

PROJECT SPECIAL PROVISIONS

SCOPE OF WORK

Location and Description of Bridge

Bridge No. 7 Tyrrell County was built in 1960 and is located on US 64 across the Alligator River between the towns of Columbia and East Lake, NC. The bridge has an overall length of 1492 feet and consists of 292 approach spans of variable length consisting of reinforced deck girders and prestressed concrete girders with a concrete deck; and a moveable span section consisting of two 131' steel spans with concrete filled steel grid deck.

Description of Work

This work shall consist of furnishing all labor, materials and equipment to repair concrete spalls on caps, girders, diaphragms, and under deck areas; install reinforced concrete pile jackets around deteriorated concrete and steel H piles, remove and replace existing failed concrete jackets; and clean and paint the structural steel portion of the moveable span as shown in the contract documents and plans. Contractor shall provide all necessary access; boats, underdeck platforms, scaffolding, ladders, etc.; provide all traffic control (both vehicular and navigational); coordinate all navigation channel work with the US Coast Guard; provide all staging area, material storage, boat storage and boat access; provide environmental controls to limit loss of materials into water and air; jacking equipment, sawing equipment, and chipping equipment; and all else necessary to complete the work.

The contractor shall be responsible for fulfilling all requirements of the NCDOT Standard Specifications for Roads and Structures dated July 2006, except as otherwise specified herein.

SECURING OF VESSELS

(10-12-01)

Secure vessels in accordance with Section 107 of the Standard Specifications and the following provision.

When utilizing barges, tugboats or other vessels, take all necessary precautions to ensure that such vessels are securely anchored or moored when not in active operation. Take all necessary measures to ensure that the vessels are operated in a manner that avoids damage to or unnecessary contact with bridges and other highway structures and attachments. If severe weather conditions are anticipated, or should be anticipated through reasonable monitoring of weather forecasts, take additional measures to protect bridges and other highway structures and attachments from extreme conditions. The Contractor is strictly liable for damages to any bridge or other highway structure or attachment caused by a vessel owned or controlled by the Contractor. The Contractor is also liable to third parties for property damages and loss of revenue caused by vessels under the Contractor's control.

COORDINATION WITH THE U. S. COAST GUARD

(SPECIAL)

At no time during work will the waterway be closed or narrowed to navigation without prior approval from the Coast Guard. The contractor is required to maintain close and regular contact with the Coast Guard, Sector North Carolina to keep them informed to activities in the waterway with Steve Lyons at (252)-247-4525 or email Stephen.w.lyons2@uscg.mil. Also must contact the 5th Coast Guard District with Bill Brazier at (757) 398-6422 or email at Bill.H.Brazier@uscg.mil.

All waterway narrowing or closures shall be requested in writing and shall be received by the District Commander of the Coast Guard at least 30 days in advance of the closure so that the appropriate marine notifications can be made.

All work shall be conducted so that free navigation of the waterway is not unreasonably interfered with and the present navigable depths are not impaired. Timely notice of any and all events that affect navigation shall be given to the District Commander during the work on the moveable span. The channel shall be promptly cleared of all obstructions placed therein or caused by the contractor.

WORK IN, OVER OR ADJACENT TO NAVIGABLE WATERS: (SPECIAL)

All work in, over, or adjacent to navigable waters shall have no adverse effect on navigation of the waterway including traffic flow, navigational depths, and horizontal and vertical clearances without approval from the U.S. Coast Guard, U.S. Army Corps of Engineers, or other authority having jurisdiction.

The Contractor shall prepare drawings necessary to obtain any permits which may be required for his operations including but not limited to excavation and dumping, constructing wharves, piers, ramps, and other structures connecting to bank or shore, and drawings for constructing falsework, cofferdams, sheeting, temporary bridges, and any other construction within the waterway. Submittals shall show locations of such work with respect to the navigational opening. The Contractor shall coordinate the submittal of drawings with the Engineer.

All construction shall progress and be maintained in a safe and timely manner. Temporary construction facilities shall be removed completely and promptly upon discontinuation of their useful purpose.

The Contractor shall immediately notify the appropriate authorities and take corrective measures as needed when any situation occurs that imposes a threat to the public. He shall also immediately correct any acts or occurrences that contradict or violate any requirements in the plans, special provisions, or permits when corrective measures can be performed in a safe manner. The Contractor shall notify the appropriate authorities when such corrective measures cannot be performed in a safe manner.

All costs incurred by the Contractor in complying with the above requirements shall be included in the prices bid for the various pay items and no additional payment will be made.

ACCESS/TRAFFIC CONTROL

Repairs to the Alligator River Bridge can be accomplished from the top side or from underneath. Means and methods are the Contractor's responsibility and will not be established by NCDOT. All access either from above or below is considered incidental to the other items of work and the cost of providing access shall be included in the unit prices bid for the various pay items of the work.

The Contractor shall provide all traffic control for this project in accordance with the latest MUTCD and NCDOT standards. Contractor shall also provide all notices, signs, buoys, lighting, communication equipment, and all else to provide safe passage to boat traffic.

No more than two lane closures of 1000' length each are permitted on the bridge at any time, and simultaneous lane closures shall be in the same direction of traffic (either eastbound or westbound). If two work zones are utilized, separate the lane closures by a minimum of ½ mile. If the two work zones are within ½ mile of each other, incorporate them into a single work zone.

When conducting work related to the repair to piles, caps, diaphragms, under deck, girders, and piles, the contractor shall adhere to the time restrictions associated with ICT#1.

When conducting work related to cleaning and painting, the removable span, the contractor shall adhere to the restrictions associated with ICT#2 and ICT#3.

Submit traffic control plans to the Engineer for approval prior to beginning work.

No separate payment will be made for traffic control, as it is considered incidental to the other items of work and the cost of providing traffic control shall be included in the unit prices bid for the various pay items of the work.

SUBMITTAL OF WORKING DRAWINGS

SPECIAL

General

Submit working drawings in accordance with Article 105-2 of the *Standard Specifications* and this provision. For this provision, "submittals" refers to only those listed in this provision. The list of submittals contained herein does not represent a list of required submittals for the project. Submittals are only necessary for those items as required by the contract. **Make submittals that are not specifically noted in this provision directly to the Resident Engineer.**

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If a submittal contains variations from plan details or specifications or significantly affects project cost, field construction or operations, discuss the submittal with and submit all copies to the Resident Engineer. State the reason for the proposed variation in the submittal. To minimize review time, make sure all submittals are complete when initially submitted. Provide a contact name and information with each submittal. Direct any questions regarding submittal requirements to the Resident Engineer or State Bridge Management Unit.

In order to facilitate in-plant inspection by NCDOT and approval of working drawings, provide the name, address and telephone number of the facility where fabrication will actually be done if different than shown on the title block of the submitted working drawings. This includes, but is not limited to, precast concrete items, prestressed concrete items and fabricated steel or aluminum items.

Addresses and Contacts

Mail submittals to:

Mr. Rick Nelson, PE
Asst. State Bridge Management Engineer
NC Dept. of Transportation
State Bridge Management Unit
4809 Beryl Drive
Raleigh, NC 27606
Fax: 919.733.2348
Ph: 919.733.4362
Email: enelson@ncdot.gov

Submittal Copies

Furnish one complete copy of each submittal, including all attachments, to the Resident Engineer. At the same time, submit the number of hard copies shown below of the same complete submittal directly to the Structure Design Unit and/or the Geotechnical Engineering Unit.

The table below covers "Structure Submittals". The Resident Engineer will receive review comments and drawing markups for these submittals from the State Bridge Management Unit.

Unless otherwise required, submit one set of supporting calculations to either the Structure Design Unit or the Geotechnical Engineering Unit unless both units require submittal copies in which case submit a set of supporting calculations to each unit. Provide additional copies of any submittal as directed by the Engineer.

STRUCTURE SUBMITTALS

Submittal	Copies Required by SBMU	Copies Required by Geotechnical Engineering Unit	Contract Reference Requiring Submittal ¹
Falsework & Forms ² (substructure)	5	0	Article 420-3 & "Falsework and Formwork"
Falsework & Forms (superstructure)	5	0	Article 420-3 & "Falsework and Formwork"
Placement of Equipment on Structures (cranes, blasting/painting equip., etc.)	5	0	Article 420-20
Painting Platforms and Containment	5	0	SP

FOOTNOTES

1. References are provided to help locate the part of the contract where the submittals are required. References in quotes refer to the provision by that name. Articles and subarticles refer to the *Standard Specifications*.
2. Submittals for these items are necessary only when required by a note on plans.
3. Submittals for these items may not be required. A list of pre-approved sequences is available from the producer or the Materials & Tests Unit.
4. The fabricator may submit these items directly to the Structure Design Unit.
5. The two sets of preliminary submittals required by Article 1072-10 of the *Standard Specifications* are not required for these items.
6. Submittals for Fabrication Drawings are not required. Submittals for Catalogue Cuts of Proposed Material are required. See Section 5.A of the referenced provision.
7. Submittals are necessary only when the top slab thickness is 18" or greater.

FALSEWORK AND FORMWORK

(7-18-06)

Description

Use this Special Provision as a guide to develop temporary works submittals required by the Standard Specifications or other provisions; no additional submittals are required herein. Such temporary works include, but are not limited to, falsework and formwork.

Falsework is any temporary construction used to support the permanent structure until it becomes self-supporting. Formwork is the temporary structure or mold used to retain plastic or fluid concrete in its designated shape until it hardens. Access scaffolding is a temporary structure that functions as a work platform that supports construction personnel, materials, and tools, but is not intended to support the structure. Scaffolding systems that are used to temporarily support permanent structures (as opposed to functioning as work platforms) are considered to be falsework under the definitions given. Shoring is a component of falsework such as horizontal, vertical, or inclined support members. Where the term "temporary works" is used, it includes all of the temporary facilities used in bridge construction that do not become part of the permanent structure.

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Design and construct safe and adequate temporary works that will support all loads imposed and provide the necessary rigidity to achieve the lines and grades shown on the plans in the final structure.

Materials

Select materials suitable for temporary works; however, select materials that also ensure the safety and quality required by the design assumptions. The Engineer has authority to reject material on the basis of its condition, inappropriate use, safety, or nonconformance with the plans. Clearly identify allowable loads or stresses for all materials or manufactured devices on the plans. Revise the plan and notify the Engineer if any change to materials or material strengths is required.

Design Requirements

Working Drawings:

Provide working drawings for items as specified in the contract, or as required by the Engineer, with design calculations and supporting data in sufficient detail to permit a structural and safety review of the proposed design of the temporary work.

When concrete placement is involved, include data such as the drawings of proposed sequence, rate of placement, direction of placement, and location of all construction joints. Submit the number of copies as called for by the contract.

When required, have the drawings and calculations prepared under the guidance of, and sealed by, a North Carolina Registered Professional Engineer who is knowledgeable in temporary works design.

Design falsework and formwork requiring submittals in accordance with the 1995 AASHTO Guide Design Specifications for Bridge Temporary Works except as noted herein.

Wind Loads:

Table 2.2 of Article 2.2.5.1 is modified to include wind velocities up to 110 mph (177 km/hr). In addition, Table 2.2A is included to provide the maximum wind speeds by county in North Carolina.

Table 2.2 - Wind Pressure Values

Height Zone feet (m) above ground	Pressure, lb/ft ² (kPa) for Indicated Wind Velocity, mph (km/hr)				
	70 (112.7)	80 (128.7)	90 (144.8)	100 (160.9)	110 (177.0)
0 to 30 (0 to 9.1)	15 (0.72)	20 (0.96)	25 (1.20)	30 (1.44)	35 (1.68)
30 to 50 (9.1 to 15.2)	20 (0.96)	25 (1.20)	30 (1.44)	35 (1.68)	40 (1.92)
50 to 100 (15.2 to 30.5)	25 (1.20)	30 (1.44)	35 (1.68)	40 (1.92)	45 (2.15)
over 100 (30.5)	30 (1.44)	35 (1.68)	40 (1.92)	45 (2.15)	50 (2.39)

Time of Removal:

The following requirements replace those of Article 3.4.8.2.

Do not remove forms until the concrete has attained strengths required in Article 420-16 of the Standard Specifications and these Special Provisions.

Do not remove forms until the concrete has sufficient strength to prevent damage to the surface.

Table 2.2A - Steady State Maximum Wind Speeds by Counties in North Carolina

COUNTY	25 YR (mph) (km/hr)	COUNTY	25 YR (mph) (km/hr)	COUNTY	25 YR (mph) (km/hr)
Alamance	70 (112.7)	Franklin	70 (112.7)	Pamlico	100 (160.9)
Alexander	70 (112.7)	Gaston	70 (112.7)	Pasquotank	100 (160.9)
Alleghany	70 (112.7)	Gates	90 (144.8)	Pender	100 (160.9)
Anson	70 (112.7)	Graham	80 (128.7)	Perquimans	100 (160.9)
Ashe	70 (112.7)	Granville	70 (112.7)	Person	70 (112.7)
Avery	70 (112.7)	Greene	80 (128.7)	Pitt	90 (144.8)
Beaufort	100 (160.9)	Guilford	70 (112.7)	Polk	80 (128.7)
Bertie	90 (144.8)	Halifax	80 (128.7)	Randolph	70 (112.7)
Bladen	90 (144.8)	Harnett	70 (112.7)	Richmond	70 (112.7)
Brunswick	100 (160.9)	Haywood	80 (128.7)	Robeson	80 (128.7)
Buncombe	80 (128.7)	Henderson	80 (128.7)	Rockingham	70 (112.7)
Burke	70 (112.7)	Hertford	90 (144.8)	Rowan	70 (112.7)
Cabarrus	70 (112.7)	Hoke	70 (112.7)	Rutherford	70 (112.7)
Caldwell	70 (112.7)	Hyde	110 (177.0)	Sampson	90 (144.8)
Camden	100 (160.9)	Iredell	70 (112.7)	Scotland	70 (112.7)
Carteret	110 (177.0)	Jackson	80 (128.7)	Stanley	70 (112.7)
Caswell	70 (112.7)	Johnston	80 (128.7)	Stokes	70 (112.7)
Catawba	70 (112.7)	Jones	100 (160.9)	Surry	70 (112.7)
Cherokee	80 (128.7)	Lee	70 (112.7)	Swain	80 (128.7)
Chatham	70 (112.7)	Lenoir	90 (144.8)	Transylvania	80 (128.7)
Chowan	90 (144.8)	Lincoln	70 (112.7)	Tyrell	100 (160.9)
Clay	80 (128.7)	Macon	80 (128.7)	Union	70 (112.7)
Cleveland	70 (112.7)	Madison	80 (128.7)	Vance	70 (112.7)
Columbus	90 (144.8)	Martin	90 (144.8)	Wake	70 (112.7)
Craven	100 (160.9)	McDowell	70 (112.7)	Warren	70 (112.7)
Cumberland	80 (128.7)	Mecklenburg	70 (112.7)	Washington	100 (160.9)
Currituck	100 (160.9)	Mitchell	70 (112.7)	Watauga	70 (112.7)
Dare	110 (177.0)	Montgomery	70 (112.7)	Wayne	80 (128.7)
Davidson	70 (112.7)	Moore	70 (112.7)	Wilkes	70 (112.7)
Davie	70 (112.7)	Nash	80 (128.7)	Wilson	80 (128.7)
Duplin	90 (144.8)	New Hanover	100 (160.9)	Yadkin	70 (112.7)
Durham	70 (112.7)	Northampton	80 (128.7)	Yancey	70 (112.7)
Edgecombe	80 (128.7)	Onslow	100 (160.9)		
Forsyth	70 (112.7)	Orange	70 (112.7)		

Note on the working drawings any anchorages, connectors, inserts, steel sleeves or other such devices used as part of the falsework or formwork that remains in the permanent structure. If the plan notes indicate that the structure contains the necessary corrosion protection required for a Corrosive Site, epoxy coat, galvanize, metallize or otherwise protect these devices as directed by the Engineer. Any coating required by the Engineer will be considered incidental to the various pay items requiring temporary works.

Review and Approval

The Engineer is responsible for the review and approval of temporary works' drawings. Submit the working drawings sufficiently in advance of proposed use to allow for their review, revision (if needed), and approval without delay to the work.

Do not start construction of any temporary work for which working drawings are required until the drawings have been approved. Such approval does not relieve the Contractor of the responsibility for the accuracy and adequacy of the working drawings.

The time period for review of the working drawings does not begin until complete drawings and design calculations, when required, are received by the Engineer.

On the drawings, show all information necessary to allow the design of any component to be checked independently as determined by the Engineer.

If requested by the Engineer, submit with the working drawings manufacturer's catalog data listing the weight of all construction equipment that will be supported on the temporary work. Show anticipated total settlements and/or deflections of falsework and forms on the working drawings. Include falsework footing settlements, joint take-up, and deflection of beams or girders. Falsework hangers that support concentrated loads and are installed at the edge of thin top flange concrete girders (such as bulb tee girders) shall be spaced so as not to exceed 75% of the manufacturer's stated safe working load. Use of dual leg hangers (such as Meadow Burke HF-42 and HF-43) are not allowed. Design the falsework and forms supporting deck slabs and overhangs on girder bridges so that there will be no differential settlement between the girders and the deck forms during placement of deck concrete.

Construction Requirements

All requirements of Section 420 of the Standard Specifications apply.

Construct temporary works in conformance with the approved working drawings. Ensure that the quality of materials and workmanship employed is consistent with that assumed in the design of the temporary works. Do not weld falsework members to any portion of the permanent structure unless approved. Show any welding to the permanent structure on the approved construction drawings.

ADHESIVELY ANCHORED ANCHOR BOLTS OR DOWELS

(6-11-07)

General

Installation and Testing of Adhesively anchored anchor bolts and dowels shall be in accordance with Section 420-13, 420-21 and 1081-1 of the Standard Specifications except as modified in this provision.

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Installation

Installation of the adhesive anchors shall be in accordance with manufacturer's recommendations and shall occur when the concrete is above 40 degrees Fahrenheit and has reached its 28 day strength.

The anchors shall be installed before the adhesive's initial set ('gel time').

Field Testing

Replace the third paragraph of Section 420-13 (C) with the following:

"In the presence of the Engineer, field test the anchor bolt or dowel in accordance with the test level shown on the plans and the following:

Level One Field testing: Test a minimum of 1 anchor but not less than 10% of all anchors to 50% of the yield load shown on the plans. If less than 60 anchors are to be installed, install and test the required number of anchors prior to installing the remaining anchors. If more than 60 anchors are to be installed, test the first 6 anchors prior to installing the remaining anchors, then test 10% of the number in excess of 60 anchors.

Level Two Field testing: Test a minimum of 2 anchors but not less than 10% of the all anchors to 80% of the yield load shown on the plans. If less than 60 anchors are to be installed, install and test the required number of anchors prior to installing the remaining anchors. If more than 60 anchors are to be installed, test the first 6 anchors prior to installing the remaining anchors, then test 10% of the number in excess of 60 anchors.

Testing should begin only after the Manufacturer's recommended cure time has been reached. For testing, apply and hold the test load for three minutes. If the jack experiences any drop in gage reading, the test must be restarted. For the anchor to be deemed satisfactory, the test load must be held for three minutes with no movement or drop in gage reading."

Removal and Replacement of Failed Test specimens:

Remove all anchors and dowels that fail the field test without damage to the surrounding concrete. Redrill holes to remove adhesive bonding material residue and clean the hole in accordance with specifications. For reinstalling replacement anchors or dowels, follow the same procedures as new installations. Do not reuse failed anchors or dowels unless approved by the Engineer.

Usage

The use of adhesive anchors for overhead installments is not permitted without written permission from the Engineer.

Basis of Payment

No separate measurement or payment will be made for furnishing, installing, and testing anchor bolts/dowels. Payment at the contract unit prices for the various pay items will be full compensation for all materials, equipment, tools, labor, and incidentals necessary to complete the work.

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REPAIRS TO CAPS, DIAPHRAMS, AND UNDER DECK

SPECIAL

Description

Work includes removal of concrete in spalled areas of the existing caps, diaphragms, and under deck areas in reasonably close conformity with the lines, depth, and details shown on the plans, described herein and as established by the Engineer. This work also includes straightening, cleaning, and replacement of reinforcing steel, dowelling new reinforcing steel, removing all loose materials, removing and disposing of debris, applying repair material, and protecting adjacent areas of the bridge and environment from material leakage. The repair material shall be one of the below described materials unless otherwise noted in the plans or provisions.

The location and extent of repairs shown on the plans described herein are general in nature. The Engineer determines the exact extent of removal in the field based on an evaluation of the condition of the exposed surfaces.

Repair, to the Engineer's satisfaction, any portion of the structure that is damaged from construction operations. No extra payment is provided for these repairs.

A. Polymer Modified Concrete Repair Material

Materials

Repair material shall be polymer modified cement mortar for vertical or overhead applications and shall be suitable for applications in marine environments. Material shall be approved for use by NCDOT. Submit repair material to the Engineer for review and approval prior to beginning the work. Color of repair material shall be concrete gray.

Surface Preparation

Prior to the application of repair mortar, square up edges in repair areas, thoroughly clean surfaces to be repaired and remove all loose materials. Remove grease, wax, salt, and oil contaminants by scrubbing with an industrial grade detergent or degreasing compound followed by a mechanical cleaning. Remove weak or deteriorated concrete to sound concrete by bush hammering, gritblasting, scarifying, waterblasting, or other approved methods. Remove dirt, dust, laitance and curing compounds by gritblasting, sanding, or etching with 15% hydrochloric acid. Only acid etch if approved and follow it by scrubbing and flushing with copious amounts of clean water. Check the cleaning using moist pH paper. Water cleaning is complete when the paper reads 10 or higher. Follow all mechanical cleaning with vacuum cleaning.

Application

When surface preparation is completed, mix and apply repair mortar in accordance with manufacturer's recommendations. Use aggregate that is washed, kiln-dried, and bagged. Apply bonding agent to all repair areas immediately prior to placing repair mortar. Repair areas shall be formed unless otherwise approved by the Engineer. Form areas to establish the original neat lines of the member being repaired.

Apply repair mortar to damp surfaces only when approved. In such instances, remove all free water by air-blasting. After applying the repair mortar, remove excessive material and provide a smooth, flush surface.

B. Shotcrete Repair Material

Shotcrete

Qualification of Shotcrete Contractor

Shotcrete Contractors are not acceptable as a Prime Contractor or Subcontractor unless all of following requirements are met:

The Shotcrete Contractor furnishes proof that his or her company has a minimum of 5 years experience in shotcrete repair work on jobs of similar size and character.

The Shotcrete Contractor furnishes five references who were responsible for supervision of similar projects and testifies to the successful completion of these projects. Include name, address, and telephone number. Prior to starting work, the Contractor's nozzle men are required to pass a test demonstrating their competence. This test is conducted at the job site and approximates actual working conditions as near as possible. For test requirements, see ACI 506.3R, Chapters 2.5 and 3. Only workmanship demonstration is tested.

General

When shotcreting, meet all requirements of ACI 506.2, published by the American Concrete Institute, Detroit, Michigan, except as modified by the requirements of this Special Provision.

Prior to beginning any repair work, provide a sufficiently sized temporary work platform at each repair location as required. Design steel members meeting the requirements of the American Institute of Steel Construction Manual. Design timber members in accordance with the "National Design Specification for Stress-Grade Lumber and Its Fastenings" of the National Forest Products Association. Submit the platform structure design for review and approval. Do not install the platform until the design is approved. Do not drill holes into the superstructure. When the platform is removed, remove all anchorages made in the substructure and repair the substructure at no additional cost to the Department.

Material

Use materials conforming to the requirements of the applicable sections of the Standard Specifications and the following provisions:

Use Type II Cement.

Replace ten percent by weight of the cement with silica fume.

Do not use admixtures without approval.

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Produce shotcrete cores with a compressive strength of 5000 psi (34.5 MPa) at 28 days. The provisions of ACI 506.2, Section 1.6.3.3, Paragraph 2, do not apply.

Submit the shotcrete mix design, including the source of the material, to the Engineer for acceptance before using it.

Use size 2S or 2MS fine aggregate unless otherwise approved.

Finish

Slightly build up and trim the shotcrete surface to the final surface by cutting with the leading edge of a sharp trowel. Use a rubber float to float any imperfections. Limit work on the finished surface to correcting imperfections caused by trowel cutting.

Testing

Each day shotcreting takes place, have each nozzleman shoot one 18" x 18" x 3" (460 mm x 460 mm x 75 mm) Test Panel. Shoot the panel in the same position as the repair work that is being done. The panel demonstrates whether the shotcrete is being properly applied and furnishes cores for testing compressive strength. Drill three 3" (76 mm) diameter cores from each test panel and also drill cores from the repair areas as directed by the Engineer. Do not take cores from repaired areas until the shotcrete has cured for 7 days. Drill a core that penetrates into the existing substructure concrete at least 2 inches (50 mm). These cores are inspected for delaminations and sand pockets and tested for bond strength and/or compressive strength. If a core taken from a repaired area indicates unsatisfactory application or performance of the shotcrete, take additional cores from the applicable repair area(s) for additional evaluation and testing as directed by the Engineer. No extra payment is provided for drilling extra cores. Patch all core holes in the repaired substructure units to the satisfaction of the Engineer.

All material, sample, and core testing is done by the Materials and Tests Unit of North Carolina Department of Transportation.

Mixture

Mix the shotcrete in the proportions of one part of portland cement to four parts of sand, and as directed by the Engineer.

Measure this mixture by volume in the dry loose state. Check batching equipment daily or at the discretion of the Engineer.

Repair Method and Operations

Prior to starting the repair operation, delineate all surfaces and areas assumed to be deteriorated by visually examining and by sounding the concrete surface with a hammer or any other alternative approved method. The Engineer is the sole judge in determining the limits of deterioration.

Remove all deteriorated concrete to sound concrete with a 17 lb (7.7 kg) (maximum) pneumatic hammer with points that do not exceed the width of the shank or with hand picks or chisels as directed by the Engineer. Do not cut or remove the existing reinforcing steel. Do not remove more existing concrete than required to expose the surface of the sound concrete. Unless specifically directed by the Engineer, do not remove concrete deeper than 6 inches (150 mm) or deeper than 1 inch (25 mm) below the reinforcing steel.

If sound concrete is encountered before existing reinforcing steel is exposed, prepare and repair the surface without removing any more concrete. However, if the reinforcing steel is wholly or partially exposed, remove the deteriorated and/or sound concrete to a minimum clearance of 1 inch (25 mm) all around the reinforcing steel.

Sandblast all exposed concrete surfaces and existing reinforcing steel in repair areas to remove all debris, loose concrete, loose mortar, rust, scale, etc. Use a wire brush to clean all exposed reinforcing steel surfaces facing away from the sandblast nozzle to remove all dust and loose particles.

All material removed becomes the Contractor's. Use an approved method to dispose of the material.

Restore all repaired members, including chamfered edges, as close as practicable to their original "As Built" dimensions and configuration. Provide a minimum of 2" (50 mm) shotcrete cover over reinforcing steel exposed during repair. Finish the shotcrete by cutting the surface to final grade with the leading edge of a trowel.

Provide welded wire fabric at each repair area larger than 1 ft² (0.1 m²). Provide a minimum 2" x 2" (50 mm x 50 mm) - 12 gage galvanized welded wire fabric. Rigidly secure the welded wire fabric to existing steel or to 3/16" (4.76 mm) minimum diameter adequately spaced galvanized hook fasteners to prevent sagging. Encase the welded wire fabric in shotcrete to a minimum depth of 1½ inches (38 mm).

If preferred, use steel or synthetic fiber reinforcement as an alternate to welded wire fabric.

Work only with experienced personnel. Always work under the direction of an experienced superintendent. The superintendent is required to show a certified experience record indicating at least 5 years experience on work of similar type. No nozzleman is deemed experienced unless they have worked on several other jobs similar to that specified herein and have passed the required pre-qualification test listed in this Special Provision.

Before applying the shotcrete to the surface, thoroughly clean the surface of all dirt, grease, oil or foreign matter, and remove all loose or weakened material.

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Wash the roughened existing concrete surface with fresh potable water and an air blast, or with a “stiff” hose stream of fresh water until all loosened materials and salt water spray are removed. Perform this operation 30 minutes to 1 hour prior to applying the shotcrete.

Maximum time allowed between removal of deteriorated concrete and shotcrete application is 5 days. If the time allowance is exceeded it will be necessary to prepare the surface again using the methods described above before shotcrete can be applied.

Apply shotcrete in layers. The properties of the applied shotcrete determine the proper thickness of each layer or lift.

If a work stoppage longer than 2 hours takes place on any shotcrete layer prior to the time it has been built up to required thickness, thoroughly wash the surface with a fresh water stream and air hose as outlined previously, prior to continuing with the remaining shotcrete course. Do not apply shotcrete to a dry surface.

Have the nozzleman hold the nozzle 3 – 4 feet (0.9 to 1.2 m) from the surface being covered in a position that ensures the stream of flowing material strikes at approximately right angles to the surface being covered without excessive impact. Have the nozzleman control the water content so it never exceeds 3½ gallons (13.25 liters) per sack of cement. Direct the nozzlemen to maintain the water at a practicable minimum, dependent on weather conditions, so that the mix properly adheres. Control water content so that it does not become high enough to cause the mix to sag or fall from vertical or inclined surfaces, or to separate in horizontal layers.

Use shooting strips or guide wires that do not entrap rebound sand to bring the finished work to approximate shape. Use guide wires to provide a positive means of checking the total thickness of the shotcrete applied. Remove the guide wires prior to the final finish coat.

Blow or rake off sand that rebounds and does not fall clear of the work, or which collects in pockets in the work, to avoid leaving sand pockets in the shotcrete. Do not reuse rebound material in the work.

Apply shotcrete only when the air temperature is at least 40°F (4°C) and rising, but less than 95°F (35°C). Do not apply shotcrete to frosted surfaces. Maintain shotcrete at a minimum temperature of 40°F (4°C) for 3 days.

Testing Shotcrete Surfaces

Immediately after bringing shotcrete surfaces to final thickness, thoroughly check them for sags, bridging, and other deficiencies. Approximately 3 days after completing the final shotcrete placement, thoroughly test it again with a hammer. At this time, the shotcrete should have sufficient strength for all sound sections to ring sharply. Remove

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and replace any unsound portions of the work found during this 3 day old inspection period, or at any other time prior to the final inspection of the work. No additional compensation is provided for removal and replacement of concrete during or after the 3 day old inspection.

Curing

Begin curing as soon as the finished shotcrete surface withstands the curing operation without damage in accordance with Section 3.7 of ACI 506.2.

Measurement and Payment

Repairs to Caps, Diaphragms, and Under Deck will be measured and paid for at the contract unit price bid per cubic foot and will be full compensation for removal, containment and disposal off-site of unsound concrete including the cost of materials, labor, tools, equipment and incidentals necessary to accomplish removal. Depth will be measured from a place at the original outside concrete face. The Contractor and Engineer will measure repair quantities after removal of unsound concrete and before application of repair material. Such payment will also include the cost of sandblasting, surface cleaning and preparation, cleaning of reinforcing steel, placement of new reinforcing steel and/or dowells, cost of temporary work platform, testing of the soundness of the exposed concrete surface, furnishing and installation of repair mortar material, curing and sampling of concrete, and protection/cleaning of adjacent areas from splatter or leakage.

Reinforcing Steel will be measured and paid for at the contract unit price bid per pound and will be full compensation for furnishing reinforcing steel for replacement of damaged rebar.

Payment will be made under:

Pay Item	Pay Unit
Repairs to Caps, Diaphragms, and Under Deck	Cubic Feet
Reinforcing Steel	Pound

REPAIRS TO PRESTRESSED CONCRETE GIRDERS

SPECIAL

Description

Work includes removal of concrete in spalled areas of the existing prestressed concrete girders in reasonably close conformity with the lines, depth, and details shown on the plans, described herein and as established by the Engineer; removing all loose materials, removing and disposing of debris, applying repair material, and protecting adjacent areas of the bridge and environment from material leakage.

The location and extent of repairs shown on the plans described herein are general in nature. The Engineer determines the exact extent of removal in the field based on an evaluation of the condition of the exposed surfaces.

Repair, to the Engineer's satisfaction, any portion of the structure that is damaged from construction operations. No extra payment is provided for these repairs.

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Materials

Repair material shall be polymer modified cement mortar for vertical or overhead applications and shall be suitable for applications in marine environments. Material shall be approved for use by NCDOT. Submit repair material to the Engineer for review and approval prior to beginning the work. Color of repair material shall be concrete gray.

Surface Preparation

Prior to the application of repair mortar, square up edges in repair areas, thoroughly clean surfaces to be repaired and remove all loose materials. Remove grease, wax, salt, and oil contaminants by scrubbing with an industrial grade detergent or degreasing compound followed by a mechanical cleaning. Remove weak or deteriorated concrete to sound concrete by bush hammering, gritblasting, scarifying, waterblasting, or other approved methods. Remove dirt, dust, laitance and curing compounds by gritblasting, sanding, or etching with 15% hydrochloric acid. Only acid etch if approved and follow it by scrubbing and flushing with copious amounts of clean water. Check the cleaning using moist pH paper. Water cleaning is complete when the paper reads 10 or higher. Follow all mechanical cleaning with vacuum cleaning.

Application

When surface preparation is completed, mix and apply repair mortar in accordance with manufacturer's recommendations. Use aggregate that is washed, kiln-dried, and bagged. Apply bonding agent to all repair areas immediately prior to placing repair mortar. Repair areas shall be formed unless otherwise approved by the Engineer. Form areas to establish the original neat lines of the member being repaired.

Apply repair mortar to damp surfaces only when approved. In such instances, remove all free water by air-blasting. After applying the repair mortar, remove excessive material and provide a smooth, flush surface.

Measurement and Payment

Repairs to Prestressed Concrete Girders will be measured and paid for at the contract unit price bid per cubic foot and will be full compensation for removal, containment and disposal off-site of unsound concrete including the cost of materials, labor, tools, equipment and incidentals necessary to accomplish removal. Depth will be measured from a place at the original outside concrete face. The Contractor and Engineer will measure repair quantities after removal of unsound concrete and before application of repair material. Such payment will also include the cost of sandblasting, surface cleaning and preparation, cleaning of reinforcing steel, cost of temporary work platform, testing of the soundness of the exposed concrete surface, furnishing and installation of repair mortar material, curing and sampling of concrete, and protection/cleaning of adjacent areas from splatter or leakage.

Payment will be made under:

Pay Item Repairs to Prestressed Concrete Girders	Pay Unit Cubic Feet
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PILE JACKETS

SPECIAL

Description

This specification establishes the minimum requirements for furnishing installing permanent outer pile forms, standoffs, steel reinforcement bands and steel bracing collars for the installation of pile jackets for pile repair. It is intended to ensure that the supplier's forms and installation/reinforcing details shall allow the forms to be filled with concrete without failure, and provide durable, corrosion resistant pile protection.

Materials

Forms:

The form shall be fabricated from fiberglass and polyester resins, or other inert materials that are compatible with Portland cement and produce a form with equal levels of corrosion resistance and durability. The inside face of the forms shall have a texture equal to that provided by sandblasting, and shall have no bond inhibiting agents in contact with cementitious materials. Forms shall include polymer standoffs of sufficient number and spacing to maintain a minimum space of 2" between the reinforcing steel and the jacket. Provide forms with dimensions in accordance with the sizing chart shown in the plans. For pumped applications provide ports in accordance with the plans. The minimum allowable thickness of the forms is 1/8". Upon opening to place around a pile, the form shall be capable of returning to its original shape without assistance or damage. It shall have an interlocking joint along one side, which will permit the form to be assembled and sealed in place around the pile. Contractor shall submit form details to the Engineer for approval prior to beginning work.

Transverse joints (if any) shall be of overlapping configuration.

The material furnished must meet the following physical property requirements:

- (a) Water Absorption (ASTM-D570)1% Maximum
- (b) Ultimate Tensile Strength (ASTM-D638)* 9,000 psi Minimum
- (c) Flexural Strength (ASTM-D796)* 16,000 psi Minimum
- (d) Flexural Modulus of Elasticity (ASTM-D790) 700,000 psi Minimum
- (e) Barcol Hardness (ASTM-D2583)30 – 40
- (f) Color Similar to Federal Color Standard 595 Number 36622 – the color shall be integral in the form material.
- (g) Accelerated Weathering ... The fabricated form material shall be subjected to a 500 hour exposure test in a Twin-Carbon-ARC-Weather-Ometer (ASTM G-23, Type D) at an operating temperature of 145°F. Said test to be made at twenty minute cycles consisting of seventeen minutes of

light and three minutes of water spray plus light. At the end of the exposure test the exposed samples shall not show any chipping, flaking or peeling. The test panels shall be prepared from the materials meeting the physical property requirements above, and they shall be in accordance with the manufacturer's recommendations.

“*”= On original specimen whose flat surfaces are not machined to disturb the fiberglass.

Epoxy Gel Sealant

Use an approved marine epoxy gel to adhere the outer jacket seams. The epoxy paste must be a two component epoxy compound, capable of being applied underwater. The ratio of resin component to hardener component is 1:1 by volume. To assist in evaluating the thoroughness of job site mixing, each component must be of sharply contrasting color.

Concrete

Concrete shall meet the requirements of the *Standard Specs* for Drilled Pier Concrete. Use a pea gravel mix suitable for pumped applications. An anti-washout admixture may be used for in water applications. Concrete mix design shall be submitted for approval prior to beginning work.

Reinforcement Bands

Provide reinforcement bands similar to that shown in the plans. The bands shall be reusable, and shall be equipped with quick release fasteners.

Submit details and calculations showing design loads and the placement of the bands on the pile form necessary to reinforce the form against failure from the concrete pressure, or any other loading the form may experience, including its use on battered piles.

Steel Bracing Collar

For forms to be supported on temporary falsework provide a steel bracing collar which will reinforce the bottom of the form and allow connection to the falsework.

Construction Methods

Prior to jacket installation, remove all deteriorated concrete and thoroughly clean pile of marine growth, oil, grease, mud, rust, and any other deleterious material which might prevent proper bonding between the concrete and the pile. Accomplish pile preparation and cleaning by chipping, grit blasting, high pressure water blasting, or by divers using powered rotary abraders. Any method that produces the quality of cleaning necessary to meet the bond requirements of this specification may be considered. When necessary, perform the pile cleaning in 2 phases where active marine growth occurs. In the first phase, a maximum of 7 days before the encapsulation, remove marine growth, oil, grease, rust, and broken concrete, etc. In the second phase, a maximum of 48 hrs before

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placement of concrete in the outer pile jacket, perform a final surface preparation, removing all remaining deleterious substances including micro-organisms.

Place the jacket assembly and position it around the pile in such a manner as to assure that no damage to stand-offs and rebar cage occur. Ensure there will be no detrimental movement of the joints while joint adhesive is curing. The placement of the jacket is to be determined by the location of the affected pile and in accordance with the contract drawings.

Seal the longitudinal and transverse seams, if any, with marine epoxy paste as described above.

Place concrete using a pump or tremie. Free fall placement of concrete will not be allowed if any part of the jacket is submerged below water. For above water applications free fall placement will only be allowed for jacket lengths up to 10'. Cope the top of the repair to drain water.

No tainted water above pH 9.0 will be allowed to discharge from the work site. Monitoring of pH levels inside and outside of the jacket is required during the pumping operation. Perimeter monitoring site should be no more than 10 ft down flow from the work area. If the pH of the water within the jacket exceeds then pump to a container and hold until the pH level returns to 9.0.

Concrete shall attain a minimum strength of 3000 psi prior to removing form work.

Submittals

Submit shop drawings and calculations to the Engineer for approval prior to start of fabrication. Submittal shall include form dimensions, standoffs, pump ports (where applicable), reinforcing cage installation, reinforcement bands, collars, temporary falsework, methods to seal the form, form installation, and sequence of concrete placement.

Material Certification

For materials to be used, the Supplier shall furnish a certificate to the Engineer attesting that the materials meet all the requirements contained herein and that they conform in all respects to the materials subjected to the tests required. Copies of current test reports shall be attached to the certificate. No test report for tests made more than one year prior to shipment will be accepted for the form material.

Measurement and Payment

Concrete Pile Jackets will be measured and paid for at the contract unit price bid per linear foot of concrete encased pile jacket and will be full compensation for removal, containment and disposal off-site of unsound concrete including the cost of materials, labor, tools, equipment and incidentals necessary to accomplish removal; cleaning the pile, installation of reinforcement, jacket installation, falsework; furnishing and

placement of concrete including pumping equipment, pH monitoring, pollution control, and all else required to repair existing deteriorated concrete piles using pile jackets.

H-Pile Jackets will be measured and paid for at the contract unit price bid per linear foot of concrete encased pile jacket and will be full compensation for removal, containment and disposal off-site of unsound concrete including the cost of materials, labor, tools, equipment and incidentals necessary to accomplish removal; cleaning the pile, installation of reinforcement, jacket installation, falsework; furnishing and placement of concrete including pumping equipment, pH monitoring, pollution control, and all else required to repair existing deteriorated steel H-Piles using pile jackets.

Rejackets will be measured and paid for at the contract unit price bid per linear foot of concrete encased pile jacket and will be full compensation for removal, containment and disposal off-site of existing pile jackets, unsound concrete including the cost of materials, labor, tools, equipment and incidentals necessary to accomplish removal; cleaning the pile, installation of reinforcement, jacket installation, falsework; furnishing and placement of concrete including pumping equipment, pH monitoring, pollution control, and all else required to repair existing jackets.

Pay Item	Pay Unit
Concrete Pile Jackets	Linear Feet
H-Pile Jackets	Linear Feet
Rejackets	Linear Feet

CLEANING AND PAINTING EXISTING STRUCTURE

SPECIAL

GENERAL

This work shall consist of furnishing all labor, equipment, and materials to clean and paint the structural steel of the existing bridge. Work includes: removing, containment and disposal of the existing paint system; preparation of the surfaces to be painted; applying the new paint system; and portable lighting.

All structural steel of the bridge shall be painted. All machinery that has previously been painted shall be painted. All machinery and other areas not to be painted shall be protected from blasting and painting. Equipment bearings, seals, and gear faces shall be sealed to keep out blasting medium and paint. Blast the underside of the concrete filled grid floor and paint the exposed grid floor members.

The moveable span consists of two main variable depth girders with plate girder floor beams and W16 I-Beam stringers. The bridge has a clear roadway width of 26'. The existing paint system has red lead primer. The approximate steel area to be painted is 30,935 square feet.

Paint on the bridge (regardless of color) contains red lead and other hazardous constituents. All cleaning and surface preparation activities must prevent dispersion of debris into the environment.

Surface area shown is approximate and may vary from the actual quantity to be painted. The Contractor is responsible for determining the actual area to be painted.

The bridge will be operated by NCDOT during the project.

SPECIALTY ITEMS:

Work Schedule – Prior to beginning work, the Contractor shall submit his work schedule to the Engineer. Schedule shall be kept up to date, with a copy of the revised schedule being provided to the Engineer in a timely manner.

SSPC QP-2 Certification - The existing paint systems include toxic substances such as red lead oxide, which are considered hazardous if improperly removed. Only contractors who are currently SSPC QP-2, Category A certified, and have successfully¹ completed lead paint removal on similar structures within 18 months prior to this bid, may perform this work. **The apparent low bidder or sub-contractor for the apparent low bidder shall complete and submit to the Assistant State Bridge Management Engineer a “Lead Abatement Affidavit” by 12:00 noon of the third day following the opening of bids. This form may be downloaded from:**

http://www.ncdot.gov/doh/operations/dp_chief_eng/maintenance/bridge/

Failure to provide the required affidavit by 12:00 noon of the third day following the opening of bids will cause the bid to be deemed irregular and may be cause for rejection.

Twelve-month Observation Period - The Contractor maintains responsibility for the coating system for a twelve (12) month observation period beginning upon the satisfactory completion of all the work required in the plans or as directed by the Engineer. The Contractor must guarantee the coating system under the payment and performance bond (refer to Article 109-10). To successfully complete the observation period, the coating system must meet the following requirements after twelve (12) months service:

- No visible rust, contamination or application defect is observed in any coated area.
- Painted surfaces have a uniform color and gloss.
- Painted surfaces have an adhesion that meets an ASTM D-3359, 3A rating.

Final acceptance is made only after the paint system meets the above requirements.

Containment Plan - No work begins until the Contractor furnishes the Engineer with a containment plan for surface preparation and coating operations and the Engineer reviews and responds in writing about the acceptability of said plan. Such plan must meet or exceed the requirements of a Class 2A containment in accordance with SSPC Guide 6. Enclosure drawings and loads supported by the structure must be prepared, signed and

¹ Successfully: Lead abatement work completed in accordance with contract specifications, free of citation from safety or environmental agencies. Lead abatement work shall include but not be limited to: abrasive blasting; waste handling, storage and disposal; worker safety during lead abatement activities (fall protection, PPE, etc.); and containment. This requirement is in addition to the contractor pre-qualification requirements covered by NCDOT Std. Specification, Section 102-2.

sealed by a Registered North Carolina Professional Engineer. **The containment plan must take into consideration the span weight balance.**

In the containment plan describe how debris are contained and collected. Describe the type of tarpaulin and bracing materials and the maximum designed wind load. Describe the dust collection system and how a negative pressure of 0.03 inches of water column is maintained inside the enclosure while blasting operations are being conducted. Describe how the airflow inside the containment structure is designed to meet all applicable OSHA Standards. Describe how water run-off from rain will be routed by or through the enclosure. Describe how wash water will be contained and paint chips separated. Describe what physical containment will be provided during painting application to protect vehicles and areas not to be painted.

Wash water Sampling and Disposal Plan - No work begins until the Contractor furnishes the Engineer with a containment plan for surface preparation and coating operations and the Engineer reviews and approves in writing said plan. All wash water shall be collected and sampled prior to disposal. Representative sampling and testing methodology shall conform to 15A NCAC 02B.0103, "Analytical Procedures". Wash water shall be tested for pollutants listed in 15A NCAC 02B.0211 (3), 15A NCAC 02T.0505 (b)(1) and 15A NCAC 2T.0905 (h) (See NCDOT Guidelines for Managing Bridge Wash Water). Depending on the test results, wash water disposal methods shall be described in the disposal plan. Wash water shall be disposed of in accordance with all current state and federal regulations.

Waste Handling of Paint and Abrasives – Use a company from the below list of approved waste management companies. Immediately after award of the contract, the Contractor arranges for waste containers, transportation and disposal of all waste. No work begins until the Contractor furnishes the Engineer with a written waste disposal plan. Any alternative method for handling waste must be pre-approved by the Engineer.

Southern Logistics, Inc. – 312 Orvil Wright Blvd, Greensboro, NC 27409 (Ph. 336-662-0292)

A&D Environmental – 2718 Uwharrie Rd., Archdale, NC 27263 (Ph. 336-434-7750)

All removed paint shall be considered a hazardous waste. The Contractor has the option of furnishing the Engineer certified test reports showing Toxicity Characteristic Leaching Procedure (TCLP) results of the paint chips stored on site, with disposal being in accordance with "Flowchart on Lead Waste Identification and Disposal"

(www.wastenotnc.org/hwhome/guidance/guidance.htm).

If the Contractor elects to have TCLP testing done, samples shall be taken from at least 10% of the barrels to be disposed of, with at least one sample being from each bridge.

Once the waste has been collected and the quantity determined, the Contractor prepares the appropriate shipping documents and manifests and presents them to the Engineer for waste shipment and disposal. The Engineer will verify the type and quantity of waste and

obtain a Temporary Waste Disposal Identification Number (TWDIN) from the NC Hazardous Waste Section.

NC Hazardous Waste Section
PO Box 27687, Raleigh, NC 27611-7687
(919) 733-2178 FAX (919) 733-4810

At the time of shipping the Engineer will sign, date and add the TWDIN in the appropriate section on the manifest. The cost for waste disposal (including any lab fees) is included in the bid price for this contract. Note NC Hazardous Waste Management Rules (15A NCAC 13A) for more information.

Equipment Mobilization - The equipment used in any travel lanes and paved shoulder must be mobile equipment on wheels that has the ability to be moved on/off the roadway and **swing span** in less than 30 minutes. All work conducted in travel lanes must be from truck or trailer supported platforms and all equipment must be self propelled or attached to a tow vehicle at all times.

SUBLETTING OF CONTRACT:

Only contractors certified to meet SSPC QP-2, Category A, and have successfully completed lead paint removal on similar structures within 18 months prior to this bid are qualified for this work. Work is only sublet by approval of the Engineer.

SPECIFICATIONS:

The North Carolina Department of Transportation (NCDOT) Standard Specifications for Roads and Structures dated January 1, 2006, together with these Special Provisions apply to this project. Surface preparation and painting are performed in accordance with Section 442 except where otherwise noted in these Special Provisions. The Paint materials must meet the applicable materials specifications under Section 1080. Materials approvals are in accordance with 4.0 Materials of this Special Provision.

1.0 PREPARATION OF SURFACES:

- 1.1 Power washing with low pressure water – Before any other surface preparation are conducted, all surfaces shall be power washed to remove dust, salts, and other contaminants.
- 1.2 Blasting is done with recyclable steel grit meeting the requirements of Section 1080-15. The profile must be between 1.0 and 3.0 mils when measured on a smooth steel surface.
- 1.3 Before the contractor departs from the work site at the end of the work day, all debris generated during surface preparation are collected in approved containers.
- 1.4 The Contractor cleans a two square foot area at each structure to demonstrate the specified finish and the inspector preserves this area by covering it with

tape, plastic or some other suitable means so that it can be retained as a site standard.

- 1.5 Any area of corroded steel (steel which has lost more than 50% of its original thickness) must not be painted until the Engineer observes its condition.
- 1.6 All parts of the bridges not to be painted, and the travelling public, shall be protected from overspray.
- 1.7 Minimum surface chloride levels for painting are 7 PPM or below.

2.0 PAINTING OF STEEL:

Paint System 1, as specified in these special provisions and Section 442 of NCDOT's Standard Specifications, is to be used for this work, with the following exception. **For this project, the IOZ primer shall be replaced with an approved Organic Zinc Primer (See section 3.0 Materials).** The top coats shall remain in accordance with Section 442.

Any area where newly applied paint fails to meet the specifications must be repaired or replaced by the Contractor. The Engineer approves all repair processes before the repair is made. Repaired areas must meet the specifications. The Contractor applies an additional finish coat of paint to areas where the tape adhesion test is conducted.

Do not apply any coating below 40 F or when a temperature of the air, surface, substrate or material is 40 F or below is predicted during the drying and curing period of the paint. Do not apply any coating above or below the manufacturers recommended application temperatures or during a period when an ambient temperature outside the recommended range is predicted during the drying and curing period of the paint. During adverse weather, use enclosures that control atmospheric conditions artificially inside within limits suitable for painting during the painting operation and until each coat of paint is cured or until weather conditions permit its exposure in the open.

All steel to steel contact in the swing span will be sealed with a suitable caulking.

Should the contractor elect to apply Organic Zinc Primer at or below 50 F the recoat window shall be a minimum of 24 hours or as necessary for the solvent to fully evaporate from the coating.

No application of acrylic coats (intermediate, stripe or top) shall be applied when the air or substrate is below 50 F.

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3.0 MATERIALS:

All **Organic Zinc-Rich** coatings **submitted for use** shall be evaluated for performance through the National Transportation Product Evaluation program (NTPEP) for Structural Steel Coatings as part of a Coating System that appears on one of the North East Protective Coatings Committee's (NEPCOAT) Qualified Products Lists. Submission of products through AASHTO/NTPEP is a continuous process and manufacturers may submit systems at any time. Prior to the start of work, a 1 quart unmixed sample kit (including the zinc dust) of the Organic Zinc Primer shall be sent to the Materials and Tests Unit for verification testing.

Only paint suppliers that have a NCDOT qualified inorganic zinc primer may furnish paints for this project. Choose a pre-qualified paint supplier from the list at this web address:

<http://www.ncdot.org/doh/operations/materials/chemical/aproducts.html>.

Only companies with a "4" listed under NCDOT Paints Furnished have approved inorganic Zinc Primers. All paints applied to a structure must be from the same supplier. Before any paints are applied the Contractor provides the Engineer a manufacturer's certification that each batch of paint meets the requirements of the applicable Section 1080 of NCDOT's Standard Specifications.

The inspector randomly collects a one quart sample of each paint product used on the project. Additional samples may be collected as needed to verify compliance to the specifications.

3.1 REQUIREMENTS FOR SUITABLE CAULKING

Submit for approval to Division 1, Engineer of Record and Materials & Tests Unit, Chemical Testing Engineer

A. APPROVAL

1. Manufacturer's letter certifying the Caulking Compound supplied to the project will meet or exceed Department expectations and will be adequate for the intended use
2. Contractor shall submit for approval necessary paperwork at a minimum of 7 working days prior to Pre-Construction Meeting:
 - Product data sheet
 - MSDS sheet
 - Letter from paint manufacturer stating caulking material is suitable and compatible with the type and brand of paint being used on the project
3. Show typical markings on the packaging and any date markings.
4. Provide application instructions and temperature limitations.
5. State effective product life

B. WORK SEQUENCE

Contractor shall apply suitable caulking in accordance the manufacturer's product data sheet. In no case shall caulking be applied to moist, damp or frost bearing surfaces or if temperature will fall below freezing for more than 48 hours.

Suitable caulking shall be applied to all steel to steel contact surfaces after application of the primer coat.

C ACCEPTANCE BY DEPARTMENT

The Engineer of Record & Chemical Testing Engineer will review the manufacturer's submittal for compliance with specification requirements.

D. FIELD DOCUMENTATION REQUIREMENTS

Accepted suitable caulking products will be accepted if they can be identified in the field by manufacturer name and (in applicable) brand name.

4.0 INSPECTION:

Quality Assurance Inspection - The Contractor furnishes all necessary apparatus such as ladders, scaffolds and platforms as required for the inspector to have reasonable and safe access to all parts of the work. The contractor illuminates the surfaces to be inspected to a minimum of 50-foot candles of light.

Contractor must insure that chloride levels on the surfaces prior to each coat of paint are below 7 PPM using an acceptable sample method in accordance with SSPC Guide 15. The frequency of testing shall be 5 tests per 1000 square feet until a pattern of acceptable results are achieved then reduced to 1 test per 1000 square feet.

If the Contractor elects to use an approved organic zinc rich primer and is approved in accordance with "3.0 Materials" of this specification, verification of the cure of organic zinc rich primer shall be tested by the following:

ASTM D 3363- 2H
ASTM D 4541- 400PSI

NCDOT reserves the right for ongoing QA (Quality Assurance) inspection to include but not limited to surface contamination testing, adhesion pull testing and DFT readings as necessary to assure quality.

The contractor informs the Engineer of all scheduled and unannounced inspections from SSPC, OSHA, EPA and/or others that come on site.

Inspection Instruments - The Contractor furnishes at least the following calibrated instruments at site and conducts the quality control testing:

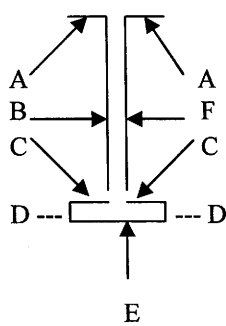
Sling Psychrometer - ASTM E-337 – bulb type
Surface Temperature Thermometer
Wind Speed Indicator
Tape Profile Tester – ASTM D-4417 Method C
Surface Condition Standards – SSPC VIS-1 and VIS-3
Wet Film Thickness Gage – ASTM D-4414
Dry Film Thickness Gage – SSPC-PA2 Modified

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- Solvent Rub Test Kit – ASTM D-4752
- Adhesion Test Kit – ASTM D-3359
- Elcometer and dollies
- Surface Contamination Analysis Kit or (Chloride Level Test Kit)

The contractor maintains a daily quality control record in accordance with Section 442-12 and such records must be available at the job site for review by the inspector and be submitted to the Engineer as directed. In addition to the information required on M&T-610, the Contractor shall submit all DFT readings as required on M&T611.

- A. The dry film thickness is measured at each spot as indicated on the attached diagram at no less than three random locations along each girder in each span. Also dry film thickness is measured at no less than six random spots per span on diaphragms/“K” frames. Each spot is an average of three to five readings in accordance with SSPC PA-2.



	A	B	C	D	E	F
Loc. 1	_____	_____	_____	_____	_____	_____
Loc. 2	_____	_____	_____	_____	_____	_____
Loc. 3	_____	_____	_____	_____	_____	_____

Randomly select one A, one C and one D spot along with B, E and F.

- B. Two random adhesion tests per span are conducted on interior surfaces after the paint has been properly cured, and will be touched up by the Contractor. One random Cut Tape adhesion test per span is conducted on interior surface after the finish coat is cured, and will be touched up by the Contractor.

5.0 SAFETY AND ENVIRONMENTAL COMPLIANCE PLANS:

Personnel access boundaries are delineated for each work site using signs, tape, cones or other approved means. Submit copies of safety and environmental compliance plans that comply with SSPC QP-2 Certification requirements.

6.0 ENVIRONMENTAL MONITORING:

Comply with Section 442–13(B) of NCDOT’s Standard Specifications.

A “Competent Person²” is on site during all surface preparation activities and monitors the effectiveness of containment and dust collection systems. Any visible emissions outside the containment enclosure or pump monitoring results exceeding the level of 30

² **Competent Person** as defined in OSHA 29 CFR 1926.62 is one who is capable of identifying existing and predictable hazards in the surroundings or working conditions which are unsanitary, hazardous, or dangerous to employees, and who have authorization to take prompt corrective measures to eliminate them.

$\mu\text{g}/\text{m}^3$ TWA is justification to suspend the work. Before any work begins the Contractor provides a written summary of the responsible person's safety training.

7.0 HEALTH AND SAFETY RESPONSIBILITY:

Comply with Section 442-13(C) of NCDOT's Standard Specifications. Insure employee blood sampling test results are less than 50 micrograms per deciliter. Remove employees with a blood sampling test of 50 or more micrograms per deciliter from work activities involving any lead exposure.

An employee who has been removed with a blood level of 50 micrograms per deciliter or more shall have two consecutive blood sampling tests indicating that the employee's blood lead level is at or below 40 micrograms per deciliter before returning to work activities involving any lead exposure.

8.0 STORAGE OF PAINT AND EQUIPMENT:

The Prime Contractor provides a location for materials, equipment and waste storage. Tarpaulins are spread over all pavements and surfaces underneath equipment utilized for abrasive recycling and other lead handling equipment or containers.

9.0 UTILITIES:

The Contractor protects all utility lines or mains which may be supported on, under, or adjacent to bridge work sites from damage and paint over-spray.

10.0 PAYMENT:

The cost of inspection, surface preparation and repainting the existing structure is included in the lump sum price bid for *Cleaning and Painting Existing Structure*. This price is full compensation for furnishing all inspection equipment, all paint, cleaning abrasives, cleaning solvents and all other materials; preparing and cleaning surfaces to be painted; applying paint in the field; protecting work, traffic and property; and furnishing blast cleaning equipment, paint spraying equipment, brushes, rollers and any other hand or power tools and any other equipment.

Pollution Control will be paid for at the contract lump sum price which price will be full compensation for all collection, handling and disposal of debris and wash water, all personal protective equipment, and all personal hygiene requirements, and all equipment, material and labor necessary to fully contain the blast debris; daily collection of the blast debris into specified containers; and any measures necessary to ensure conformance to all safety and environments regulations as directed by the Engineer.

Payment will be made under:

Pay Item	Pay Unit
Cleaning and Painting Existing Structure	Lump Sum
Pollution Control	Lump Sum

UNDER STRUCTURE WORK PLATFORM

SPECIAL

Description

Prior to any painting operations on the structure, the Contractor shall design and install an understructure work platform which will be used to provide access to the work to be done as well as serve as containment for the cleaning and painting of the bridge. The Contractor shall determine the capacity of the platform which will be required, but the capacity shall not be less than that required by State or Federal regulations. The platform shall remain in place until all work is completed on the moveable span. Platform shall be constructed of materials capable of withstanding damage from any of the work required on this project. The platform shall be fireproof. Drawings of the platform and loads supported by the platform shall be sealed by a North Carolina Registered Professional Engineer. Submit drawings to the Engineer for approval prior to beginning work on the platform. Platform shall be cleaned after each work day to prevent materials from falling or washing into the river.

The Contractor shall furnish and install counterweights as required to keep the swing span balanced and shall maintain the balance of the bridge to allow for normal opening and closing. The platform used for painting the structure may be installed under both spans simultaneously to assist with maintaining the balance. The Contractor shall furnish, install and maintain 6 warning signs at least 48"x48" indicating the reduced clearance resulting from the platform. The signs shall be installed at the ends of each bridge fender.

Payment

Under Structure Work Platform will be paid for at the lump sum contract price and will be full compensation for the design, installation, maintenance, and removal of the platform and warning signs.

Payment will be made under:

Pay Item	Pay Unit
Under Structure Work Platform	Lump Sum

MANAGING BRIDGE WASH WATER**1.0 Description**

Collect and properly dispose of Bridge Wash Water from bridge decks.

2.0 Construction Methods

(A) Prepare a written Bridge Wash Water management plan in accordance with the Guidelines for Managing Bridge Wash Water available at <http://www.ncdot.org/doh/preconstruct/ps/contracts/letting.html>. Submit plan and obtain approval from the Engineer prior to beginning of the bridge cleaning operation.

(B) Prior to final payment, submit a paper copy of all completed records pertaining to disposal of Bridge Wash Water.

3.0 Measurement and Payment

Payment for collecting, sampling, testing, pH adjustment, monitoring, handling, discharging, hauling, disposing of the bridge wash water, documentation, record keeping, and obtaining permits if applicable, shall be included in the payment for other items.