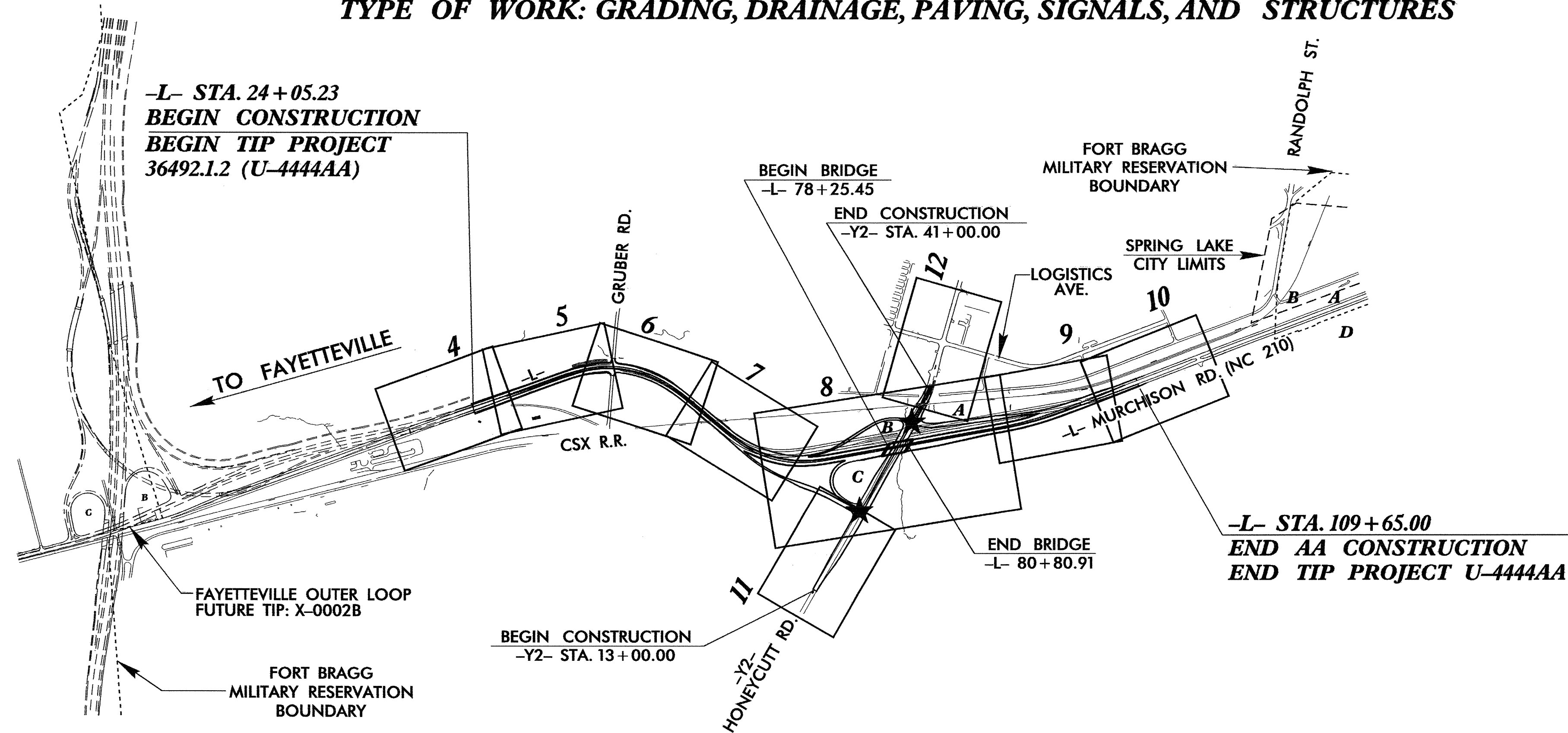
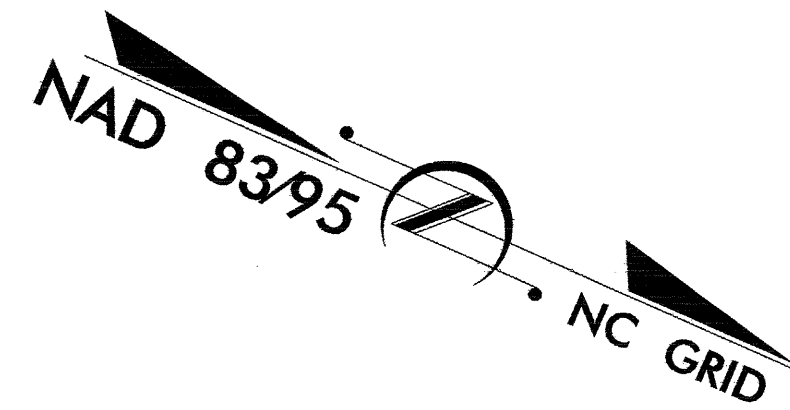


**TIP PROJECT: U-4444AA**

STATE OF NORTH CAROLINA  
 DIVISION OF HIGHWAYS  
 PLAN FOR PROPOSED  
 HIGHWAY EROSION CONTROL  
**CUMBERLAND COUNTY**

**LOCATION: NC 210 (MURCHISON ROAD) FROM FAYETTEVILLE OUTER LOOP  
 TO JUST NORTH OF HONEYCUTT RD.**

**TYPE OF WORK: GRADING, DRAINAGE, PAVING, SIGNALS, AND STRUCTURES**



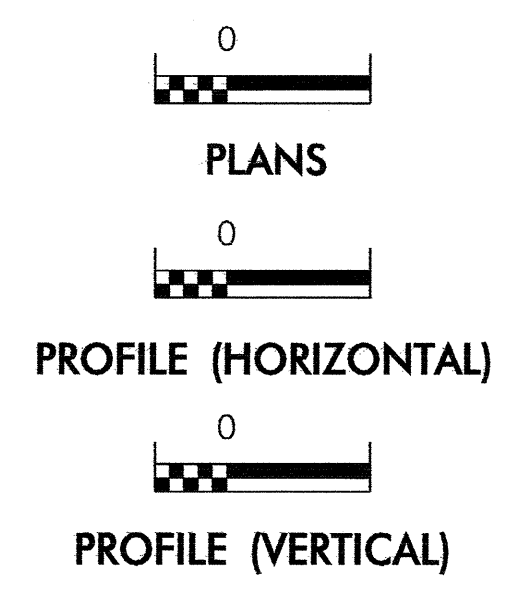
STATE	STATE PROJECT REFERENCE NO.	SHEET NO.	TOTAL SHEETS
N.C.	U-4444AA	EC-1	
STATE PROJ. NO.	F.A. PROJ. NO.	DESCRIPTION	

**EROSION AND SEDIMENT CONTROL MEASURES**

Std. #	Description	Symbol
1630.03	Temporary Silt Ditch	---
1630.05	Temporary Diversion	TD
1605.01	Temporary Silt Fence	
1606.01	Special Sediment Control Fence	
1622.01	Temporary Berms and Slope Drains	--->
1630.01	Riser Basin	⊙
	Silt Basin Type B	⊙
1633.01	Temporary Rock Silt Check Type-A	⊗
	Temporary Rock Silt Check Type-B	⊗
	Wattle	⊙
1634.01	Temporary Rock Sediment Dam Type-A	⊗
1634.02	Temporary Rock Sediment Dam Type-B	⊗
1635.01	Rock Pipe Inlet Sediment Trap Type-A	⊗
1635.02	Rock Pipe Inlet Sediment Trap Type-B	⊗
1630.04	Stilling Basin	⊗
1630.06	Special Stilling Basin	⊗
Rock Inlet Sediment Trap:		
1632.01	Type A	A
1632.02	Type B	B
1632.03	Type C	C
	Skimmer Basin	⊗
	Tiered Skimmer Basin	⊗
	Infiltration Basin	⊗

**THIS PROJECT CONTAINS  
 EROSION CONTROL PLANS  
 FOR CLEARING AND  
 GRUBBING PHASE OF  
 CONSTRUCTION.**

**GRAPHIC SCALE**



ROADSIDE ENVIRONMENTAL UNIT  
 DIVISION OF HIGHWAYS  
 STATE OF NORTH CAROLINA

Prepared In the Office of:  
**ROADSIDE ENVIRONMENTAL UNIT**  
 1 South Wilmington St.  
 Raleigh, NC 27611  
**2006 STANDARD SPECIFICATIONS**

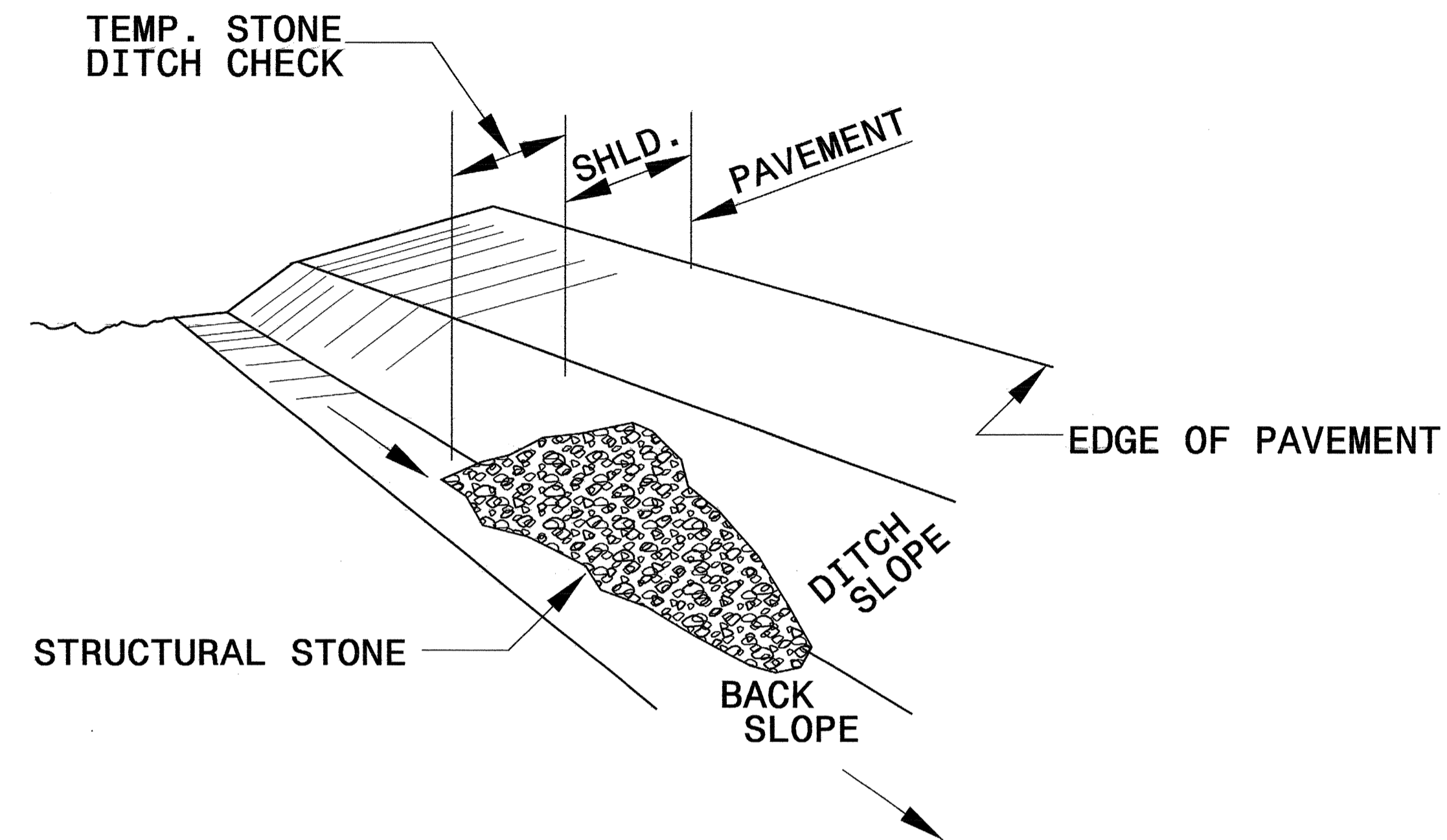
Roadway Standard Drawings  
 The following roadway english standards as appear in "Roadway Standard Drawings"- Roadway Design Unit - N. C. Department of Transportation - Raleigh, N. C., dated July 18, 2006 and the latest revision thereto are applicable to this project and by reference hereby are considered a part of these plans.

1605.01 Temporary Silt Fence	1632.01 Rock Inlet Sediment Trap Type A
1606.01 Special Sediment Control Fence	1632.02 Rock Inlet Sediment Trap Type B
1607.01 Gravel Construction Entrance	1632.03 Rock Inlet Sediment Trap Type C
1622.01 Temporary Berms and Slope Drains	1633.01 Temporary Rock Silt Check Type A
1630.03 Temporary Silt Ditch	1634.01 Temporary Rock Sediment Dam Type A
1630.05 Temporary Diversion	1635.01 Rock Pipe Inlet Sediment Trap Type A
	1635.02 Rock Pipe Inlet Sediment Trap Type B

02-JUN-2009 09:55  
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 jagooskin

PROJECT REFERENCE NO. U-4444AA	SHEET NO. EC-2
RW SHEET NO.	
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER

# TEMPORARY ROCK SILT CHECK TYPE 'B' DETAIL

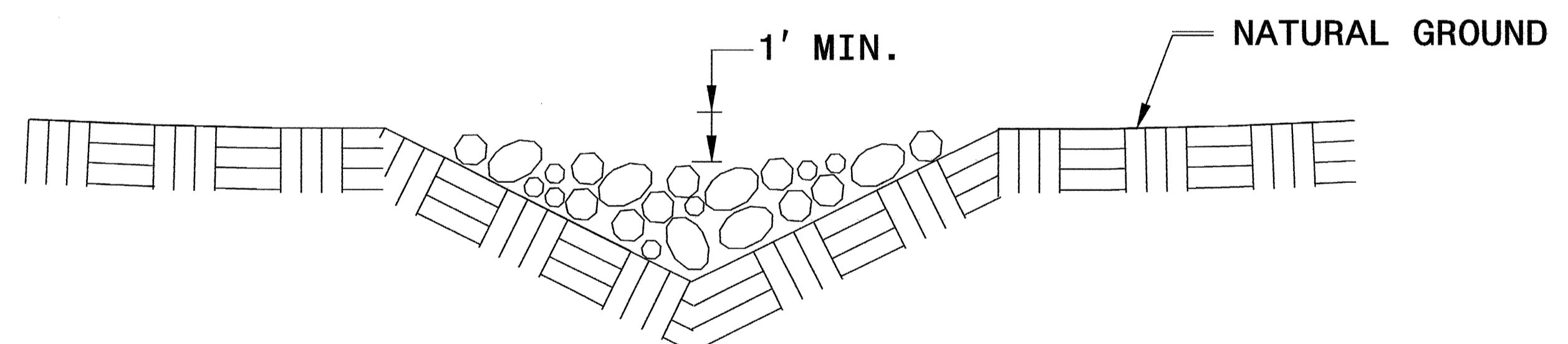


**ISOMETRIC VIEW**

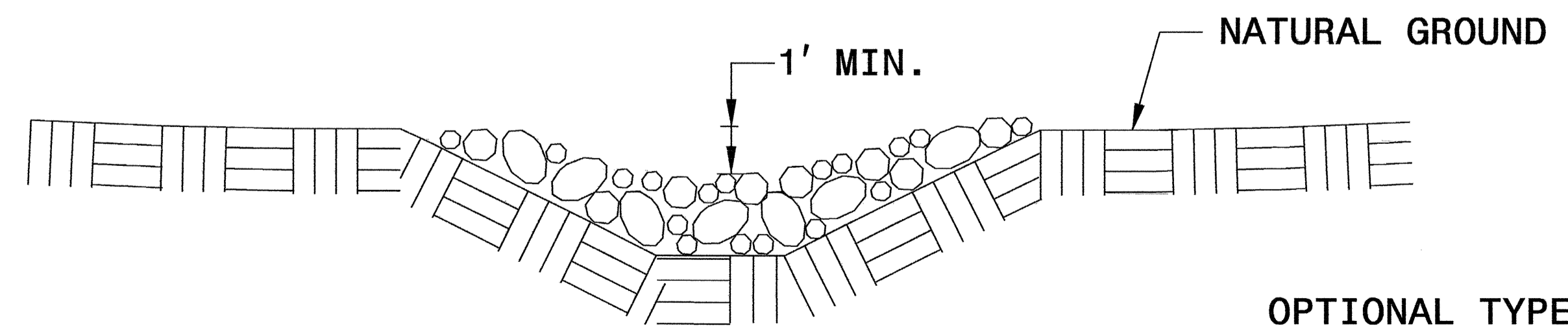
**NOTES:**

USE CLASS 'B' EROSION CONTROL STONE FOR STRUCTURAL STONE.

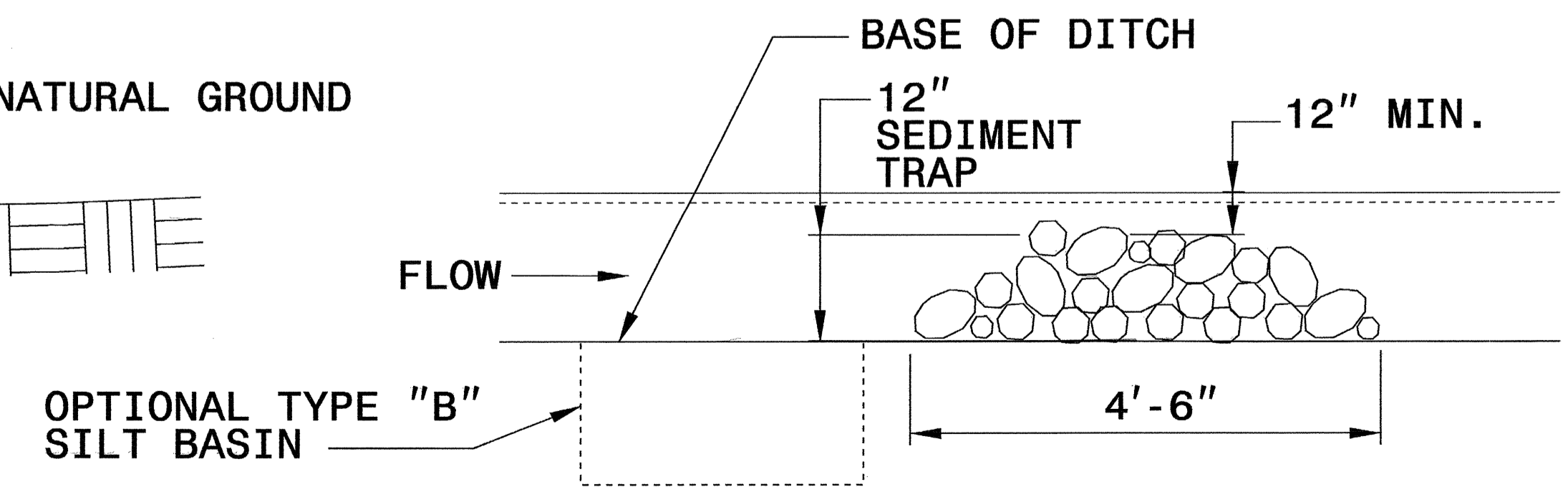
THE ENGINEER MAY DIRECT THE OPTION OF CLASS "A" STONE FOR SITES HAVING LESS THAN ONE (1) ACRE DRAINAGE AREA AND A DITCH GRADE LESS THAN 3%.



**CROSS SECTION VEE DITCH**



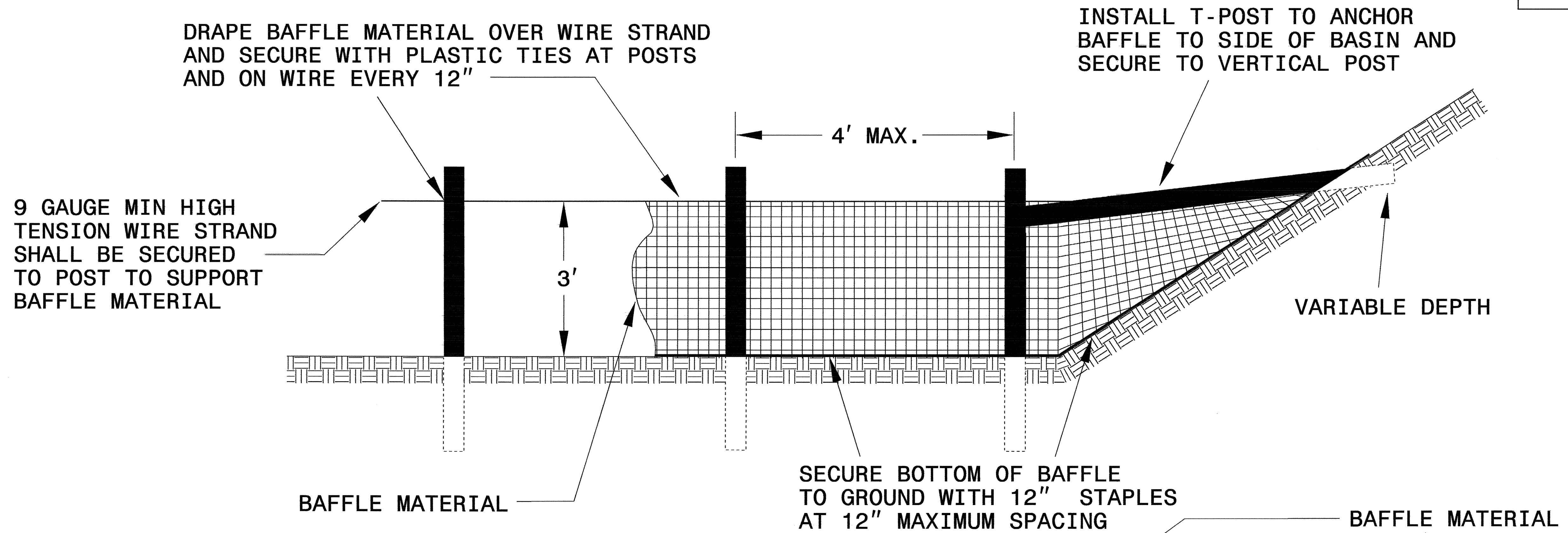
**CROSS SECTION TRAPEZOIDAL DITCH**



**ELEVATION VIEW**

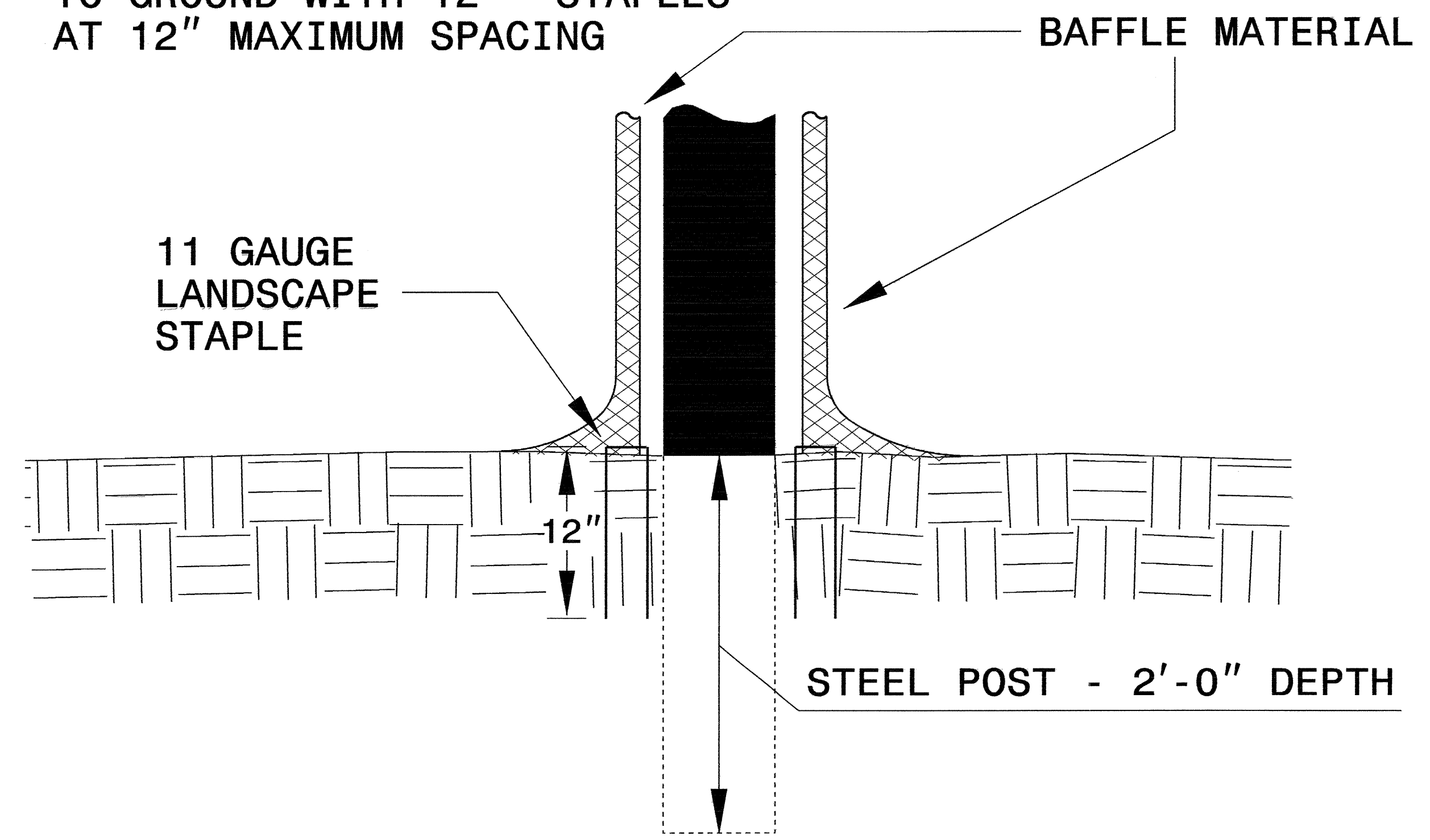
PROJECT REFERENCE NO. U-4444AA	SHEET NO. EC-2A
RW SHEET NO.	
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER

# COIR FIBER BAFFLE DETAIL



## NOTES:

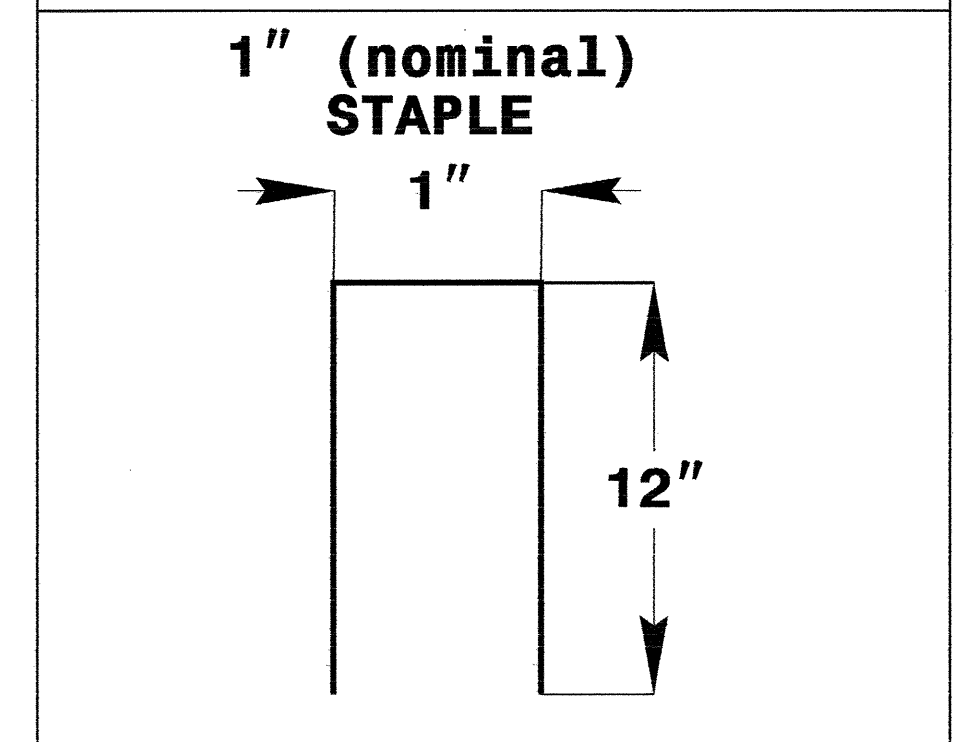
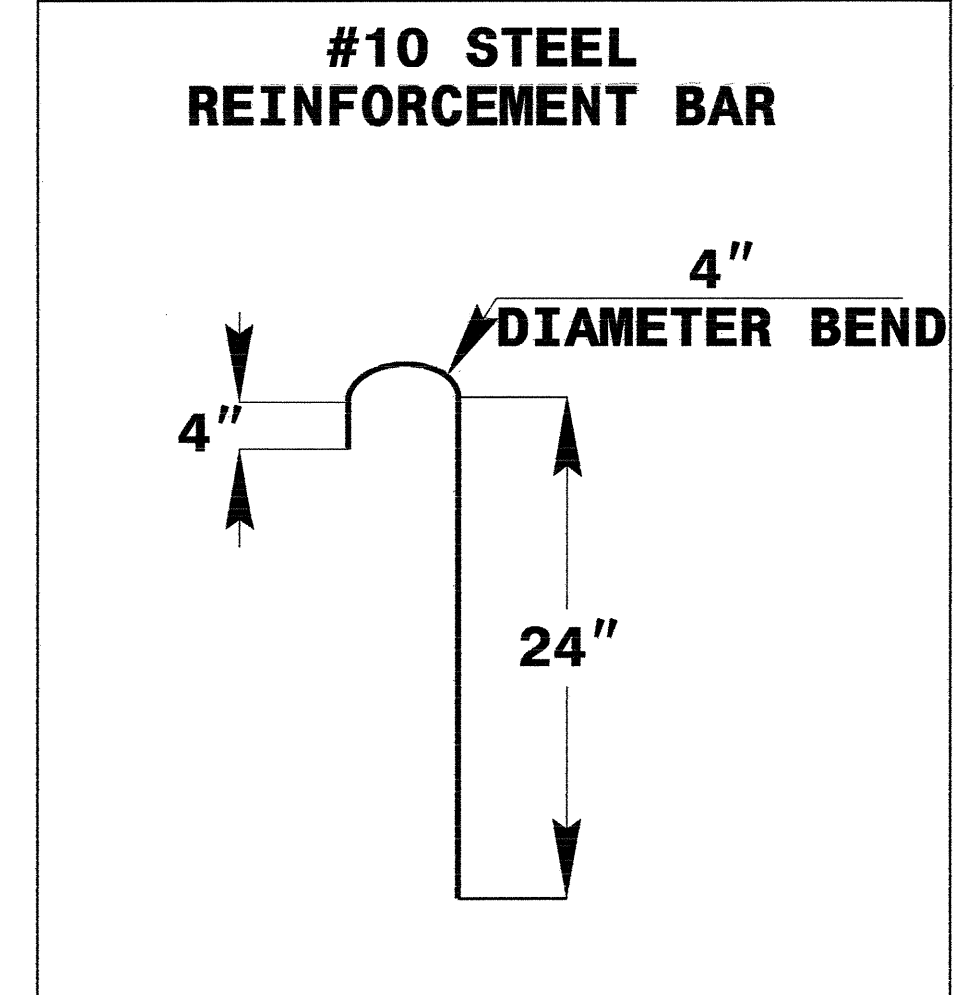
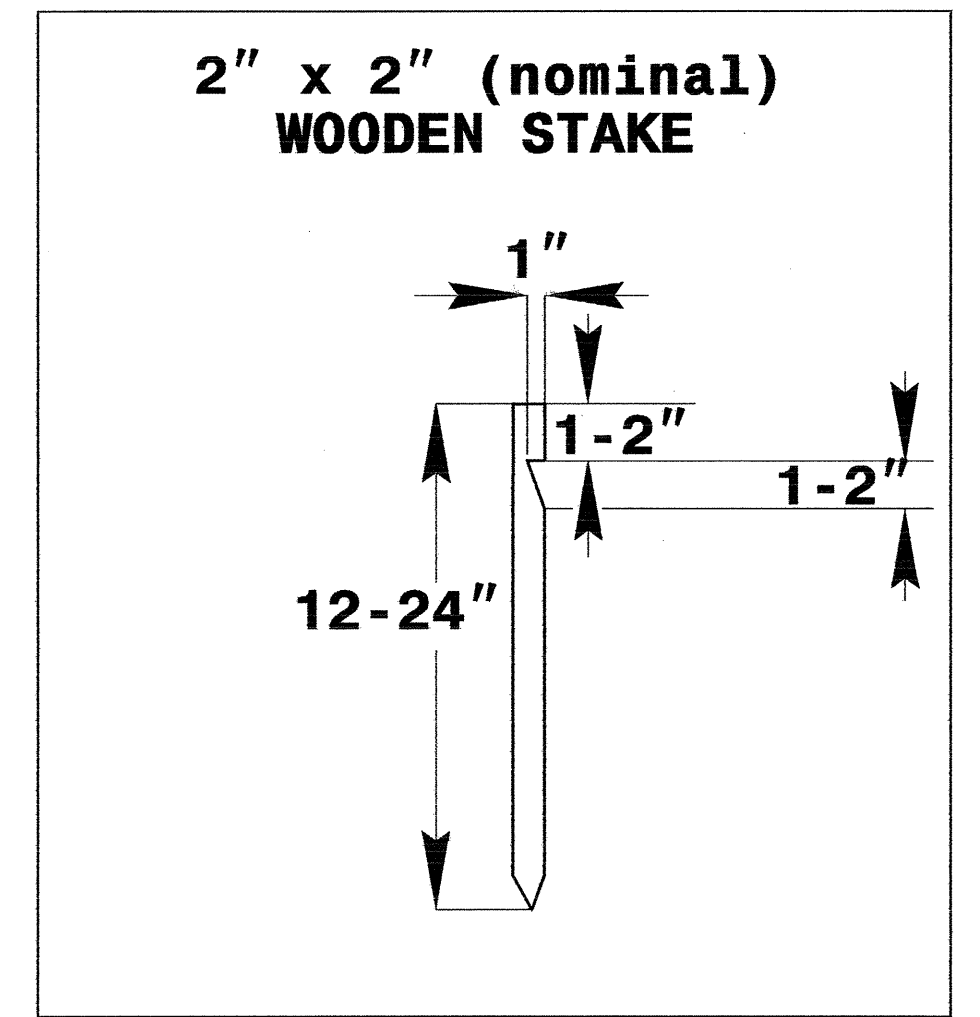
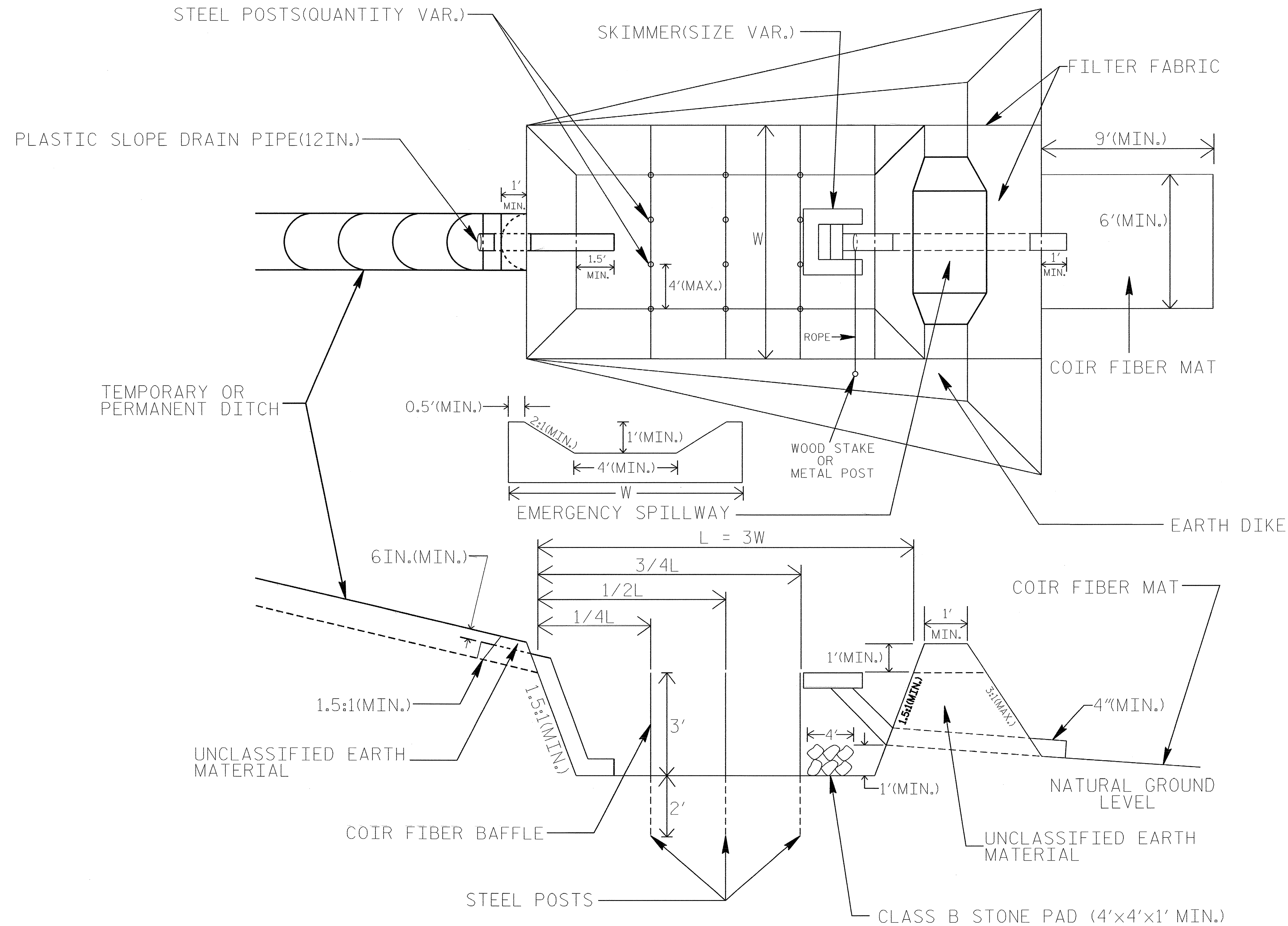
1. INSTALL THREE(3) COIR FIBER BAFFLES IN SILT BASINS AND SEDIMENT DAMS AT DRAINAGE OUTLETS WITH A SPACING OF  $\frac{1}{4}$  THE BASIN LENGTH.
2. TWO(2) COIR FIBER BAFFLES CAN BE INSTALLED IN SILT BASINS AND DAMS LESS THAN 20 FT. IN LENGTH WITH A SPACING OF  $\frac{1}{3}$  THE BASIN LENGTH.
3. TOP HEIGHT OF COIR FIBER BAFFLES SHALL NOT BE BELOW BASE OF EMERGENCY SPILLWAY ELEVATION.



BAFFLE MATERIAL SHALL BE SECURED TO THE BOTTOM AND SIDES OF BASIN USING 12" LANDSCAPE STAPLES

# SKIMMER BASIN WITH BAFFLES DETAIL

PROJECT REFERENCE NO. U-444AA	SHEET NO. EC-2B
RW SHEET NO.	
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER



## COIR FIBER MAT ANCHOR OPTIONS

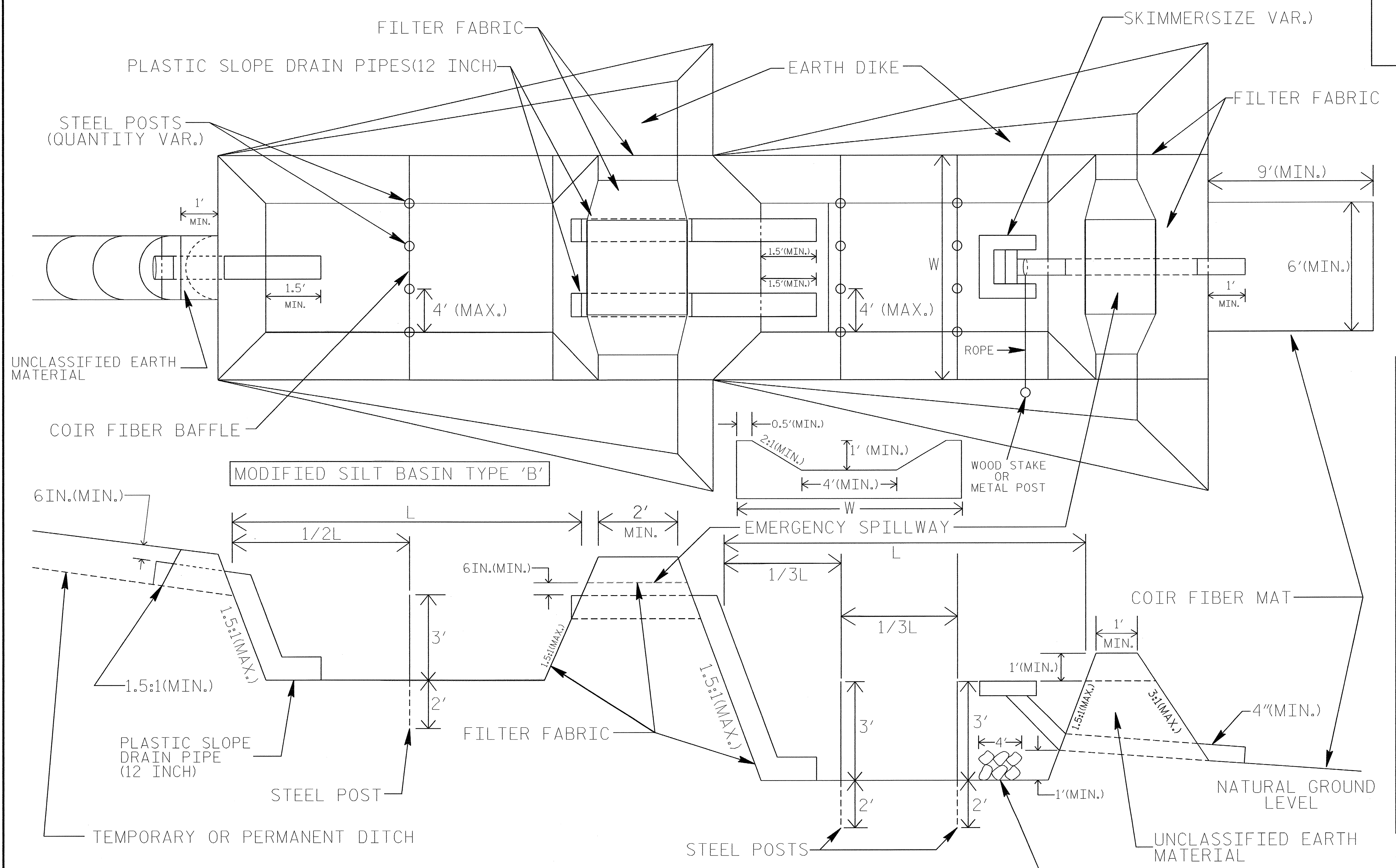
### NOTES

1. SEED AND PLACE MATTING FOR EROSION CONTROL ON INTERIOR SIDESLOPES.
2. LIMIT EARTH DIKE HEIGHT TO 5 FT.
3. THE MINIMUM BASIN WIDTH SHALL BE 9 FT.
4. DETERMINE EMERGENCY SPILLWAY LENGTH (FT.) USING  $Q/0.8$ , WHERE Q IS FLOW RATE (CFS) INTO BASIN.

NOT TO SCALE

PROJECT REFERENCE NO. U-4444AA	SHEET NO. EC-2C
RW SHEET NO.	
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER

# TIERED SKIMMER BASIN DETAIL



**NOTES**

1. SEED AND PLACE MATTING FOR EROSION CONTROL ON INTERIOR SIDESLOPES OF BASINS.
2. LIMIT HEIGHT OF EARTH DIKES TO 5 FT.
3. ADDITIONAL MODIFIED SILT BASINS TYPE 'B' MAY BE NEEDED DEPENDING ON SLOPE.
4. THE MINIMUM BASIN WIDTHS SHALL BE 9 FT.
5. DETERMINE EMERGENCY SPILLWAY LENGTHS (FT.) USING  $Q/0.8$ , WHERE Q IS FLOW RATE (CFS) INTO UPPER BASIN.

**2" x 2" (nominal) WOODEN STAKE**

**#10 STEEL REINFORCEMENT BAR**

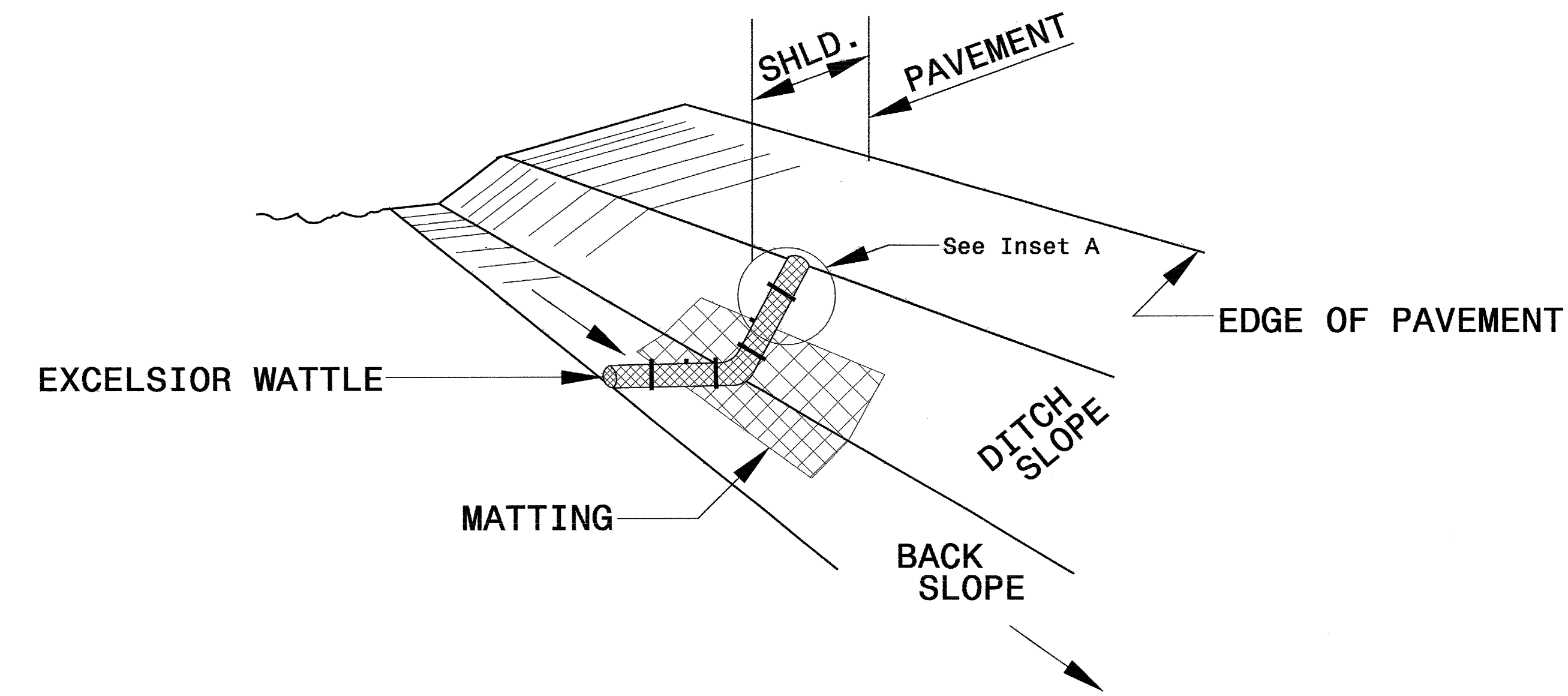
**1" (nominal) STAPLE**

**COIR FIBER MAT ANCHOR OPTIONS**

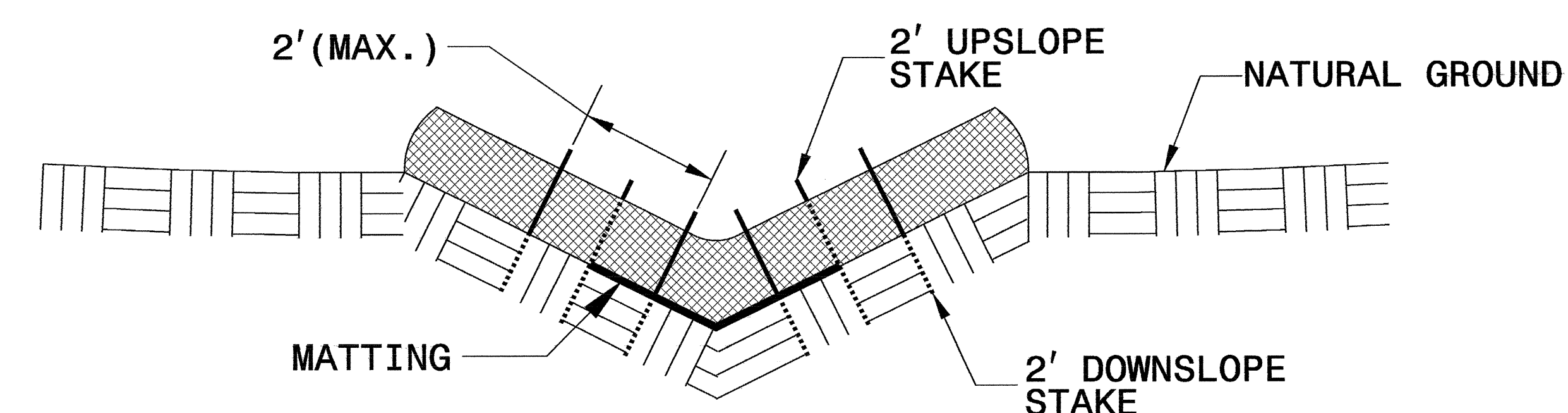
NOT TO SCALE

PROJECT REFERENCE NO. U-4444AA	SHEET NO. EC-2D
RW SHEET NO.	
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER

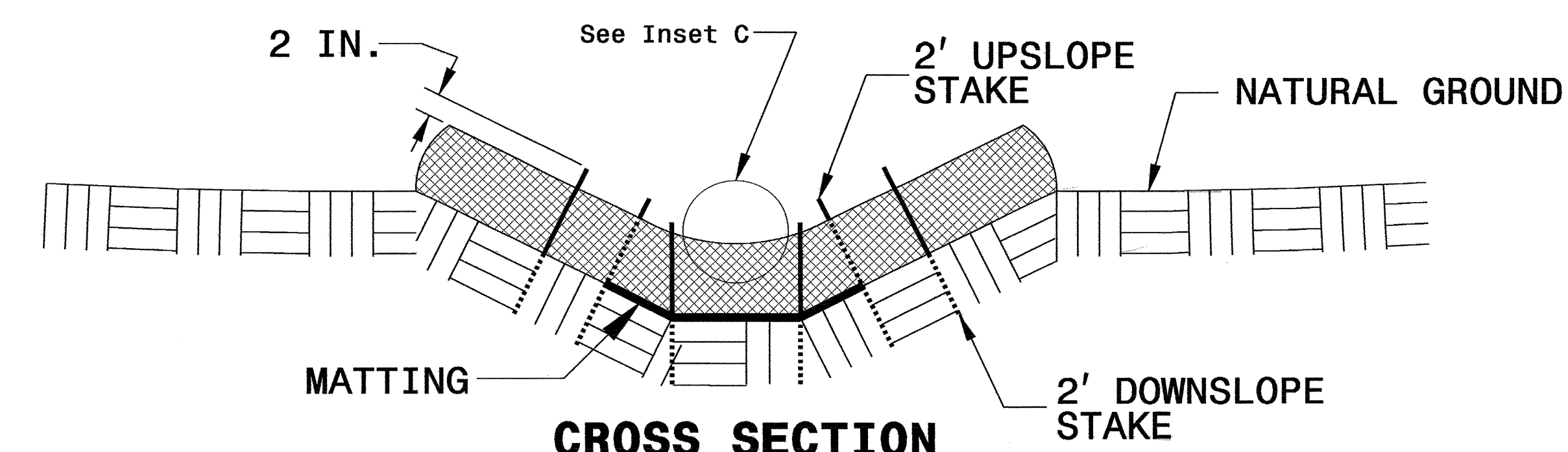
# WATTLE WITH POLYACRYLAMIDE DETAIL



**ISOMETRIC VIEW**



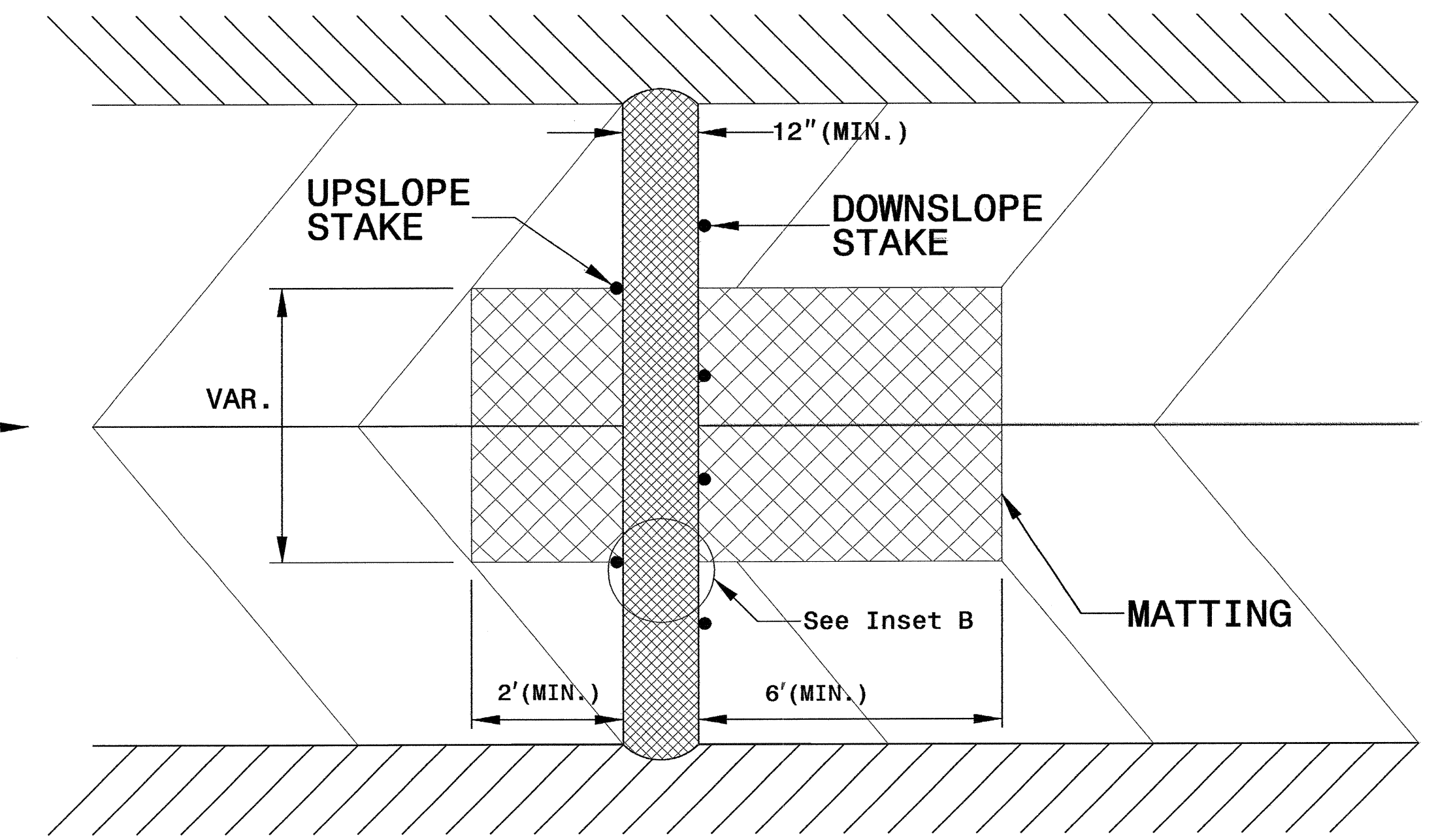
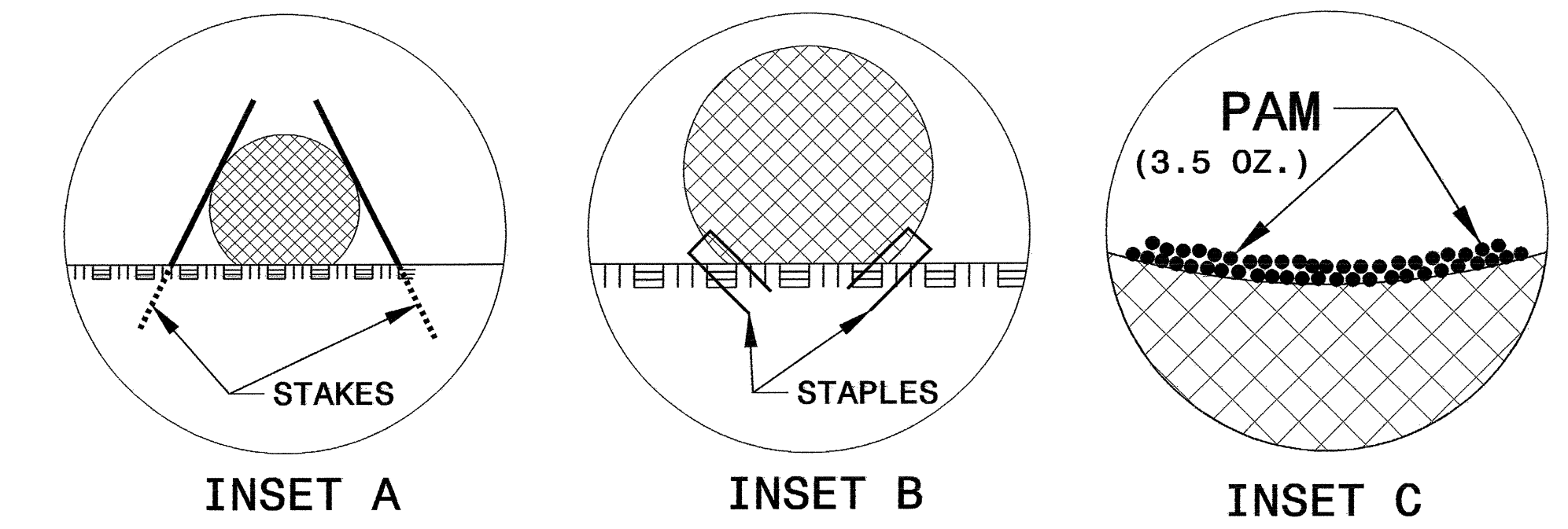
**CROSS SECTION VEE DITCH**



**CROSS SECTION TRAPEZOIDAL DITCH**

**NOTES:**

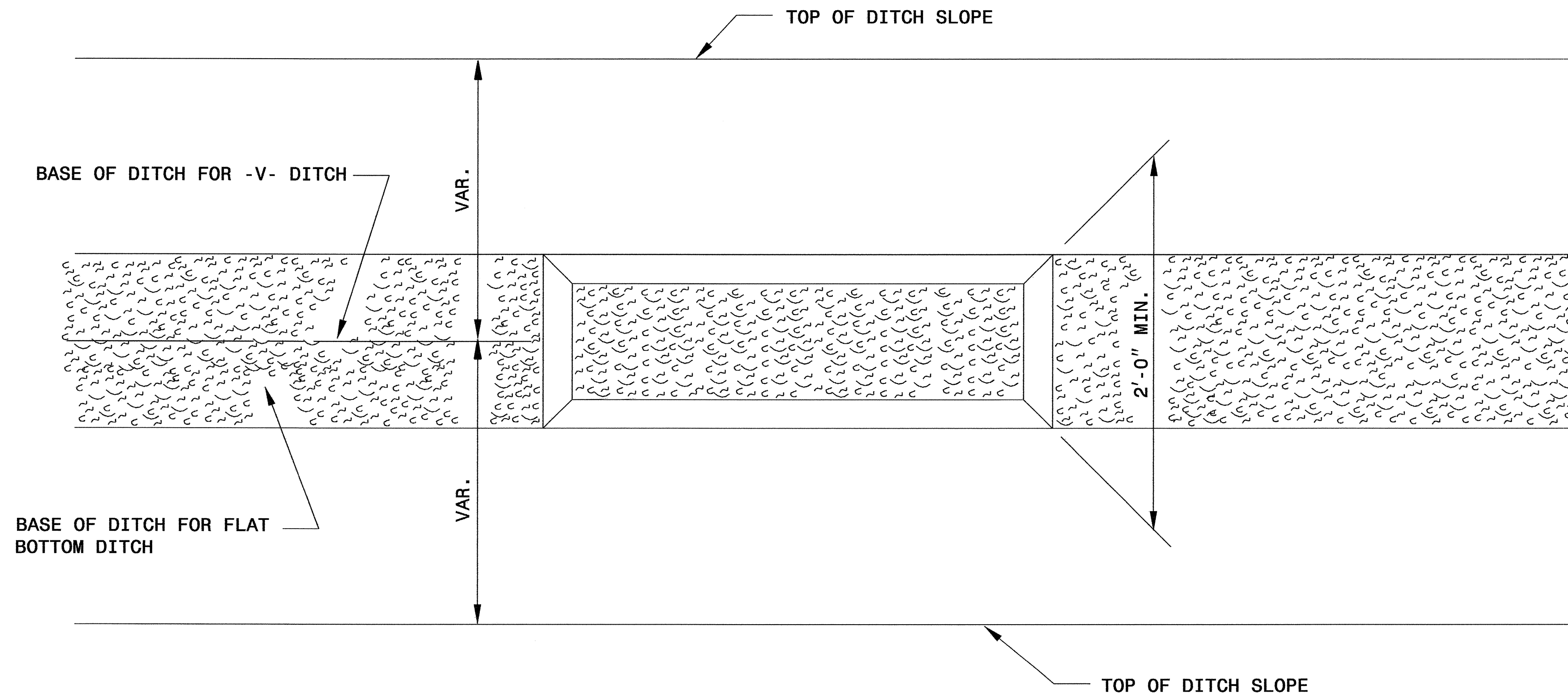
- USE MINIMUM 12 IN. DIAMETER EXCELSIOR WATTLE.
- USE 2 FT. WOODEN STAKES WITH A 2 IN. BY 2 IN. CROSS SECTION.
- ONLY INSTALL WATTLE(S) TO A HEIGHT IN DITCH SO FLOW WILL NOT WASH AROUND WATTLE AND SCOUR DITCH SLOPES AND AS DIRECTED.
- INSTALL A MINIMUM OF 2 UPSLOPE STAKES AND 4 DOWNSLOPE STAKES AT AN ANGLE TO WEDGE WATTLE TO BOTTOM OF DITCH.
- PROVIDE STAPLES MADE OF 0.125 IN. DIAMETER STEEL WIRE FORMED INTO A U SHAPE NOT LESS THAN 12" IN LENGTH.
- INSTALL STAPLES APPROXIMATELY EVERY 1 LINEAR FOOT ON BOTH SIDES OF WATTLE AND AT EACH END TO SECURE IT TO THE SOIL.
- INSTALL MATTING IN ACCORDANCE WITH SECTION 1631 OF THE STANDARD SPECIFICATIONS.
- PRIOR TO POLYACRYLAMIDE (PAM) APPLICATION, OBTAIN A SOIL SAMPLE FROM PROJECT LOCATION, AND FROM OFFSITE MATERIAL, AND ANALYZE FOR APPROPRIATE PAM FLOCCULANT TO BE APPLIED TO EACH WATTLE.
- INITIALLY APPLY 3.5 OUNCES OF ANIONIC OR NEUTRALLY CHARGED POLYACRYLAMIDE (PAM) OVER WATTLE WHERE WATER WILL FLOW AND AFTER EVERY RAINFALL EVENT THAT IS EQUAL TO OR EXCEEDS 0.50 IN.



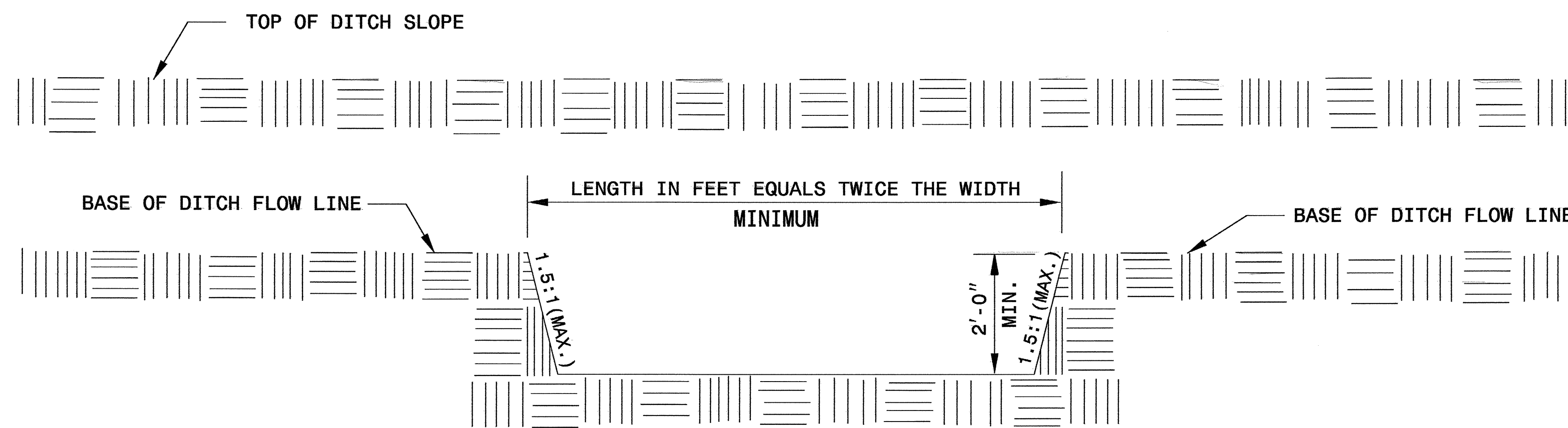
**TOP VIEW**

PROJECT REFERENCE NO. U-4444AA	SHEET NO. EC-2E
RW SHEET NO.	
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER

# SILT BASIN 'B' DETAIL



PLAN



ELEVATION

# BORROW PIT DEWATERING BASIN DETAIL

PROJECT REFERENCE NO. U-4444AA	SHEET NO. EC-2F
RW SHEET NO.	
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER

## GENERAL NOTES:

DETERMINE BORROW PIT DEWATERING BASIN SIZE USING  $V = 8.0203 * Q * T$ , WHERE V IS VOLUME (FT<sup>3</sup>), Q IS PUMP FLOW RATE (GPM), AND T IS DEWATERING TIME (HR). USE MAXIMUM FLOW RATE OF 1000 GPM AND A MINIMUM DEWATERING TIME OF 2 HOURS.

RISER SHALL BE A NON-PERFORATED, SMOOTH OR CORRUGATED MATERIAL WITH A FLASHBOARD OPTION.

CONSTRUCT THE COIR FIBER BAFFLE WITH A MATERIAL THAT MEETS THE SPECIFICATIONS OF THE COIR FIBER MAT SPECIAL PROVISION PROVIDED IN THE CONTRACT.

PROVIDE 5' STEEL POSTS OF THE SELF-FASTENER ANGLE STEEL TYPE. INSTALL STEEL POSTS WITH NO MORE THAN 3' OF THE POST APPEARING ABOVE THE GROUND.

ATTACH THE COIR FIBER MAT TO THE STEEL POSTS WITH WIRE OR OTHER ACCEPTABLE MEANS AND STAPLED INTO THE BOTTOM AND SIDE SLOPES OF THE BASIN WITH 12" STAPLES.

INSTALL TYPE 2 FILTER FABRIC ON SIDESLOPES AND BOTTOM OF BASIN AT INLET AS SHOWN IN THE DETAIL.

USE THE TYPICAL SECTION SHOWN FOR THE BORROW PIT DEWATERING BASIN AS A GUIDE. THE BASIN MAY HAVE ANY TYPE CONFIGURATION AS LONG AS SUFFICIENT VOLUME IS PROVIDED AND PROVISIONS ARE MADE FOR A NON-PERFORATED RISER.

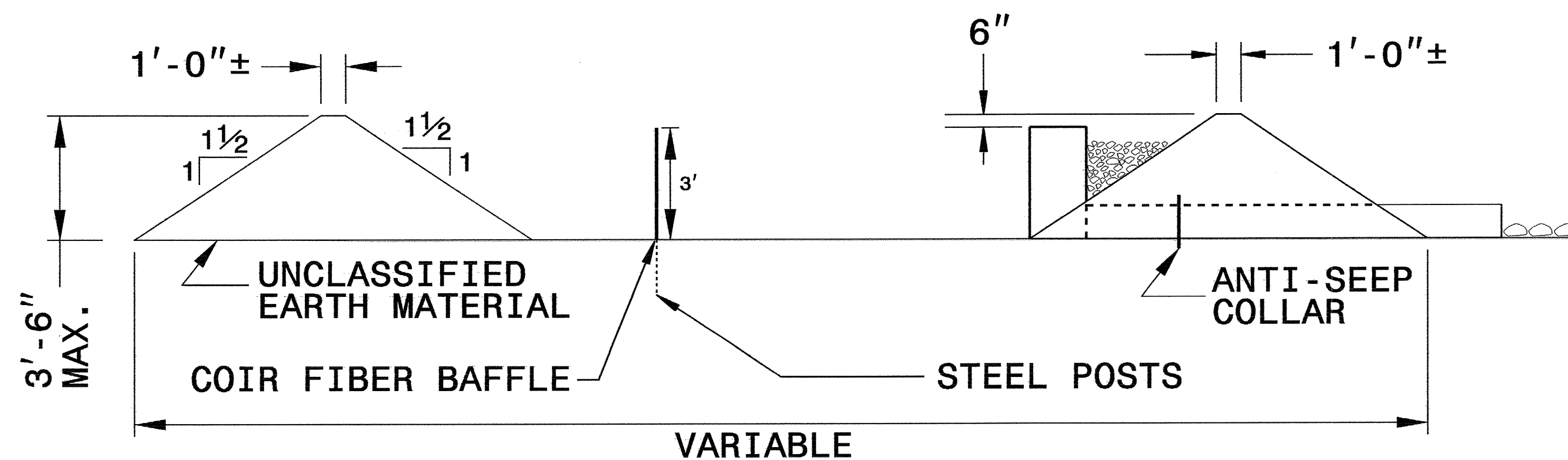
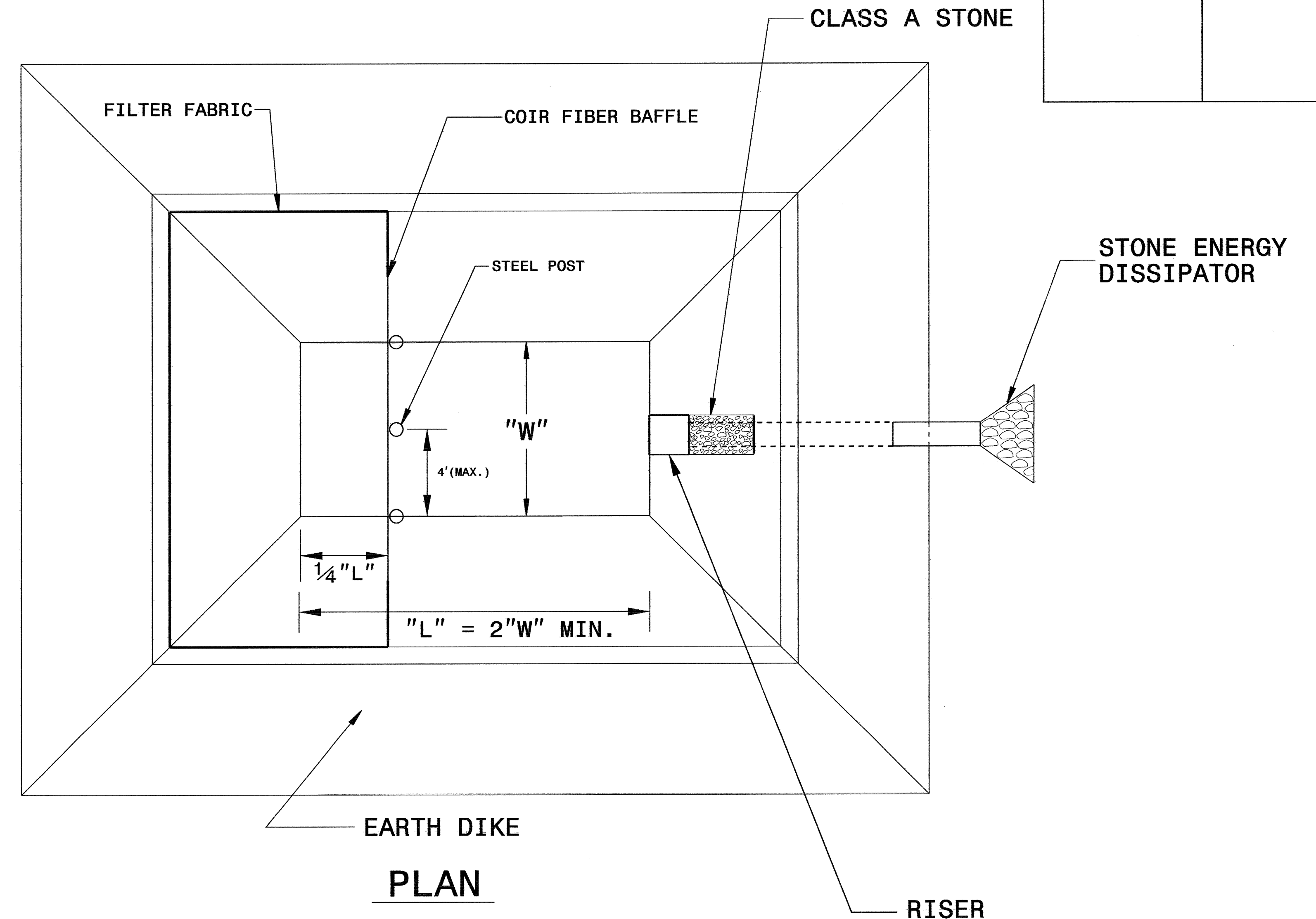
DO NOT EXCEED 3 1/2 FT. IN HEIGHT FOR THE EARTH DIKES REQUIRED FOR BORROW PIT DEWATERING BASIN.

THE BORROW PIT DEWATERING BASIN SIZE IS VARIABLE AND DEPENDENT ON SPECIFIC SITE REQUIREMENTS AS WELL AS PROPOSED CONSTRUCTION OPERATIONS.

SUBMIT THE SIZE, LOCATION AND RISER PIPE MATERIAL FOR APPROVAL PRIOR TO CONSTRUCTION.

PUMP THE EFFLUENT INTO THE BORROW PIT DEWATERING BASIN TO A MAXIMUM DEPTH OF 6 IN. BELOW TOP OF EARTH DIKE.

PROVIDE A STONE ENERGY DISSIPATOR PAD AT THE OUTLET OF THE PUMP DISCHARGE HOSE AND OUTLET OF THE RISER BARREL IN ACCORDANCE WITH ROADWAY STANDARD DRAWING 876.02 FOR OUTLET W/O DITCH.



TYPICAL SECTION VIEW

NOT TO SCALE



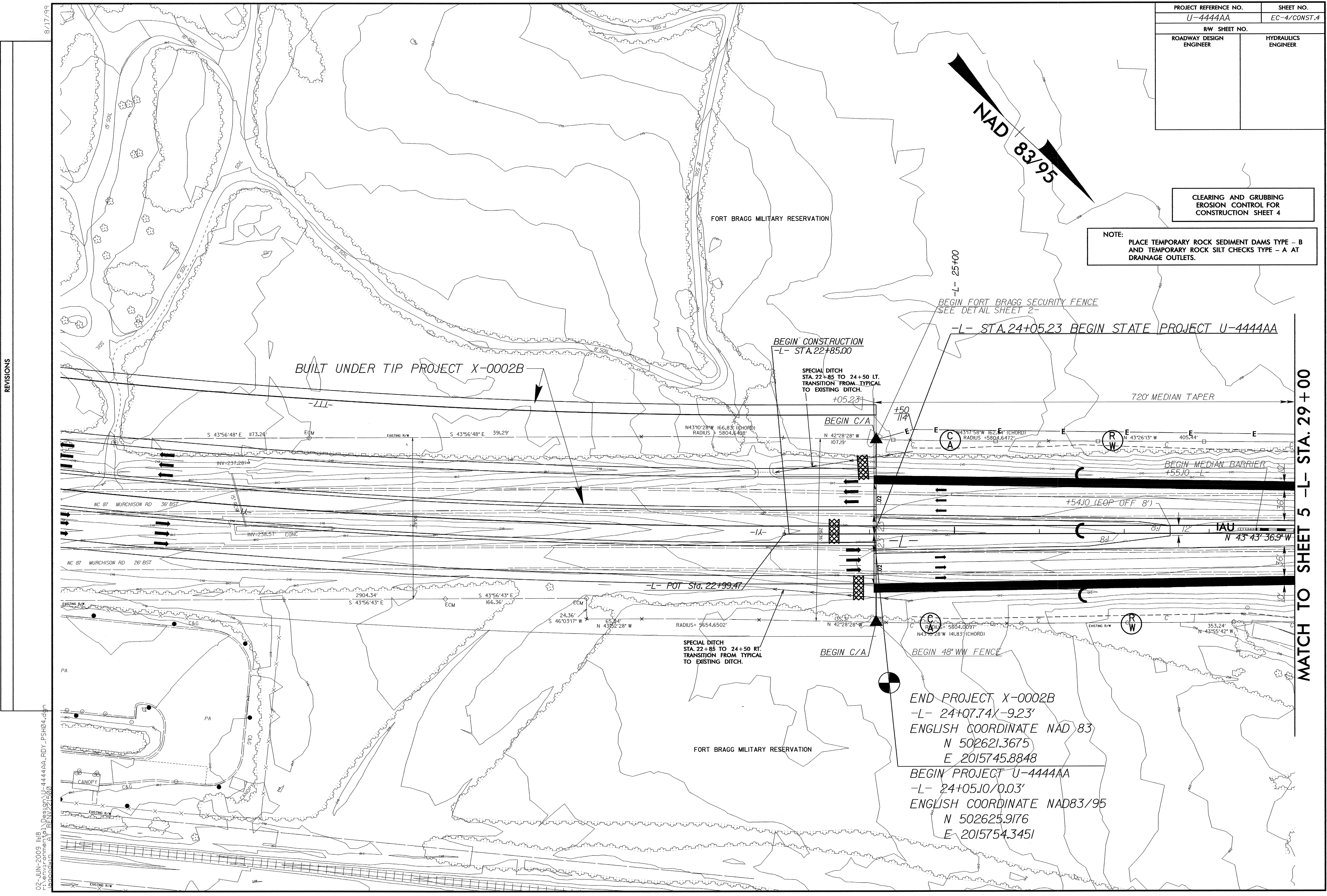
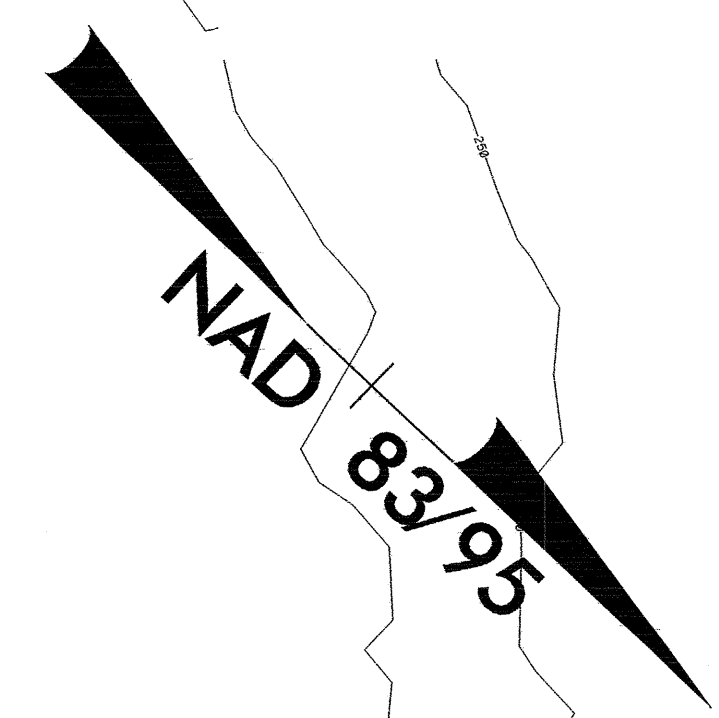




PROJECT REFERENCE NO. U-4444AA	SHEET NO. EC-4/CONST.4
RW SHEET NO.	
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER

CLEARING AND GRUBBING  
EROSION CONTROL FOR  
CONSTRUCTION SHEET 4

NOTE:  
PLACE TEMPORARY ROCK SEDIMENT DAMS TYPE - B  
AND TEMPORARY ROCK SILT CHECKS TYPE - A AT  
DRAINAGE OUTLETS.

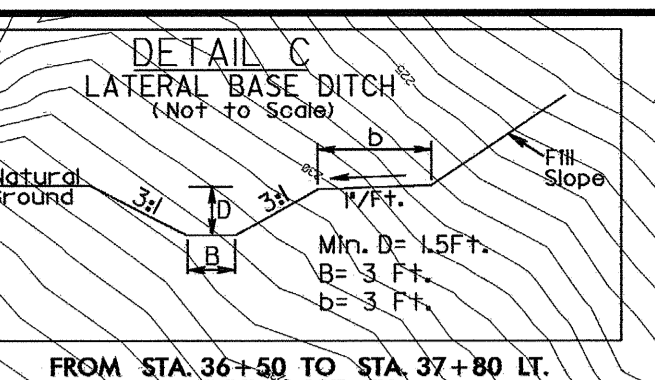
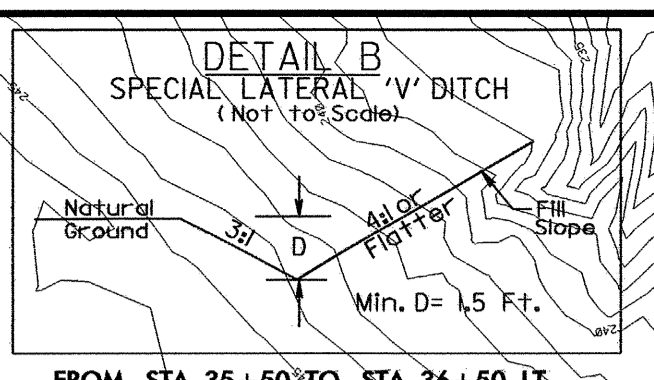
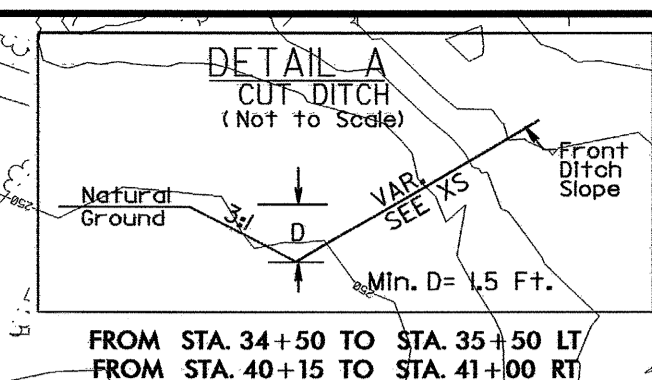


REVISIONS

MATCH TO SHEET 5 -L- STA. 29+00

END PROJECT X-0002B  
-L- 24+07.74X-9.23'  
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N 502621.3675  
E 2015745.8848  
BEGIN PROJECT U-4444AA  
-L- 24+05.10/0.03'  
ENGLISH COORDINATE NAD83/95  
N 502625.9176  
E 2015754.3451

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REVISIONS AT RENY21500



**Modified Silt Basin  
Type 'B'  
37 x 23 x 3  
(See Tiered Skimmer  
Basin Detail)  
ID 5.1**

**37 x 23 x 3  
1.5 inch Skimmer  
with 1.0 inch  
Orifice Diameter  
15 ft. weir  
(See Tiered Skimmer  
Basin Detail)  
ID 5.1**

**24 x 63 x 3  
1.5 inch Skimmer  
with 1.125 inch  
Orifice Diameter  
16 ft. weir  
ID 5.2**

PI Sta 37+02.54 Os = 9° 28' 33.0" Ls = 430.00' LT = 287.08' ST = 143.77'	PI Sta 39+52.05 Δ = 9° 22' 26.2" (RT) D = 4° 24' 26.5" L = 212.69' T = 106.58' R = 1,300.00' e = 8%	PI Sta 44+73.31 Δ = 31° 44' 48.2" (RT) D = 5° 55' 27.7" L = 808.96' T = 415.16' R = 1,460.00' e = 8%	PI Sta 50+27.53 Os = 9° 25' 06.5" Ls = 480.00' LT = 320.45' ST = 160.41'
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MATCH TO SHEET 4 -L- STA. 29+00

MATCH TO SHEET 6 -L- STA. 41+00

REVISIONS

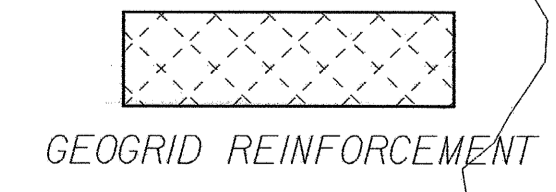
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CLEARING AND GRUBBING  
EROSION CONTROL FOR  
CONSTRUCTION SHEET 05

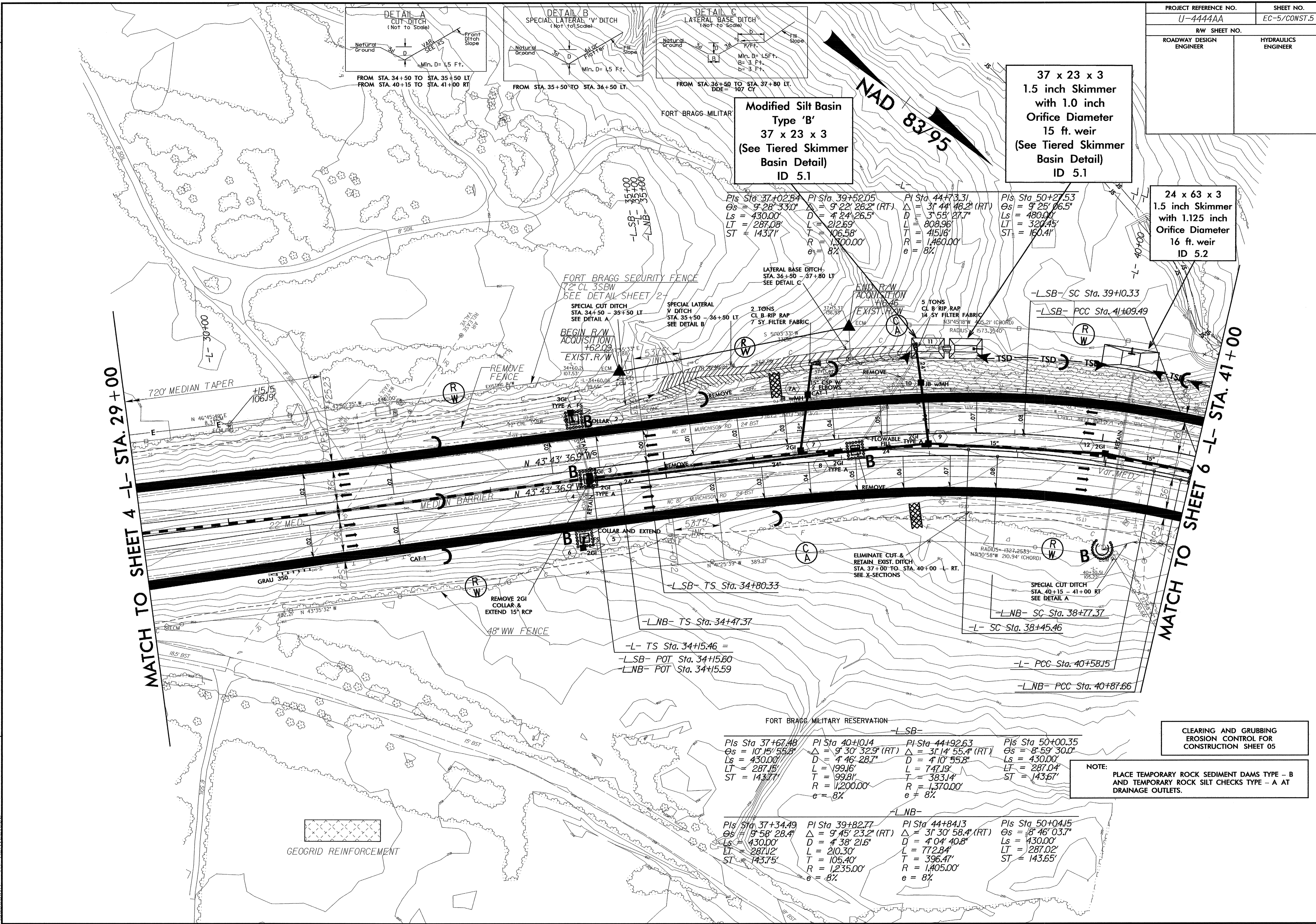
NOTE:  
PLACE TEMPORARY ROCK SEDIMENT DAMS TYPE - B  
AND TEMPORARY ROCK SILT CHECKS TYPE - A AT  
DRAINAGE OUTLETS.

PIs Sta 37+67.48 Os = 10° 15' 56.8" Ls = 430.00' LT = 287.15' ST = 143.77'	PI Sta 40+10.14 Δ = 9° 30' 32.9" (RT) D = 4° 46' 28.7" L = 99.16' T = 99.81' R = 1,200.00' e = 8%	PI Sta 44+92.63 Δ = 31° 14' 55.4" (RT) D = 4° 10' 55.8" L = 747.19' T = 383.14' R = 1,370.00' e = 8%	PIs Sta 50+00.35 Os = 8° 59' 30.0" Ls = 430.00' LT = 287.04' ST = 143.67'
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PIs Sta 37+34.49 Os = 9° 58' 28.4" Ls = 430.00' LT = 287.12' ST = 143.75'	PI Sta 39+82.77 Δ = 9° 45' 23.2" (RT) D = 4° 38' 21.6" L = 210.30' T = 105.40' R = 1,235.00' e = 8%	PI Sta 44+84.13 Δ = 31° 30' 58.4" (RT) D = 4° 04' 40.8" L = 772.84' T = 396.47' R = 1,405.00' e = 8%	PIs Sta 50+04.15 Os = 8° 46' 03.7" Ls = 430.00' LT = 287.02' ST = 143.65'
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GEOGRID REINFORCEMENT

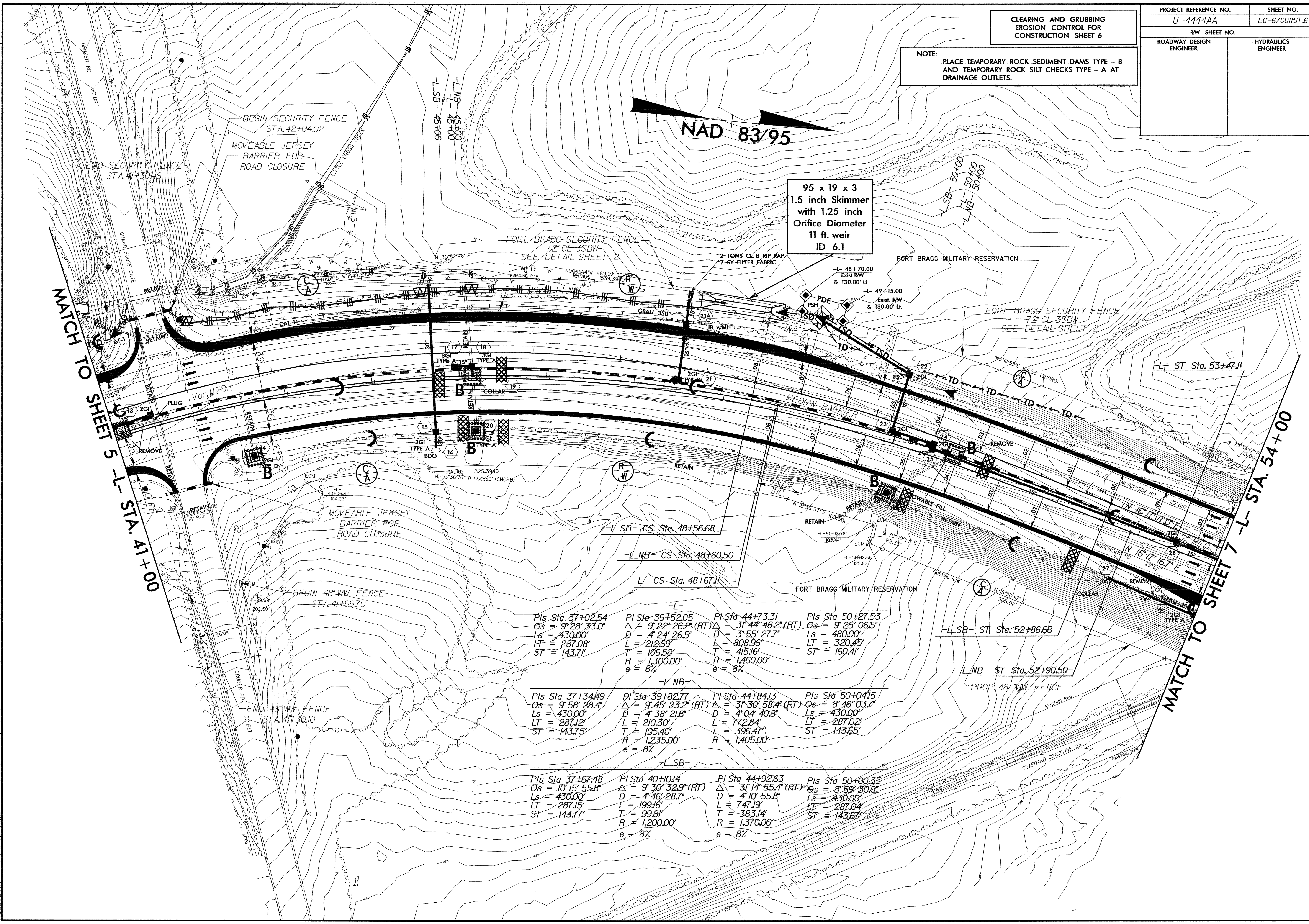


**CLEARING AND GRUBBING  
EROSION CONTROL FOR  
CONSTRUCTION SHEET 6**

NOTE:  
PLACE TEMPORARY ROCK SEDIMENT DAMS TYPE - B  
AND TEMPORARY ROCK SILT CHECKS TYPE - A AT  
DRAINAGE OUTLETS.



95 x 19 x 3  
1.5 inch Skimmer  
with 1.25 inch  
Orifice Diameter  
11 ft. weir  
ID 6.1



Pls Sta 37+02.54 Os = 9' 28' 33.0" Ls = 430.00' LT = 287.08' ST = 143.71'	Pls Sta 39+52.05 Δ = 9' 22' 26.2" (RT) D = 4' 24' 26.5" L = 212.69' T = 106.58' R = 1,300.00' e = 8%	Pls Sta 44+73.31 Δ = 31' 44' 48.2" (RT) D = 3' 55' 27.7" L = 808.96' T = 415.16' R = 1,460.00' e = 8%	Pls Sta 50+27.53 Os = 9' 25' 06.5" Ls = 480.00' LT = 320.45' ST = 160.41'
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Pls Sta 37+34.49 Os = 9' 58' 28.4" Ls = 430.00' LT = 287.12' ST = 143.75'	Pls Sta 39+82.77 Δ = 9' 45' 23.2" (RT) D = 4' 38' 21.6" L = 210.30' T = 105.40' R = 1,235.00' e = 8%	Pls Sta 44+84.13 Δ = 31' 30' 58.4" (RT) D = 4' 04' 40.8" L = 772.84' T = 396.47' R = 1,405.00'	Pls Sta 50+04.15 Os = 8' 46' 03.7" Ls = 430.00' LT = 287.02' ST = 143.65'
---	--	---	---

Pls Sta 37+67.48 Os = 10' 15' 55.8" Ls = 430.00' LT = 287.15' ST = 143.77'	Pls Sta 40+10.14 Δ = 9' 30' 32.9" (RT) D = 4' 46' 28.7" L = 199.16' T = 99.81' R = 1,200.00' e = 8%	Pls Sta 44+92.63 Δ = 31' 14' 55.4" (RT) D = 4' 10' 55.8" L = 747.19' T = 383.14' R = 1,370.00'	Pls Sta 50+00.35 Os = 8' 59' 30.0" Ls = 430.00' LT = 287.04' ST = 143.67'
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REVISIONS

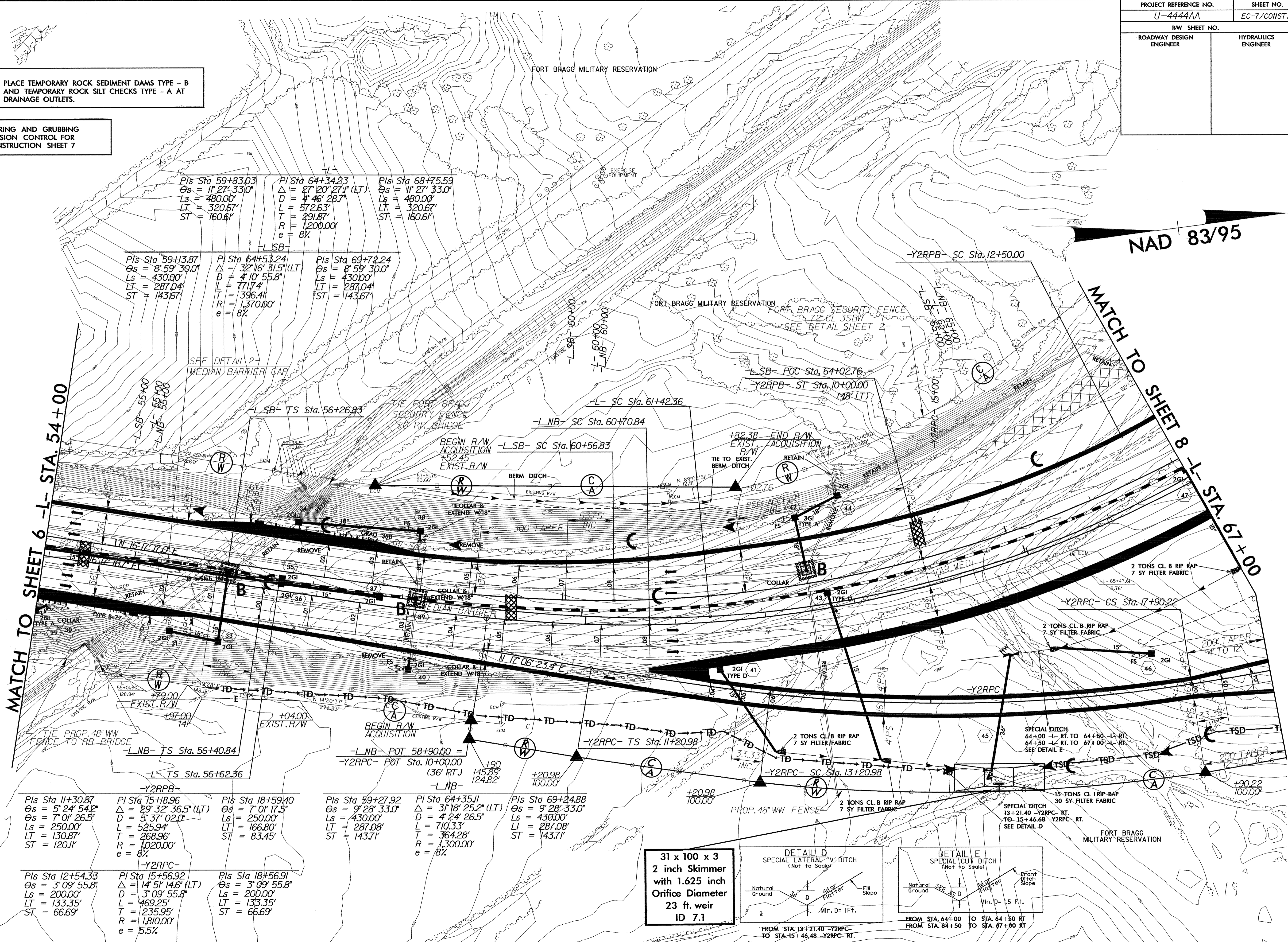
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PROJECT REFERENCE NO. U-4444AA	SHEET NO. EC-7/CONST.7
RW SHEET NO.	
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER

NOTE:  
PLACE TEMPORARY ROCK SEDIMENT DAMS TYPE - B AND TEMPORARY ROCK SILT CHECKS TYPE - A AT DRAINAGE OUTLETS.

CLEARING AND GRUBBING  
EROSION CONTROL FOR  
CONSTRUCTION SHEET 7

NAD 83/95



MATCH TO SHEET 6 -L- STA. 54+00

MATCH TO SHEET 8 -L- STA. 67+00

Pls Sta 11+30.87  
 $\Theta_s = 5^\circ 24' 54.2''$   
 $\Theta_s = 7^\circ 01' 26.5''$   
 $L_s = 250.00'$   
 $LT = 130.87'$   
 $ST = 120.11'$

Pls Sta 15+18.96  
 $\Delta = 29^\circ 32' 36.5''$  (LT)  
 $D = 5^\circ 37' 02.0''$   
 $L = 525.94'$   
 $T = 268.96'$   
 $R = 1,020.00'$   
 $e = 8\%$

Pls Sta 18+59.40  
 $\Theta_s = 7^\circ 01' 17.5''$   
 $\Theta_s = 250.00'$   
 $LT = 166.80'$   
 $ST = 83.45'$

Pls Sta 59+27.92  
 $\Theta_s = 9^\circ 28' 33.0''$   
 $L_s = 430.00'$   
 $LT = 287.08'$   
 $ST = 143.71'$

Pls Sta 64+35.11  
 $\Delta = 31^\circ 18' 25.2''$  (LT)  
 $D = 4^\circ 24' 26.5''$   
 $L = 710.33'$   
 $T = 364.28'$   
 $R = 1,300.00'$   
 $e = 8\%$

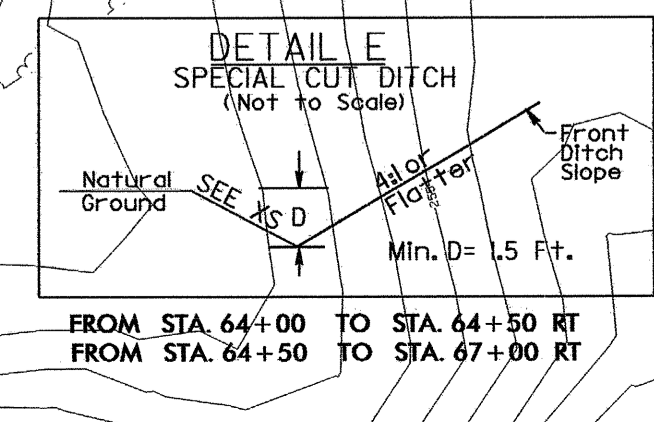
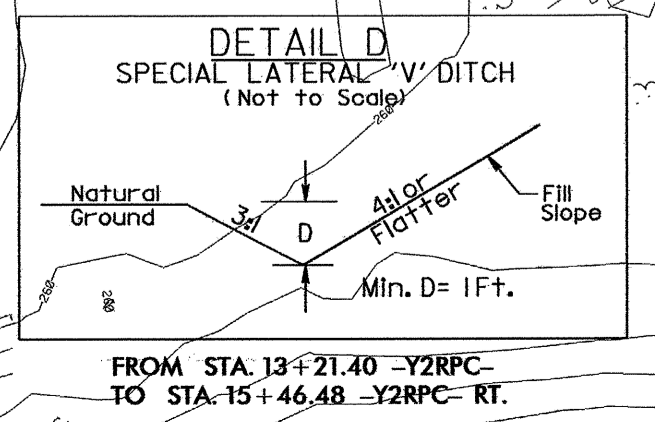
Pls Sta 69+24.88  
 $\Theta_s = 9^\circ 28' 33.0''$   
 $L_s = 430.00'$   
 $LT = 287.08'$   
 $ST = 143.71'$

Pls Sta 12+54.33  
 $\Theta_s = 3^\circ 09' 55.8''$   
 $L_s = 200.00'$   
 $LT = 133.35'$   
 $ST = 66.69'$

Pls Sta 15+56.92  
 $\Delta = 14^\circ 51' 14.6''$  (LT)  
 $D = 3^\circ 09' 55.8''$   
 $L = 469.25'$   
 $T = 235.95'$   
 $R = 1,810.00'$   
 $e = 5.5\%$

Pls Sta 18+56.91  
 $\Theta_s = 3^\circ 09' 55.8''$   
 $L_s = 200.00'$   
 $LT = 133.35'$   
 $ST = 66.69'$

31 x 100 x 3  
 2 inch Skimmer  
 with 1.625 inch  
 Orifice Diameter  
 23 ft. weir  
 ID 7.1



FROM STA. 13+21.40 -Y2RPC-  
TO STA. 15+46.68 -Y2RPC- RT.

FROM STA. 64+00 TO STA. 64+50 RT  
FROM STA. 64+50 TO STA. 67+00 RT

REVISIONS

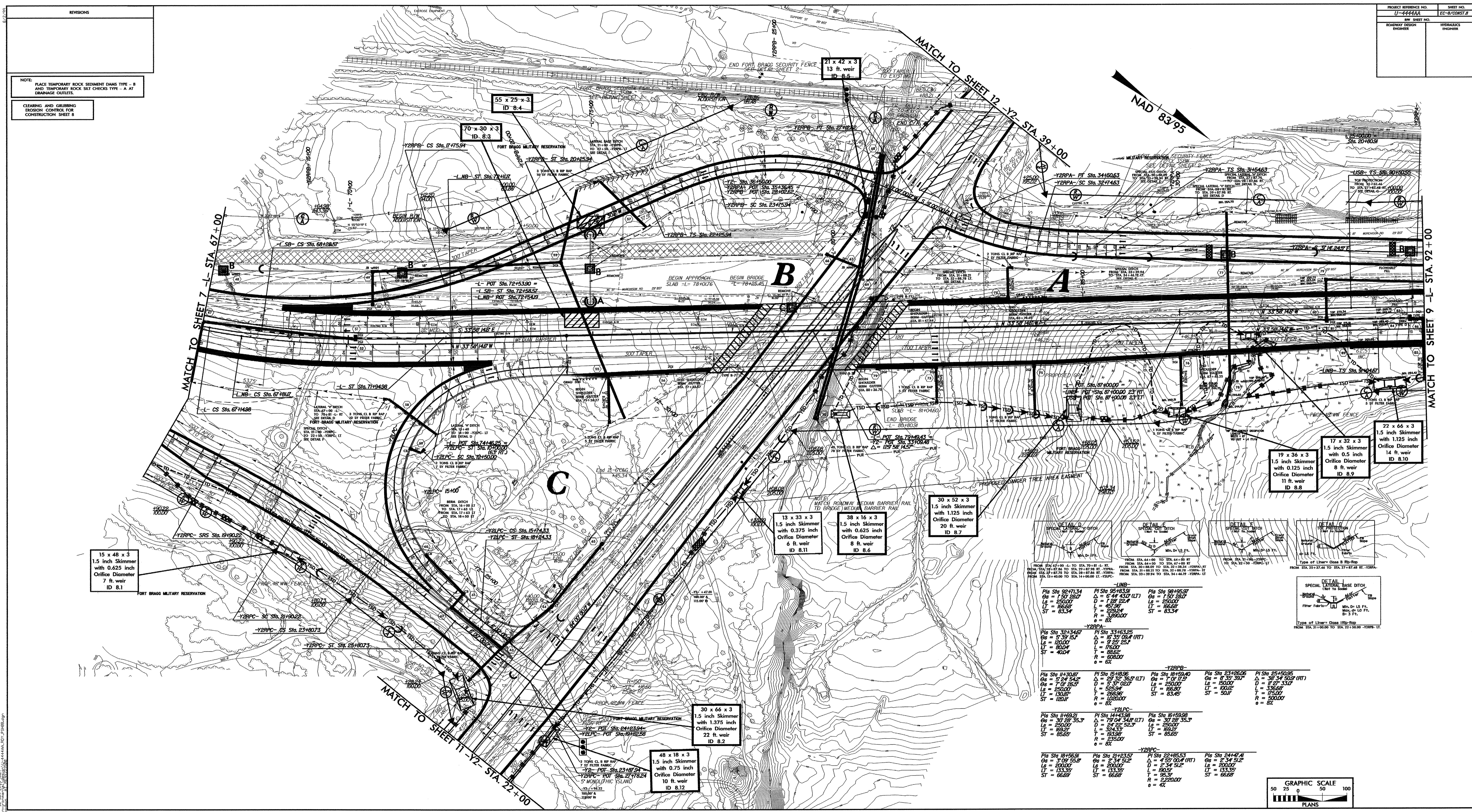
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REVISIONS

NOTE:  
 PLACE TEMPORARY ROCK SEDIMENT DAMS TYPE - B  
 AND TEMPORARY ROCK SET CHECKS TYPE - A AT  
 DRAINAGE OUTLETS.

CLEARING AND GRUBBING  
 POSITION CONTROL FOR  
 CONSTRUCTION SHEET 8

PROJECT REFERENCE NO.	SHEET NO.
U-4444A	EC-8/CONST# 8
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER



15 x 48 x 3  
 1.5 inch Skimmer  
 with 0.625 inch  
 Orifice Diameter  
 7 ft. weir  
 ID 8.1

55 x 25 x 3  
 ID 8.4

70 x 30 x 3  
 ID 8.3

21 x 42 x 3  
 13 ft. weir  
 ID 8.5

13 x 33 x 3  
 1.5 inch Skimmer  
 with 0.375 inch  
 Orifice Diameter  
 6 ft. weir  
 ID 8.11

38 x 16 x 3  
 1.5 inch Skimmer  
 with 0.625 inch  
 Orifice Diameter  
 8 ft. weir  
 ID 8.6

30 x 52 x 3  
 1.5 inch Skimmer  
 with 1.125 inch  
 Orifice Diameter  
 20 ft. weir  
 ID 8.7

19 x 36 x 3  
 1.5 inch Skimmer  
 with 0.125 inch  
 Orifice Diameter  
 11 ft. weir  
 ID 8.8

17 x 32 x 3  
 1.5 inch Skimmer  
 with 1.125 inch  
 Orifice Diameter  
 14 ft. weir  
 ID 8.10

22 x 66 x 3  
 1.5 inch Skimmer  
 with 1.125 inch  
 Orifice Diameter  
 14 ft. weir  
 ID 8.10

30 x 46 x 3  
 1.5 inch Skimmer  
 with 1.375 inch  
 Orifice Diameter  
 22 ft. weir  
 ID 8.2

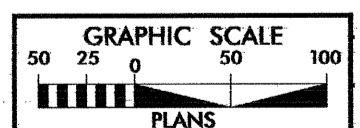
48 x 18 x 3  
 1.5 inch Skimmer  
 with 0.75 inch  
 Orifice Diameter  
 10 ft. weir  
 ID 8.12

Pile Sta 92+17.34 $\Delta = 150' 28.0'$ $D = 6' 4" 43.0'$ (LT) $L = 150' 28.0'$ $LT = 65.00'$ $ST = 83.34'$ $R = 3300.0'$ $\theta = 8.0'$	Pile Sta 92+18.31 $\Delta = 6' 4" 43.0'$ (LT) $L = 150' 28.0'$ $LT = 65.00'$ $ST = 83.34'$ $R = 3300.0'$ $\theta = 8.0'$	Pile Sta 92+19.27 $\Delta = 150' 28.0'$ $D = 6' 4" 43.0'$ (LT) $L = 150' 28.0'$ $LT = 65.00'$ $ST = 83.34'$ $R = 3300.0'$ $\theta = 8.0'$
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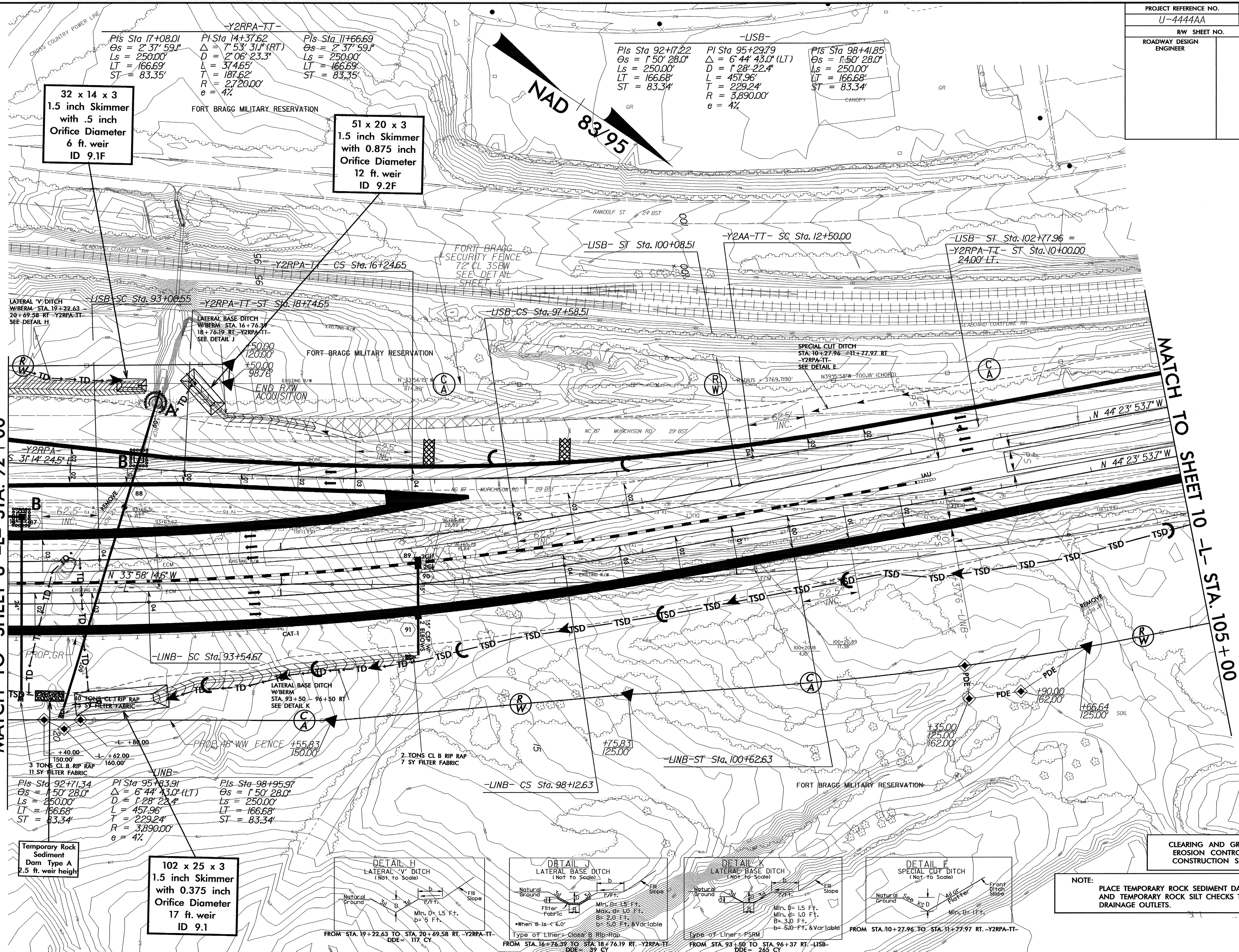
Pile Sta 32+34.67 $\Delta = 5' 39' 15.7"$ $D = 120.00'$ $L = 80.04'$ $LT = 40.04'$ $R = 66.67'$ $\theta = 6.0'$	Pile Sta 32+36.25 $\Delta = 16' 39' 09.4'$ (RT) $D = 9' 25' 25.4"$ $L = 176.00'$ $LT = 88.00'$ $ST = 88.00'$ $R = 66.67'$ $\theta = 6.0'$	Pile Sta 15+10.96 $\Delta = 29' 32' 36.5'$ (LT) $D = 5' 37' 02.7"$ $L = 526.94'$ $LT = 120.00'$ $ST = 120.00'$ $R = 1200.00'$ $\theta = 8.0'$	Pile Sta 15+10.40 $\Delta = 7' 07' 17.5"$ $D = 150.00'$ $L = 166.80'$ $LT = 83.45'$ $ST = 83.45'$ $R = 5000.00'$ $\theta = 8.0'$	Pile Sta 23+26.06 $\Delta = 39' 34' 50.9'$ (RT) $D = 11' 27' 53.0"$ $L = 336.84'$ $LT = 175.00'$ $ST = 50.00'$ $R = 5000.00'$ $\theta = 8.0'$
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Pile Sta 11+68.21 $\Delta = 30' 22' 35.3"$ $D = 250.00'$ $L = 168.51'$ $LT = 66.67'$ $ST = 66.67'$ $R = 66.67'$ $\theta = 8.0'$	Pile Sta 14+13.98 $\Delta = 7' 07' 17.5"$ $D = 324.51'$ $L = 168.51'$ $LT = 83.45'$ $ST = 83.45'$ $R = 2350.00'$ $\theta = 8.0'$	Pile Sta 16+19.98 $\Delta = 30' 22' 35.3"$ $D = 250.00'$ $L = 168.51'$ $LT = 66.67'$ $ST = 66.67'$ $R = 66.67'$ $\theta = 8.0'$	Pile Sta 16+19.98 $\Delta = 30' 22' 35.3"$ $D = 250.00'$ $L = 168.51'$ $LT = 66.67'$ $ST = 66.67'$ $R = 66.67'$ $\theta = 8.0'$
--	---	--	--

Pile Sta 18+56.51 $\Delta = 3' 09' 55.0"$ $D = 200.00'$ $L = 133.33'$ $LT = 66.67'$ $ST = 66.67'$ $R = 66.67'$ $\theta = 8.0'$	Pile Sta 21+21.57 $\Delta = 2' 34' 51.2"$ $D = 200.00'$ $L = 133.33'$ $LT = 66.67'$ $ST = 66.67'$ $R = 66.67'$ $\theta = 8.0'$	Pile Sta 22+55.53 $\Delta = 4' 59' 03.4'$ (RT) $D = 2' 34' 51.2"$ $L = 200.00'$ $LT = 133.33'$ $ST = 66.67'$ $R = 66.67'$ $\theta = 8.0'$	Pile Sta 24+47.41 $\Delta = 2' 34' 51.2"$ $D = 200.00'$ $L = 133.33'$ $LT = 66.67'$ $ST = 66.67'$ $R = 66.67'$ $\theta = 8.0'$
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PROJECT REFERENCE NO.	SHEET NO.
U-4444AA	EC-9/CONST.9
R/W SHEET NO.	HYDRAULICS ENGINEER
ROADWAY DESIGN ENGINEER	

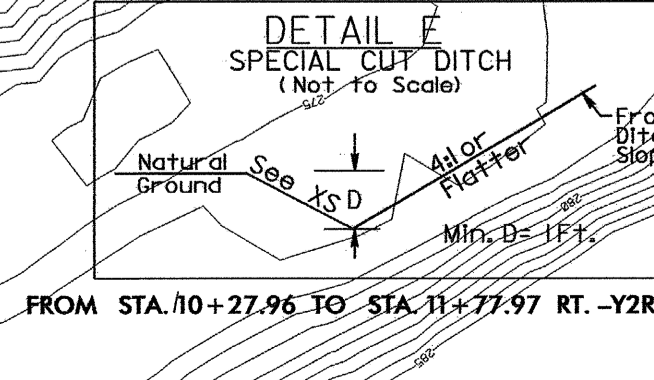
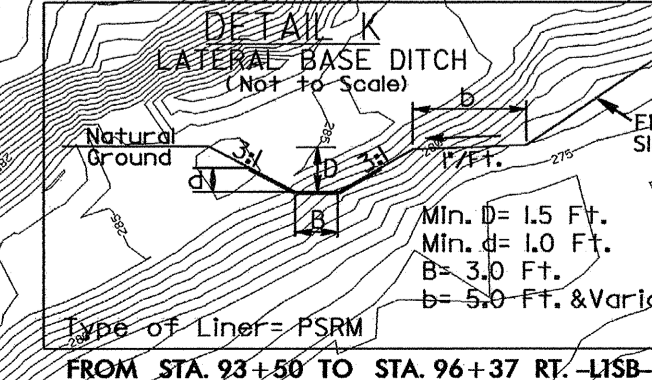
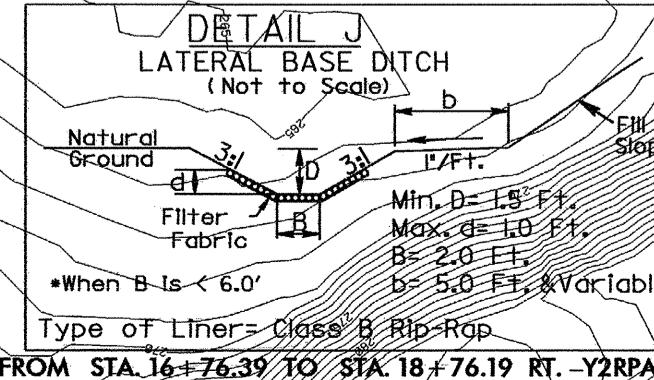
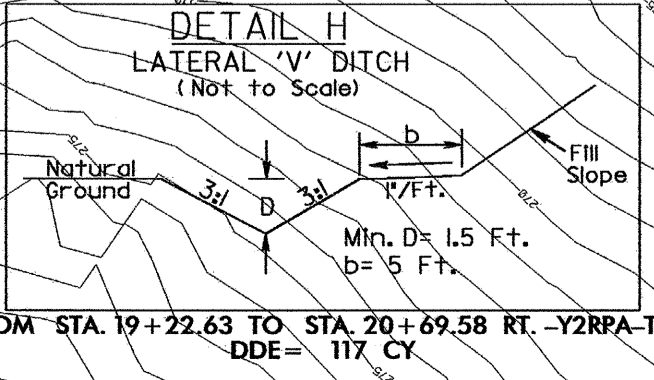


32 x 14 x 3  
1.5 inch Skimmer  
with .5 inch  
Orifice Diameter  
6 ft. weir  
ID 9.1F

51 x 20 x 3  
1.5 inch Skimmer  
with 0.875 inch  
Orifice Diameter  
12 ft. weir  
ID 9.2F

Temporary Rock  
Sediment  
Dam Type A  
2.5 ft. weir height

102 x 25 x 3  
1.5 inch Skimmer  
with 0.375 inch  
Orifice Diameter  
17 ft. weir  
ID 9.1



CLEARING AND GRUBBING  
EROSION CONTROL FOR  
CONSTRUCTION SHEET 9

NOTE:  
PLACE TEMPORARY ROCK SEDIMENT DAMS TYPE - B  
AND TEMPORARY ROCK SILT CHECKS TYPE - A AT  
DRAINAGE OUTLETS.

MATCH TO SHEET 8 - L- STA. 92+00

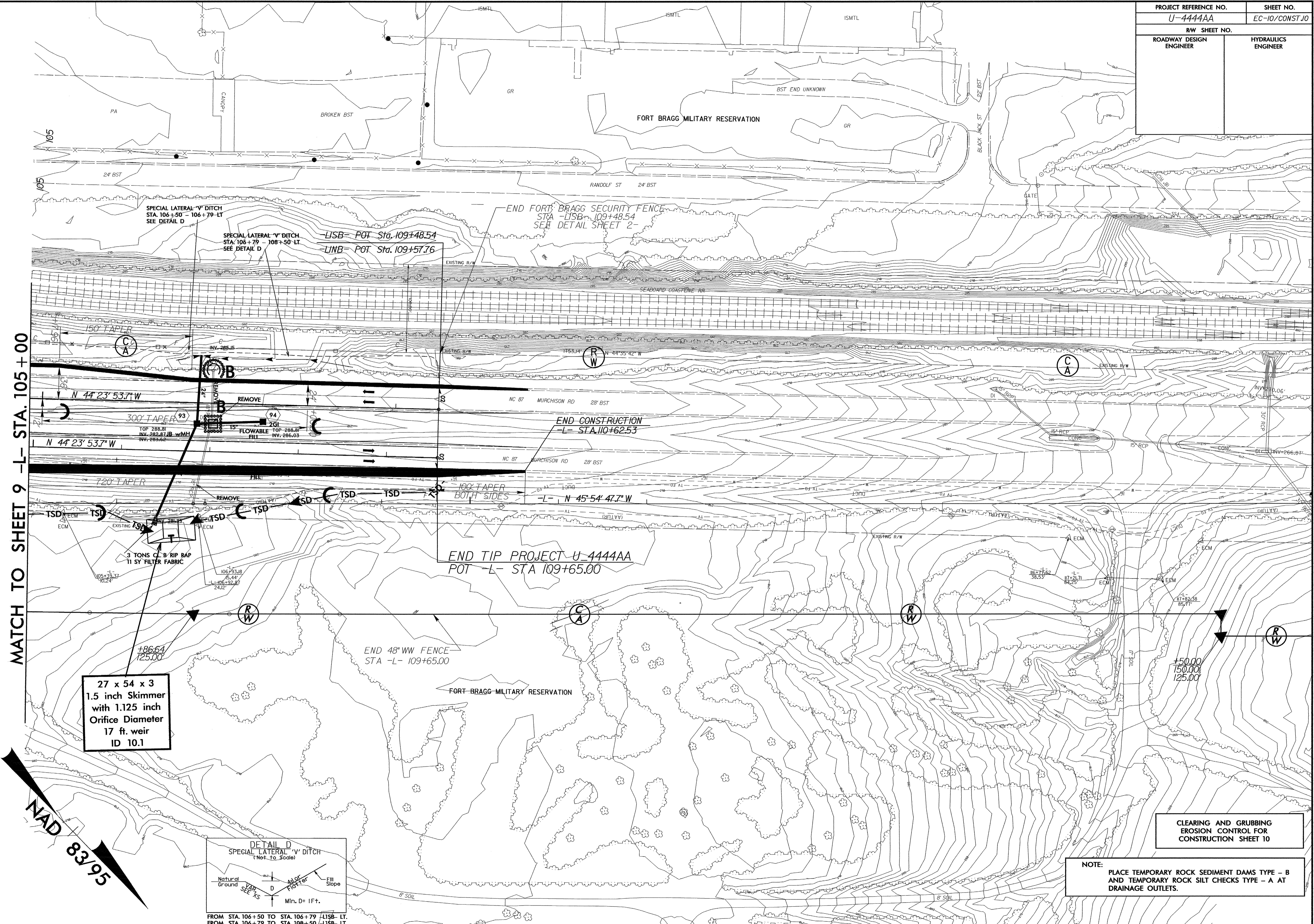
MATCH TO SHEET 10 - L- STA. 105+00

REVISIONS

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8/17/99  
A1: RENV21500

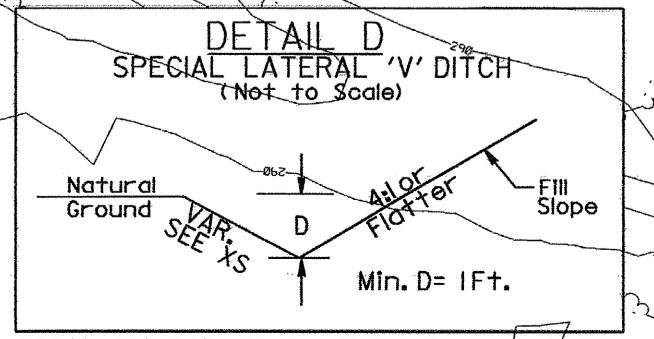


PROJECT REFERENCE NO. U-4444AA	SHEET NO. EC-10/CONST.10
RW SHEET NO.	
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER



MATCH TO SHEET 9 -L- STA. 105 + 00

27 x 54 x 3  
1.5 inch Skimmer  
with 1.125 inch  
Orifice Diameter  
17 ft. weir  
ID 10.1



NOTE:  
PLACE TEMPORARY ROCK SEDIMENT DAMS TYPE - B  
AND TEMPORARY ROCK SILT CHECKS TYPE - A AT  
DRAINAGE OUTLETS.

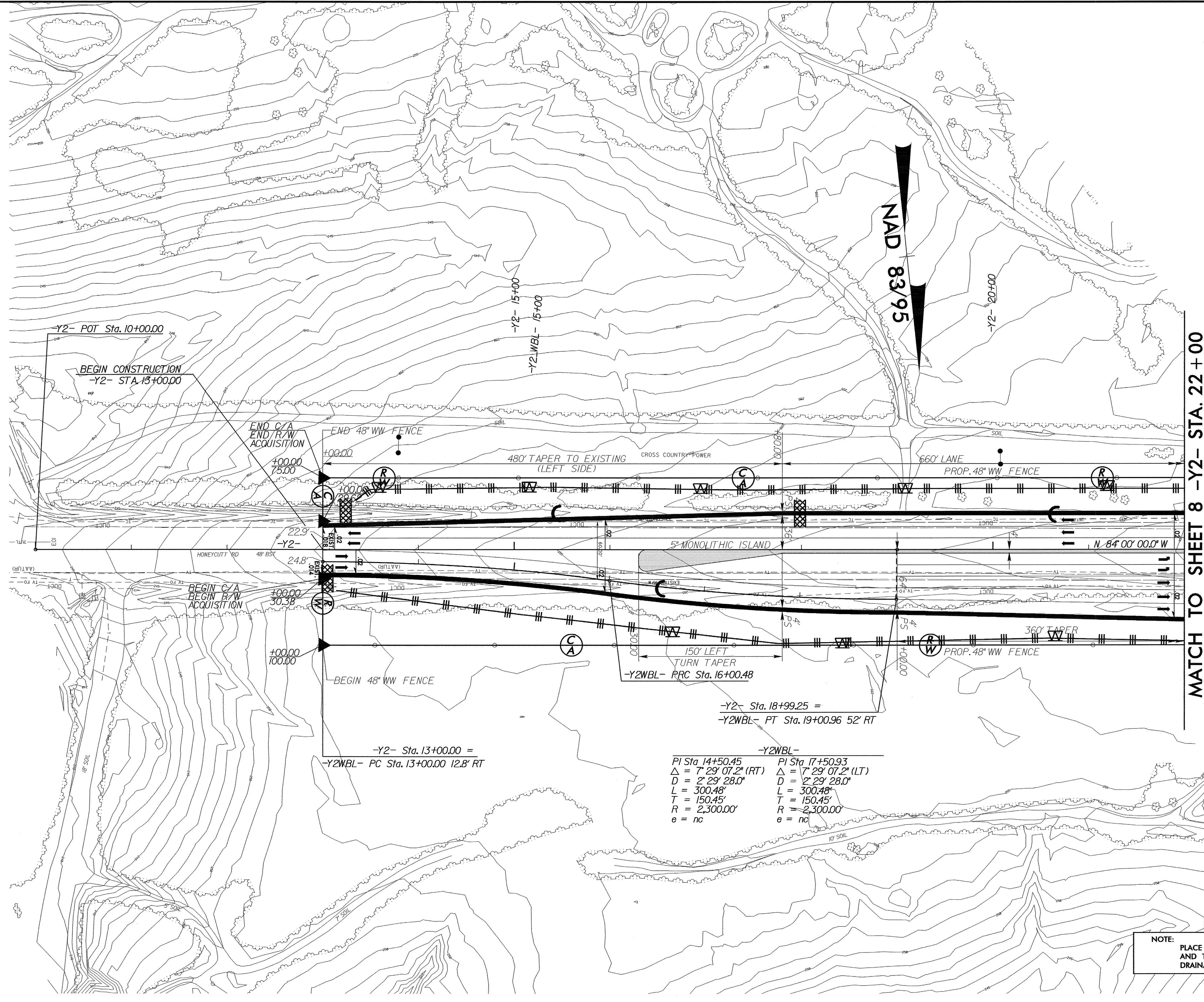
CLEARING AND GRUBBING  
EROSION CONTROL FOR  
CONSTRUCTION SHEET 10

NAD 83/95

REVISIONS

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PROJECT REFERENCE NO.	SHEET NO.
U-4444AA	EC-II/CONST.II
R/W SHEET NO.	HYDRAULICS ENGINEER
ROADWAY DESIGN ENGINEER	



-Y2WBL- PI Sta 14+50.45 Δ = 7° 29' 07.2" (RT) D = 2° 29' 28.0" L = 300.48' T = 150.45' R = 2,300.00' e = nc	-Y2WBL- PI Sta 17+50.93 Δ = 7° 29' 07.2" (LT) D = 2° 29' 28.0" L = 300.48' T = 150.45' R = 2,300.00' e = nc
--	--

CLEARING AND GRUBBING  
EROSION CONTROL FOR  
CONSTRUCTION SHEET 11

NOTE:  
PLACE TEMPORARY ROCK SEDIMENT DAMS TYPE - B  
AND TEMPORARY ROCK SILT CHECKS TYPE - A AT  
DRAINAGE OUTLETS.

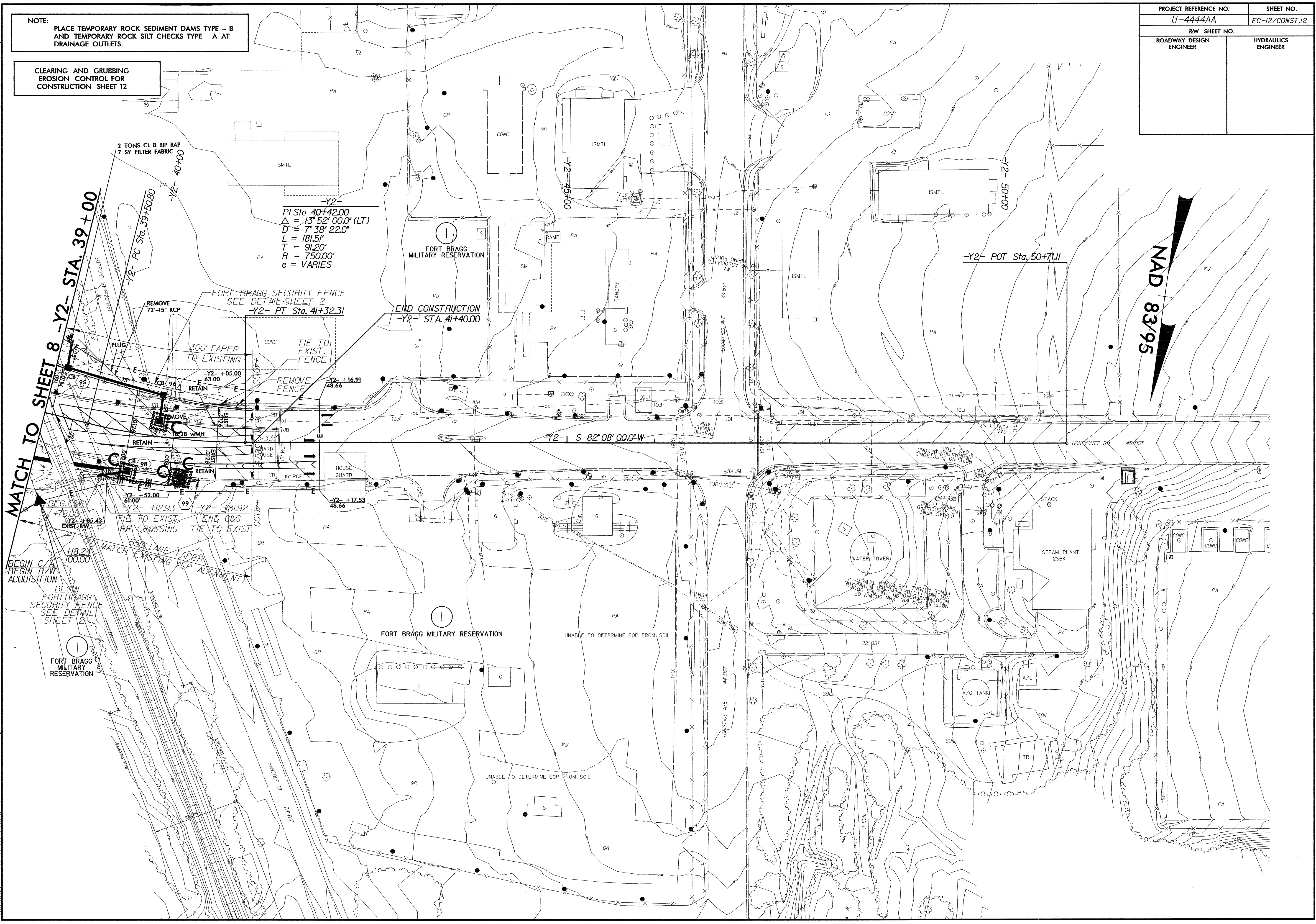
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 PL RENV21500

PROJECT REFERENCE NO.	SHEET NO.
U-4444AA	EC-12/CONST 12
RW SHEET NO.	
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER

NOTE: PLACE TEMPORARY ROCK SEDIMENT DAMS TYPE - B AND TEMPORARY ROCK SILT CHECKS TYPE - A AT DRAINAGE OUTLETS.

CLEARING AND GRUBBING EROSION CONTROL FOR CONSTRUCTION SHEET 12

8/17/99



-Y2-  
 PI Sta 40+42.00  
 $\Delta = 13^{\circ} 52' 00.0"$  (LT)  
 $D = 7^{\circ} 38' 22.0"$   
 $L = 181.5'$   
 $T = 91.20'$   
 $R = 750.00'$   
 $e = \text{VARIES}$

NAD 83/95

MATCH TO SHEET 8 -Y2- STA. 39+00

END CONSTRUCTION -Y2- STA. 41+40.00

BEGIN C/A ACQUISITION

FORT BRAGG MILITARY RESERVATION

FORT BRAGG MILITARY RESERVATION

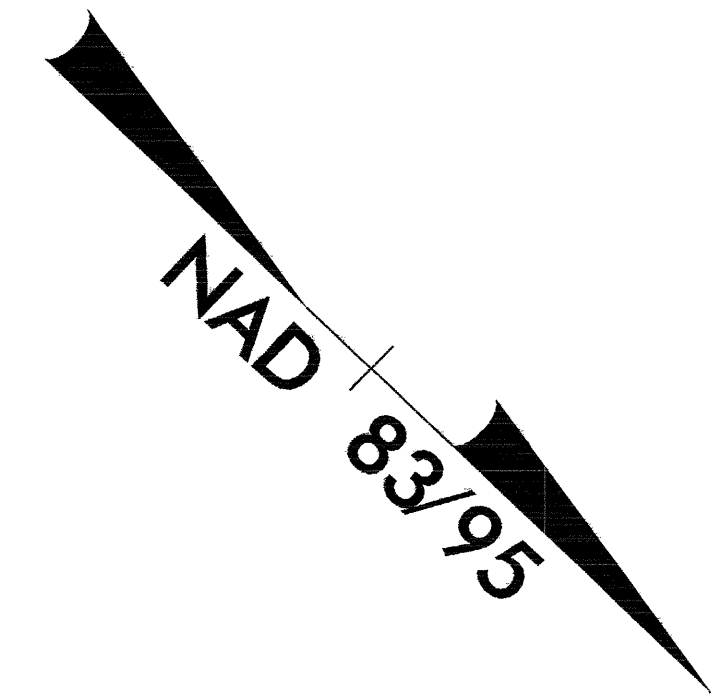
UNABLE TO DETERMINE EOP FROM SOIL

UNABLE TO DETERMINE EOP FROM SOIL

REVISIONS

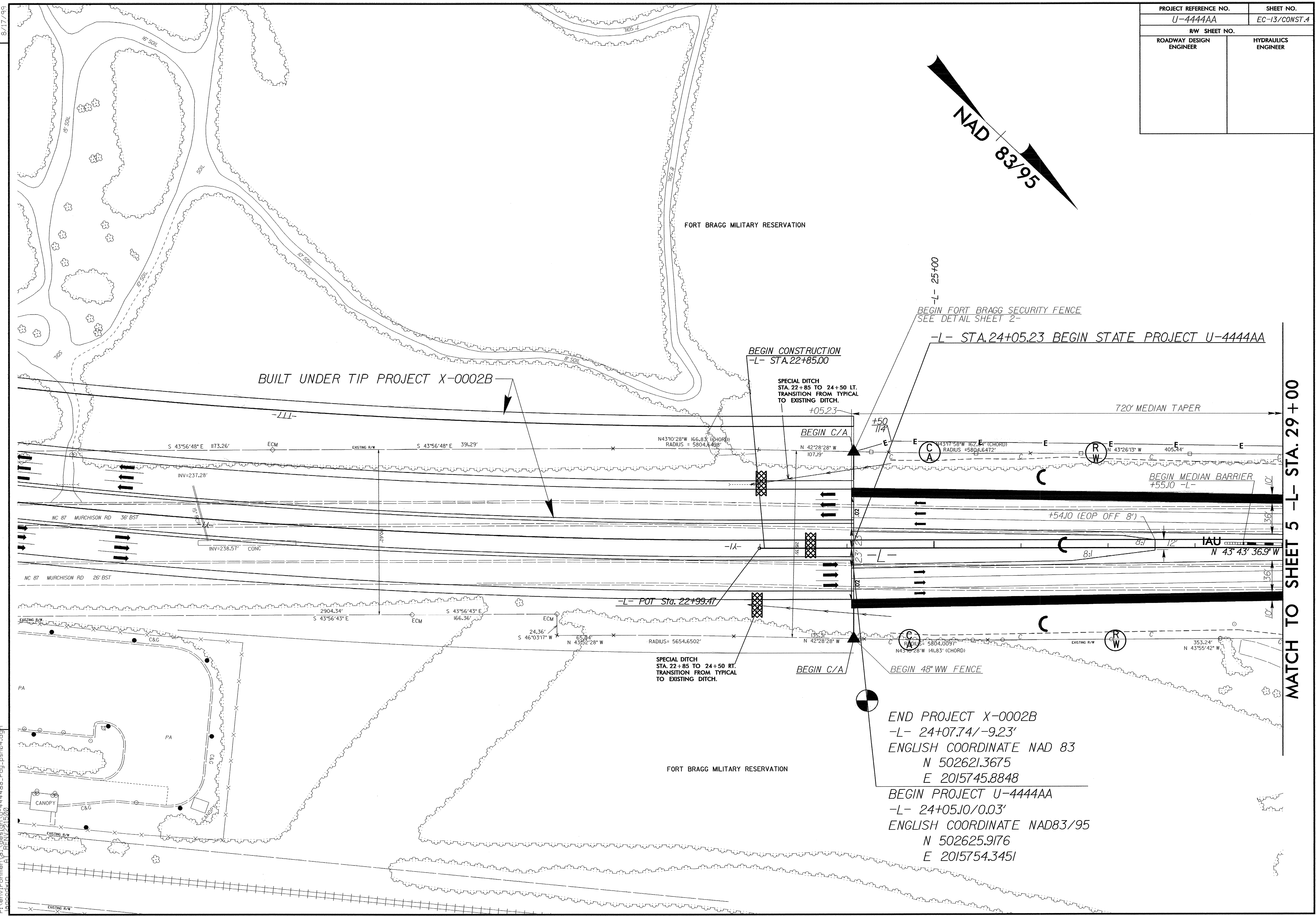
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PROJECT REFERENCE NO. U-4444AA	SHEET NO. EC-13/CONST.4
RW SHEET NO.	
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER



8/17/99

REVISIONS



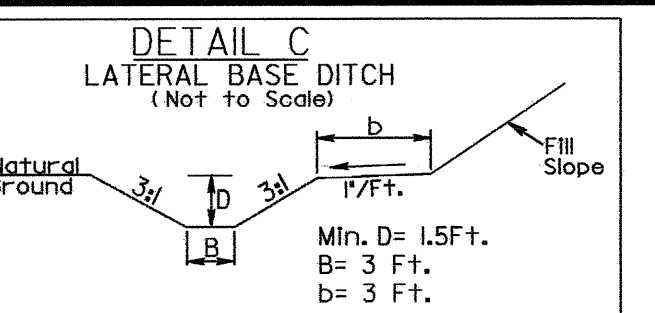
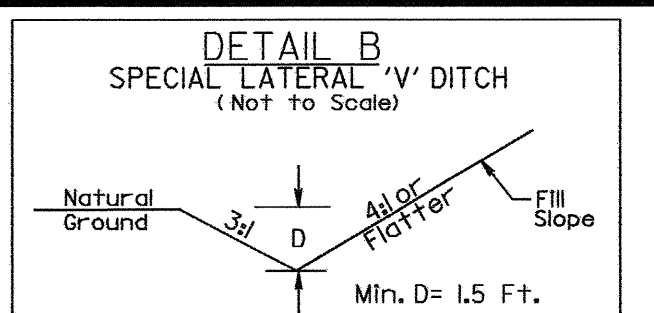
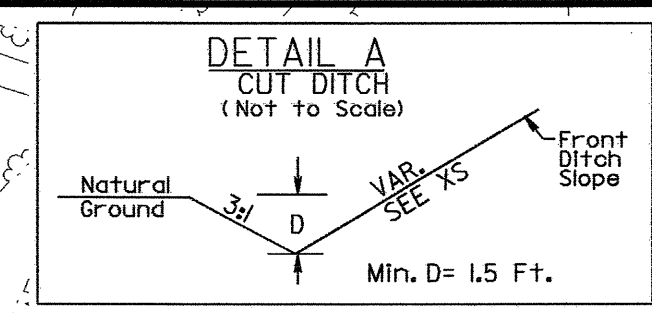
MATCH TO SHEET 5 -L- STA. 29+00

END PROJECT X-0002B  
 -L- 24+07.74/-9.23'  
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 N 502621.3675  
 E 2015745.8848  
 BEGIN PROJECT U-4444AA  
 -L- 24+05.10/0.03'  
 ENGLISH COORDINATE NAD83/95  
 N 502625.9176  
 E 2015754.3451

02-JUN-2009 14:45  
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8/17/99

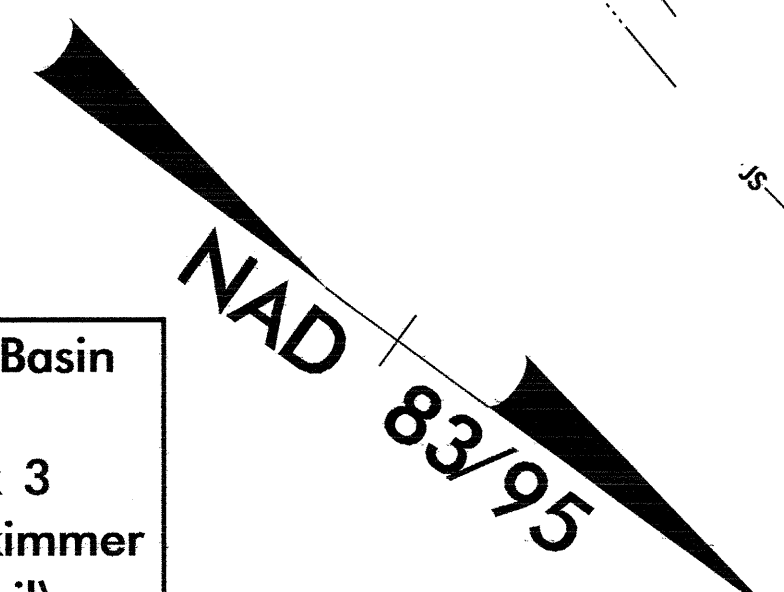
PROJECT REFERENCE NO. U-4444AA	SHEET NO. EC-14/CONST.5
RW SHEET NO.	
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER



FROM STA. 34+50 TO STA. 35+50 LT  
FROM STA. 40+15 TO STA. 41+00 RT

FROM STA. 35+50 TO STA. 36+50 LT

FROM STA. 36+50 TO STA. 37+80 LT.  
DDE = 107 CY



Modified Silt Basin  
Type 'B'  
37 x 23 x 3  
(See Tiered Skimmer  
Basin Detail)  
ID 5.1

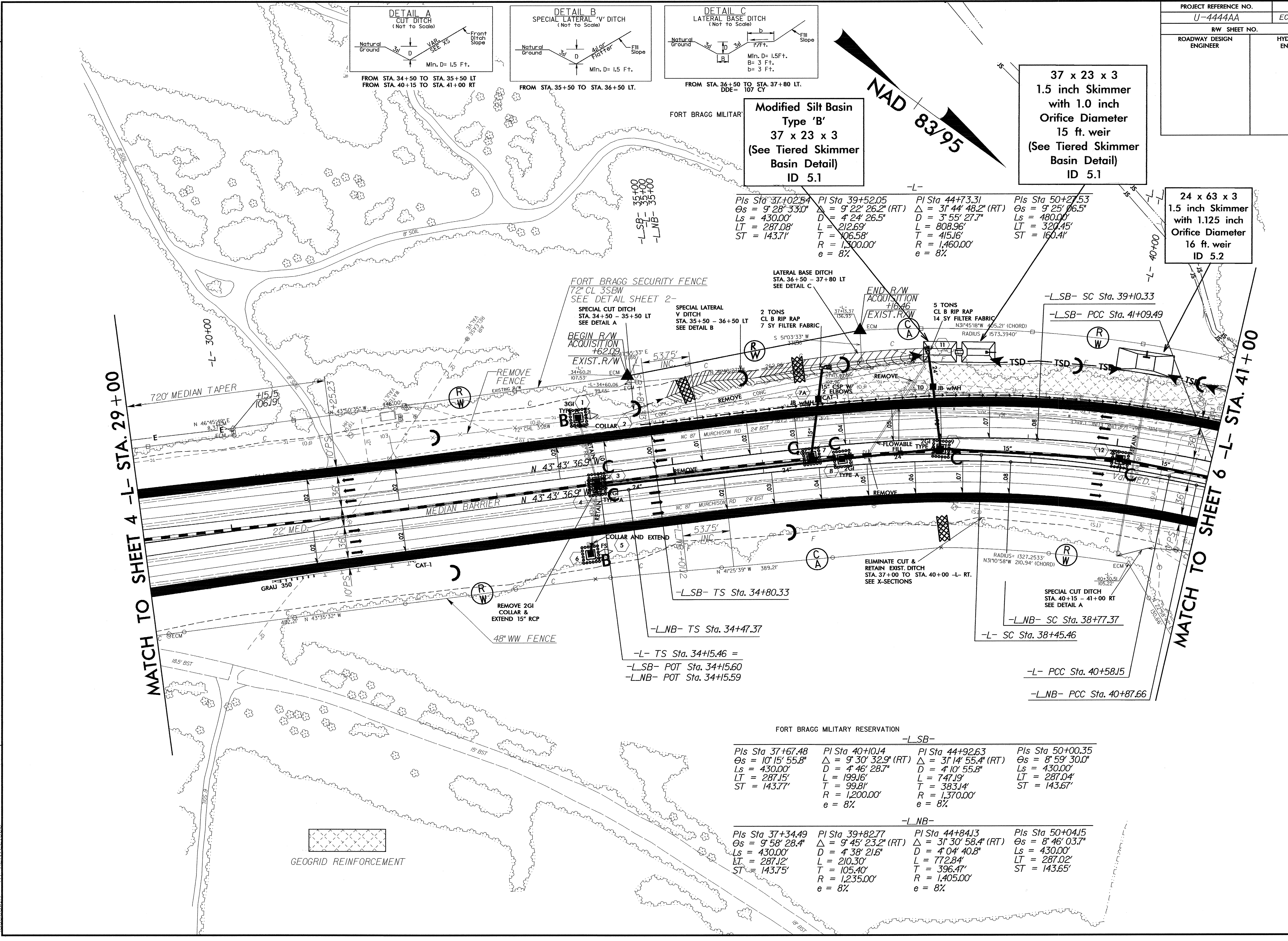
37 x 23 x 3  
1.5 inch Skimmer  
with 1.0 inch  
Orifice Diameter  
15 ft. weir  
(See Tiered Skimmer  
Basin Detail)  
ID 5.1

24 x 63 x 3  
1.5 inch Skimmer  
with 1.125 inch  
Orifice Diameter  
16 ft. weir  
ID 5.2

Pls Sta 37+02.54 Os = 9' 28" 33.0" Ls = 430.00' LT = 287.08' ST = 143.71'	PI Sta 39+52.05 Δ = 9' 22" 26.2" (RT) D = 4' 24" 26.5" L = 212.69' T = 106.58' R = 1,300.00' e = 8%	PI Sta 44+73.31 Δ = 3' 44" 48.2" (RT) D = 3' 55" 27.7" L = 808.96' T = 415.16' R = 1,460.00' e = 8%	Pls Sta 50+27.53 Os = 9' 25" 06.5" Ls = 480.00' LT = 320.45' ST = 160.41'
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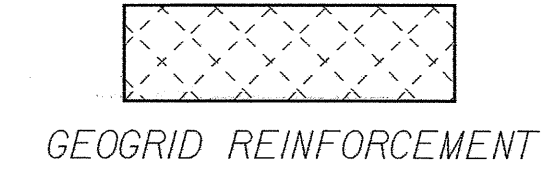
MATCH TO SHEET 4 -L- STA. 29+00

MATCH TO SHEET 6 -L- STA. 41+00



FORT BRAGG MILITARY RESERVATION

Pls Sta 37+67.48 Os = 10' 15" 55.8" Ls = 430.00' LT = 287.15' ST = 143.77'	PI Sta 40+10.14 Δ = 9' 30" 32.9" (RT) D = 4' 46" 28.7" L = 199.16' T = 99.81' R = 1,200.00' e = 8%	PI Sta 44+92.63 Δ = 3' 14" 55.4" (RT) D = 4' 10" 55.8" L = 747.19' T = 383.14' R = 1,370.00' e = 8%	Pls Sta 50+00.35 Os = 8' 59" 30.0" Ls = 430.00' LT = 287.04' ST = 143.67'
Pls Sta 37+34.49 Os = 9' 58" 28.4" Ls = 430.00' LT = 287.12' ST = 143.75'	PI Sta 39+82.77 Δ = 9' 45" 23.2" (RT) D = 4' 38" 21.6" L = 210.30' T = 105.40' R = 1,235.00' e = 8%	PI Sta 44+84.13 Δ = 3' 30" 58.4" (RT) D = 4' 04" 40.8" L = 772.84' T = 396.47' R = 1,405.00' e = 8%	Pls Sta 50+04.15 Os = 8' 46" 03.7" Ls = 430.00' LT = 287.02' ST = 143.65'



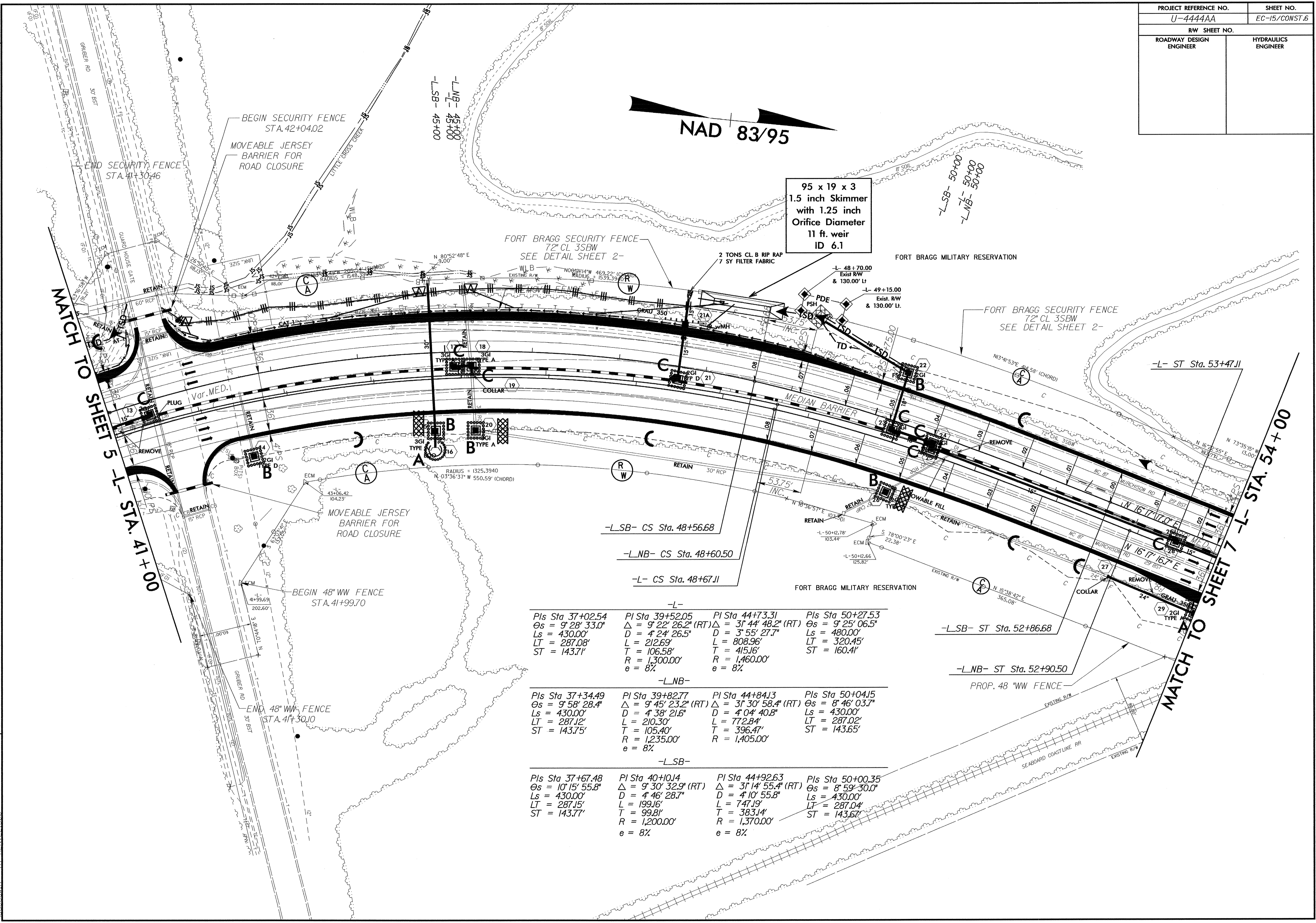
REVISIONS

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PROJECT REFERENCE NO.		SHEET NO.	
U-4444AA		EC-15/CONST.6	
RW SHEET NO.			
ROADWAY DESIGN ENGINEER		HYDRAULICS ENGINEER	

NAD 83/95

95 x 19 x 3  
1.5 inch Skimmer  
with 1.25 inch  
Orifice Diameter  
11 ft. weir  
ID 6.1



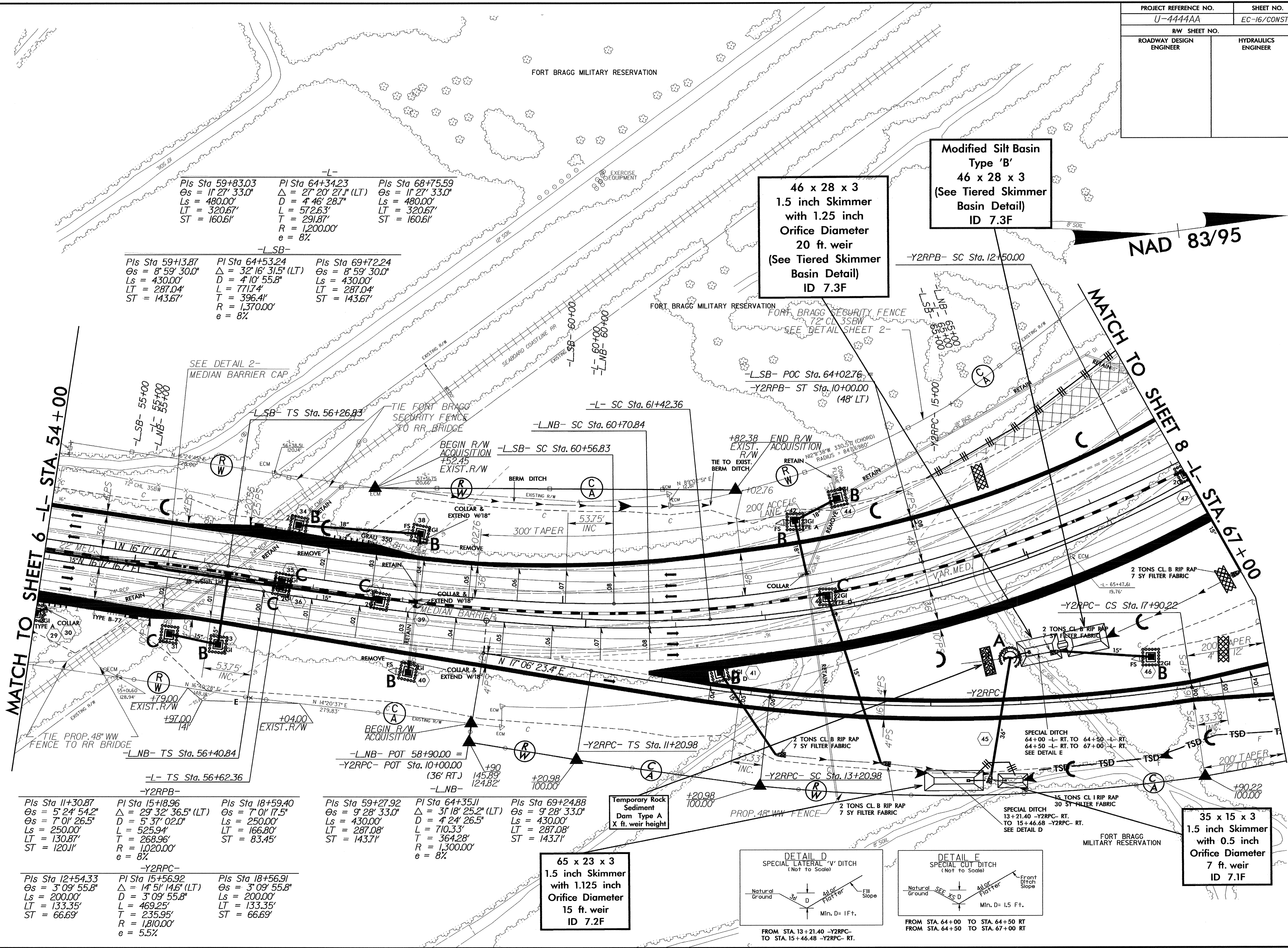
<p>-L-</p> <p>PIs Sta 37+02.54  <math>\Theta_s = 9^\circ 28' 33.0''</math>  <math>L_s = 430.00'</math>  <math>LT = 287.08'</math>  <math>ST = 143.71'</math></p>	<p>PI Sta 39+52.05  <math>\Delta = 9^\circ 22' 26.2''</math> (RT)  <math>D = 4' 24' 26.5''</math>  <math>L = 212.69'</math>  <math>T = 106.58'</math>  <math>R = 1,300.00'</math>  <math>e = 8\%</math></p>	<p>PI Sta 44+73.31  <math>\Delta = 3^\circ 44' 48.2''</math> (RT)  <math>D = 3' 55' 27.7''</math>  <math>L = 808.96'</math>  <math>T = 415.16'</math>  <math>R = 1,460.00'</math>  <math>e = 8\%</math></p>	<p>PI Sta 50+27.53  <math>\Theta_s = 9^\circ 25' 06.5''</math>  <math>L_s = 480.00'</math>  <math>LT = 320.45'</math>  <math>ST = 160.41'</math></p>
-LNB-			
<p>PIs Sta 37+34.49  <math>\Theta_s = 9^\circ 58' 28.4''</math>  <math>L_s = 430.00'</math>  <math>LT = 287.12'</math>  <math>ST = 143.75'</math></p>	<p>PI Sta 39+82.77  <math>\Delta = 9^\circ 45' 23.2''</math> (RT)  <math>D = 4' 38' 21.6''</math>  <math>L = 210.30'</math>  <math>T = 105.40'</math>  <math>R = 1,235.00'</math>  <math>e = 8\%</math></p>	<p>PI Sta 44+84.13  <math>\Delta = 3^\circ 30' 58.4''</math> (RT)  <math>D = 4' 04' 40.8''</math>  <math>L = 772.84'</math>  <math>T = 396.47'</math>  <math>R = 1,405.00'</math>  <math>e = 8\%</math></p>	<p>PI Sta 50+04.15  <math>\Theta_s = 8^\circ 46' 03.7''</math>  <math>L_s = 430.00'</math>  <math>LT = 287.02'</math>  <math>ST = 143.65'</math></p>
-LSB-			
<p>PIs Sta 37+67.48  <math>\Theta_s = 10^\circ 15' 55.8''</math>  <math>L_s = 430.00'</math>  <math>LT = 287.15'</math>  <math>ST = 143.77'</math></p>	<p>PI Sta 40+10.14  <math>\Delta = 9^\circ 30' 32.9''</math> (RT)  <math>D = 4' 46' 28.7''</math>  <math>L = 199.16'</math>  <math>T = 99.81'</math>  <math>R = 1,200.00'</math>  <math>e = 8\%</math></p>	<p>PI Sta 44+92.63  <math>\Delta = 3^\circ 14' 55.4''</math> (RT)  <math>D = 4' 10' 55.8''</math>  <math>L = 747.19'</math>  <math>T = 383.14'</math>  <math>R = 1,370.00'</math>  <math>e = 8\%</math></p>	<p>PIs Sta 50+00.35  <math>\Theta_s = 8^\circ 59' 30.0''</math>  <math>L_s = 430.00'</math>  <math>LT = 287.04'</math>  <math>ST = 143.67'</math></p>

REVISIONS

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A1 GENW221600

PROJECT REFERENCE NO.	SHEET NO.
U-4444AA	EC-16/CONST.7
RW SHEET NO.	
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER

NAD 83/95



PIs Sta 59+83.03 Δs = 11° 27' 33.0" Ls = 480.00' LT = 320.67' ST = 160.61'	PI Sta 64+34.23 Δ = 27° 20' 27.1" (LT) D = 4' 46" 28.7" L = 572.63' T = 291.87' R = 1,200.00' e = 8%	PIs Sta 68+75.59 Δs = 11° 27' 33.0" Ls = 480.00' LT = 320.67' ST = 160.61'
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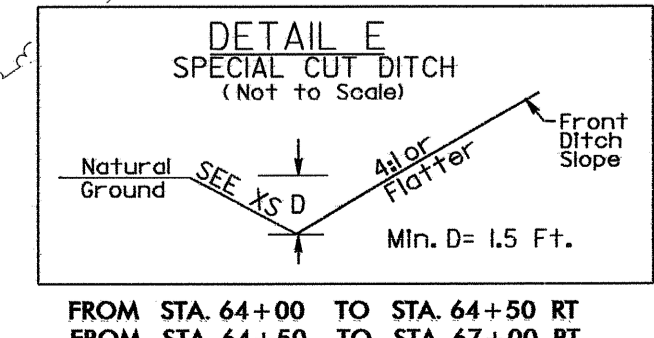
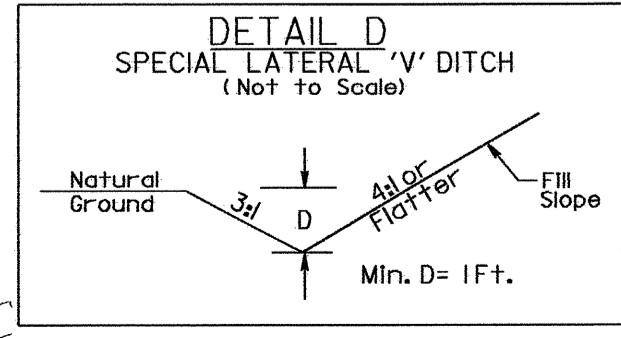
PIs Sta 59+13.87 Δs = 8° 59' 30.0" Ls = 430.00' LT = 287.04' ST = 143.67'	PI Sta 64+53.24 Δ = 32° 16' 31.5" (LT) D = 4' 10" 55.8" L = 771.74' T = 396.41' R = 1,370.00' e = 8%	PIs Sta 69+72.24 Δs = 8° 59' 30.0" Ls = 430.00' LT = 287.04' ST = 143.67'
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PIs Sta 11+30.87 Δs = 5° 24' 54.2" Ls = 250.00' LT = 130.87' ST = 120.11'	PI Sta 15+18.96 Δ = 29° 32' 36.5" (LT) D = 5' 37" 02.0" L = 525.94' T = 268.96' R = 1,020.00' e = 8%	PIs Sta 18+59.40 Δs = 7° 01' 17.5" Ls = 250.00' LT = 166.80' ST = 83.45'
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PIs Sta 12+54.33 Δs = 3° 09' 55.8" Ls = 200.00' LT = 133.35' ST = 66.69'	PI Sta 15+56.92 Δ = 14° 51' 14.6" (LT) D = 3' 09" 55.8" L = 469.25' T = 235.95' R = 1,810.00' e = 5.5%	PIs Sta 18+56.91 Δs = 3° 09' 55.8" Ls = 200.00' LT = 133.35' ST = 66.69'
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PIs Sta 59+27.92 Δs = 9° 28' 33.0" Ls = 430.00' LT = 287.08' ST = 143.71'	PI Sta 64+35.11 Δ = 31° 18' 25.2" (LT) D = 4' 24" 26.5" L = 710.33' T = 364.28' R = 1,300.00' e = 8%	PIs Sta 69+24.88 Δs = 9° 28' 33.0" Ls = 430.00' LT = 287.08' ST = 143.71'
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65 x 23 x 3  
1.5 inch Skimmer  
with 1.125 inch  
Orifice Diameter  
15 ft. weir  
ID 7.2F



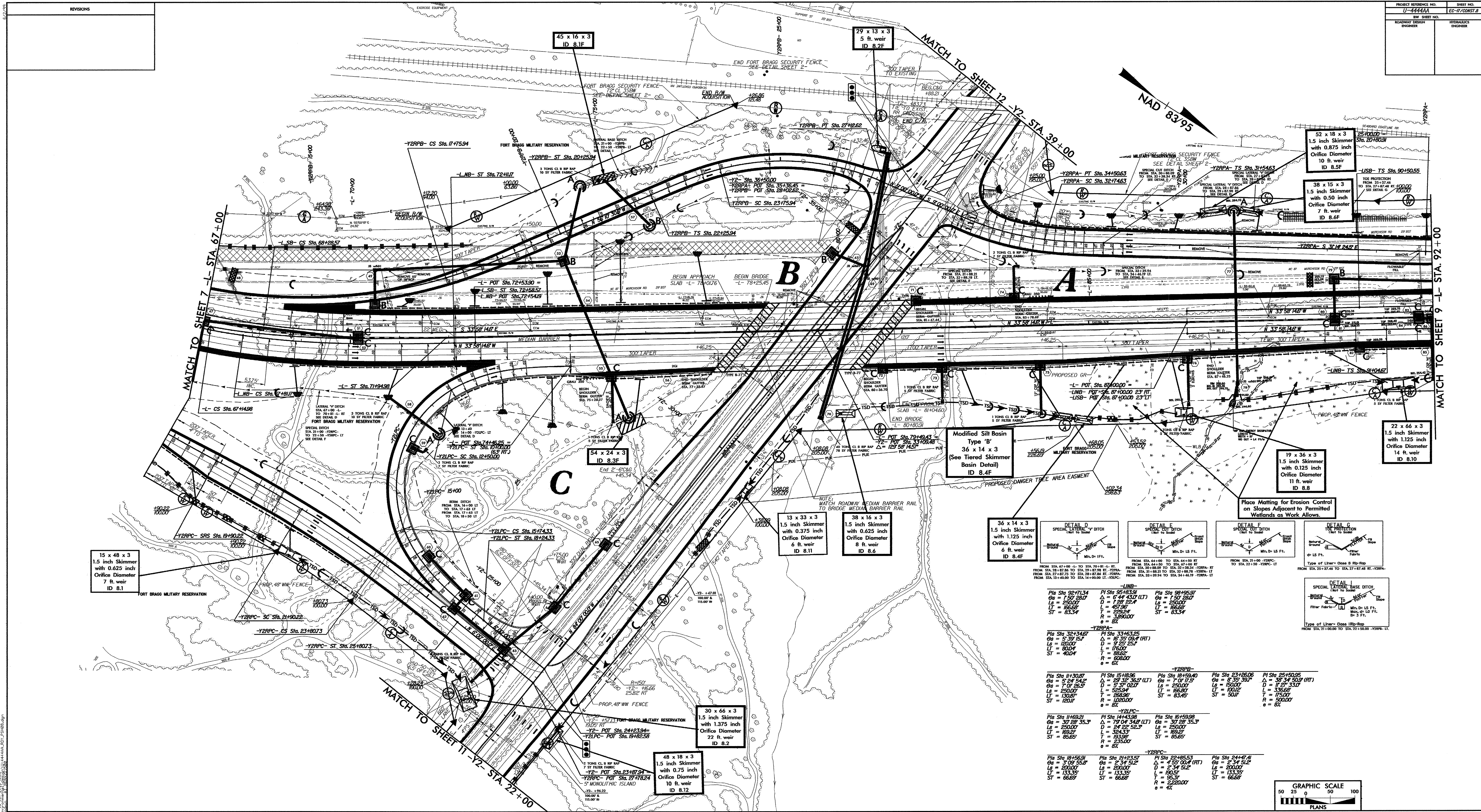
35 x 15 x 3  
1.5 inch Skimmer  
with 0.5 inch  
Orifice Diameter  
7 ft. weir  
ID 7.1F

REVISIONS

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REVISIONS

PROJECT REFERENCE NO. U-4444A	SHEET NO. EC-17/CONSTR
DATE 	
ROADWAY DESIGN ENGINEER 	HYDRAULICS ENGINEER 



15 x 48 x 3  
1.5 inch Skimmer  
with 0.625 inch  
Orifice Diameter  
7 ft. weir  
ID 8.1

54 x 24 x 3  
ID 8.3F

48 x 18 x 3  
1.5 inch Skimmer  
with 0.75 inch  
Orifice Diameter  
10 ft. weir  
ID 8.12

13 x 33 x 3  
1.5 inch Skimmer  
with 0.375 inch  
Orifice Diameter  
6 ft. weir  
ID 8.11

38 x 16 x 3  
1.5 inch Skimmer  
with 0.625 inch  
Orifice Diameter  
8 ft. weir  
ID 8.6

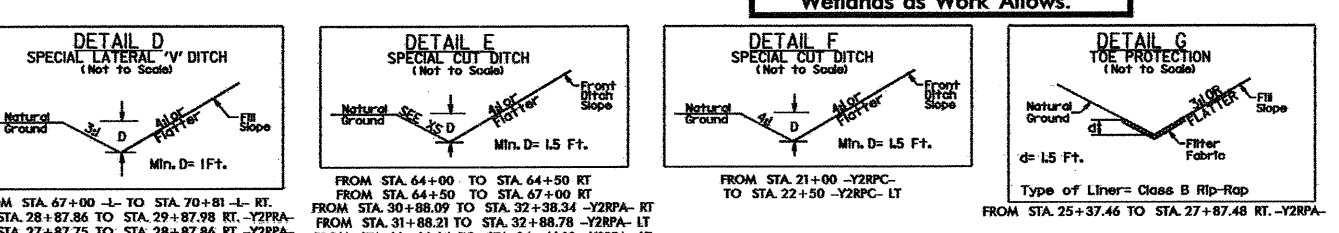
Modified Silt Basin  
Type 'B'  
36 x 14 x 3  
(See Tiered Skimmer  
Basin Detail)  
ID 8.4F

36 x 14 x 3  
1.5 inch Skimmer  
with 1.125 inch  
Orifice Diameter  
6 ft. weir  
ID 8.4E

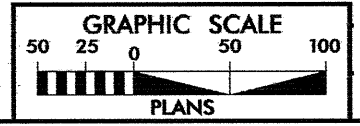
19 x 36 x 3  
1.5 inch Skimmer  
with 0.125 inch  
Orifice Diameter  
11 ft. weir  
ID 8.8

22 x 66 x 3  
1.5 inch Skimmer  
with 1.125 inch  
Orifice Diameter  
14 ft. weir  
ID 8.10

Place Matting for Erosion Control  
on Slopes Adjacent to Permitted  
Wetlands as Work Allows.

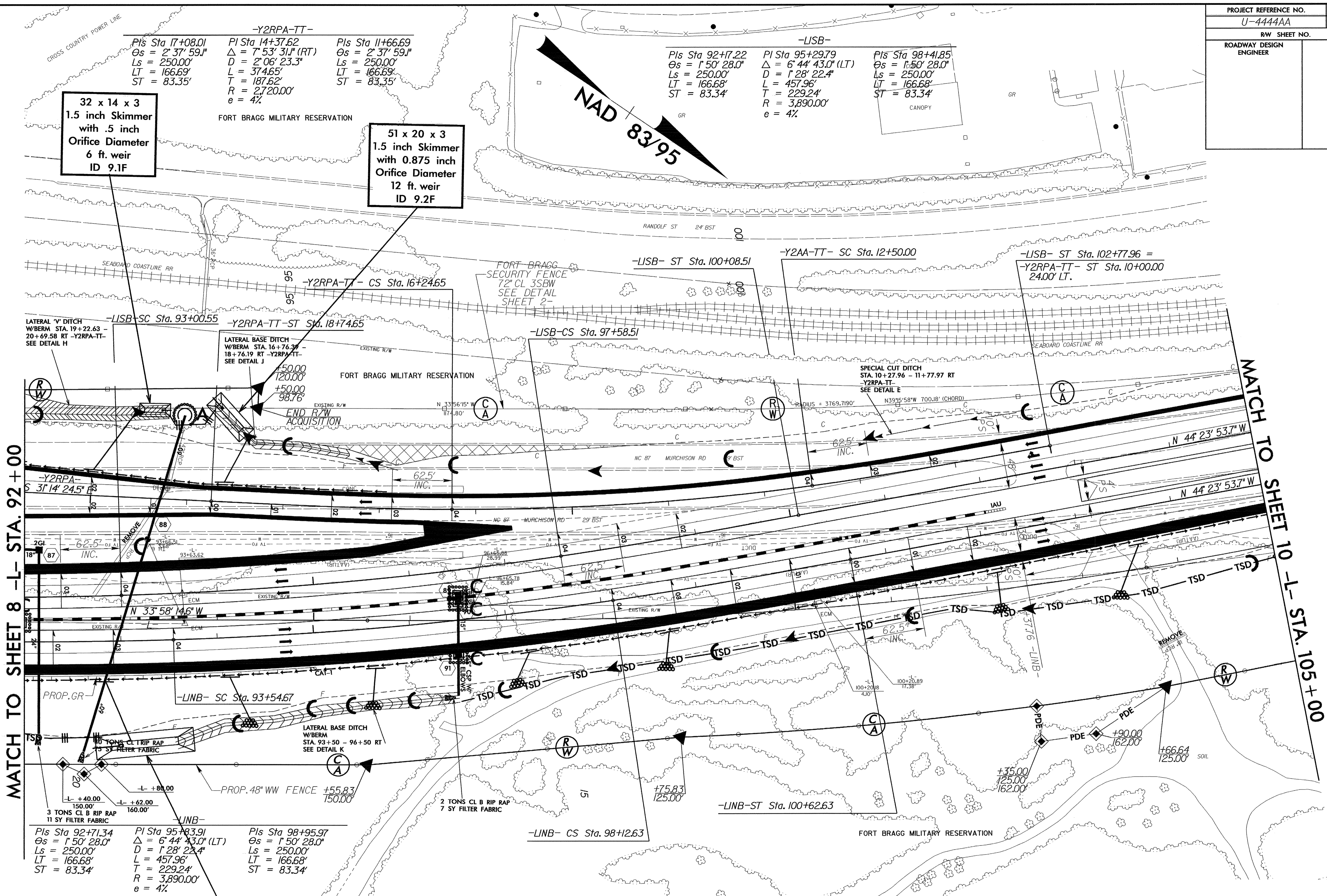


-Y2RPA- PI Sta 92+71.34 Gs = 150 28.07 Ls = 250.00 LT = 65.59 ST = 83.34 e = 6X	-LNB- PI Sta 92+43.57 Gs = 6 44 43.0 (LT) Ls = 1 38 22.4 LT = 250.00 ST = 83.34 e = 6X	PI Sta 98+58.57 Gs = 150 28.07 Ls = 250.00 LT = 65.59 ST = 83.34 e = 6X	-Y2RPA- PI Sta 32+34.67 Gs = 5 39 15.7 Ls = 120.00 LT = 60.04 ST = 40.04 e = 6X	PI Sta 33+63.25 Gs = 16 35 09.4 (RT) Ls = 9 25 25.4 LT = 176.00 ST = 60.00 e = 6X	-Y2RPA- PI Sta 11+30.07 Gs = 5 24 54.2 Ls = 7 07 26.5 LT = 250.00 ST = 120.01 e = 6X	PI Sta 15+18.56 Gs = 25 32 30.5 (LT) Ls = 5 37 02.7 LT = 250.00 ST = 120.00 e = 6X	PI Sta 18+59.40 Gs = 7 07 17.5 Ls = 1 00.00 LT = 165.00 ST = 83.45 e = 6X	PI Sta 23+06.06 Gs = 8 30 39.7 Ls = 1 00.00 LT = 100.00 ST = 50.00 e = 6X	PI Sta 25+50.26 Gs = 36 34 50.2 (RT) Ls = 1 00.00 LT = 336.68 ST = 175.00 e = 6X	-Y2RPA- PI Sta 11+69.21 Gs = 30 28 35.3 Ls = 2 00.00 LT = 169.21 ST = 65.67 e = 6X	PI Sta 14+43.98 Gs = 7 07 34.8 (LT) Ls = 24 22 52.3 LT = 324.33 ST = 153.59 e = 6X	PI Sta 16+59.98 Gs = 7 07 34.8 (LT) Ls = 250.00 LT = 169.21 ST = 65.67 e = 6X	-Y2RPA- PI Sta 18+56.51 Gs = 3 02 55.9 Ls = 2 00.00 LT = 133.59 ST = 66.68 e = 6X	PI Sta 21+23.57 Gs = 2 34 51.2 Ls = 2 00.00 LT = 133.59 ST = 66.68 e = 6X	PI Sta 22+65.53 Gs = 4 50 00.4 (RT) Ls = 2 34 51.2 LT = 160.00 ST = 90.31 e = 2220.00 e = 6X	PI Sta 24+47.41 Gs = 2 34 51.2 Ls = 2 00.00 LT = 133.59 ST = 66.68 e = 6X
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PROJECT REFERENCE NO. U-4444AA	SHEET NO. EC-18/CONST-9
RW SHEET NO.	
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER



32 x 14 x 3  
1.5 inch Skimmer  
with .5 inch  
Orifice Diameter  
6 ft. weir  
ID 9.1F

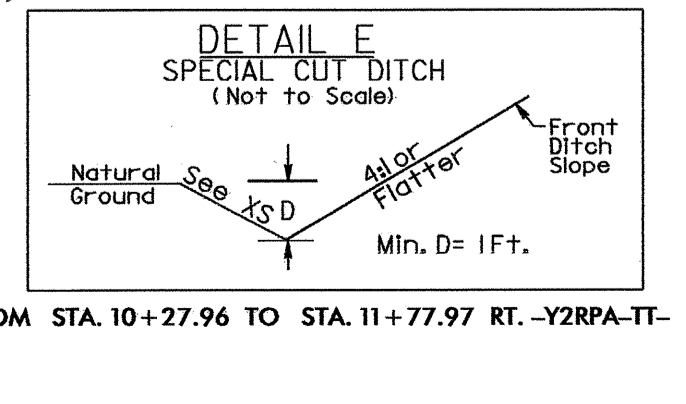
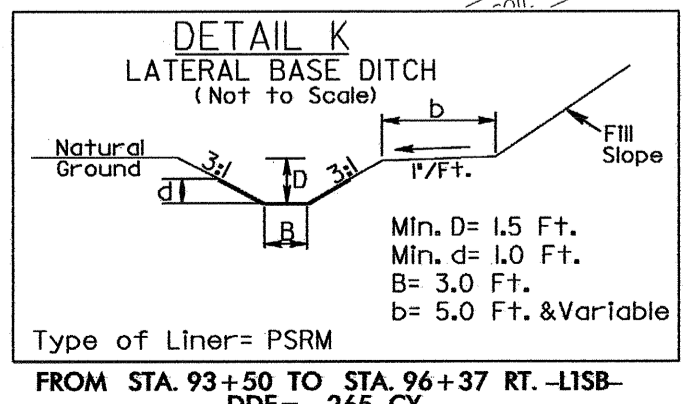
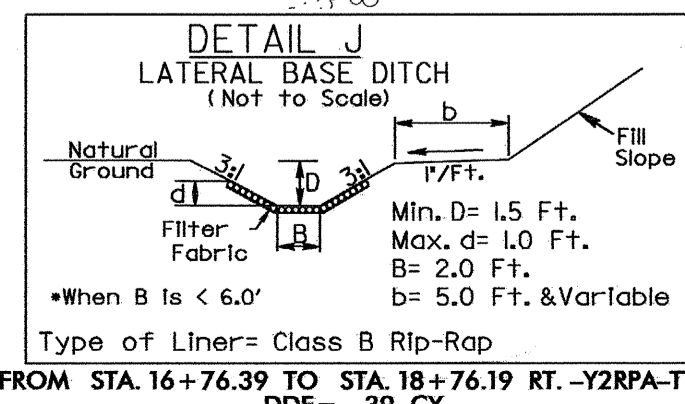
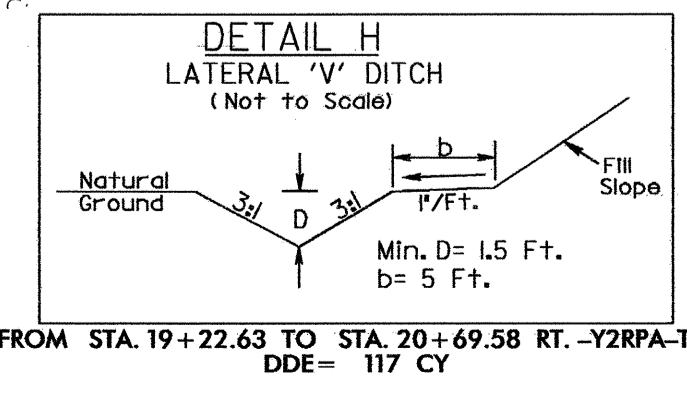
51 x 20 x 3  
1.5 inch Skimmer  
with 0.875 inch  
Orifice Diameter  
12 ft. weir  
ID 9.2F

**-Y2RPA-TT-**  
 Pls Sta 17+08.01  
 $\Delta = 2' 37' 59.1''$   
 $L_s = 250.00'$   
 $L = 166.69'$   
 $T = 83.35'$   
 $e = 4\%$   
 Pls Sta 14+37.62  
 $\Delta = 7' 53' 31.1''$  (RT)  
 $D = 2' 06' 23.3''$   
 $L = 374.65'$   
 $T = 187.62'$   
 $R = 2720.00'$   
 $e = 4\%$   
 Pls Sta 11+66.69  
 $\Delta = 2' 37' 59.1''$   
 $L_s = 250.00'$   
 $L = 166.69'$   
 $T = 83.35'$

**-LISB-**  
 Pls Sta 92+17.22  
 $\Delta = 1' 50' 28.0''$   
 $L_s = 250.00'$   
 $L = 166.68'$   
 $T = 83.34'$   
 Pls Sta 95+29.79  
 $\Delta = 6' 44' 43.0''$  (LT)  
 $D = 1' 28' 22.4''$   
 $L = 457.96'$   
 $T = 229.24'$   
 $R = 3890.00'$   
 $e = 4\%$   
 Pls Sta 98+41.85  
 $\Delta = 1' 50' 28.0''$   
 $L_s = 250.00'$   
 $L = 166.68'$   
 $T = 83.34'$

**-LINB-**  
 Pls Sta 92+17.14  
 $\Delta = 1' 50' 28.0''$   
 $L_s = 250.00'$   
 $L = 166.68'$   
 $T = 83.34'$   
 Pls Sta 95+18.91  
 $\Delta = 6' 44' 43.0''$  (LT)  
 $D = 1' 28' 22.4''$   
 $L = 457.96'$   
 $T = 229.24'$   
 $R = 3890.00'$   
 $e = 4\%$   
 Pls Sta 98+19.97  
 $\Delta = 1' 50' 28.0''$   
 $L_s = 250.00'$   
 $L = 166.68'$   
 $T = 83.34'$

102 x 25 x 3  
1.5 inch Skimmer  
with 0.375 inch  
Orifice Diameter  
17 ft. weir  
ID 9.1



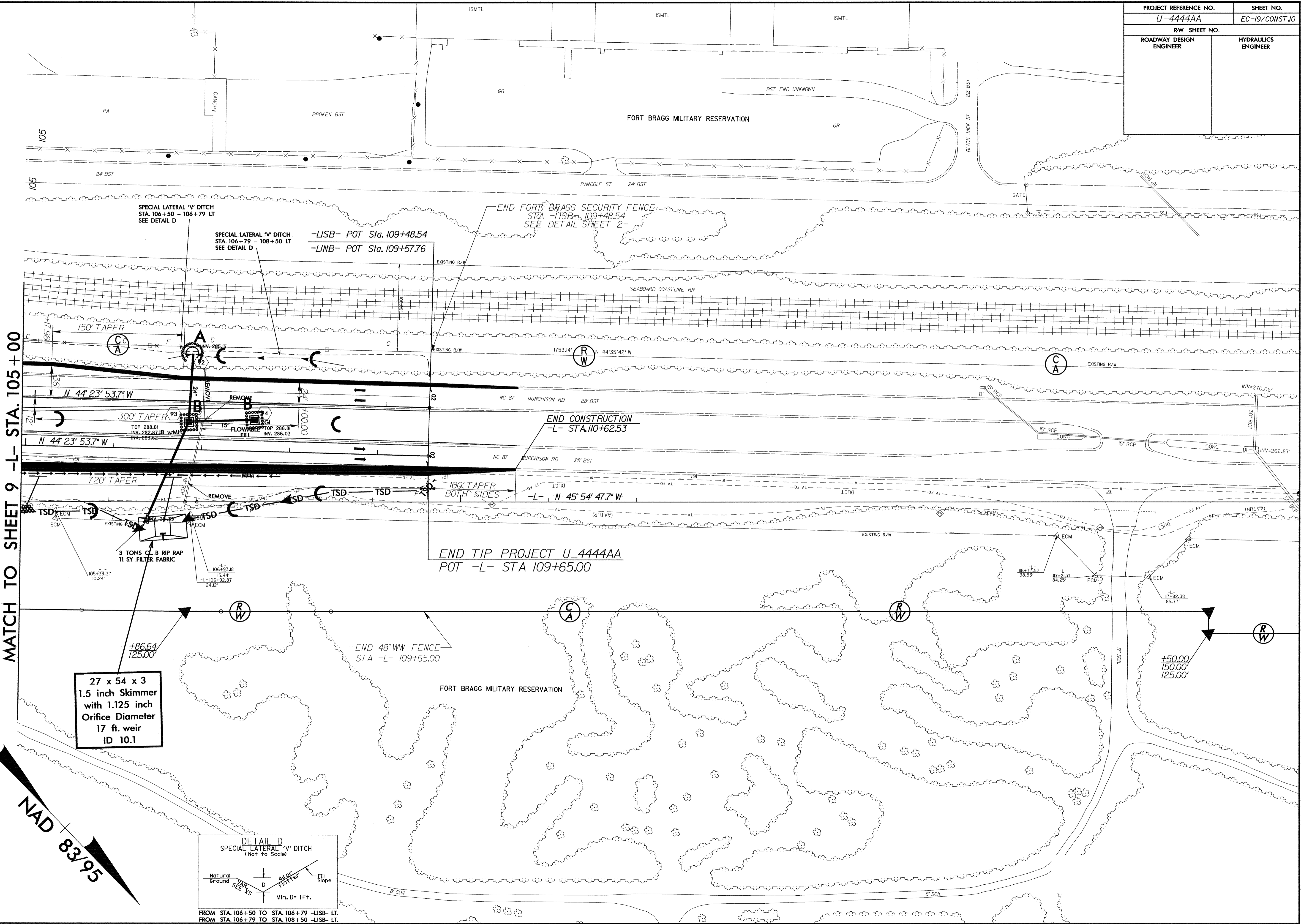
REVISIONS

MATCH TO SHEET 8 -L- STA. 92+00

MATCH TO SHEET 10 -L- STA. 105+00

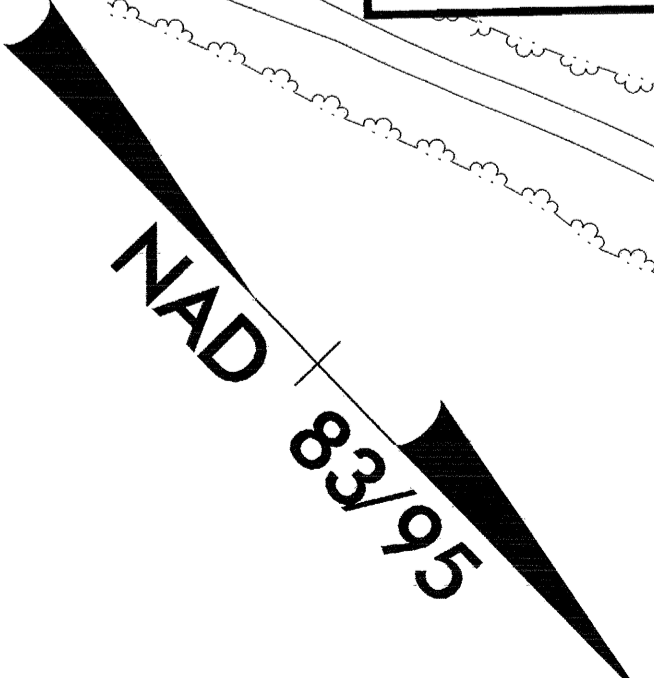
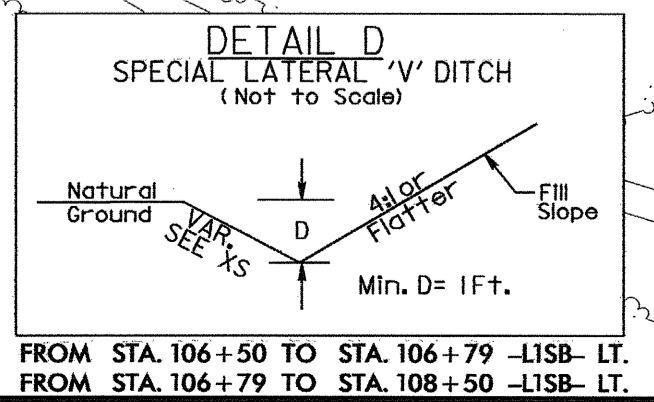
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 8/17/99

PROJECT REFERENCE NO. U-4444AA	SHEET NO. EC-19/CONST.10
RW SHEET NO.	
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER



MATCH TO SHEET 9 -L- STA. 105 + 00

27 x 54 x 3  
1.5 inch Skimmer  
with 1.125 inch  
Orifice Diameter  
17 ft. weir  
ID 10.1

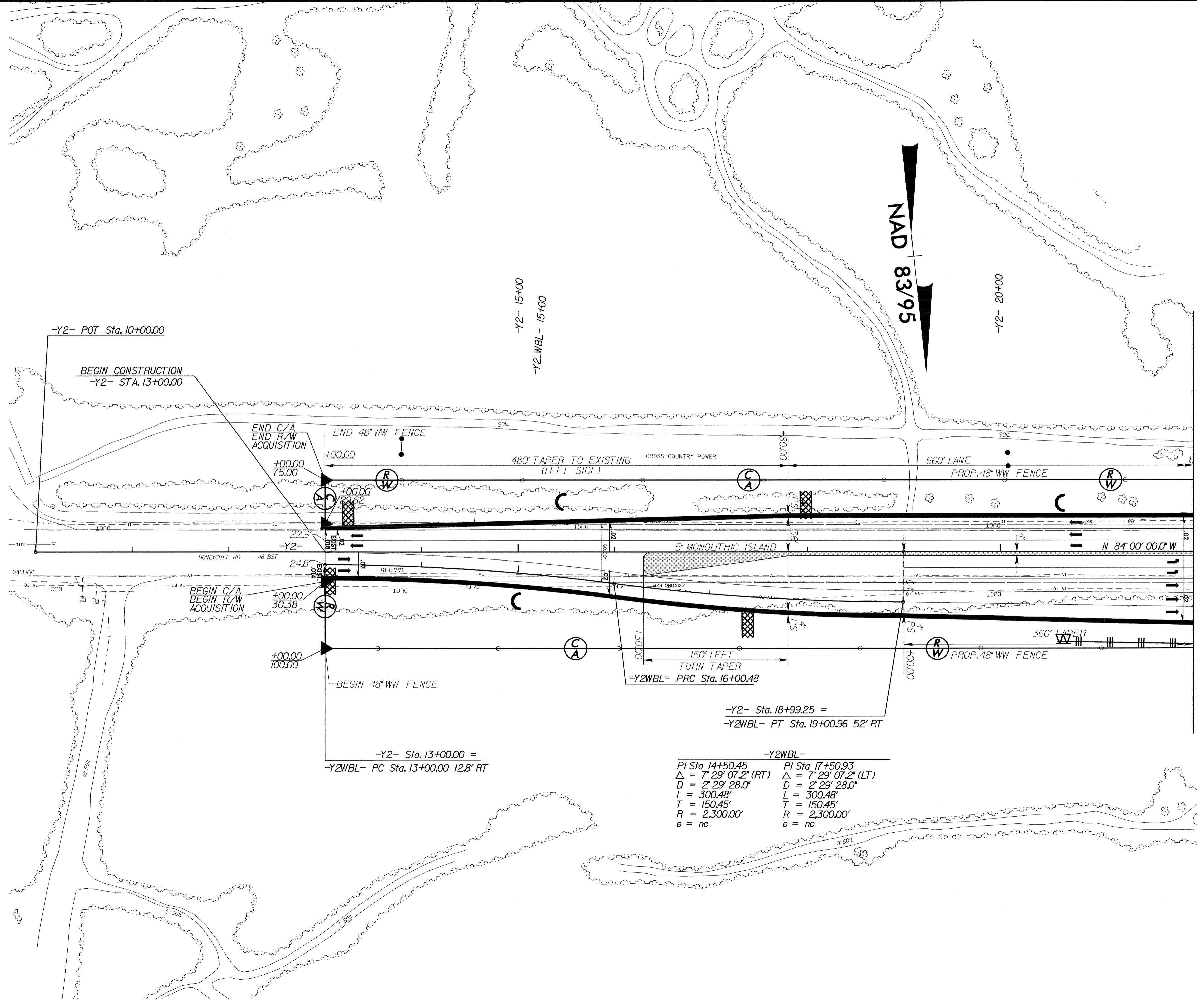


REVISIONS

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PROJECT REFERENCE NO.	SHEET NO.
U-4444AA	EC-20/CONST.II
RW SHEET NO.	
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER

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 RACASWID AT RENW21500



-Y2- Sta. 13+00.00 =  
 -Y2WBL- PC Sta. 13+00.00 12.8' RT

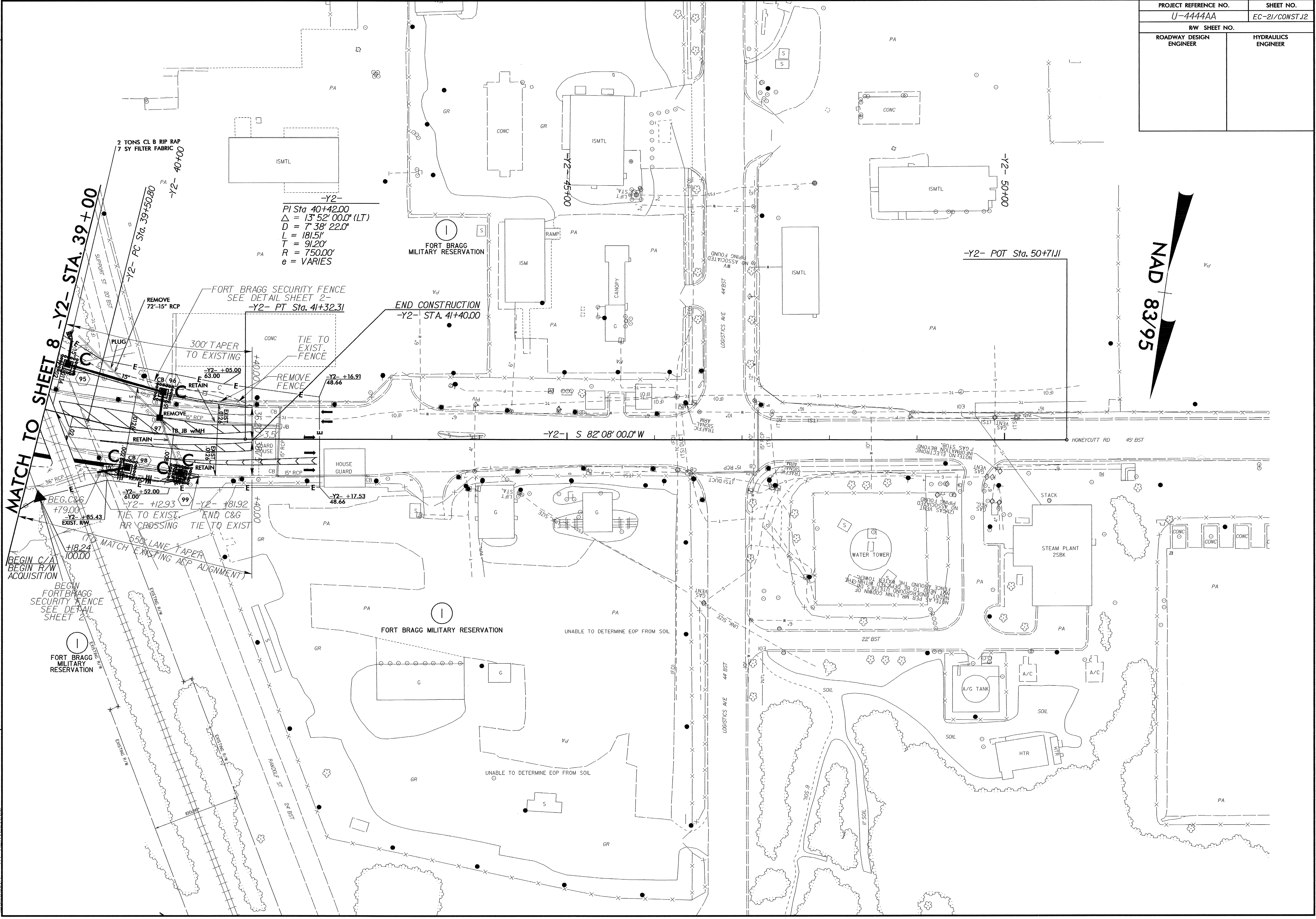
-Y2- Sta. 18+99.25 =  
 -Y2WBL- PT Sta. 19+00.96 52' RT

-Y2WBL-	
PI Sta 14+50.45	PI Sta 17+50.93
$\Delta = 7^{\circ} 29' 07.2''$ (RT)	$\Delta = 7^{\circ} 29' 07.2''$ (LT)
D = 2' 29' 28.0"	D = 2' 29' 28.0"
L = 300.48'	L = 300.48'
T = 150.45'	T = 150.45'
R = 2,300.00'	R = 2,300.00'
e = nc	e = nc

PROJECT REFERENCE NO. U-444AA	SHEET NO. EC-21/CONST J2
RW SHEET NO.	
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER

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02-JUN-2009 13:46  
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AT REN221500

REVISIONS



**MATCH TO SHEET 8 -Y2- STA. 39+00**

-Y2-  
 $PI\ Sta\ 40+42.00$   
 $\Delta = 13^\circ 52' 00.0" (LT)$   
 $D = 7' 38" 22.0"$   
 $L = 181.5'$   
 $T = 91.20'$   
 $R = 750.00'$   
 $e = VARIES$

**END CONSTRUCTION**  
 -Y2- STA. 41+40.00

**NAD 83/95**

**MATCH TO SHEET 8 -Y2- STA. 39+00**

BEG. C&G  
 +79.00  
 -Y2- +85.43  
 EXIST. RW

BEGIN C/A  
 BEGIN R/W  
 ACQUISITION

BEGIN  
 FORT BRAGG  
 SECURITY FENCE  
 SEE DETAIL  
 SHEET 2

550' LANE TAPER  
 MATCH EXISTING ACP ALIGNMENT

UNABLE TO DETERMINE EOP FROM SOIL

FORT BRAGG  
 MILITARY  
 RESERVATION

FORT BRAGG MILITARY RESERVATION

UNABLE TO DETERMINE EOP FROM SOIL

UNABLE TO DETERMINE EOP FROM SOIL

NOTE: PER MR. LYNN COOKING OF  
 THE ARMY ENGINEERING CENTER  
 NOT UNDERGROUND UTILITIES OR  
 FENCE AROUND THE WATER TOWER.

WATER TOWER

STACK

STEAM PLANT  
 25BK

A/G TANK

HTR

SOIL

SOIL

SOIL

CONC

CONC

CONC