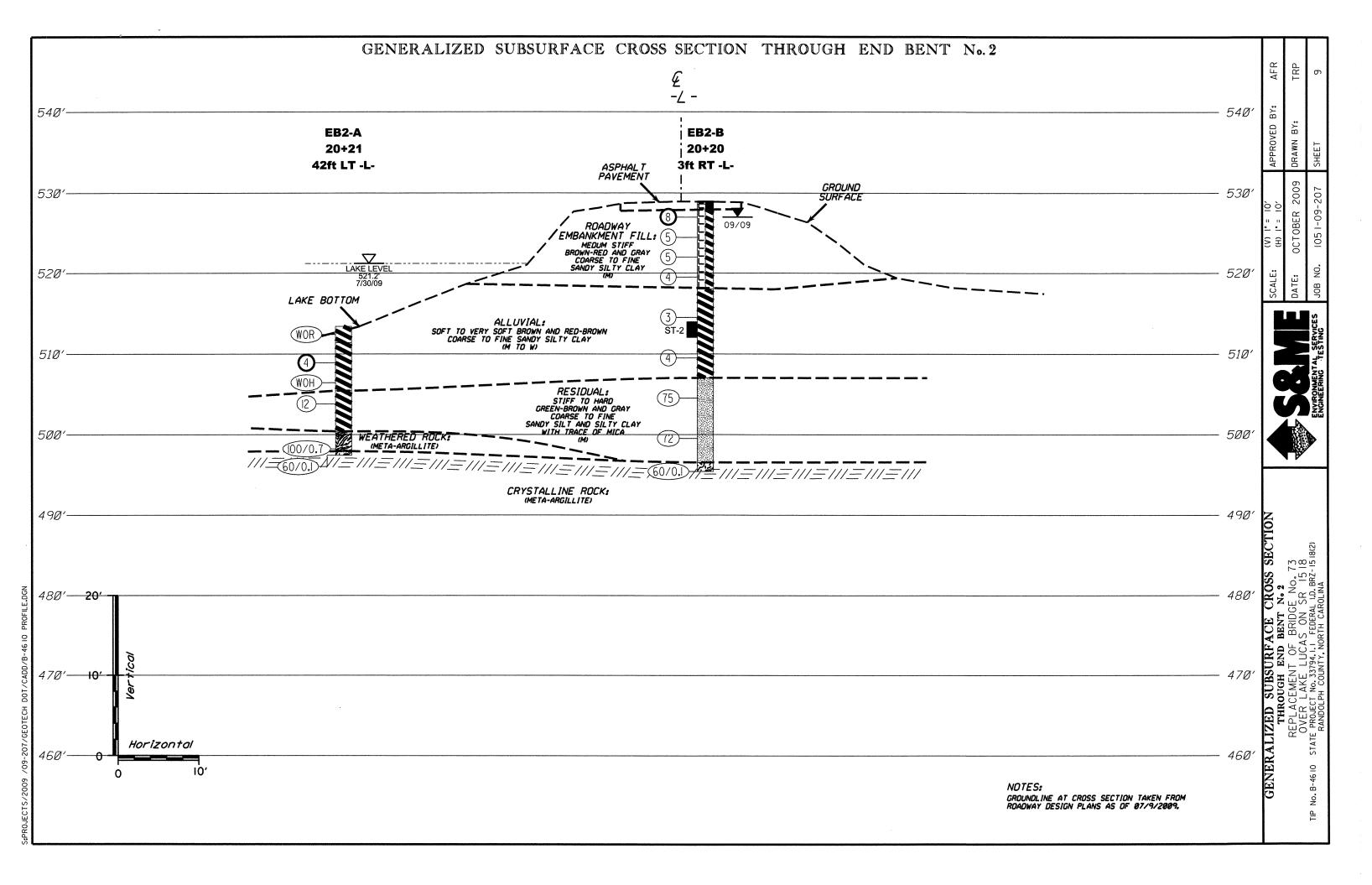
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		Branch
		Lake Greenes
		5500
	Back Creek	
	Back Cree	Back Cree
	522 Section 522	Back Creb 700 Asheboro
	ye Ch 1587	
	Charlotte	BIN 600
		628
		548 (Cerr)
-SITEVIC	170	
TIS-		580
0.0		Serie Valley Control of the Control
- B-4 - 4-4		
OO OV		
XX		
/GEOTECH/CADD/B-46 10		533
77 GE		
9-207	A CONTROL OF THE CONT	THE COUNTY CONTROL OF STREET AND ADDRESS OF THE PROPERTY OF TH
60/6	SCALE: 1:24,000	SITE VICINITY MAP
Ä 🎚	CHECKED BY: AFR	BRIDGE NO 73 OVER LAKE LLICAS
α	DRAWN BY: TRP	BRIDGE No. 73 OVER LAKE LUCAS ON SR 1518
0	DATE: OCTOBER 2009 ENVIRONMENTAL SERVICES ENGINEERING *TESTING	STATE PROJECT NO. 33794.I.I TIP NO. B-46 IO FEDERAL I.D. NO. BRZ- 15 I8(2) RANDOLPH COUNTY, NORTH CAROLINA
4.	JOB NO. 1051-09-207	RANDOLPH COUNTY, NORTH CAROLINA

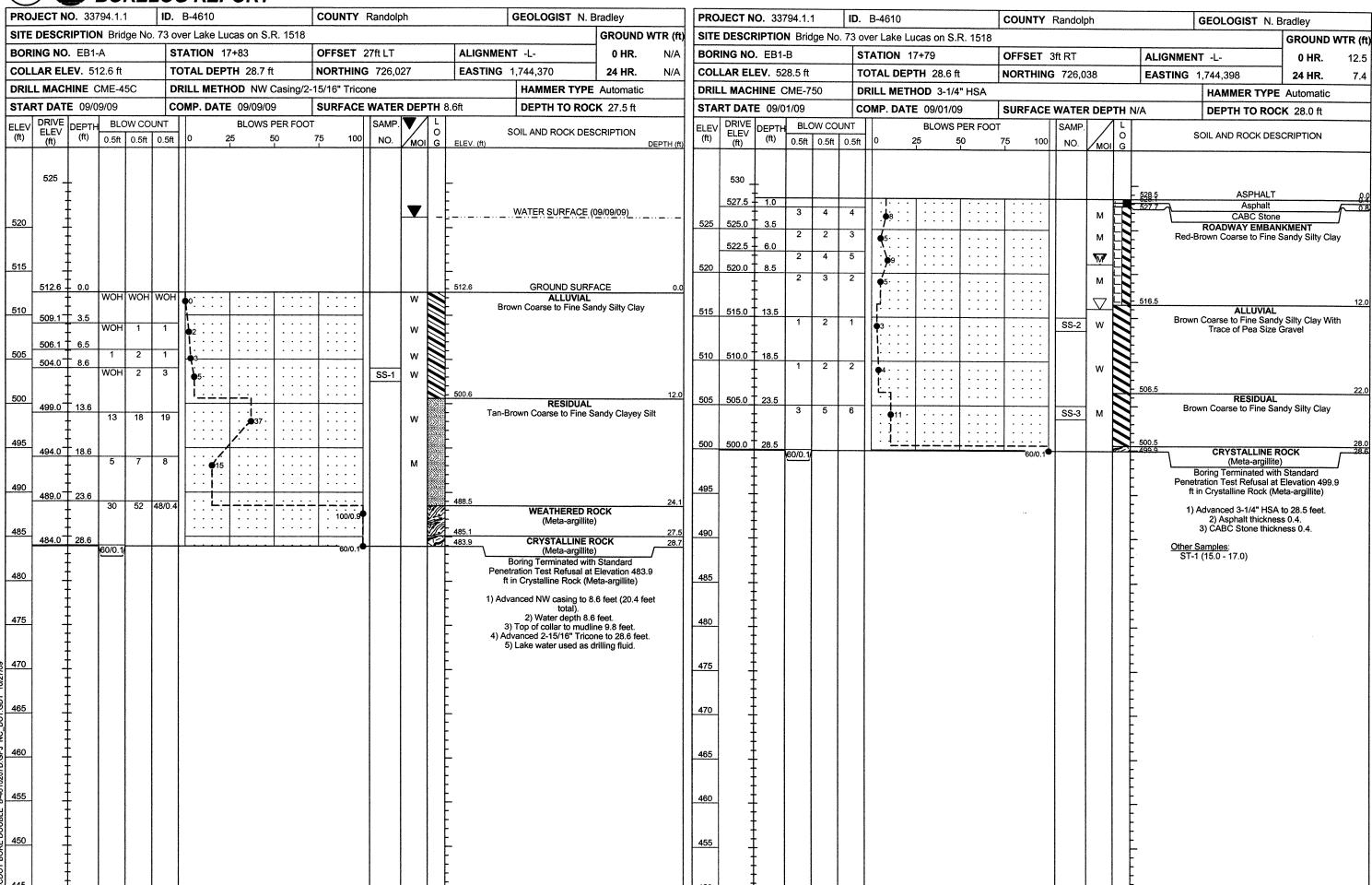












SHEET 11 OF 25

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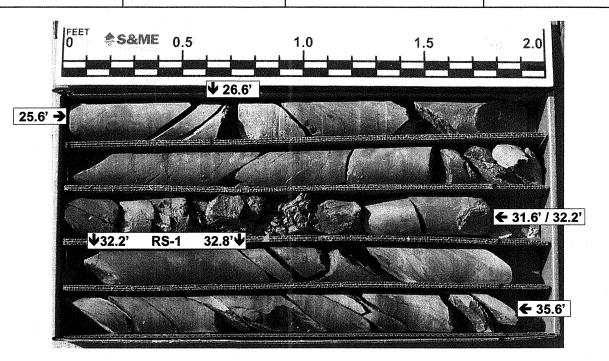
PRO	JECT N	O . 337	794.1.	1	ID.	В	3-4610			COUN	ITY	Randol	ph			GEOLOGIST N. E	Bradley	
SITE	DESC	RIPTIO	N Brid	dge No	o. 73 o	vei	r Lake Luca	as on S.F	R. 1518	•	***********	**************					GROUND WT	R (ft
BOR	ING NO	. B1-A	١		s	TA	ATION 18+	·55		OFFS	ET 2	24ft LT			ALIGNMEN"	Γ -L-	0 HR.	N/A
COL	LAR EL	EV . 50)4.4 ft		Т	ОТ	TAL DEPTH	1 41.6 ft		NORT	HING	726,0	83		EASTING 1	,744,332	24 HR.	N/A
DRIL	L MAC	HINE (CME-4	5C	D	RII	LL METHO	NW C	Casing/2-	15/16"	Trico	ne/NW[04 Co	re		HAMMER TYPE	Automatic	
STA	RT DAT	E 09/1	0/09		С	ON	MP. DATE	09/10/09)	SURF	ACE	WATE	R DEF	TH	16.8ft	DEPTH TO ROC	K 25.6 ft	
ELEV	DRIVE ELEV	DEPTH	'	ow co		41			PER FOOT			SAMP.		L	S	OIL AND ROCK DES	CRIPTION	
(ft)	(ft)	(ft)	0.5ft	0.5ft	0.5ft	\coprod	0 25	5	60 L	75 	100	NO.	MOI					
											ĺ							
	525 _	_				Ш									_			
	-	-													<u> </u>			
520	-	-				Ш							Y	_	<u> </u>	WATER SURFACE (09/10/09)	
1020	-	-				П					ŀ				_			
	-	_													_			
515	-	-									1				E			
	-	-													_			
510	-	-				Ш									_			
010	-	-													_			
į	-	-													-			
505	_	•													-504.4	GROUND SURF	ACF	0.0
	503.3	1.1	WOH	WOH	WOH	\prod				T : : :			100			ALLUVIAL Gray Fine Sandy Cla		
500	501.0	3.4	WOH		7	1	0			:	::		W		- 500.5	Gray Fine Sandy Cla	iyey Siit	3.9
300	498.4 -	6.0	WOR	3	′	lF	10			 		SS-4	W		_	RESIDUAL an-Brown Fine Sandy	Clause Cilt	
	-	-	1	2	15	11	. i7			1 : : :	::		w		ţ	in-brown Fine Sandy	Clayey Sill	
495	495.4	9.0	100/0.3]		$\ \ $	· · · !			+	0/0.3			110	495.9	WEATHERED RO	OCK	8.5
	493.9	10.0	100/0.3	נ						1	0/0.3				_	(Meta-argillite)		
490	-	-								: : :	::				_			
430	488.9	15.5] -				+					489.9 Bro	own-Gray Fine Sandy	Clavev Silt	14.5
	-	-	19	32	25				57 · ·		::		М		_	,	,	
485	400.0					$\ \ $			· i · · ·	<u> </u>		İ			484.9			19.5
	483.9	20.5	16	33	67/0.3					1:::	::[_	WEATHERED RO (Meta-argillite)	OCK	
480	-	-								. 100	0/0.8				-	(a. a g	,	
460	478.9	25.5				$\ \cdot\ $				+					 478.8			25.6
	-	-	60/0.1						· · · ·	60	0/0.1				- - 476.7	CRYSTALLINE R (Meta-argillite)		27.7
475	_	- -				$\ \ $	• • • •			<u> </u>	• •			10		WEATHERED RO	OCK	
	472.8	31.6														(Meta-argillite))	
470	-	_	70	30/0.1			: : : :			. 100	0/0.6	RS-1			472.2	CRYSTALLINE R		32.2
470	-	-				$\ \cdot\ $				+					468.8	(Meta-argillite))	35.6
	-	-								: : :	::				467.4	WEATHERED RO (Meta-argillite)		37.0
465	-	-				$\ \ $				<u> </u>		RS-2			<u> </u>	CRYSTALLINE R	OCK	
	-	_				Ш				1 : : :		110-2			462.8	(Meta-argillite))	41.6
400	-	•				Γ									_ Boring	Terminated at Eleva Crystalline Rock (Meta	tion 462.8 ft in	
460	-	-													-	vanced NW casing to		
	_	_													-	feet total). ranced 2-15/16" Tricor	•	
455	_	<u>-</u>														anced NWD4 core bar		
	-	-														41.6 feet. Lake water used as d		
450	-	-													5) 1	Γορ of collar to mudlin	e 17.8 feet.	
450	-	<u>-</u> -													_			
	-	-													-			
445	-					L									-			

NCDOT GEOTECHNICAL ENGINEERING UNIT

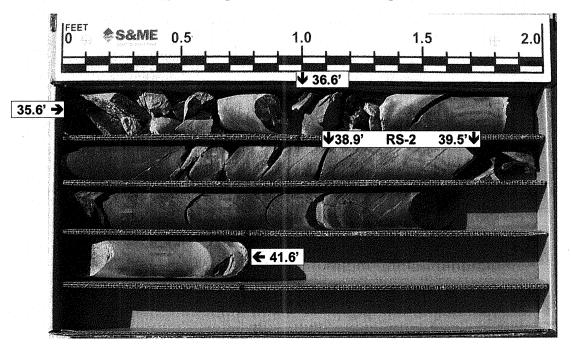
PRO	JECT NO	5 . 337	94.1.1	1	D. B-	4610				CO	NTY Randolph GEOLO	GIST N. B		
SITE	DESCR	IPTION	Brid	ge No. 7	3 over	Lake	Lucas on	S.R.	1518			:	GROUND W	TR (ft
BOR	NG NO.	B1-A			STA	TION	18+55			OF	SET 24ft LT ALIGNMENT -L-		0 HR.	N/A
COLI	AR ELE	EV. 50	4.4 ft		TOT	AL DE	PTH 41.	.6 ft		NO	THING 726,083 EASTING 1,744,33	32	24 HR.	N/A
DRIL	L MACH	INE C	ME-4	5C	DRIL	L MET	THOD N	W Cas	sing/2	-15/1	'Tricone/NWD4 Core HAMN	WER TYPE	Automatic	
STAF	RT DATE	09/1	0/09		COM	P. DA	TE 09/1	0/09		su	FACE WATER DEPTH 16.8ft DEPT	H TO ROCK	< 25.6 ft	
COR	ESIZE	NWD4	ļ				N 15.4 f			 	LER J. White			
ELEV (ft)	RUN ELEV (ft)	DEPTH (ft)	RUN (ft)	DRILL RATE (Min/ft)	REC. (ft)	JN RQD (ft) %	SAMP. NO.	REC. (ft) %	RQD (ft) %	L O G	DESCRIPTION AND REMA	ARKS	С	DEPTH (
	478.8 478.8 477.8 7	25.6	1.0	2:30/1.0	(0.6)	(0.4)		(2.0)	(1.4)		Begin Coring @ 25.6 478.8 CRYSTALLINE ROC	6 ft CK		25.
	477.8.7	26.6	5.0	2:30/1.0 2:30/1.0	60% (4.8)	40% (1.5)		95% (4.6)	67% N/A		476.7 Hard to Moderately Hard Moderate to Moderate Meta-argillite with Close Fracture Spaci			<u>27.</u>
475	_			2:45/1.0 2:30/1.0	96%	30%		88%	18/		WEATHERED ROC (Meta-argillite)			, .
	472.8	31.6 32.2		2:15/1.0 W=100/0 6	(4.4)	(4.2)	RS-1	(2.4)	(0.6)		472.2 CRYSTALLINE ROC	<u> </u>		32.
470	_	Ė	4.4	1:00/0.4 2:30/1.0	(4.1) 93%	(1.2) 27%	K3-1	(3.4) 100%	(0.6)		Hard to Moderately Hard Moderately to S	Slightly Weathe		
	467.8	36.6		3:00/1.0 4:00/1.0 2:30/1.0	<u> </u>			(1.0)	N/A		468.8 Meta-argillite with Close to Very Close Fracture WEATHERED ROC		Jonnes (W 40-00	35. 37.
465	_	<u> </u>	5.0	2:30/1.0 2:45/1.0 2:30/1.0 2:30/1.0	(4.6) 92%	(2.4) 48%		(3.1)	(1.8) 39%	1	(Meta-argillite) CRYSTALLINE ROO			
405	462.8 _	41.6		2:30/1.0			RS-2	67%			Hard to Moderately Hard Moderately Weathers Moderately Severely Weathered Search from 3	38.0-38.5 feet	Close to Very	41.
	402.0	41.0		2:45/1.0	 	<u> </u>					Close Fracture Spacing, 9 Joints @ 40	0-50°, 1 joint (@ 70°	T .
460	_	F									KSF Axial=31.7 R1=0, R2=8, R3=10, R4=6	6, R5=0		
	-	F									RMR=24 Rock Type E			
455	_	Ē									Boring Terminated at Elevation 462.8 ft in Crys	stalline Rock	(Meta-argillite)	•
											1) Advanced NW casing to 25.5 fee 2) Advanced 2-15/16" Tricone		tal).	
				-	l deservi						Advanced NWD4 core barrel from Ablake water used as drilli	n 25.6 to 41.6	feet.	
450		-									5) Top of collar to mudline			
	_	<u> </u>												
445	-	<u> </u>												
	-	ļ .												
440	-	ļ												
	-	F												
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435		F												
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430		E												
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425	٠	-												
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420	-	Ŀ						•						
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445	-	_												
415	-	-								ŀ				
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410	-	-												
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405	-	 												
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400	_													

SHEET 12 OF 25

Project No.: 1051-09-207	ID No.: B-4610	Location: Randolph Co., N.C.	Boring No.: B1-A
Site Description: Replacem	nent of Bridge No. 73 over La	ake Lucas on SR 1518	Driller: J. White
Collar Elev.: 504.4 ft.	Core Size: NWD4	Equipment: CME-45C	Geologist: N. Bradley
Elev. at T.D.: 462.8 ft.	Total Depth: 41.6 ft.	Total Run: 15.4 ft.	Date: 9/10/09



Box 1 of 2 Top of Box @ 25.6 feet; Bottom of Box @ 35.6 feet



Box 2 of 2 Top of Box @ 35.6 feet; Bottom of Box @ 41.6 feet

PROJECT NO. 33794.1.1	ID. B-4610	COUNTY Randolph	GEOLOGIST N.			CT NO. 33	~		B-4610	COUNTY R	landolph	l		GEOLOGIST N. B	
	. 73 over Lake Lucas on S.R. 151			GROUND WTR (ft)					over Lake Lucas on S.R. 1518						GROUND WTR
BORING NO. B1-B	STATION 18+57		IGNMENT -L-	0 HR. N/A	l	G NO. B2-/			TATION 19+45	OFFSET 24	4ft LT		ALIGNME	NT -L-	0 HR. N
COLLAR ELEV. 503.6 ft	TOTAL DEPTH 13.5 ft		ASTING 1,744,352	24 HR. N/A		AR ELEV. 5			OTAL DEPTH 9.1 ft	NORTHING		6	EASTING	1,744,278	24 HR. N
DRILL MACHINE CME-750	DRILL METHOD NW Casing	·	HAMMER TYPE			MACHINE			RILL METHOD NW Casing/2					HAMMER TYPE	Automatic
START DATE 09/02/09	COMP. DATE 09/02/09	SURFACE WATER DEPTH 17.7ft	DEPTH TO ROC	K 12.0 ft		DATE 09/			OMP. DATE 09/10/09	SURFACE W		EPTH 1	17.4ft	DEPTH TO ROCK	6 8.0 ft
DRIVE DEPTH BLOW COU	JNT BLOWS PER FO 0.5ft 0 25 50	75 100 NO. MOI G ELE	SOIL AND ROCK DES	CRIPTION DEPTH (ft)		ORIVE ELEV (ft) (ft)	0.5ft 0.5		BLOWS PER FOO 0 25 50	75 100	SAMP.	MOI G		SOIL AND ROCK DESC	CRIPTION
525		■	WATER SURFACE (09/02/09)	<u>520</u> <u>515</u> <u>510</u>	525					,	▼ _		WATER SURFACE (0	9/10/09)
505 503.6 0.0		- 503			505	503.8 0.0							- - - 503.8	GROUND SURFA	ACE
	1 2		ALLUVIAL Gray Fine Sandy Cla			‡	WOR WO	OR WOR		1 1 1	SS-6	w	501.8 [ALLUVIAL Brown Coarse to Fine San	dy Silty Clay
00 499.7 3.9					500	500.3 + 3.5	4 4	6				vaz 🌅	-	RESIDUAL Brown Gray Fine Sandy	
497.8 5.8 WOH WOH	T * · · · · · · · · · ·	:: ::::	•			497.8 6.0			. •10		ĺ	w	497.8		
95 495.2 8.4 3 20	80/0.3	100/0.8	WEATHERED R	OCK 6.8	495	‡	81 19/0	0.1		100,0.01	į		495.8 494.7	WEATHERED RO (Felsic Metavolca	nic)
100/0.2		100/0.8	(Meta-argillite	e)	495	494.8 <u>9.0</u>	60/0.1/			60/0.1			494.7	CRYSTALLINE RO (Felsic Metavolca	
<u> </u>			6	12.0		1							<u> </u>	Boring Terminated with	Standard
490.2 13.4		60/0.1	CRYSTALLINE F	ROCK 13.5		<u></u>							Pe fti	netration Test Refusal at I in Crystalline Rock (Felsic	Elevation 494.7 Metavolcanic)
485		-	Boring Terminated with Penetration Test Refusal at ft in Crystalline Rock (M	h Standard Elevation 490.1 leta-argillite)	485	‡ ‡							2)	Advanced NW casing to 6. total).) Advanced 2-15/16" Trico 3) Top of Collar to Mudlin	ne to 9.0 feet. le 18.4 feet.
80			feet). 2) Advanced 2-15/16" Tricc 3) Lake water used fe 4) Bridge deck to mudlir	one to 13.4 feet.	480	‡ ‡							- - - -	4) Lake water used as d	rilling fluid.
		[. •			‡							_		
5					475	† †							- - - [
70					470	‡							- - - -		
5 +					465	<u> </u>							- - -		
					460	‡							- - -		
5 -					455	‡							- - - -		
50					450	‡ ‡							<u>-</u>		
5 +					445	‡							<u>-</u> -		

SHEET 14 OF 25

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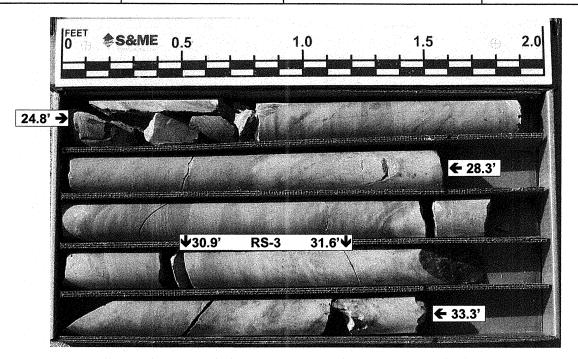
PKO.	JECT N	O. 33	794.1.	1	ID.	E	B-4610		***	C	OUNTY	Randol	nh			GEOLOGIS	T N. Bradley		
							er Lake Lu	cas on S	.R. 1518				r.,			CLOLOGIS	GROU	ND W	TP /
	ING NO						ATION 19				FFSET :	3ft RT			ALIGNA	MENT -L-	0 H		N/A
COLI	LAR EL	EV . 50	06.3 ft	***********			TAL DEP		ft		ORTHING		74			IG 1,744,298	24 H		N/A
	L MACI						ILL METH							re.	LACTIN		R TYPE Autom		A
	RT DAT						MP. DATE				URFACE				5.0ft		O ROCK 22.5		
ELEV	DRIVE	DEPTH	BLO	ow co		Ť			PER FO		J. T. A.O.L.	SAMP.		1		DEFINI	O ROCK 22.5	11.	***************************************
(ft)	ELEV (ft)	(ft)	0.5ft	0.5ft	0.5ft		0 2	25	50	75	100	NO.	MO	O		SOIL AND RO	CK DESCRIPTIC	N	
	525 _	-													-				
520	-													-[WATER SUF	RFACE (09/02/09)		
515	- - -															*,			
	-	-													-				
510	1 1 1	-													_	r			
505	506.3	0.0	WOH	WOH	WOH	Н		 	,	• 1		ļ	ļ		506.3		SURFACE		0.
303	-		"	"	"		0		 	\pm			W		- G	ray-Brown Coarse	. UVIAL to Fine Sandy Sil	ty Clay	
}	502.8 _	3.5	3	5	7	$\ $	12				: : : :		w		502.8	RES	IDUAL		3.
500	500.3	6.0	3	4	5	-	1		<u> </u>	<u>: </u>					_	Brown Coarse to F		y Silt	
	497.6 -	- - 8.7					: t 9		-:-:-	÷Ļ			M	-	498.3	\A/F A 711F	RED ROCK		8.
495	495.3	11.0	E4	38	62/0.4	1			1:::		100/0.9	-					etavolcanic)		
730	-		29	26	74/0.3	3			 	\pm	100/0.8				_	•			
	-	-			ļ			· · · ·		:									
490	490.1	16.2	9	13	26	-		· · · L =	+						<u>491.1</u> -	Gray Fine to Coa	rse Sandy Silty C	lay	15.2
	•	-		"	20			· · •39	<u> </u>			SS-7	М		487.8				18.
485	485.1	21.2							: : :								RED ROCK etavolcanic)		
700	100.1		100/0.	5					+	.	. 100/0.5	ľ			483.8				22.
	481.6	- 24.7							: : :		::::[481.5		LINE ROCK etavolcanic)		24.8
480		-	60/0.1								- 60/0.1			对		CRYSTAL	LINE ROCK etavolcanic)		_ == == :
	1	-							: : :							(1 01010 111	otavologino)		
475	1	-								:	: : : :								
	7	-							1	-		RS-3			-				
	1	-					: : : :		: : :										
470		-							ļ · · ·						-				
-					<u> </u>	Ш			<u> </u>	\perp	· · · · · <u> </u>			S	468.0				38.3
465	1	-												t	E	Boring Terminated Crystalline Rock (at Elevation 468.0 (Felsic Metavolca	J ft in nic)	
.50	=	<u> </u>													- 1	I) Advanced NW ca		(total	
	‡	<u> </u>														50.4 2) Bridge deck to	l feet). o mudline 22.5 fee	et.	
460	4	-												<u> </u>	- 3 - 4)) Advanced 2-15/1 Advanced NWD4	6" Tricone to 24.7	feet.	
	-	<u> </u>												<u> </u>	.,	38.3	3 feet. used for drilling.		
AEE	4	-												1 E		o, Lake Waler	assa ioi unining.		
455	=	-												E	-				
	-	<u> </u>												E					
450		<u> </u>												E	_				
l	1	-												F					
445	}	ŀ												F					

NCDOT GEOTECHNICAL ENGINEERING UNIT

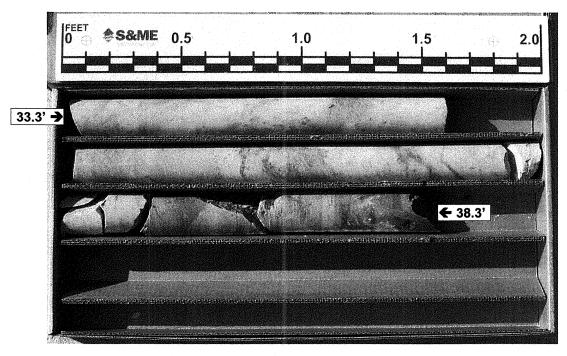
1	2	DV	D	CO	RE B	OR	NI	RE	PO	RT						
	PRO	JECT N	D. 337	94.1.	1 I	D . B	4610				СО	UNTY [*] Randolph		GEOLOGIST N. E	Bradley	
	SITE	DESCR	IPTION	Brid	lge No. 7	3 over	Lake	Lucas on	S.R.	1518			,		GROUND W	/TR (ft)
	BOR	NG NO	B2-B			STA	TION	19+48			OF	FSET 3ft RT	ALIGNMENT	-L-	0 HR.	N/A
	COL	AR EL	EV. 50	6.3 ft		TOT	AL DE	PTH 38.	.3 ft		NO	RTHING 726,174	EASTING 1	,744,298	24 HR.	N/A
	DRIL	L MACH	IINE C	ME-7	50	DRIL	L MET	THOD N	W Cas	sing/2	-15/1	6" Tricone/NWD4 Core		HAMMER TYPE	Automatic	
	STAF	RT DATI	E 09/0	2/09		COM	P. DA	TE 09/0:	2/09		su	RFACE WATER DEPTH 15	5.0ft	DEPTH TO ROCK	C 22.5 ft	
	COR	E SIZE	NWD4			1		N 13.5 f			DR	ILLER J. White			A44	
	ELEV (ft)	RUN ELEV (ft)	DEPTH (ft)	RUN (ft)	DRILL RATE (Min/ft)	REC. (ft) %	RQD (ft) %	SAMP. NO.	STR REC. (ft) %	RQD (ft) %	LOG	D ELEV. (ft)	ESCRIPTION A	ND REMARKS		DEPTH (ft)
		481.5					<u></u>		(1.5.5)				Begin Coring	@ 24.8 ft		
	480	481.5 - 478.0	 	3.5	5:30/1.0 5:30/1.0 2:30/1.0 1:30/0.5	(3.3) 94%	(3.1) 89%		(13.0) 96%	(11.7) 87%		- 481.5 - Hard to Moderately - Felsic Metavolcanic	CRYSTALL Hard Fresh to V c With Moderate	ery Slightly Weathered ly Close to Close Fract	d Green-Gray ture Spacing	24.8
	175		-	5.0	4:00/1.0 4:30/1.0 4:30/1.0	94%	(4.0) 80%					- - R1	KSF Axia	l=4619.5 =20, R4=20, R5=0		
	475	473.0	33.3		4:30/1.0 5:00/1.0 4:00/1.0			RS-3				-	RMF Rock T	R=72		
		473.0	- 00.0	5.0	6:00/1.0 6:30/1.0	(5.0) 100%	(4.6) 92%					• •		,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,		
	470	-	ļ.		6:30/1.0 4:00/1.0	100%	3270					- · · · · · · · · · · · · · · · · · · ·				
		468.0	38.3		5:00/1.0		-				1	468.0 Boring Terminate	ed at Flevation 4	68.0 ft in Crystalline Ro	nck (Felsic	38.3
	465		†					:				- Bonng Tommate	Metavo		JOIN (1 C1010	
			ļ .									1) Advanc	ed NW casing to	26.0 feet (total 50.4 fe mudline 22.5 feet.	eet).	
			F									- 3) Ad	Ivanced 2-15/16"	Tricone to 24.7 feet. arrel from 24.8 to 38.3	feet	
	460	-	F										5) Lake water u	ised for drilling.	1001.	
							-									
	455											- -				
						ì						-				
1												- -				
ł	450	-	-													
			Ė									<u>.</u>				
-	445	-										-				
ı			-									• •				
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1	700	-	-									- -				
NCDOT		-	-									-				

SHEET 15 OF 25

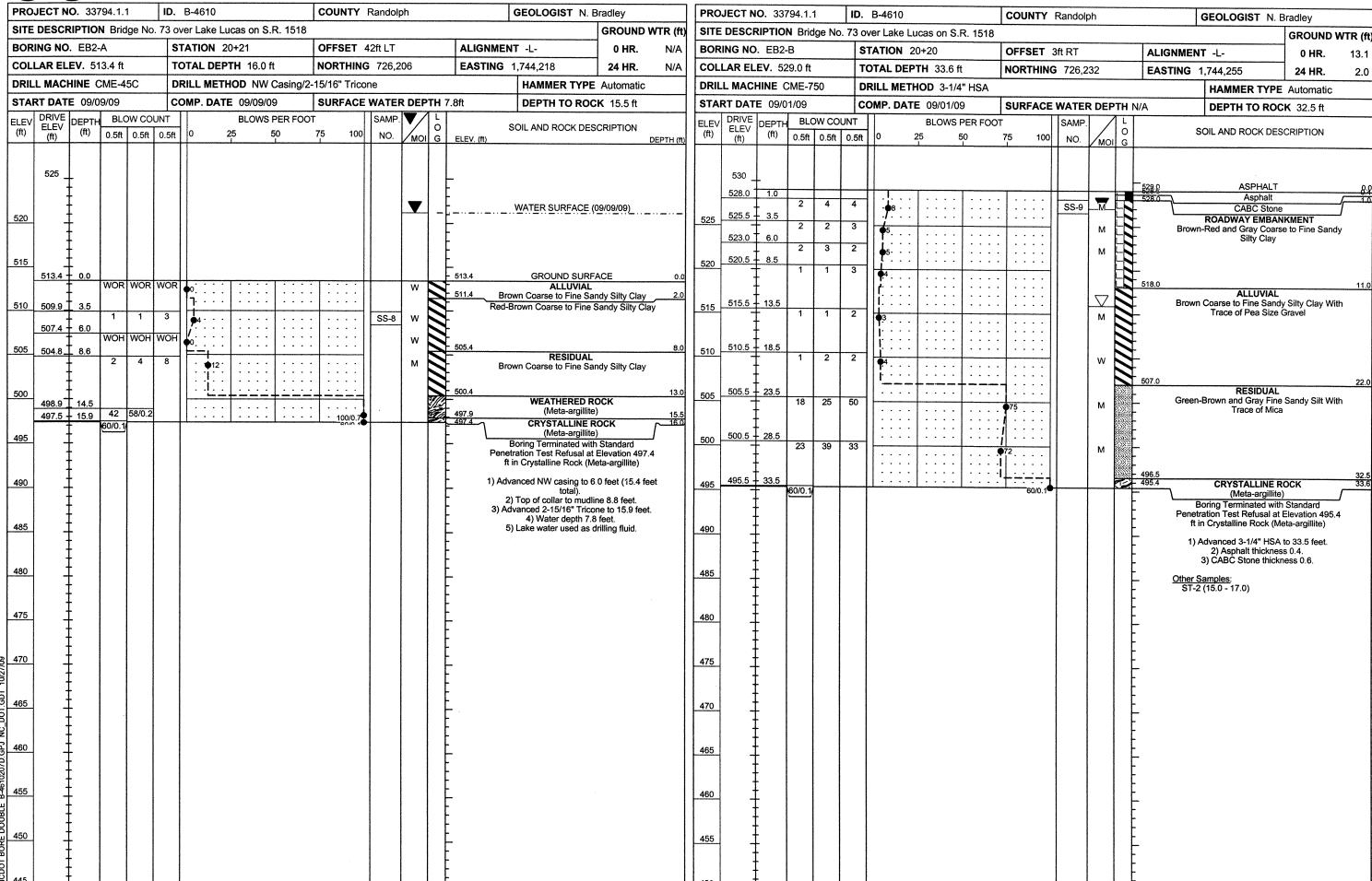
Project No.: 1051-09-207	ID No.: B-4610	Location: Randolph Co., N.C.	Boring No.: B2-B
Site Description: Replacen	nent of Bridge No. 73 over La	ake Lucas on SR 1518	Driller: J. White
Collar Elev.: 506.3 ft.	Core Size: NWD4	Equipment: CME-750	Geologist: N. Bradley
Elev. at T.D.: 468.0 ft.	Total Depth: 38.3 ft.	Total Run: 13.5 ft.	Date: 9/2/09



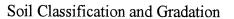
Box 1 of 2 Top of Box @ 24.8 feet; Bottom of Box @ 33.3 feet



Box 2 of 2 Top of Box @ 33.3 feet; Bottom of Box @ 38.3 feet



SUMMARY OF LABORATORY TEST DATA





S&ME Project #:	1051-09-207			Test Date(s):	09/04 - 09/25/2009
State Project No.:	33794.1.1	County:	Randolph	Report Date:	9/25/2009
Federal ID No.:	BRZ-1518(12)	TIP No. B-4	610		
Project Name:	Bridge No. 73 over La	ke Lucas on SR1518			
Client Name:	NCDOT				
Client Address:	Raleigh, North Carolin	ıa			

		Sample	AASHTO		Total % Passing				Total Mortar Fraction					Moisture		
Boring	Sample	Depth	Classification		Siev	ve#			Coarse	Fine			LL	PL	PI	Content
No.	No.	(feet)		10	40	60	200	270	Sand	Sand	Silt	Clay				%
EB1-A	SS-1	8.6 - 10.1	A-6 (17)	100	99	98	95.1	92.4	2	6	56	36	39	22	17	ND
EB1-B	SS-2	13.5-15.0	A-7-5 (18)	100	99	99	92.3	90.0	1	9	58	32	46	30	16	ND
EB1-B	ST-1	15.0 - 17.0	A-7-6 (24)	100	98	96	86.8	84.2	4	12	40	44	53	29	24	35
EB1-B	SS-3	23.5 - 25.0	A-7-6 (16)	100	99	99	95.8	95.0	1	4	59	36	42	28	14	ND
B1-A	SS-4	3.4 - 4.9	A-4 (7)	100	100	99	90.7	84.6	1	14	58	27	29	20	9	ND
B1-B	SS-5	0.0 - 1.5	A-4 (7)	100	99	98	87.2	82.5	2	16	54	28	30	21	9	ND
B2-A	SS-6	0.0 - 1.5	A-6 (9)	99	97	94	85.9	81.7	5	13	56	26	33	22	11	ND
B2-B	SS-7	16.2 - 17.7	A-6 (12)	99	87	84	78.3	76.6	15	8	48	29	37	21	16	ND
EB2-A	SS-8	3.5 - 5.0	A-7-6 (26)	98	95	94	91.3	90.0	4	4	40	52	48	21	27	ND
EB2-B	SS-9	1.0-2.5	A-7-5 (15)	100	96	94	88.4	87.1	6	7	63	24	45	31	14	26.8
EB2-B	ST-2	15.0 - 17.0	A-7-6 (24)	95	93	91	86.9	85.1	4	6	43	47	50	24	26	33.4

Notes: ND=Not Determined.

References:

AASHTO T88: Particle Size Analysis of Soils as Modified by the NCDOT

AASHTO T89: Determining the Liquid Limit of Soils

AASHTO T90: Determining the Plastic Limit & Plasticity Index of Soils

AASHTO T265: Laboratory Determination of Moisture Content of Soils

AASHTO M 145: The Classification of Soils and Soil Aggregate Mixtures for Highway Construction Purposes

Mal Krajan, AET

Technician Name:

104-01-0703

Certification #

Abner F. Riggs, Jr., P.E.

Technical Responsibility:

Senior Engineer

Position

PROJECT: 33794.1.1 (B-4610)

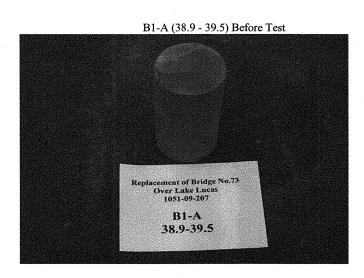
Description: Replacement of Bridge No. 73 Over Lake Lucas on S.R. 1518

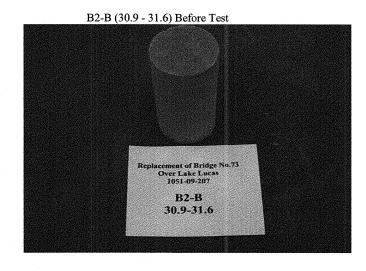
COUNTY: Randolph, N.C. F.A. No.: BRZ-1518(2) JOB No.: 1051-09-207 Date: 9/29/2009 Tested by: Jeff Webb

Sample	Boring	Depth	Rock	Run	Specimen I	Dimension, in.	Area	Unit Wt.	Loading Rate	Max. Load	Strength	Moisture
No.	Location	(ft)	Туре	RQD	Length	Diameter	(in ²)	(lb/ft ³)	(psi/min)	(lb)	(psi)	(%)
RS-1	B1-A	32.2 - 32.8	Meta-Argillite	27%	4.38	2.03	3.24	157.7	204	1,380	426	1.8
RS-2	B1-A	38.9 - 39.5	Meta-Argillite	48%	4.00	2.04	3.27	166.7	97	720	220	0.7
RS-3	B2-B	30.9 - 31.6	Felsic Metavolcanic	80%	4.31	2.04	3.27	173.5	2,319	104,900	32,080	0.1

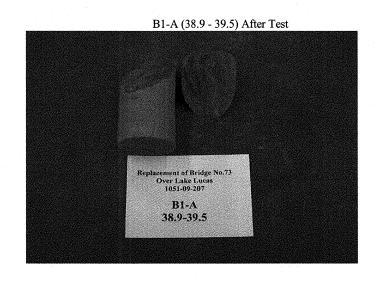
Replacement of Bridge No.73
Over Lake Lucas
1051-09-207

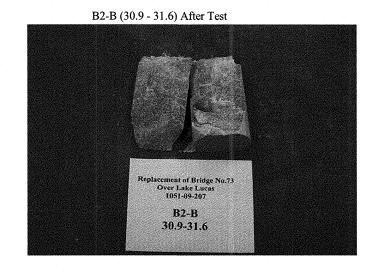
B1-A
32.2-32.8





Replacement of Bridge No.73
Over Lake Lucas
1051-09-207
B1-A
32.2-32.8







FIELD SCOUR REPORT

WBS:	33794.1.1	TIP:	B-4610	COUNTY: Randolph	
DESCRIPTION(1):	Bridge No. 73 c	over Lake L	ucas on S.R. 15	18	
			EXISTING	BRIDGE	
Information from:		nspection (explain) E	x Micr Bridge Survey&H	ofilm (reel pos:) lydraulic Design Report	
Bridge No.: Foundation Type:		: 226 FT	Total Bents: 7	Bents in Channel: 5 Bents in Floodplain:	2
EVIDENCE OF S Abutments or E	SCOUR(2) End Bent Slopes	: None obs	served		-
Interior Bents:	None observed				
Channel Bed:	None observed				
Channel Bank:	None observed				
EXISTING SCO	UR PROTECTION	ON			
Type(3):	Rock with mort	ar between	some of the roc	ks	-
Extent(4):	Past concrete v	vingwalls a	long both sides o	of causeway at bridge approaches	
Effectiveness(5):	Adequate	onalog ermuny erms smoothy or 1 happy many manner			
Obstructions(6):	None observed				

INSTRUCTIONS

- 1 Describe the specific site's location, including route number and body of water crossed.
- 2 Note scour evidence at existing end bents or abutments (e.g. undermining, sloughing, degradations).
- Note existing scour protection (e.g. rip rap).
- Describe extent of existing scour protection.
- 5 Describe whether or not the scour protection appears to be working.
- Note obstructions such as dams, fallen trees, debris at bents, etc.
- Describe the channel bed material based on observation and/or samples. Include any lab results with report.
- Describe the channel bank material based on observation and/or samples. Include any lab results with report.
- Describe the material covering the banks (e.g. grass, trees, rip rap, none).
- 10 Determine the approximate floodplain width from field observation or a topographic map.
- Describe the material covering the floodplain (e.g. grass, trees, crops).
- 12 Use professional judgement to specify if the stream is degrading, aggrading, or static.
- 13 Describe potential and direction of the stream to migrate laterally during the bridge's life (approx. 100 years).
- 14 Give the design scour elevation (DSE) expected over the life of the bridge (approx. 100 years). This elevation can be given as a range across the site, or for each bent. Discuss the relationship between the Hydraulics Unit theoritical scour and the DSE. If the DSE is dependent on scour counter measures, explain (e.g. rip rap armoring on slopes). The DSE is based on the erodability of materials, giving consideration to the influence of joints, foliation, bedding characteristics, % core recovery, % RQD, differential weathering, shear strength, observations at existing structures, other tests deemed appropriate, and overall geologic conditions at the site.

DESIGN INFORMATION

Channel Bed Material(7): Brown Coarse to Fine Sandy Silty Clay A-6(17), A-6(9) & Gray Fine Sandy Clayey Silt A-4(7)

Channel Bank Material(8): Brown Coarse to Fine Sandy Silty Clay A-7-6(16), Tan-Brown Fine Sandy Clayey Silt A-4(7) & Brown-Red & Gray Coarse to Fine Sandy Silty Clay A-7-5(24)

Channel Bank Cover(9): Brush and trees

Floodplain Width(10): Approximately 100 feet south and approximately 150 north of bridge

Floodplain Cover(11): Hardwood trees and brush

Stream is(12): Aggrading Degrading Static x

Channel Migration Tendency(13): None

Observations and Other Comments: Bridge over existing lake. Overhead utilities on east side of bridge

Reported by:

DESIGN SCOUR ELEVATIONS(14)

Feet X Meters

BENTS

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						e. Print As. 6 thouse or manufacture	AND CONTRACTOR OF SERVICE AND PROVIDE AND ADMINISTRATION OF SERVICE AND ADMINISTRATION OF SERVIC	MITTER COMPANY OF STREET BASE STREET, MITTER STREET, AND ADDRESS.		
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Comparison of DSE to Hydraulics Unit theoretical scour:

Geotechnical Engineering Unit agrees with the Hydraulic Unit's theoretical scour elevations.

DSE determined by:

SOIL ANALYSIS RESULTS FROM CHANNEL BED AND BANK MATERIAL

SOIL ANAL IS	SIS KESULI	S FROM CHAP	AIAEF DED WI	ID BANK MA	CRIAL		
Bed or Bank	Bed	Bank	Bank	Bed	Bed	Bank	
Sample No.	SS-1	SS-3	SS-4	SS-5	SS-6	SS-9	THE PERSON NAMED OF PERSONS AND ADDRESS ASSESSED ASSESSED AS A SECOND AS
Retained #4	0%	0%	0%	0%	0%	0%	And the second s
Passed #10	100%	100%	100%	100%	99%	100%	# 100.000.00 PROFES of \$4.00
Passed #40	99%	99%	100%	99%	97%	96%	ACOUNTAINE TO COME CONTEX ON A ACOUNT AND A CONTEX OF A CONTEX OF
Passed #200	95.10%	95.80%	90.70%	87.20%	85.90%	88.40%	THE STREET PRINTED STREET, AND ADDRESS AS A STREET AND ADDRESS AND
Coarse Sand	2%	1%	1%	2%	5%	6%	THE RESIDENCE OF THE PARTY OF T
Fine Sand	6%	4%	14%	16%	13%	7%	The second secon
Silt	56%	59%	58%	54%	56%	63%	F7-invalenceWesternals at conscious a special representation on program
Clay	36%	36%	27%	28%	26%	24%	
LL	39	42	29	30	33	45	
PI	17	14	9	9	11	14	
AASHTO	A-6	A-7-6	A-4	A-4	A-6	A-7-5	
Station	17+83	17+79	18+55	18+57	19+45	20+20	
Offset	27' LT	3' RT	24' LT	3' RT	24' LT	3' RT	
Depth	8.6'-10.1'	23.5'-25.0'	3.4'-4.9'	0.0'-1.5'	0.0'-1.5'	1.0'-2.5'	

Particle Size Analysis of Soils



Particle Size Analysis of Soils

AASHTO T 88 as Modified by NCDOT

\$S&ME

AASHTO T 88 as Modified by NCDOT

1051-09-207

Bridge 73 over Lake Lucas on SR1518 Randolph Co.

Report Date: 9/17/2009 Test Date(s): 09/14 - 09/16/2009

9/17/2009 S&ME Project #: 4 - 09/16/2009 Project Name:

1051-09-207

NCDOT

Bridge 73 over Lake Lucas on SR1518 Randolph Co.

Report Date: Test Date(s): 9/11/2009 09/04 - 09/10/2009

Client Name: Client Address: State Project #:

Project Name:

S&ME Project #:

NCDOT

33794.1.1

Raleigh, North Carolina

F.A. Project No: BRZ-1518(2) TIP NO: B-4610

Client Address:
State Project #:

Client Name:

Raleigh, North Carolina
33794.1.1 F.A. Project No: BRZ-1518(2)

TIP NO: B-4610

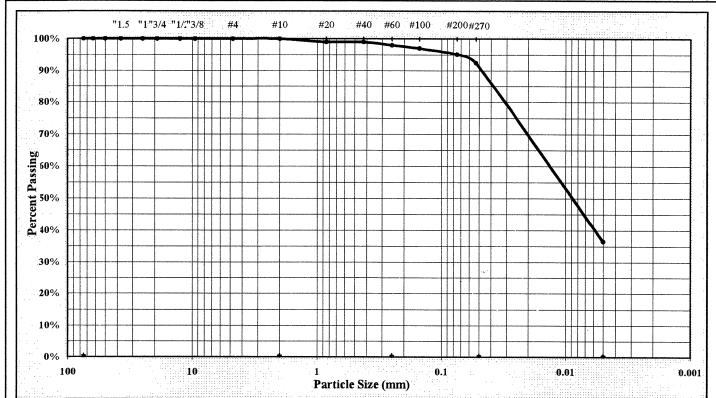
Brown Coarse to Fine Sandy Silty CLAY A-7-6 (16)

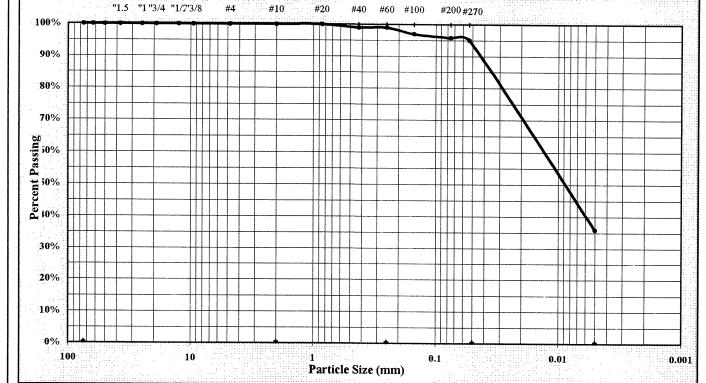
Boring #: EB1-A Sample #: SS-1 Sample Date: 09/09 - 09/11/2009 Boring #: EB1-B Sample #: SS-3 Sample Date: 09/01 - 09/03/2009

Location: 17+83 Offset: 27' LT Depth (ft): 8.6' - 10.1' Location: 17+79 -L- Offset: 3' RT Depth (ft): 23.5' - 25.0'

Sample Description: Brown Coarse to Fine Sandy Silty CLAY A-6 (17) Sample Description:

"1.5 "1"3/4 "1/"3/8 #4 #10 #20 #40 #60 #100 #200#270 "1.5 "1"3/4 "1





1								
	As Define	ed by NCDOT	Fine Sand	< 0.25 mm and > 0.05 mm	As Defin	ed by NCDOT	Fine Sand	< 0.25 mm and > 0.05 mm
	Gravel	< 75 mm and > 2.00 mm	Silt	< 0.05 and > 0.005 mm	Gravel	< 75 mm and > 2.00 mm	Silt	< 0.05 and > 0.005 mm
I	Coarse Sand	< 2.00 mm and > 0.25 mm	Clay	< 0.005 mm	Coarse Sand	< 2.00 mm and > 0.25 mm	Clay	< 0.005 mm
	Maximum Par	rticle Size #4	Coarse Sand 2%	Silt 56%	Maximum Pa	article Size #10	Coarse Sand 1%	Silt 59%
		Charrel 00/	Time Can 1 (0)	C1 2 (0)		~		

Gravel Fine Sand 6% Clay 36% Gravel 0% Fine Sand Clay 36% Apparent Relative Density Moisture Content % Passing #200 95.1% Apparent Relative Density Moisture Content % Passing #200 95.8% Liquid Limit 39 Plastic Limit 22 Plastic Index 17 **Liquid Limit** 42 Plastic Limit 28 Plastic Index 14

Soil Mortar (-#10 Sieve) Soil Mortar (-#10 Sieve)

Coarse Sand 2%	Fine Sand 6%	Silt 56%	Clay 36%	Coarse Sand 1%	Fine Sand 4%	Silt 59%	Clay 36%
Description of Sand & Gravel Particle	s: Rounded 🗆 Angular 🗖	Hard & Durable □ Soft □	Weathered & Friable □	Description of Sand & Gravel Particles	s: Rounded Angular	☐ Hard & Durable ☐ Soft ☐	Weathered & Friable □
Mechanical Stirring Apparatus (A)	Length of Dispersion Period: 1 min.	Dispersing Agent: Sodium Hexame	taphosphate: 40 g./ Liter	Mechanical Stirring Apparatus (A)	Length of Dispersion Period: 1 min.	Dispersing Agent: Sodium Heyamet	tanhoenhate: 40 a / Liter

References: AASHTO T88: Particle Size Analysis of Soils as Modified by the NCDOT AASHTO T87: Dry Preparation of Disturbed Soil and Soil Aggregate Samples for Test

AASHTO T265: Laboratory Determination of Moisture Content of Soils
AASHTO T90: Determining the Plastic Limit & Plasticity Index of Soils

AASHTO T87: Dry Preparation of Disturbed Soil and Soil Aggregate Samples for Test AASHTO T89: Determining the Liquid Limit of Soils

AASHTO T88: Particle Size Analysis of Soils as Modified by the NCDOT

AASHTO T265: Laboratory Determination of Moisture Content of Soils
AASHTO T90: Determining the Plastic Limit & Plasticity Index of Soils

AASHTO M 145: The Classification of Soils and Soil Aggregate Mixtures for Highway Construction Purposes

AASHTO M 145: The Classification of Soils and Soil Aggregate Mixtures for Highway Construction Purposes

Technician Name:

Technical Responsibility:

AASHTO T89: Determining the Liquid Limit of Soils

Mal Krajan

Mal Krajan

Certification #

104-01/-0703

<u>Laboratory Manager</u> Signature Technician Name:

References:

Technical Responsibility:

Mal Krajan

Mal Krajan

Certificatio Signature

104-01-0703

Laboratory Manager
Signature

Particle Size Analysis of Soils



Particle Size Analysis of Soils

AASHTO T 88 as Modified by NCDOT



AASHTO T 88 as Modified by NCDOT

Report Date: 9/17/2009 Test Date(s): 09/14 - 09/16/2009

S&ME Project #: Project Name:

Client Name:

1051-09-207

Raleigh, North Carolina

NCDOT

33794.1.1

Bridge 73 over Lake Lucas on SR1518 Randolph Co.

9/25/2009 9/24 - 9/25/09

Bridge 73 over Lake Lucas on SR1518 Randolph Co. Client Name: NCDOT

S&ME Project #:

Project Name:

Client Address:

State Project #:

1051-09-207

Raleigh, North Carolina 33794.1.1 F.A. Project No: BRZ-1518(2)

TIP NO: B-4610

Client Address: State Project #:

F.A. Project No: BRZ-1518(2)

TIP NO: B-4610

Sample #: SS-4 Boring #: B1-A Sample Date: 09/09 - 09/11/2009 Location: 18+55 -L-Offset: 24' LT

Boring #: B1-B Location: 18+57 -L- Sample #: SS-5 Offset: 3' RT Depth (ft): 0.0-1.5

Sample Date: 9/2/09

Depth (ft): 3.4' - 4.9' Sample Description:

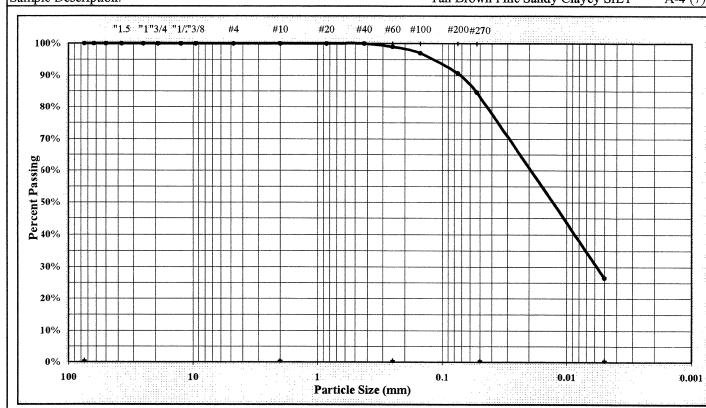
Tan Brown Fine Sandy Clayey SILT A-4 (7)

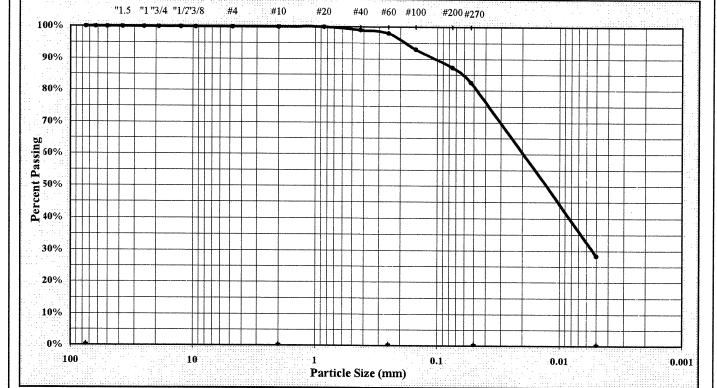
Sample Description:

Gray Fine Sandy Clayey SILT A-4(7)

Report Date:

Test Date(s):





As Defin	ed by NCDOT		e Sand	< 0.25 mm and > 0.05 mm	As Defin	ed by NCDOT	Fine Sand	< 0.25 mm and > 0.05 mm
Gravel	< 75 mm and > 2.0		Silt	< 0.05 and > 0.005 mm	Gravel	< 75 mm and > 2.00 mm	Silt	< 0.05 and > 0.005 mm
Coarse Sand	< 2.00 mm and > 0.	.25 mm	Clay	< 0.005 mm	Coarse Sand	< 2.00 mm and > 0.25 mm	Clay	< 0.005 mm
Maximum Pa	rticle Size #10	Coarse Sand	1%	Silt 58%	Maximum Pa	article Size #4 (Coarse Sand 2%	Silt 54%
	Gravel 0%	Fine Sand	14%	Clay 27%		Gravel 0%	Fine Sand 16%	Clay 28%
Apparent Relativ	ve Density	Moisture Content		% Passing #200 90.7%	Apparent Relati	ve Density Moist	ure Content	% Passing #200 87.2%

Liquid Limit 29 Plastic Limit 20 Plastic Index Liquid Limit 30 Plastic Limit 21 Plastic Index Soil Mortar (-#10 Sieve) Soil Mortar (-#10 Sieve)

Coarse Sand 1%	Fine Sand 14%	Silt 58%	Clay 27%	Coarse Sand 2%	Fine Sand 16%	Silt 54%	Clay 28%
Description of Sand & Gravel Particles:	Rounded □ Angular □	Hard & Durable □ Soft □	Weathered & Friable	Description of Sand & Gravel Particles:	Rounded	ar □ Hard & Durable □ Soft □	Weathered & Friable □
	ngth of Dispersion Period: 1 min.	Dispersing Agent: Sodium Hexameta	aphosphate: 40 g./ Liter	Mechanical Stirring Apparatus (A)	ength of Dispersion Period: 1 min.	Dispersing Agent: Sodium Hexamo	etaphosphate: 40 g./ Liter
References: AASHTO T88: Particle Size And	alysis of Soils as Modified by the NCDOT			References: AASHTO T88: Particle Size An	alysis of Soils as Modified by the NO	CDOT	
AASHTO T87: Dry Preparation of Disturbed Soil	and Soil Aggregate Samples for Test	AASHTO T265: Laboratory Determination	on of Moisture Content of Soils	AASHTO T87: Dry Preparation of Disturbed Soil	and Soil Aggregate Samples for Test	AASHTO T265: Laboratory Determin	ation of Moisture Content of Soils
AASHTO T89: Determining the Liquid Limit of S	Soils AASH	ITO T90: Determining the Plastic Limit & Plast		AASHTO T89: Determining the Liquid Limit of S		AASHTO T90: Determining the Plastic Limit & P	

3201 Spring Forest Road, Raleigh, N.C. 27616

AASHTO T89: Determining the Liquid Limit of Soils AASHTO M 145: The Classification of Soils and Soil Aggregate Mixtures for Highway Construction Purposes

> Technician Name: Mal Krajan

Laboratory Manager

Technical Responsibility:

Mal Krajan

AASHTO M 145: The Classification of Soils and Soil Aggregate Mixtures for Highway Construction Purposes

Laboratory Manager

AASHTO T90: Determining the Plastic Limit & Plasticity Index of Soils

<u>104-01/</u>0703

% Passing #200

87.2%

Technician Name:

Technical Responsibility:

Mal Krajan

Mal Krajan

Particle Size Analysis of Soils



Particle Size Analysis of Soils AASHTO T 88 as Modified by NCDOT

09/04 - 09/10/2009

AASHTO T 88 as Modified by NCDOT

1051-09-207 S&ME Project #: Bridge 73 over Lake Lucas on SR1518 Randolph Co.

Report Date: 9/25/2009 Test Date(s):

9/24 - 9/25/09 Project Name:

S&ME Project #: Bridge 73 over Lake Lucas on SR1518 Randolph Co.

1051-09-207

Report Date: Test Date(s): 9/11/2009

Client Name: NCDOT

Project Name:

Client Address:

State Project #:

Raleigh, North Carolina

10

33794.1.1

F.A. Project No: BRZ-1518(2) TIP NO: B-4610 State Project #:

Client Name:

Client Address: Raleigh, North Carolina

NCDOT

33794.1.1 F.A. Project No: BRZ-1518(2)

TIP NO: B-4610

Boring #: B2-A Sample #: SS-6 Sample Date: 9/2/09 19+45 -L-Location:

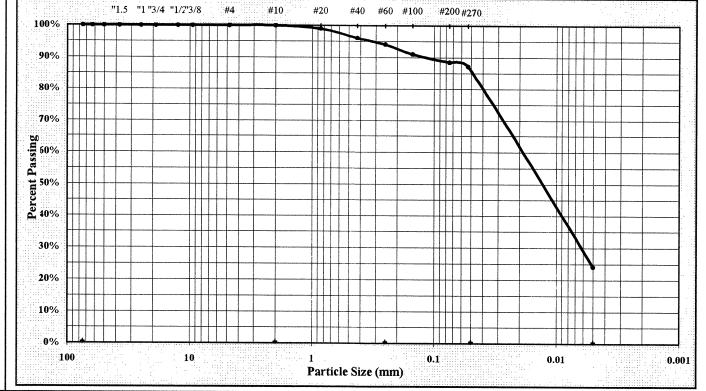
Offset: 24' LT Depth (ft): 0.0 - 1.5 ft.

Boring #: EB2-B Sample #: SS-9 Sample Date: 09/01 - 09/03/2009 Location: 20+20 -L-Offset: 3' RT Depth (ft): 1.0' - 2.5' Sample Description: Brown-Red Coarse to Fine Sandy Silty CLAY A-7-5 (15)

Sample Description: Brown Coarse to Fine Sandy Silty CLAY A-6 (9)



Particle Size (mm)



As Defi	ned by NCDOT	Fine Sand	< 0.25 mm and > 0.05 mm	As Defir	ned by NCDOT	Fine Sand	< 0.25 mm and > 0.05 mm
Gravel	< 75 mm and > 2.00 mm	Silt	< 0.05 and > 0.005 mm	Gravel	< 75 mm and > 2.00 mm	Silt	< 0.05 and > 0.005 mm
Coarse Sand	< 2.00 mm and > 0.25 mm	Clay	< 0.005 mm	Coarse Sand	< 2.00 mm and > 0.25 mm	Clay	< 0.005 mm
Maximum Pa	article Size #4	Coarse Sand 5%	Silt 56%	Maximum P	article Size #4	Coarse Sand 6%	Silt 63%
	Gravel 1%	Fine Sand 12%	Clay 26%		Gravel 0%	Fine Sand 70/	Clara 240/

Gravel Fine Sand Clay 24% Apparent Relative Density Moisture Content % Passing #200 85.9% Apparent Relative Density Moisture Content % Passing #200 88.4% Liquid Limit 33 Plastic Limit 22 Plastic Index 11 Liquid Limit 45 Plastic Limit 31 Plastic Index 14

Soil Mortar (-#10 Sieve)		Soil Mortar (-#10 Sieve)	_
		` '	

Coarse Sand 5%	Fine Sand 13%	Silt 56%	Clay 26%	Coarse Sand 6%	Fine Sand 7%	Silt	63%	Clay 24%
Description of Sand & Gravel Particle	es: Rounded	Hard & Durable □ Soft □	Weathered & Friable	☐ Description of Sand & Gravel Particles	: Rounded 🗆 A	angular 🗆 Hard & Durabl	le 🗆 Soft 🗆 Weat	hered & Friable
Mechanical Stirring Apparatus (A)	Length of Dispersion Period: 1 min.	Dispersing Agent: Sodium Hexamet	aphosphate: 40 g./ Liter	Mechanical Stirring Apparatus (A)	Length of Dispersion Period: 1	min. Dispersing Agent:	Sodium Hexametaphospha	te: 40 g./ Liter
References: AASHTO T88: Particle Size Analysis of Soils as Modified by the NCDOT References: AASHTO T88: Particle Size Analysis of Soils as Modified by the NCDOT								
AASHTO T87: Dry Preparation of Disturbed Soil and Soil Aggregate Samples for Test AASHTO T265: Laboratory Determination of Moisture Content of Soils				AASHTO T87: Dry Preparation of Disturbed Soil and Soil Aggregate Samples for Test AASHTO T265: Laboratory Determination of Moisture Content of Soils				
AASHTO T89: Determining the Liquid Limit of Soils AASHTO T90: Determining the Plastic Limit & Plasticity Index of Soils			AASHTO T89: Determining the Liquid Limit of Soils AASHTO T90: Determining the Plastic Limit & Plasticity Index of Soils					
AASHTO M 145: The Classification of Soils and Soil Aggregate Mixtures for Highway Construction Purposes				AASHTO M 145: The Classification of Soils and Soil Aggregate Mixtures for Highway Construction Purposes				

Technician Name:

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Mal Krajan

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Laboratory Manager

Technician Name:

Technical Responsibility:

Mal Krajan

Mal Krajan

Laboratory Manager

Technical Responsibility: Mal Krajan

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3201 Spring Forest Road, Raleigh, N.C. 27616

PHOTOGRAPHIC RECORD Replacement of Bridge No. 73 Over Lake Lucas on SR 1518



Photograph No. 1:
This photograph was taken from the south approach, along the centerline of the -L- alignment, looking northwest.



Photograph No. 2: This photograph was taken from the centerline of the -L- alignment, looking west, across proposed End Bent No. 1.



Photograph No. 3:
This photograph was taken from the centerline of the -L- alignment, looking east, across End Bent No. 1.



Photograph No. 4:
This photograph was taken from the centerline of the -L- alignment, looking west, across proposed Interior Bent No. 1.

PHOTOGRAPHIC RECORD Replacement of Bridge No. 73 Over Lake Lucas on SR 1518



Photograph No. 5: This photograph was taken from the centerline of the -L- alignment, looking east, across proposed Interior Bent No. 1.



Photograph No. 6: This photograph was taken from the existing bridge, looking west (downstream).



Photograph No. 7: This photograph was taken from the existing bridge, looking east (upstream).



Photograph No. 8:
This photograph was taken from the centerline of the -L- alignment, looking west, across proposed Interior Bent No. 2.



Photograph No. 9: This photograph was taken from the centerline of the -L- alignment, looking east, across proposed Interior Bent No. 2.



Photograph No. 10:
This photograph was taken from the centerline of the -L- alignment, looking west, across proposed End Bent No. 2.



Photograph No. 11: This photograph was taken from the centerline of the -L- alignment, looking east, across proposed End Bent No. 2.



Photograph No. 12:
This photograph was taken from the north approach, along the centerline of the -L- alignment, looking southeast.