

FOUNDATION LAYOUT

DIMENSIONS LOCATING PILES ARE SHOWN TO THE PILE CENTERLINE. BRACE PILES AT END BENTS ARE BATTERED 3:12

FOUNDATION NOTES

THE DRILLED PIERS FOR BENTS #1 AND #2 HAVE BEEN DESIGNED FOR BOTH SKIN FRICTION AND END BEARING. CHECK FIELD CONDITIONS FOR THE REQUIRED END BEARING CAPACITY OF 10 TSF FOR BENT #1 AND 35 TSF FOR BENT #2.

DRILLED PIERS AT BENT #1 AND #2 HAVE BEEN DESIGNED FOR AN APPLIED LOAD OF 330 TONS EACH AT THE TOP SID INSPECTIONS MAY BE REQUIRED TO INSPECT THE BOTTOM CLEANLINESS OF THE DRILLED PIERS. THE OF THE DRILLED PIER. ENGINEER WILL DETERMINE THE NEED FOR SID INSPECTIONS. SEE DRILLED PIERS SPECIAL PROVISION. OF THE DRILLED PIER.

DRILLED PIERS FOR BENT #1 SHALL EXTEND TO AN ELEVATION NO HIGHER THAN 441.000, AND BENT #2 NO HIGHER CSL TUBES ARE REQUIRED AND CSL TESTING MAY BE REQUIRED FOR THE DRILLED PIERS. THE ENGINEER WILL THAN 445.000 AND SATISFY THE REQUIRED END BEARING CAPACITY.

DETERMINE THE NEED FOR CSL TESTING. SEE CROSSHOLE SONIC LOGGING SPECIAL PROVISION.

PERMANENT STEEL CASING IS REQUIRED FOR DRILLED PIERS AT BENTS #1 AND #2.DO NOT EXTEND THE CASING BELOW ELEVATION 451.000 WITHOUT PRIOR APPROVAL FROM THE ENGINEER. SEE DRILLED PIERS SPECIAL PROVISION.

FOR DRILLED PIERS, SEE DRILLED PIERS SPECIAL PROVISION.

THE SCOUR CRITICAL ELEVATION FOR BENT #1 AND #2 IS ELEVATION 450.000. SCOUR CRITICAL ELEVATIONS ARE USED TO MONITOR POSSIBLE SCOUR PROBLEMS DURING THE LIFE OF THE STRUCTURE.

SPT TESTING IS NOT REQUIRED TO DETERMINE THE END BEARING CAPACITY OF THE DRILLED PIERS AT BENTS #1 AND #2. DO NOT USE SLURRY CONSTRUCTION FOR DRILLED PIERS AT BENTS #1 AND #2.

DRIVE PILES FOR END BENTS #1 AND #2 TO A REQUIRED MINIMUM BEARING CAPACITY OF 100 TONS PER PILE. THE REQUIRED BEARING CAPACITY IS EQUAL TO THE ALLOWABLE BEARING CAPACITY WITH A MINIMUM FACTOR OF SAFETY OF TWO.

THE ALLOWABLE BEARING CAPACITY FOR PILES AT END BENTS #1 AND #2 IS 50 TONS PER PILE.



PROJECT NO. B-4276 STANLY _ COUNTY 19+96.00 -L-

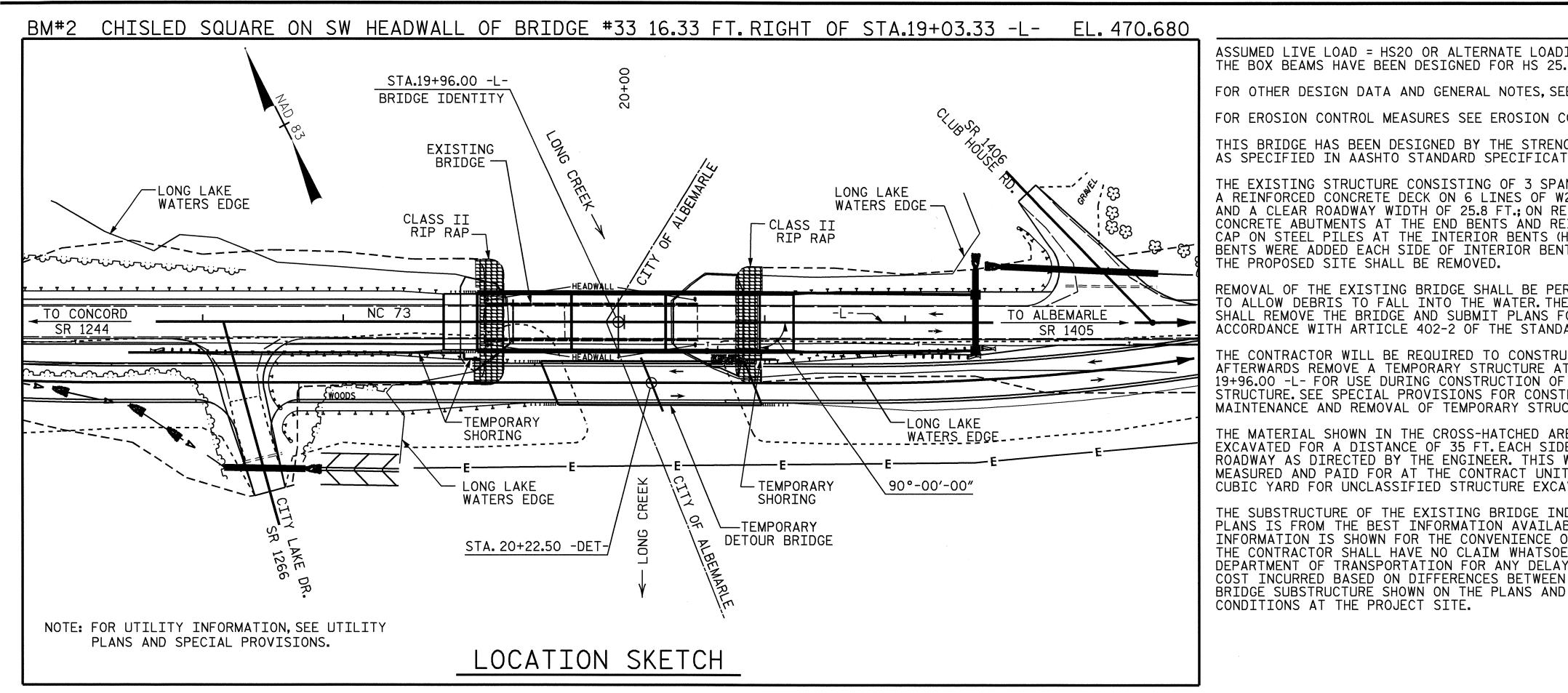
SHEET 2 OF 3

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

FOUNDATION LAYOUT

	REV:	ISION:	S		SHEET NO.
BY:	DATE:	NO.	BY:	DATE:	S-2
		3			TOTAL SHEETS
		4			21

DRAWN BY: KEITH D. LAYNE DATE: 01-10-08
CHECKED BY: H. A. LOCKLEAR DATE: 1/24/08



ASSUMED LIVE LOAD = HS20 OR ALTERNATE LOADING, EXCEPT THAT

FOR OTHER DESIGN DATA AND GENERAL NOTES, SEE SHEET SN.

FOR EROSION CONTROL MEASURES SEE EROSION CONTROL PLANS.

THIS BRIDGE HAS BEEN DESIGNED BY THE STRENGTH DESIGN METHOD AS SPECIFIED IN AASHTO STANDARD SPECIFICATIONS.

THE EXISTING STRUCTURE CONSISTING OF 3 SPANS @ 38'-01/2" WITH A REINFORCED CONCRETE DECK ON 6 LINES OF W24×80 I-BEAMS; AND A CLEAR ROADWAY WIDTH OF 25.8 FT.; ON REINFORCED CONCRETE ABUTMENTS AT THE END BENTS AND REINFORCED CONCRETE CAP ON STEEL PILES AT THE INTERIOR BENTS (HP12×53 CRUTCH BENTS WERE ADDED EACH SIDE OF INTERIOR BENTS) AND LOCATED AT REINFORCING STEEL, TWO 30 INCH SAMPLES OF EACH SIZE BAR USED. THE PROPOSED SITE SHALL BE REMOVED.

REMOVAL OF THE EXISTING BRIDGE SHALL BE PERFORMED SO AS NOT SAMPLE, PLUS A MINIMUM LAP SPLICE OF THIRTY BAR DIAMETERS. TO ALLOW DEBRIS TO FALL INTO THE WATER. THE CONTRACTOR SHALL REMOVE THE BRIDGE AND SUBMIT PLANS FOR DEMOLITION IN ACCORDANCE WITH ARTICLE 402-2 OF THE STANDARD SPECIFICATIONS. STEEL CONTAINS LEAD, THE CONTRACTOR'S ATTENTION IS DIRECTED

THE CONTRACTOR WILL BE REQUIRED TO CONSTRUCT, MAINTAIN AND AFTERWARDS REMOVE A TEMPORARY STRUCTURE AT STATION 19+96.00 -L- FOR USE DURING CONSTRUCTION OF THE PROPOSED STRUCTURE. SEE SPECIAL PROVISIONS FOR CONSTRUCTION, MAINTENANCE AND REMOVAL OF TEMPORARY STRUCTURE.

THE MATERIAL SHOWN IN THE CROSS-HATCHED AREA SHALL BE EXCAVATED FOR A DISTANCE OF 35 FT. EACH SIDE OF CENTERLINE ROADWAY AS DIRECTED BY THE ENGINEER. THIS WORK WILL BE MEASURED AND PAID FOR AT THE CONTRACT UNIT PRICE PER CUBIC YARD FOR UNCLASSIFIED STRUCTURE EXCAVATION.

THE SUBSTRUCTURE OF THE EXISTING BRIDGE INDICATED ON THE PLANS IS FROM THE BEST INFORMATION AVAILABLE. SINCE THIS INFORMATION IS SHOWN FOR THE CONVENIENCE OF THE CONTRACTOR. THE CONTRACTOR SHALL HAVE NO CLAIM WHATSOEVER AGAINST THE DEPARTMENT OF TRANSPORTATION FOR ANY DELAYS OR ADDITIONAL COST INCURRED BASED ON DIFFERENCES BETWEEN THE EXISTING BRIDGE SUBSTRUCTURE SHOWN ON THE PLANS AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

THIS STRUCTURE HAS BEEN DESIGNED IN ACCORDANCE WITH HEC 18, "EVALUATING SCOUR AT BRIDGES", MAY, 2001.

NOTES

FOR LIMITS OF TEMPORARY SHORING FOR MAINTENANCE OF TRAFFIC.

SEE TRAFFIC CONTROL PLANS.

THIS BRIDGE HAS BEEN DESIGNED IN ACCORDANCE WITH THE REQUIREMENTS OF THE AASHTO STANDARD SPECIFICATIONS FOR SEISMIC DESIGN OF HIGHWAY BRIDGES FOR SEISMIC PERFORMANCE CATEGORY A.

THE CONTRACTOR SHALL PROVIDE INDEPENDENT ASSURANCE SAMPLES OF REINFORCING STEEL AS FOLLOWS: FOR PROJECTS REQUIRING UP TO 400 TONS OF REINFORCING STEEL, ONE 30 INCH SAMPLE OF EACH SIZE BAR USED, AND FOR PROJECTS REQUIRING OVER 400 TONS OF THE BARS FROM WHICH THE SAMPLES ARE TAKEN MUST THEN BE SPLICED WITH REPLACEMENT BARS OF THE SIZE AND LENGTH OF THE

INASMUCH AS THE PAINT SYSTEM ON THE EXISTING STRUCTURAL TO ARTICLE 107-1 OF THE STANDARD SPECIFICATIONS. ANY COSTS RESULTING FROM COMPLIANCE WITH APPLICABLE STATE OR FEDERAL REGULATIONS PERTAINING TO HANDLING OF MATERIALS CONTAINING LEAD BASED PAINT SHALL BE INCLUDED IN THE BID PRICE FOR "REMOVAL OF EXISTING STRUCTURE AT STATION 19+96.00 -L-."

FOR SUBMITTAL OF WORKING DRAWINGS, SEE SPECIAL PROVISIONS.

FOR FALSEWORK AND FORMWORK, SEE SPECIAL PROVISIONS.

FOR CRANE SAFETY, SEE SPECIAL PROVISIONS.

FOR PRESTRESSED CONCRETE MEMBERS. SEE SPECIAL PROVISIONS.

FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.

FOR CONSTRUCTION, MAINTENANCE, AND REMOVAL OF TEMPORARY ACCESS, SEE SPECIAL PROVISIONS.

PRIOR TO FORMING THE BENT CAPS THE CONTRACTOR SHALL VERIFY THE WATER ELEVATION. IF THE BOTTOM OF THE CAP IS BELOW THE WATER ELEVATION, WATER TIGHT FORMS WILL BE REQUIRED. COST OF WATERTIGHT FORMS SHALL BE INCLUDED IN THE VARIOUS PAY

HYDRAULIC DATA

DESIGN DISCHARGE	. 5,109 CFS
FREQUENCY OF DESIGN FLOOD	. 50 YR
DESIGN HIGH WATER ELEVATION	469.300
DRAINAGE AREA	. 30.3 SQ.MI.
BASIC DISCHARGE (Q100)	. 5 , 791 CFS
BASIC HIGH WATER ELEVATION	469.700

OVERTOPPING FLOOD DATA

OVERTOPPING DISCHARGE	8,062 CFS
FREQUENCY OF OVERTOPPING FLOOD	500 YR
OVERTOPPING FLOOD ELEVATION	471.000

			- TOTA	AL BILL	OF MATE	ERIAL —				
	CONSTRUCTION MAINTENANCE & REMOVAL OF TEMPORARY STRUCTURE	CONSTRUCTION MAINTENANCE & REMOVAL OF TEMPORARY ACCESS	REMOVAL OF	3'-6" DIA. DRILLED PIERS IN SOIL	3'-6" DIA. DRILLED PIERS NOT IN SOIL	PERMANENT STEEL CASING FOR 3'-6" DIA. DRILLED PIER	SID INSPECTION	CROSSHOLE SONIC LOGGING	UNCLASSIFIED STRUCTURE EXCAVATION	CONCRETE WEARING SURFACE
	LUMP SUM	LUMP SUM	LUMP SUM	LIN FT.	LIN FT.	LIN FT.	EACH	EACH	CU. YDS.	SQ.FT.
SUPERSTRUCTURE										8365
END BENT #1									130	
BENT #1				43 . 5	33.0	46.5		1		
BENT #2				41.5	21.0	44.4	1	1		
END BENT #2									800	
TOTAL	LUMP SUM	LUMP SUM	LUMP SUM	85.0	54.0	90.9	1	2	930	8365

	GROOVING BRIDGE FLOORS	CLASS A CONCRETE	BRIDGE APPROACH SLABS	REINFORCING STEEL	SPIRAL COLUMN REINFORCING STEEL		2 12×53 STEEL PILES	CONCRETE BARRIER RAIL	RIP RAP CLASS II (2'-0" THICK)		ELASTOMERIC BEARINGS	EVAZOTE JOINT SEALS	PRES	" X 2'-3" STRESSED NCRETE K BEAMS
	SQ.FT.	CU. YDS.	LUMP SUM	LBS.	LBS.	NO.	LIN.FT.	LIN.FT.	TONS	SQ. YDS.	LUMP SUM	LUMP SUM	NO.	LIN.FT.
SUPERSTRUCTURE	9601		LUMP SUM					401.75			LUMP SUM	LUMP SUM	45	3009.39
END BENT #1		20.2		3279		11	220		160	180				
BENT #1		29.4		8543	1725									
BENT #2		29.4		7876	1423									
END BENT #2		20.2		3279		11	165		110	120				
TOTAL	9601	99.2	LUMP SUM	22977	3148	22	385	401.75	270	300	LUMP SUM	LUMP SUM	45	3009.39

PROJECT NO. B-4276STANLY COUNTY STATION: 19+96.00 -L-

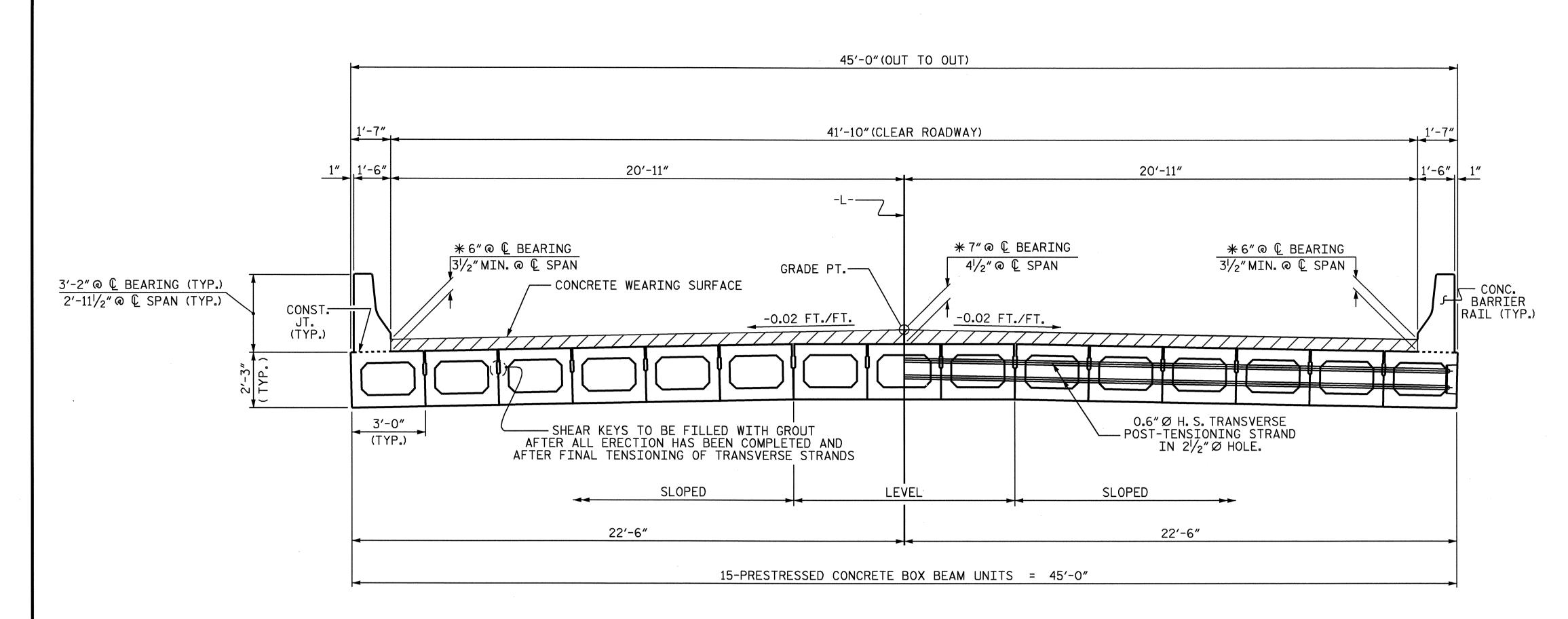
SHEET 3 OF 3

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

GENERAL DRAWING FOR BRIDGE OVER

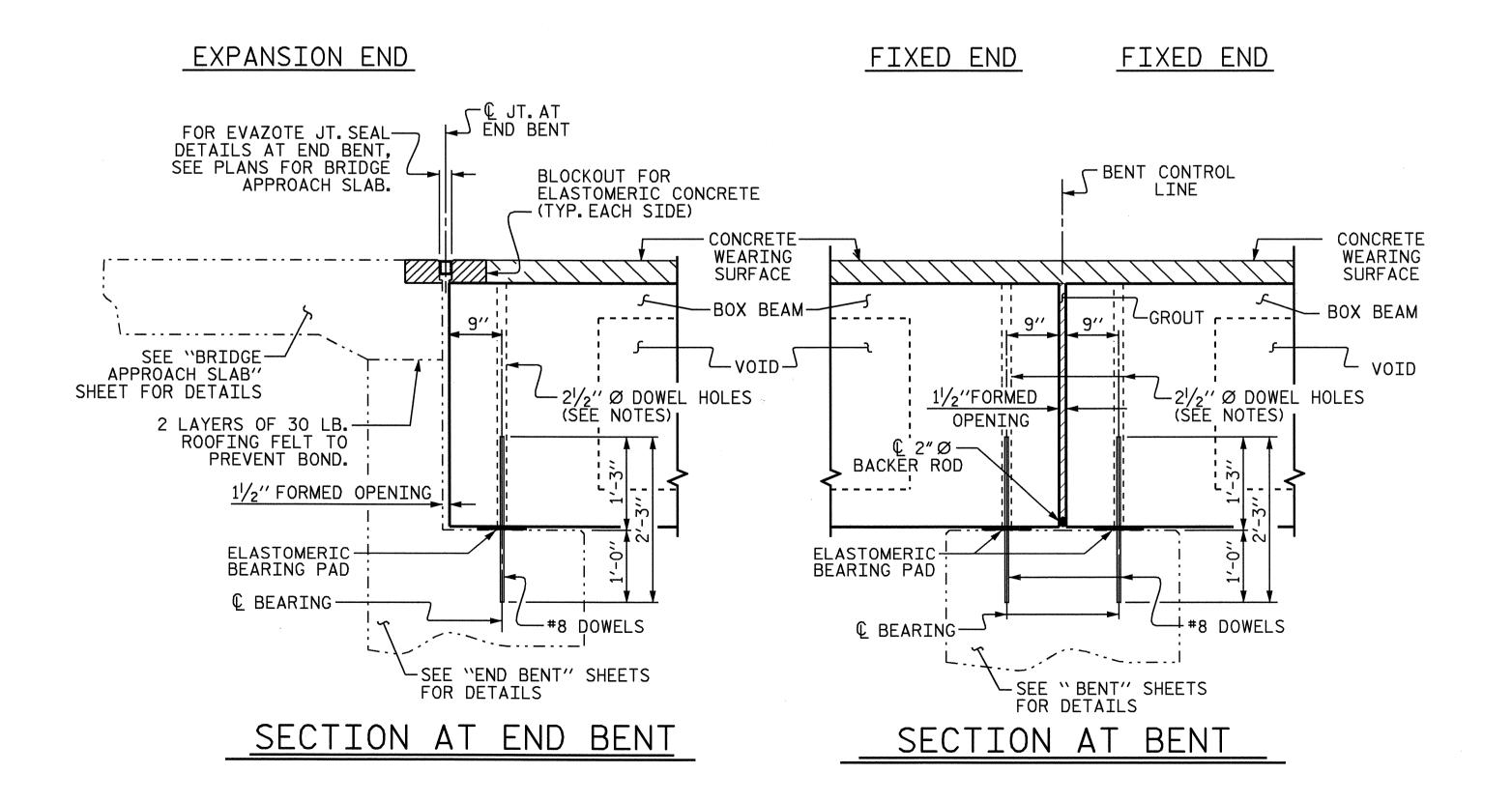
REVISIONS S-3 DATE: BY: DATE: TOTAL SHEETS 21

DRAWN BY: KEITH D. LAYNE
CHECKED BY: H. A. LOCKLEAR
DATE: 01-10-08
DATE: 1/24/08



TYPICAL SECTION

* BASED ON PREDICTED FINAL CAMBER AND THEORETICAL GRADE LINE ELEVATIONS



NOTES

ALL PRESTRESSING STRANDS SHALL BE 7-WIRE LOW RELAXATION GRADE 270 STRANDS AND SHALL CONFORM TO AASHTO M203 EXCEPT FOR SAMPLING REQUIREMENTS WHICH SHALL BE IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

ALL REINFORCING STEEL CAST WITH THE BOX BEAM SECTIONS SHALL BE GRADE 60 AND SHALL BE INCLUDED IN THE UNIT PRICE BID FOR PRESTRESSED CONCRETE BOX BEAMS.

RECESSES FOR TRANSVERSE STRANDS SHALL BE GROUTED AFTER THE TENSIONING OF THE STRANDS.

THE $2^{1}\!/_{2}$ " Ø DOWEL HOLES AT FIXED ENDS OF BOX BEAM SECTIONS SHALL BE FILLED WITH NON-SHRINK GROUT. THE $2^{1}\!/_{2}$ " Ø DOWEL HOLES AT EXPANSION ENDS OF BOX BEAM SECTIONS SHALL BE FILLED WITH JOINT SEALER MATERIAL TO $1^{1}\!/_{2}$ " ABOVE THE TOP OF DOWELS AND THEN FILLED WITH GROUT

THE JOINT SEALER MATERIAL SHALL CONFORM TO THE REQUIREMENTS OF TYPE SL LOW MODULUS SILICONE SEALANT. THE 2"Ø BACKER ROD SHALL CONFORM TO THE REQUIREMENTS OF TYPE M BOND BREAKER. SEE SECTION 1028 OF THE STANDARD SPECIFICATIONS.

THE TRANSFER OF LOAD FROM THE ANCHORAGES TO THE BOX BEAM UNIT SHALL BE DONE WHEN THE CONCRETE HAS REACHED A COMPRESSIVE STRENGTH OF NOT LESS THAN 4800 PSI.

ALL REINFORCING STEEL IN BARRIER RAILS AND CONCRETE WEARING SURFACE SHALL BE EPOXY COATED.

PRESTRESSING STRANDS SHALL BE CUT FLUSH WITH THE BOX BEAM UNIT ENDS.

APPLY EPOXY PROTECTIVE COATING TO BOX BEAM UNIT ENDS.

VERTICAL GROOVED CONTRACTION JOINTS, 1/2"IN DEPTH, SHALL BE TOOLED IN ALL EXPOSED FACES OF PARAPET AND IN ACCORDANCE WITH ARTICLE 825-10(B) OF THE STANDARD SPECIFICATIONS. THE CONTRACTION JOINTS SHALL BE LOCATED AT THIRD POINT BETWEEN PARAPET EXPANSION JOINTS. ONLY ONE CONTRACTION JOINT IS REQUIRED AT MIDPOINT OF PARAPET SEGMENTS LESS THAN 20 FEET IN LENGTH AND NO CONTRACTION JOINTS ARE REQUIRED FOR FOR THOSE SEGMENTS LESS THAN 10 FEET IN LENGTH.

FOR EVAZOTE JOINT SEALS, SEE SPECIAL PROVISIONS.

PLACEMENT OF THE CONCRETE WEARING SURFACE SHALL OCCUR AFTER CASTING THE CONCRETE RAIL. THE COST OF THE REINFORCING STEEL CAST WITH THE CONCRETE WEARING SURFACE SHALL BE INCLUDED IN THE UNIT PRICE BID FOR CONCRETE WEARING SURFACE. FOR CONCRETE WEARING SURFACE, SEE SPECIAL PROVISIONS.

FOR ELASTOMERIC CONCRETE, SEE SPECIAL PROVISIONS.

THE LOCATION OF THE VOID DRAINS MAY BE SHIFTED SLIGHTLY WHERE NECESSARY TO CLEAR PRESTRESSING STRANDS OR TRANSVERSE REINFORCING STEEL.

FOR PRESTRESSED CONCRETE MEMBERS, SEE SPECIAL PROVISIONS.

FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.

TRANSVERSE POST TENSIONING OF THE BOX BEAM SECTIONS SHALL BE DONE IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS, EXCEPT THAT THE 0.6 0.6 TRANDS SHALL BE TENSIONED TO 43,950 POUNDS.

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STANLY COUNTY

STATION: 19+96.00 -L-



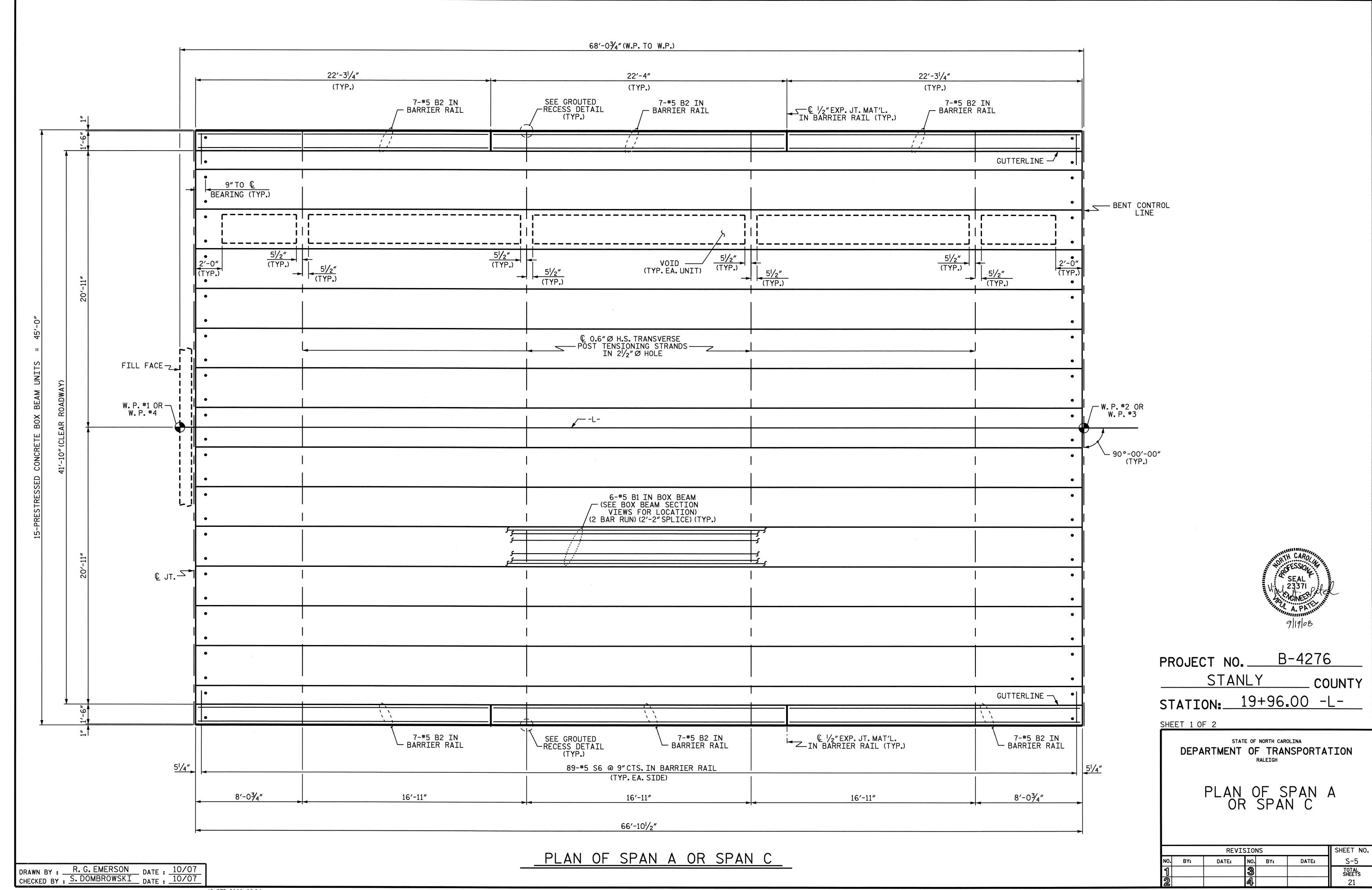
DEPARTMENT OF TRANSPORTATION
RALEIGH
STANDARD

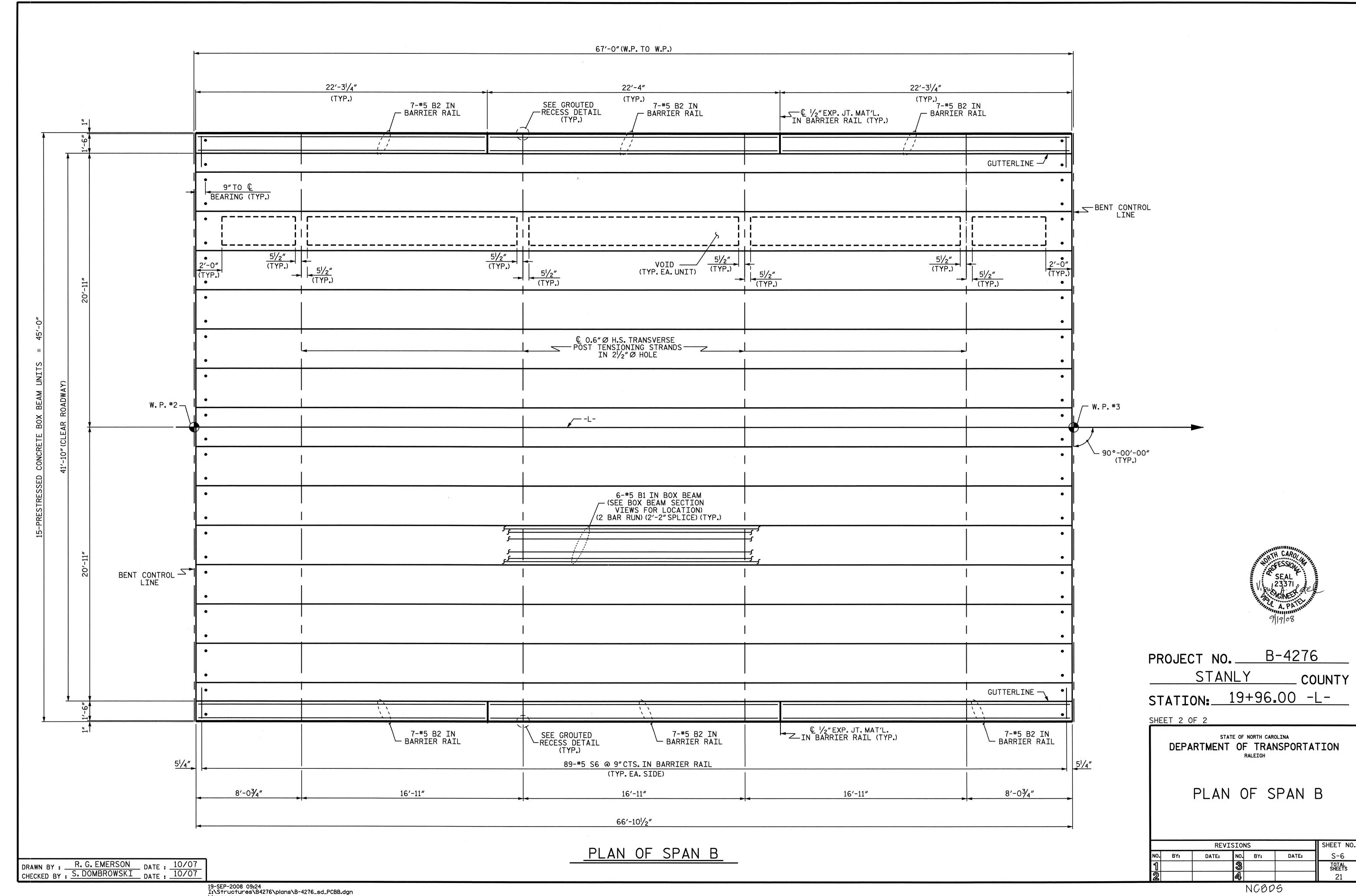
3'-0" X 2'-3"
PRESTRESSED CONCRETE
BOX BEAM UNIT

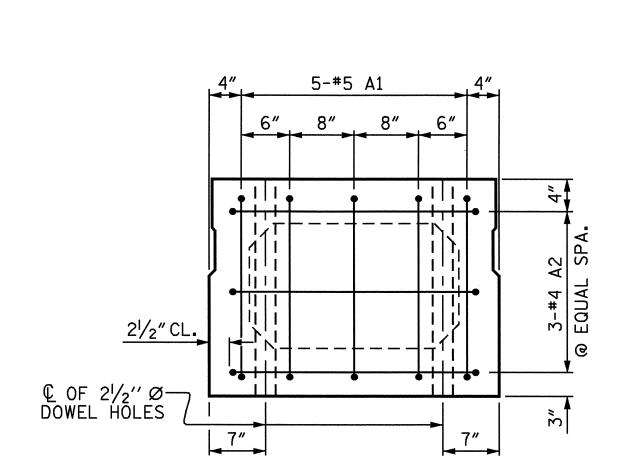
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		3			TOTAL SHEETS
		4			21
	BY:		BY: DATE: NO.	3	BY: DATE: NO. BY: DATE:

ASSEMBLED BY : R. G. EMERSON DATE : 10/07 CHECKED BY : S. DOMBROWSKI DATE : 10/07

DRAWN BY : TLA 5/05 ADDED 7/11/05R REV. 5/1/06R KMM/GM

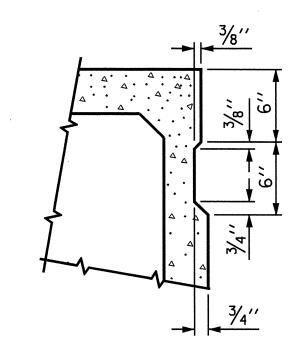






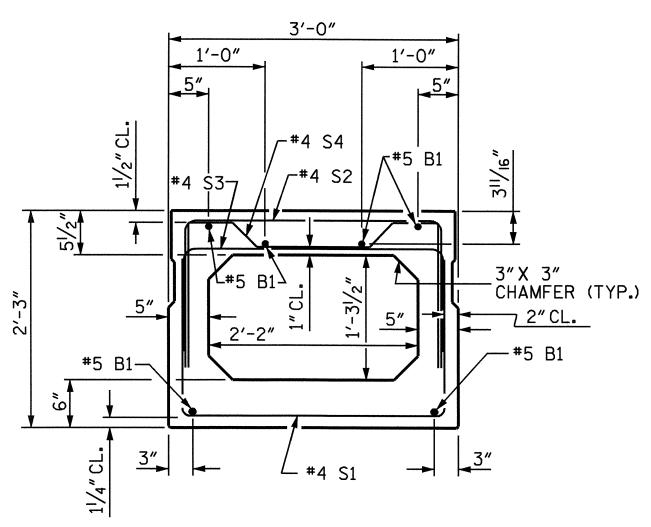
END ELEVATION

SHOWING PLACEMENT OF #5 & #4 "A" BARS
AND LOCATION OF DOWEL HOLES.
(INTERIOR BOX BEAM SECTION SHOWN-EXTERIOR
SECTION SIMILAR EXCEPT SHEAR KEY LOCATION.
STRAND LAYOUT NOT SHOWN.)



SHEAR KEY DETAIL

NOTE: OMIT SHEAR KEY ON OUTSIDE FACE OF EXTERIOR BOX BEAMS.



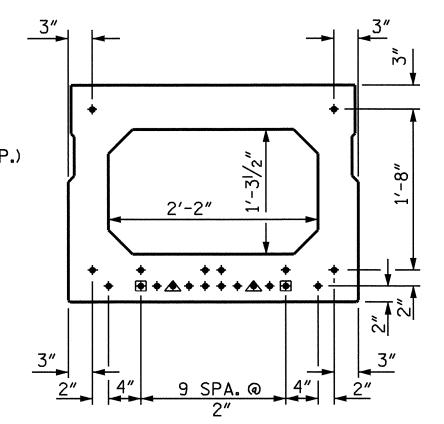
INTERIOR BOX BEAM SECTION

(STRAND LAYOUT NOT SHOWN)

#5 S5 #4 S2 #4 S3 #5 B1 3" X 3" CHAMFER (TYP.) 2" CL. #5 B1 3" X 3" CHAMFER (TYP.) 4 S1

EXTERIOR BOX BEAM SECTION (STRAND LAYOUT NOT SHOWN)

0.6" Ø LOW RELAXATION STRAND LAYOUT



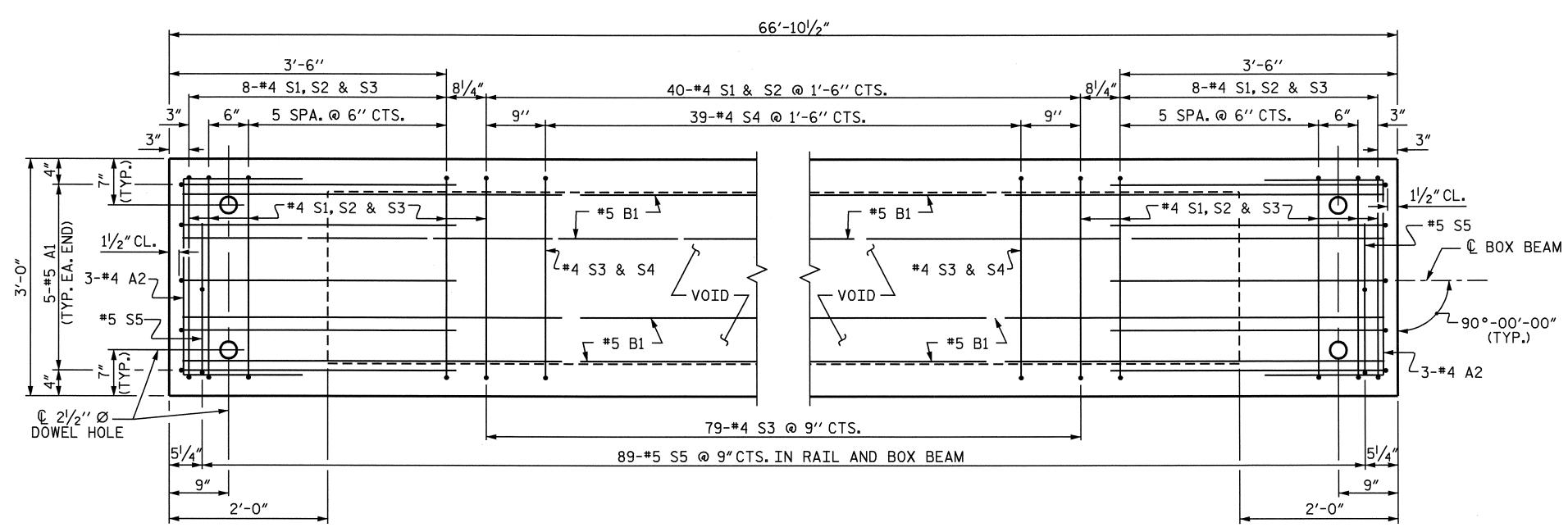
TYPICAL STRAND LOCATION (20 STRANDS REQUIRED)

(INTERIOR BOX BEAM SECTION SHOWN. EXTERIOR SECTION SIMILAR EXCEPT SHEAR KEY LOCATION)

DEBONDING LEGEND

- FULLY BONDED STRANDS
- STRANDS DEBONDED FOR 2'-0"
 FROM END OF GIRDER
- STRANDS DEBONDED FOR 4'-0" FROM END OF GIRDER

GRADE 270	STRANDS
·	0.6″Ø L.R.
AREA (SQUARE INCHES)	0.217
ULTIMATE STRENGTH (LBS.PER STRAND)	58,600
APPLIED PRESTRESS (LBS.PER STRAND)	43,950

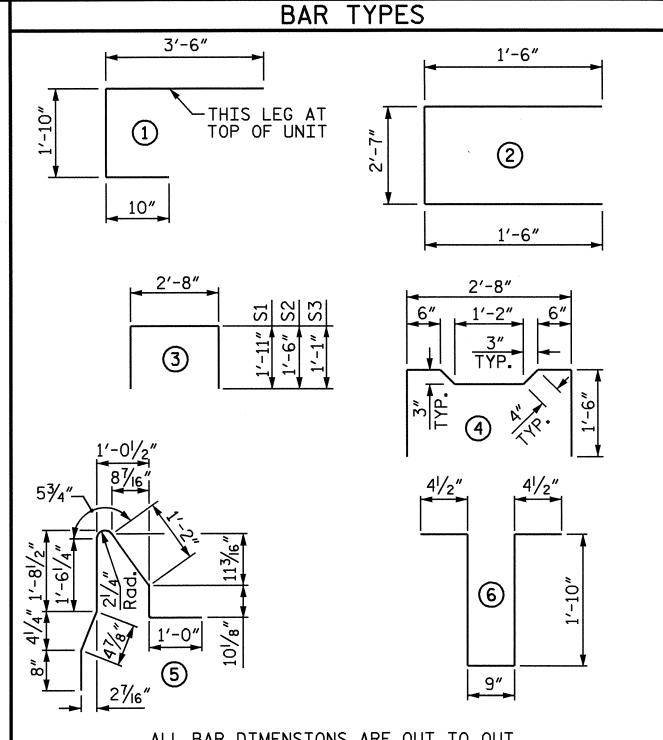


PLAN OF BOX BEAM

EXTERIOR UNIT SHOWN, INTERIOR UNIT SIMILAR EXCEPT OMIT #5 S5 BARS. FOR LOCATION OF DIAPHRAGMS, SEE PLAN OF SPANS. FOR REINFORCING STEEL IN DIAPHRAGMS, SEE DIAPHRAGM DETAILS.

ASSEMBLED BY : R. G. EMERSON DATE : 10/07 CHECKED BY : S. DOMBROWSKI DATE : 10/07

DRAWN BY : TLA 5/05 ADDED 7/11/05 REV. 5/1/06 TLA/GM



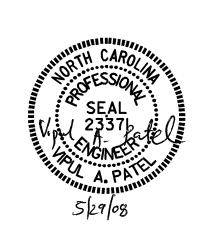
ALL BAR DIMENSIONS ARE OUT TO OUT

B.	ILL OF	- MAT	ERIAL	FOR ON	NE BOX	BEAM UN	VIT		
				EXTERI(OR UNIT	INTERIOR UNIT			
BAR	NUMBER	SIZE	TYPE	LENGTH	WEIGHT	LENGTH	WEIGHT		
A1	10	#5	1	6'-2"	64	6′-2″	64		
A2	22	#4	2	5′-7″	82	5′-7″	82		
B1	12	#5	STR	34'-5"	431	34'-5"	431		
K1	12	#4	6	5′-2″	41	5′-2″	41		
K2	8	#4	STR	2'-7"	14	2'-7"	14		
S1	56	#4	3	6′-6″	243	6′-6″	243		
S2	56	#4	3	5′-8″	212	5′-8″	212		
S3	95	#4	3	4'-10"	307	4′-10″	307		
S4	39	#4	4	5′-10″	152	5′-10″	152		
		ļ							
* S5	89	#5	5	6′-1″	565				
		STEEL			1546 LBS.	1	1546 LBS.		
	Y COATE		IF. STEEL		565 LBS.				
6,500	P.S.I. CC	NCRETE		10.	.9 CU. YDS.	10.	.9 CU. YDS.		
0.6"Ø	L.R. STR.	ANDS		No. 2	20	No. 20			

PROJECT NO. B-4276

STANLY COUNTY

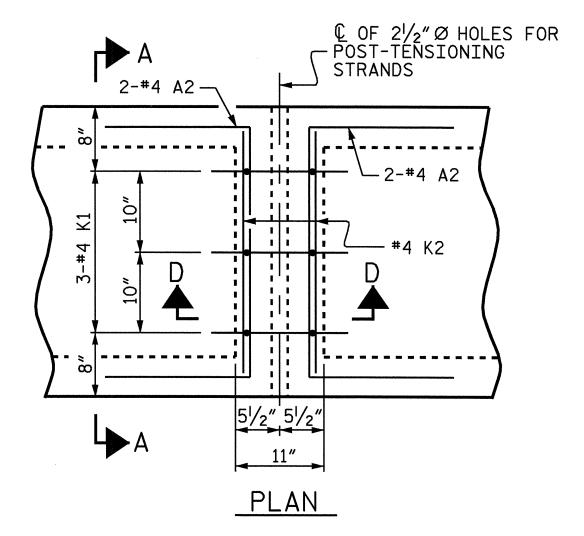
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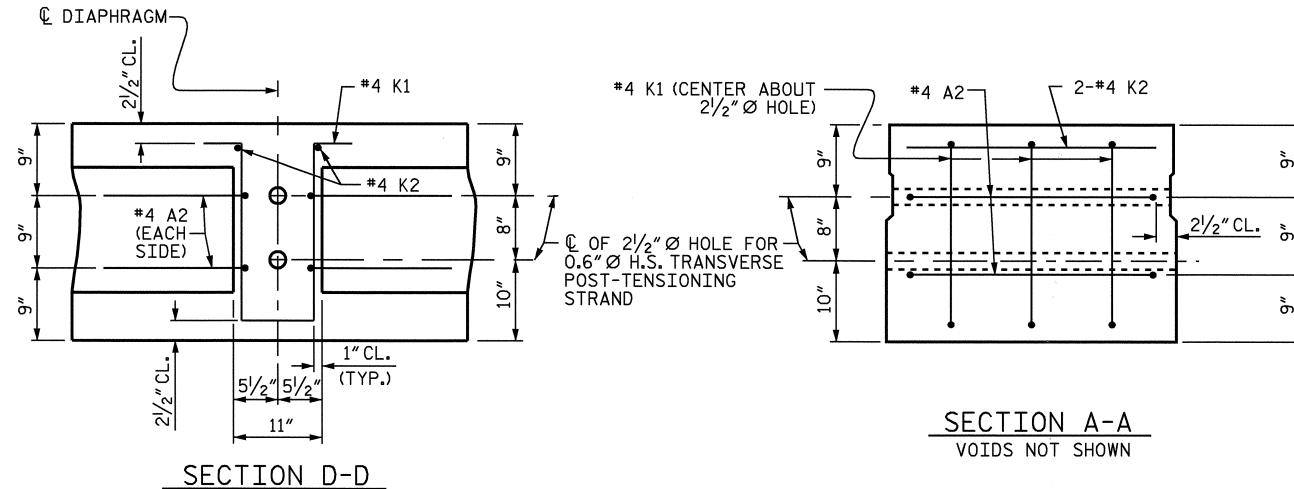


DEPARTMENT OF TRANSPORTATION
RALEIGH
STANDARD

3'-0" X 2'-3"
PRESTRESSED CONCRETE
BOX BEAM UNIT

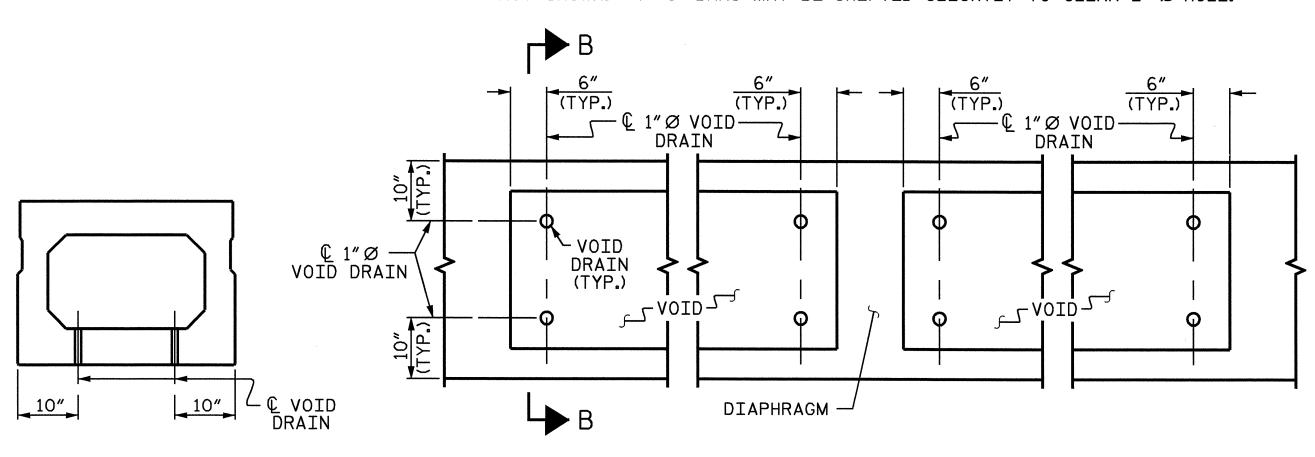
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DOUBLE DIAPHRAGM DETAILS

#4 "S" BARS NOT SHOWN. #4 "S" BARS MAY BE SHIFTED SLIGHTLY TO CLEAR 2" Ø HOLE.



VOID DRAIN DETAILS

(DIMENSIONS SHOWN ARE TYPICAL FOR EACH VOID)

ASSEMBLED BY : R. G. EMERSON DATE : 10/07
CHECKED BY : S. DOMBROWSKI DATE : 10/07

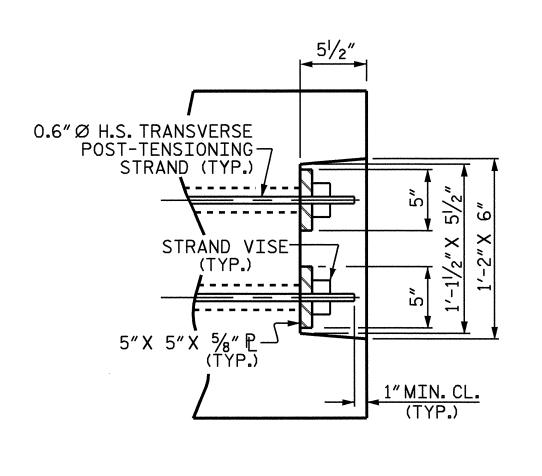
DRAWN BY : TLA 5/05
CHECKED BY : GM 6/05

ADDED 7/11/05
REV. 5/1/06

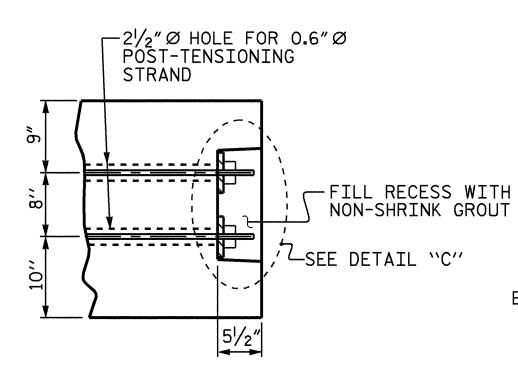
TLA/GM

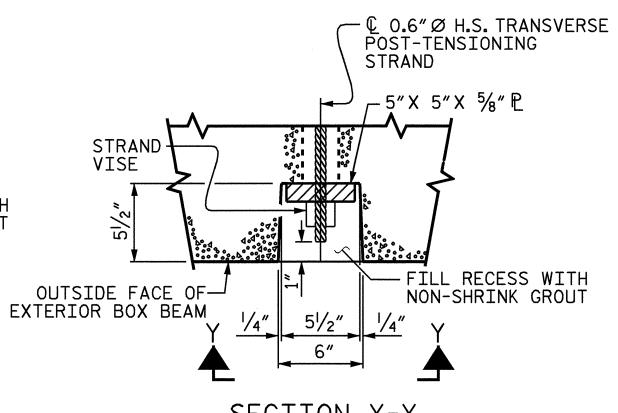
SECTION B-B

PART PLAN



DETAIL "C"





PART SECTION AT RECESS

SECTION X-X
SHOWING PLAN VIEW OF GROUTED RECESS

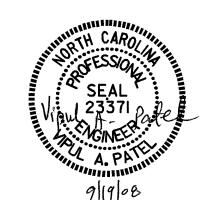
GROUTED RECESS DETAIL AT END OF POST-TENSIONED STRANDS OF EXTERIOR BOX BEAM

DEAD LOAD DEF	.	ECTTON A	ND CAMBEI				
3'-0" × 2'-3"							
0.6" Ø L.R. STRAND							
		SPAN "A"	SPAN "B"	SPAN "C"			
CAMBER (BEAM ALONE IN PLACE)	•	3 ¹ / ₁₆ "	3 ¹ /16"	31/ ₁₆ "			
DEFLECTION DUE TO CONCRETE WEARING SURFACE	↓ ▼	³ /8″	3/8"	3/8"			
FINAL CAMBER *	A	2 ¹ / ₁₆ "	2 / ₆ "	2 / ₁₆ "			
* DOES NOT INCLUDE DEFLECTION [DUE	E TO RAIL &	FUTURE WEARI	NG SURFACE			

PROJECT NO. B-4276

STANLY COUNTY

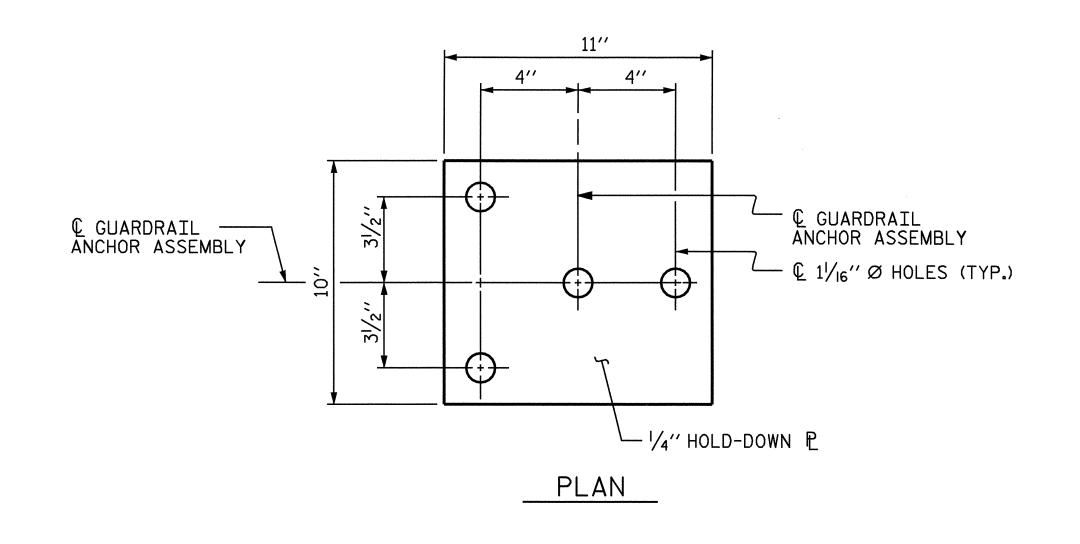
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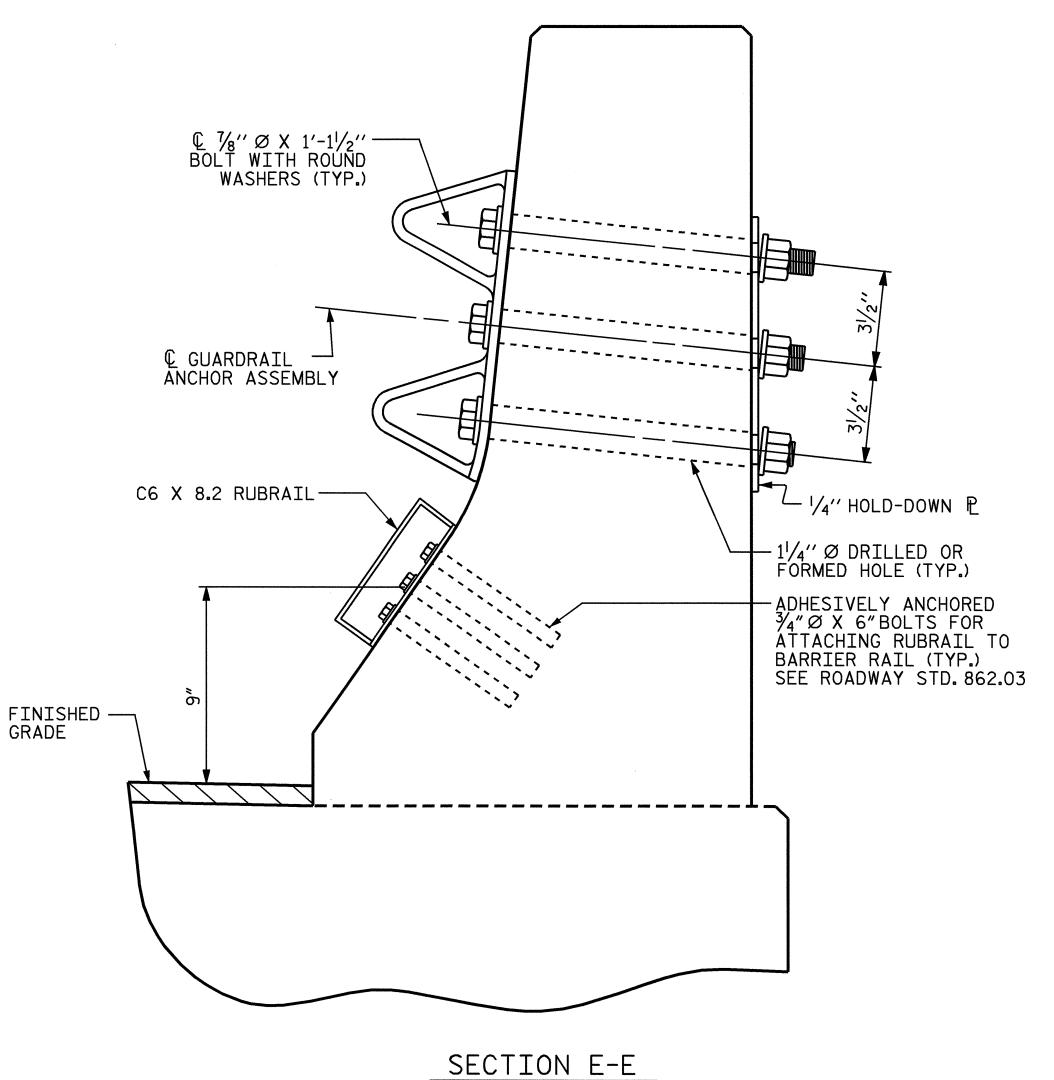


DEPARTMENT OF TRANSPORTATION
RALEIGH
STANDARD

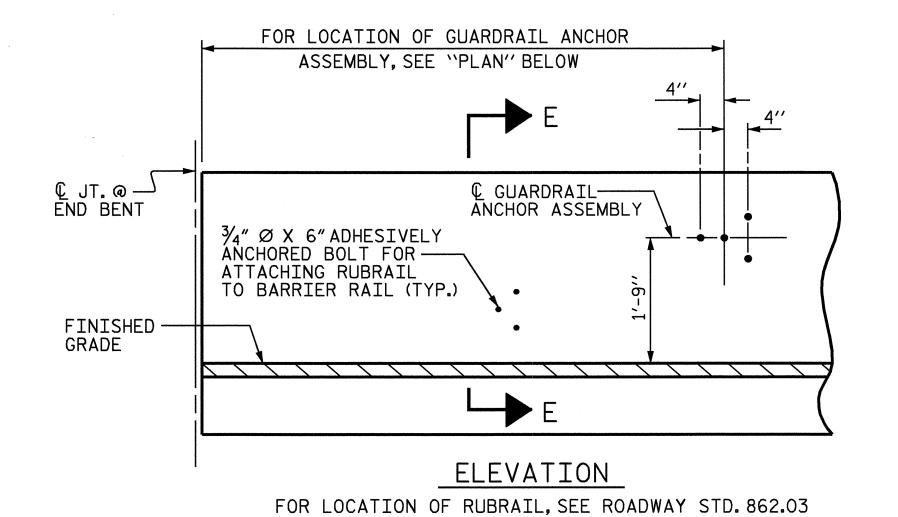
3'-0" X 2'-3"
PRESTRESSED CONCRETE
BOX BEAM UNIT

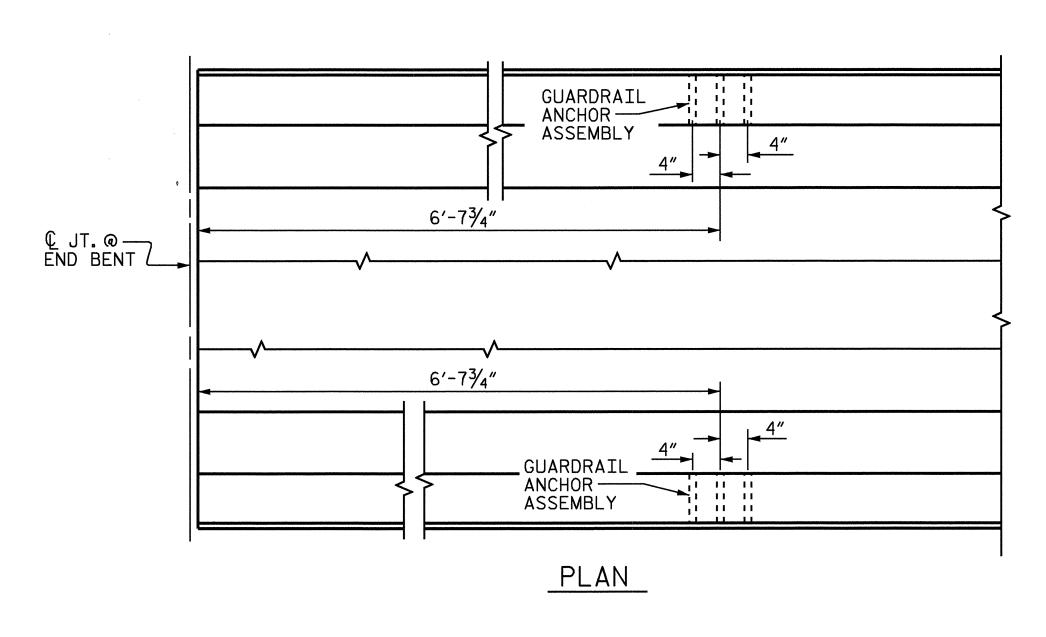
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		4			21





GUARDRAIL ANCHOR ASSEMBLY DETAILS





LOCATION OF ANCHORS FOR GUARDRAIL

END BENT #1 SHOWN, END BENT #2 SIMILAR.

NOTES

THE GUARDRAIL ANCHOR ASSEMBLY SHALL CONSIST OF A 1/4" HOLD DOWN PLATE AND 4 - 1/8" Ø BOLTS WITH NUTS AND WASHERS, RUBRAIL, AND ADHESIVELY ANCHORED

THE HOLD-DOWN PLATE SHALL CONFORM TO AASHTO M270 GRADE 36. AFTER FABRICATION, THE HOLD-DOWN PLATE SHALL BE HOT-DIP GALVANIZED IN ACCORDANCE WITH AASHTO M111.

BOLTS SHALL CONFORM TO THE REQUIREMENTS OF ASTM A307 AND NUTS SHALL CONFORM TO THE REQUIREMENTS OF AASHTO M291. BOLTS, NUTS AND WASHERS SHALL BE GALVANIZED. (AT THE CONTRACTOR'S OPTION, STAINLESS STEEL BOLTS, NUTS AND WASHERS MAY BE USED AS AN ALTERNATE FOR THE 1/8" Ø GALVANIZED BOLTS, NUTS AND WASHERS. THEY SHALL CONFORM TO OR EXCEED THE MECHANICAL REQUIREMENTS OF ASTM A307. THE USE OF THIS ALTERNATE SHALL BE APPROVED BY THE ENGINEER.)

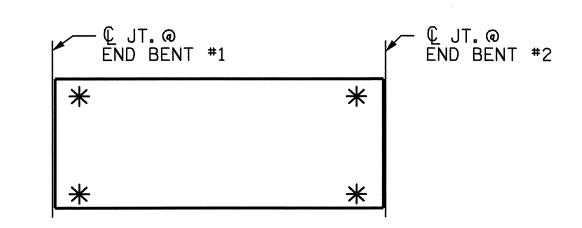
THE GUARDRAIL ANCHOR ASSEMBLY IS REQUIRED AT ALL POINTS WHERE APPROACH GUARDRAIL IS TO BE ATTACHED TO THE END OF BARRIER RAIL. FOR POINTS OF ATTACHMENT, SEE SKETCH.

AFTER INSTALLATION, THE EXPOSED THREAD OF THE BOLT SHALL BE BURRED WITH A SHARP POINTED TOOL.

THE COST OF THE GUARDRAIL ANCHOR ASSEMBLY SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR CONCRETE BARRIER RAIL.

THE 1 $\frac{1}{4}$ " Ø HOLES SHALL BE FORMED OR DRILLED WITH A CORE BIT. IMPACT TOOLS WILL NOT BE PERMITTED. ANY CONCRETE DAMAGED BY THIS WORK SHALL BE REPAIRED TO THE SATISFACTION OF THE ENGINEER.

THE C6 X 8.2 RUBRAIL IS TO BE ADHESIVELY ANCHORED TO THE RAIL USING THREE $\frac{3}{4}$ " Ø X 6"BOLTS WITH WASHERS. LEVEL ONE FIELD TESTING IS REQUIRED, AND THE YIELD LOAD OF THE $\frac{3}{4}$ " Ø BOLT IS 12 KIPS. FOR ADHESIVELY ANCHORED ANCHOR BOLTS OR DOWELS, SEE SPECIAL PROVISIONS. SEE ROADWAY STANDARD 862.03 FOR DETAILS AND LOCATION OF THE RUBRAIL.



SKETCH SHOWING POINTS OF ATTACHMENTS

* DENOTES GUARDRAIL ANCHOR ASSEMBLY

PROJECT NO. B-4276 STANLY COUNTY STATION: 19+96.00 -L-

> STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION STANDARD

GUARDRAIL ANCHORAGE FOR BARRIER RAIL





	REV	ISION	S		SHEET NO.
BY:	DATE:	NO.	BY:	DATE:	S-9
		3			TOTAL SHEETS
		4			21

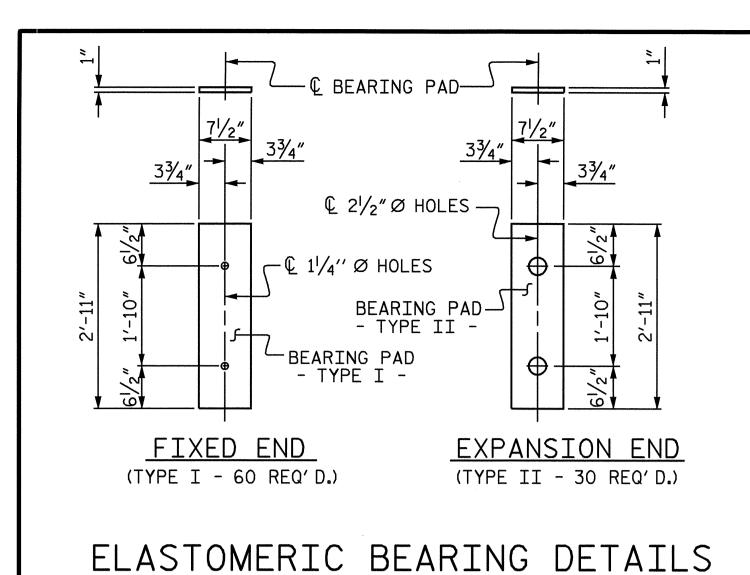
28-MAY-2008 08:48 I:\Structures\B4276\plans\B4276_sd_GR.dgn

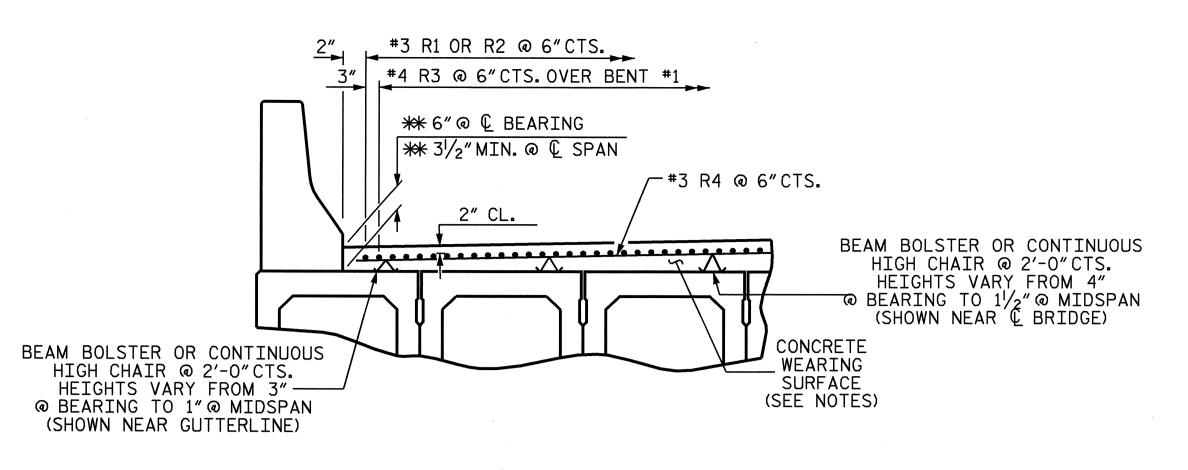
ASSEMBLED BY : S. DOMBROWSKI

CHECKED BY : V. PATEL

DRAWN BY: TLA 5/06 CHECKED BY: GM 5/06 DATE:5/8/08

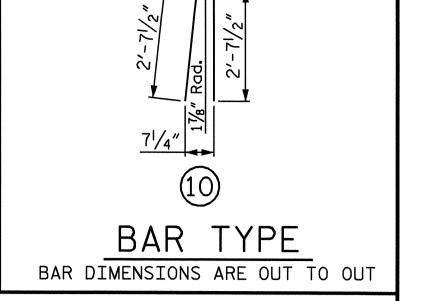
DATE:5/8/08 ADDED 5/I/OGR KMM/GM





TOTAL LENGTH NUMBER LENGTH SPAN A EXTERIOR B. B. 66'-101/2" 133.75 66'-101/2" INTERIOR B. B. 13 869.38 SPAN B EXTERIOR B. B. 66'-101/2" 133.75 INTERIOR B. B. 13 66'-101/2" 869.38 SPAN C EXTERIOR B. B. 66'-101/2" 133.75 66'-101/2" INTERIOR B.B. 13 869.39 TOTAL 45 3009.39 _____

BOX BEAM UNITS REQUIRED



	BILL	OF	MATE	RIAL	FOR	CONCR	ETE E	BARR	IER RA	4IL
BAR				SPAN		TOTAL NO.	SIZE	TYPE	LENGTH	WEIGHT
		SPAN A	SPAN B	SPAN C						
 ₩B2		42	42	42		126	#5	STR	21′-11″	2880
* S6		178	178	178		534	#5	10	5′-9″	3203
<u></u> ★ El	POXY CO	DATED F	REINFOR	RCING S	TEEL	LBS.				6083
	CLASS AA CONCRETE CU. YDS. 51.4									
TOT	AL LIN	.FT.OF	CONCRE	TE BAR	RIER R	AIL				401.75

GROOVING	BRIDGE	FL(ORS
APPROACH SLABS	18-	49	SQ.FT.
BRIDGE DECK	77!	52	SQ.FT.
TOTAL	_ 96	01	SQ.FT.

Ţ	SPLICE LEN	IGTH CHART	
	BAR SIZE	EPOXY COATED	
	#3	1′-3″	
	#4	1′-8″	

BILL OF MATERIAL FOR CONCRETE WEARING SURFACE							
BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT		
 ₩R1	588	#3	STR	29'-8"	6559		
₩R2	800	#3	STR	21′-5″	6442		
 ₩R3	166	#4	STR	20'-0"	2218		
EPOXY COATED REINFORCING STEEL LBS. 15,219							
CONCRETE	CONCRETE WEARING SURFACE SQ. FT. 8365						

SPLICE LEN	IGTH CHART
BAR SIZE	EPOXY COATED
#3	1′-3″
#4	1′-8″

BILL OF MATERIAL FOR CONCRETE WEARING SURFACE								
BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT			
 ₩R1	588	#3	STR	29'-8"	6559			
₩R2	800	#3	STR	21′-5″	6442			
 ₩R3	166	#4	STR	20'-0"	2218			
* EPOXY COATED REINFORCING STEEL LBS. 15,219								
CONCRETE	WEARIN	CONCRETE WEARING SURFACE SQ.FT. 8365						

REINFORCING FOR CONCRETE WEARING SURFACE

** BASED ON PREDICTED FINAL CAMBER AND THEORETICAL GRADE LINE ELEVATIONS

ELASTOMER IN ALL BEARINGS SHALL BE 60 DUROMETER HARDNESS

1'-93/4" 400 - #3 R2 @ 6"CTS.(2 BAR RUN) © JOINT @── END BENT #1 —€ JOINT @ END BENT #2 BENT #2 BENT #1 CONTROL LINE CONTROL LINE 90° 00′ 00″ (TYP.) 10'-0" 10'-0"

PLAN SHOWING CONCRETE WEARING SURFACE REINFORCING STEEL

© 1/2"EXP. JT. MAT'L HELD IN PLACE WITH GALVANIZED NAILS. (NOTE: OMIT EXP. JT. MAT'L. WHEN SLIP FORM IS USED) PAIL @ BENT CHAMFER CHAMFER CHAMFER CONST. JT. ELEVATION AT EXPANSION JOINTS

9"@ L BEARING 61/2"@ (SPAN CONST.--#5 S5 @ 9" CTS.

SECTION S-S

AT DAM IN OPEN JOINT (THIS IS TO BE USED ONLY WHEN SLIP FORM IS USED)

SECTION THRU RAIL

BARRIER RAIL DETAILS

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

PROJECT NO. ___

STATION:_

STANLY

STANDARD

B-4276

19+96.00 -L-

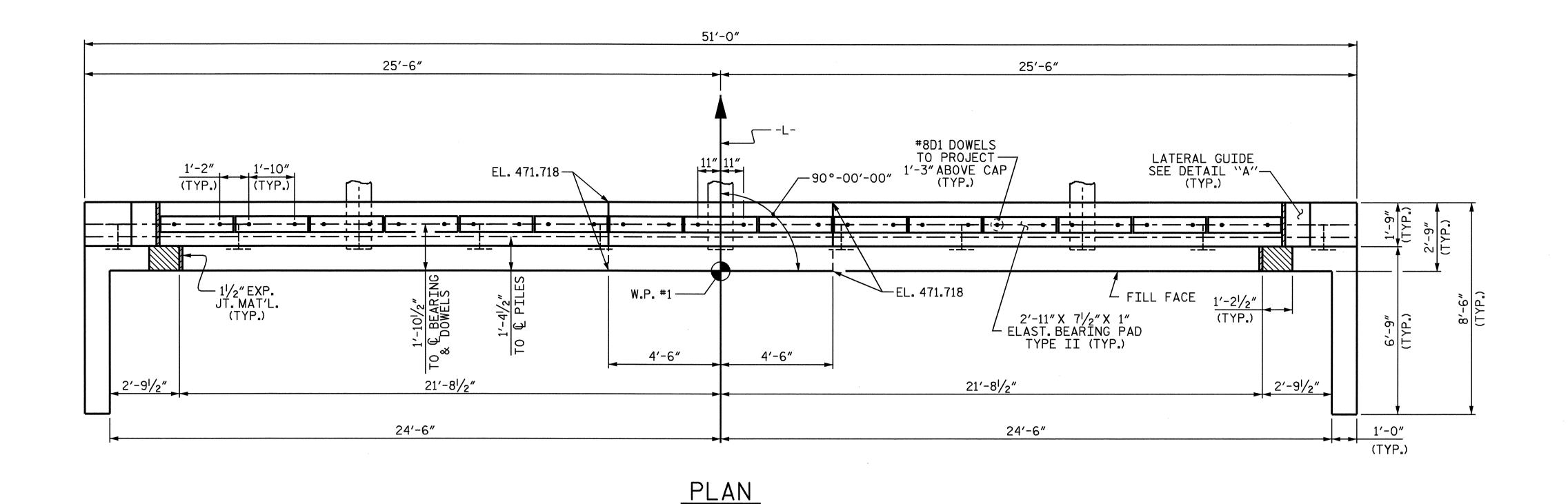
COUNTY

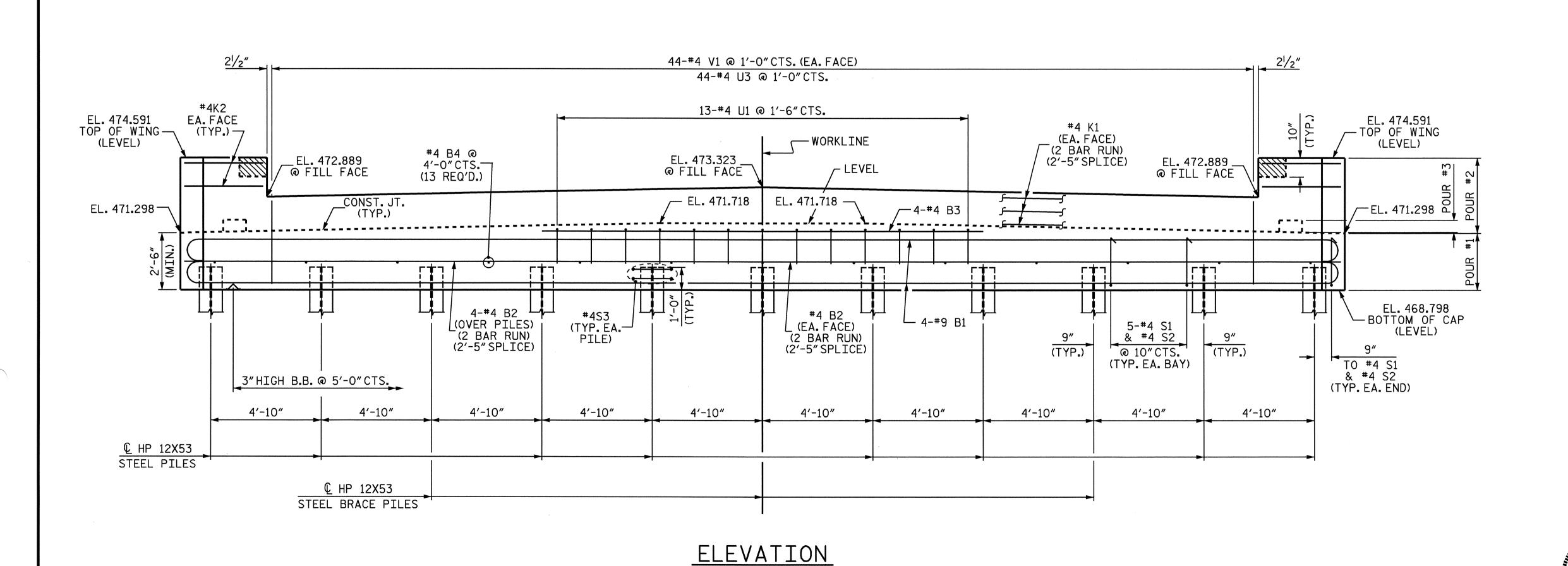
3'-0"X 2'-3"
PRESTRESSED CONCRETE
BOX BEAM UNIT DETAILS

REVISIONS					SHEET NO.	
).	BY:	DATE:	NO.	BY:	DATE:	S-10
			3			TOTAL SHEETS
2			4			21

ASSEMBLED BY : R. G. EMERSON DATE : 10/07 CHECKED BY : S. DOMBROWSKI DATE : 10/07 ADDED 7/II/05R REV. 5/I/06 DRAWN BY: TLA 5/05 CHECKED BY: GM 6/05 TLA/GM

28-MAY-2008 11:48
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DRAWN BY: S. DOMBROWSKI DATE: 10/07 CHECKED BY: R.G. EMERSON DATE: 12/07

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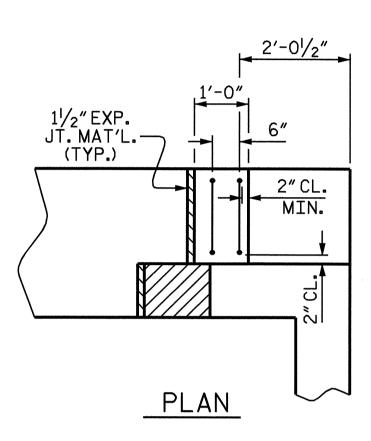
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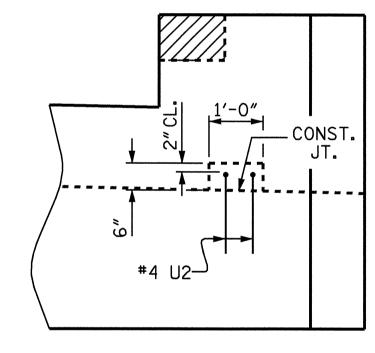
STIRRUPS IN CAP MAY BE SHIFTED AS NECESSARY TO CLEAR #8D1 DOWELS.

THE CONCRETE IN THE SHADED AREA OF THE WING SHALL BE POURED AFTER THE CONCRETE BARRIER RAIL IS CAST IF SLIP FORMING IS USED.

THE CONTRACTOR SHALL PROVIDE FOR INSTALLATION OF THE 4"DIAMETER DRAIN PIPE THROUGH THE WING WALL AS REQUIRED FOR REINFORCED BRIDGE APPROACH FILLS, SEE THE ROADWAY PLANS. REINFORCING STEEL IN THE WING WALL MAY BE SHIFTED AS NECESSARY TO CLEAR THE DRAIN PIPE.

THE LATERAL GUIDE AT EACH END OF THE CAP IS NOT TO BE POURED UNTIL AFTER CORED SLAB UNITS ARE IN PLACE.





ELEVATION DETAIL ''A''

PROJECT NO. B-4276

STANLY COUNTY

STATION: 19+96.00 -L-

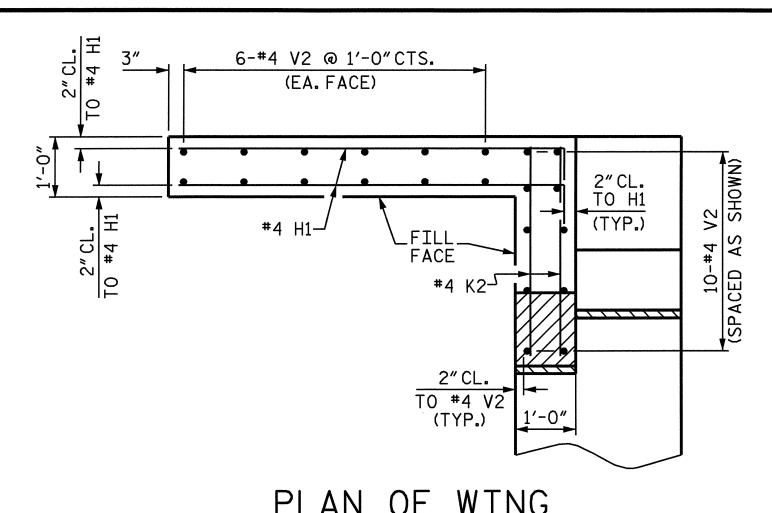
SHEET 1 OF 2

STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION

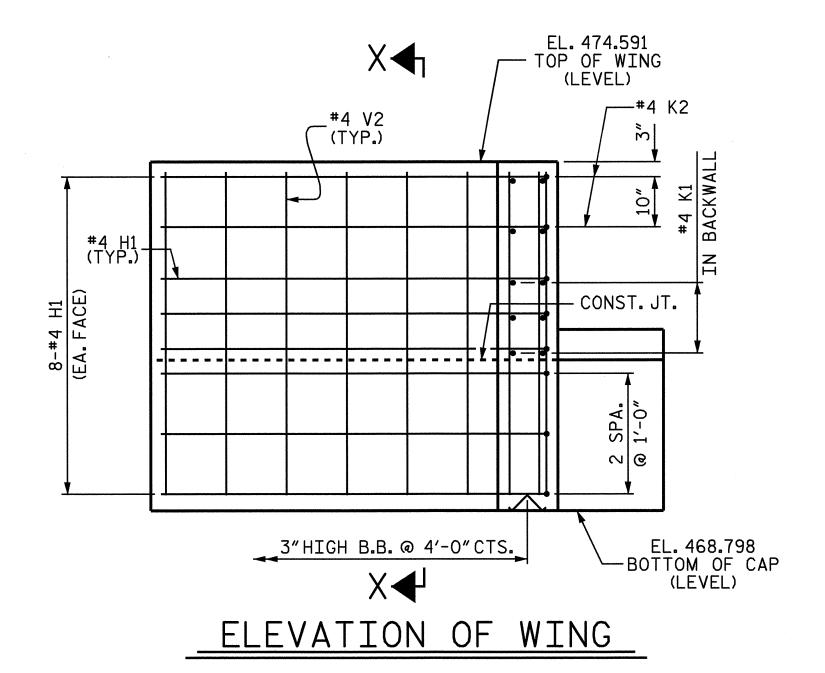
SUBSTRUCTURE END BENT #1

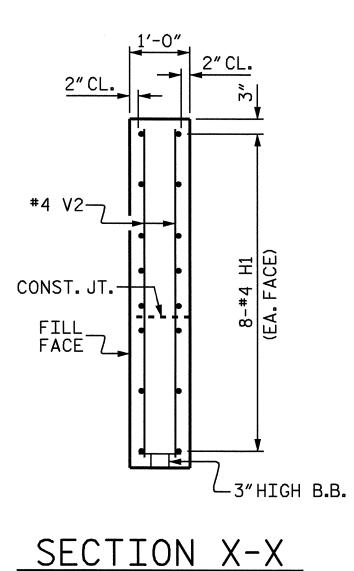


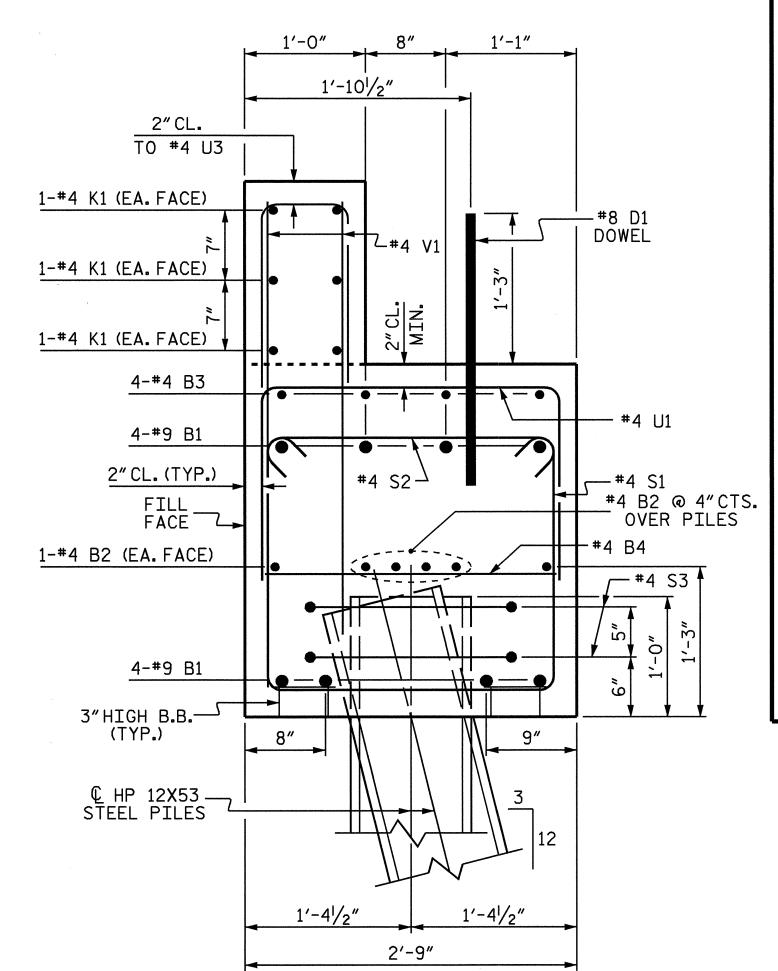
	REV:	ISION:	S		SHEET NO
BY:	DATE:	NO.	BY:	DATE:	S-11
		3			TOTAL SHEETS
					71 24



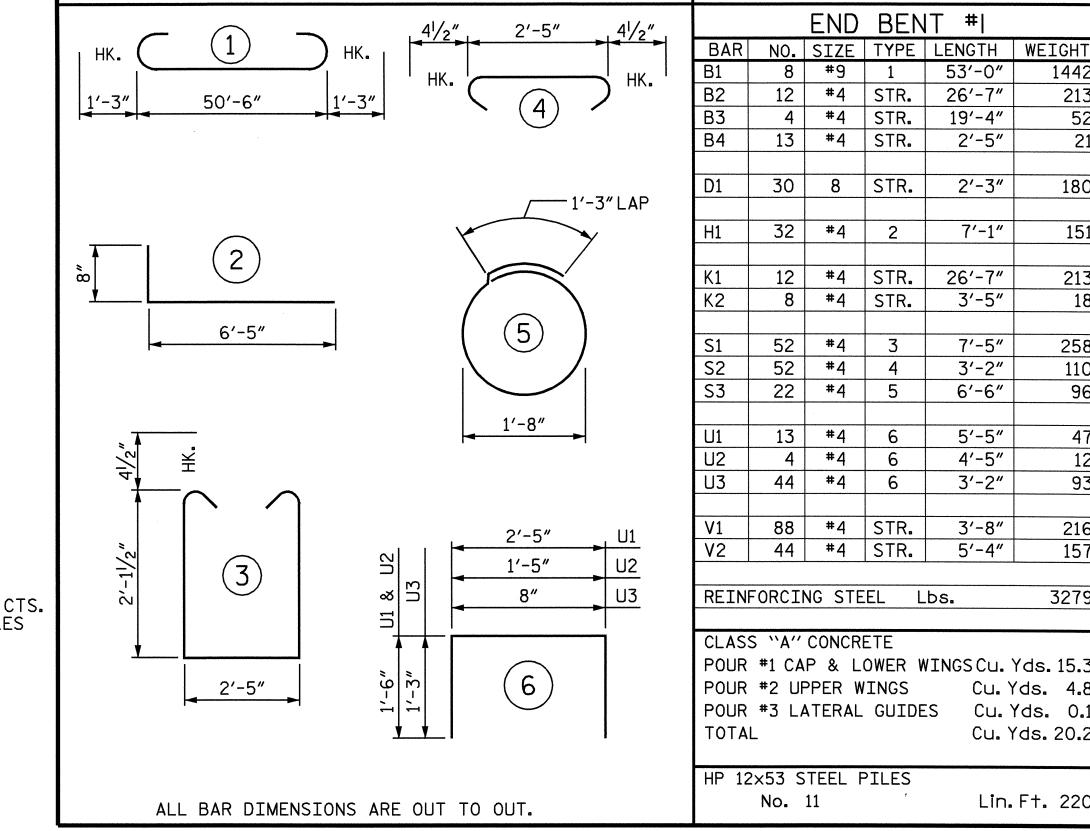




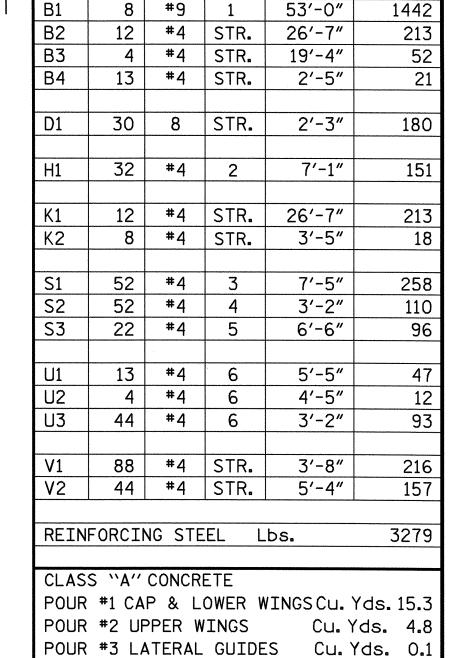




CAP SECTION



-BAR TYPES



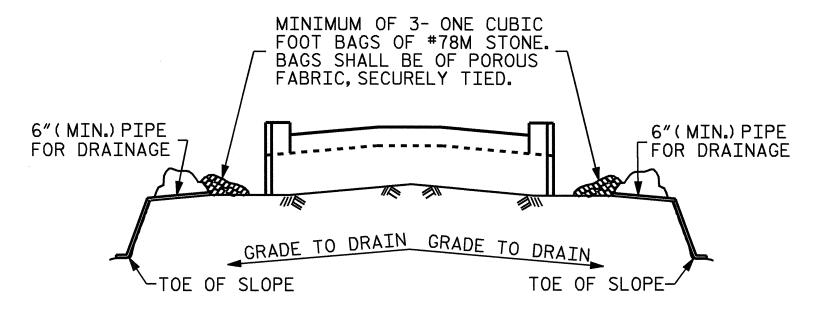
BILL OF MATERIAL

END BENT #I

TOTAL Cu. Yds. 20.2

HP 12×53 STEEL PILES

Lin. Ft. 220 No. 11

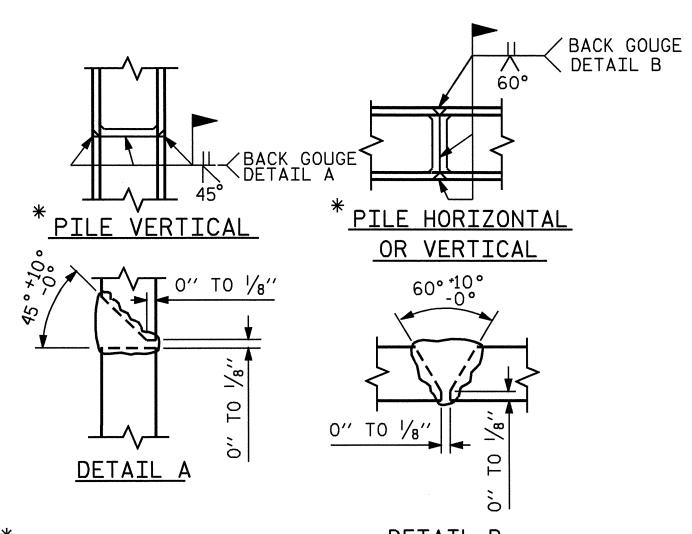


BAGGED STONE AND PIPE SHALL BE PLACED IMMEDIATELY AFTER COMPLETION OF END BENT EXCAVATION. PIPE MAY BE EITHER CONCRETE, CORRUGATED STEEL, CORRUGATED ALUMINUM ALLOY, OR CORRUGATED PLASTIC. PERFORATED PIPE WILL NOT BE ALLOWED.

BAGGED STONE SHALL REMAIN IN PLACE UNTIL THE ENGINEER DIRECTS THAT IT BE REMOVED. THE CONTRACTOR SHALL REMOVE AND DISPOSE OF SILT ACCUMULATIONS AT BAGGED STONE WHEN SO DIRECTED BY THE ENGINEER. BAGS SHALL BE REMOVED AND REPLACED WHENEVER THE ENGINEER DETER-MINES THAT THEY HAVE DETERIORATED AND LOST THEIR EFFECTIVENESS.

NO SEPARATE PAYMENT WILL BE MADE FOR THIS WORK AND THE ENTIRE COST OF THIS WORK SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR THE SEVERAL PAY ITEMS.

TEMPORARY DRAINAGE AT END BENT



POSITION OF PILE DURING WELDING. DETAIL B

PILE SPLICE DETAILS

PROJECT N	10. <u>B-4</u>	1276
STA	ANLY	_ COUNTY
STATION:_	19+96.00) -L-

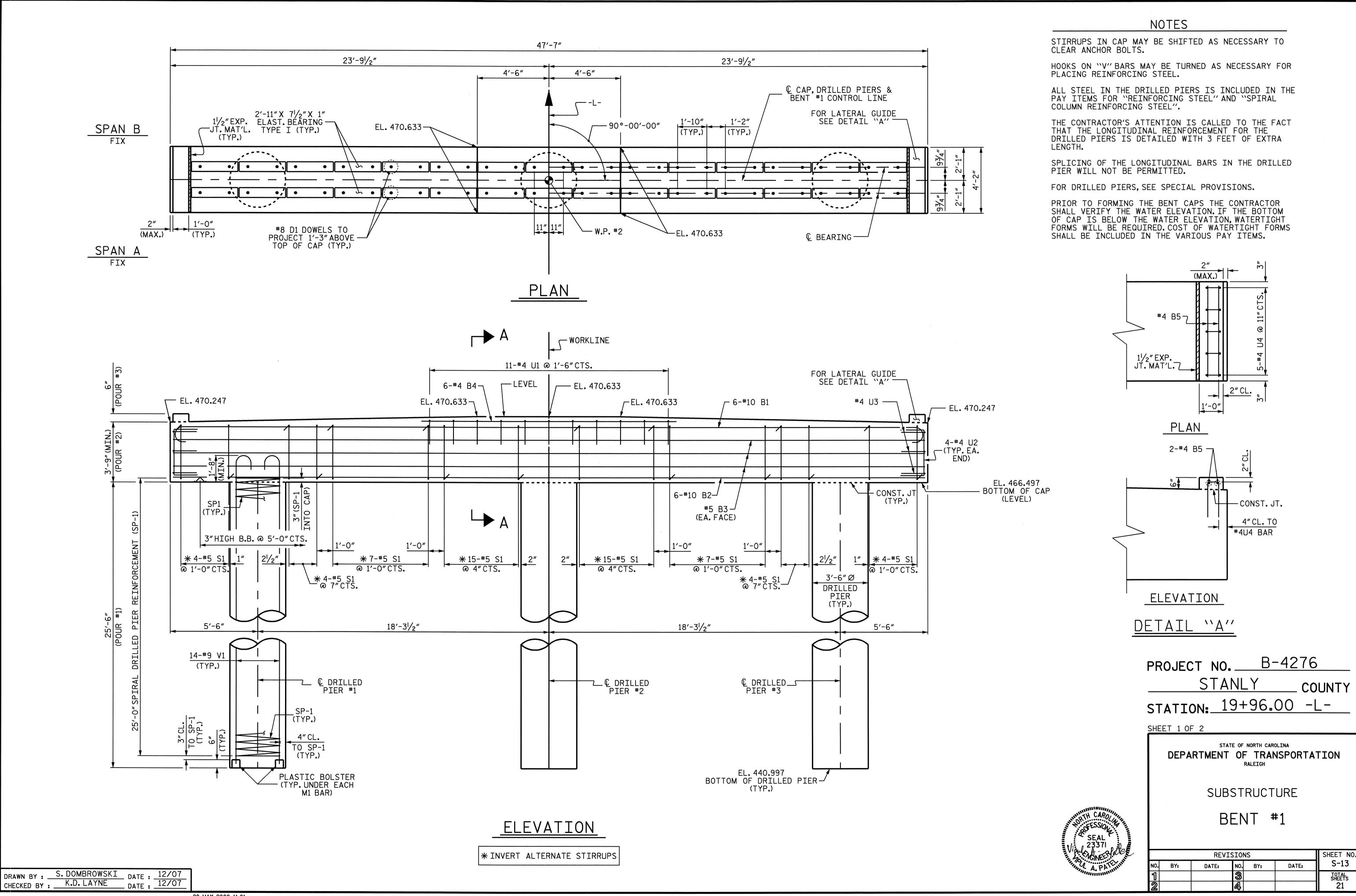
SHEET 2 OF 2

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

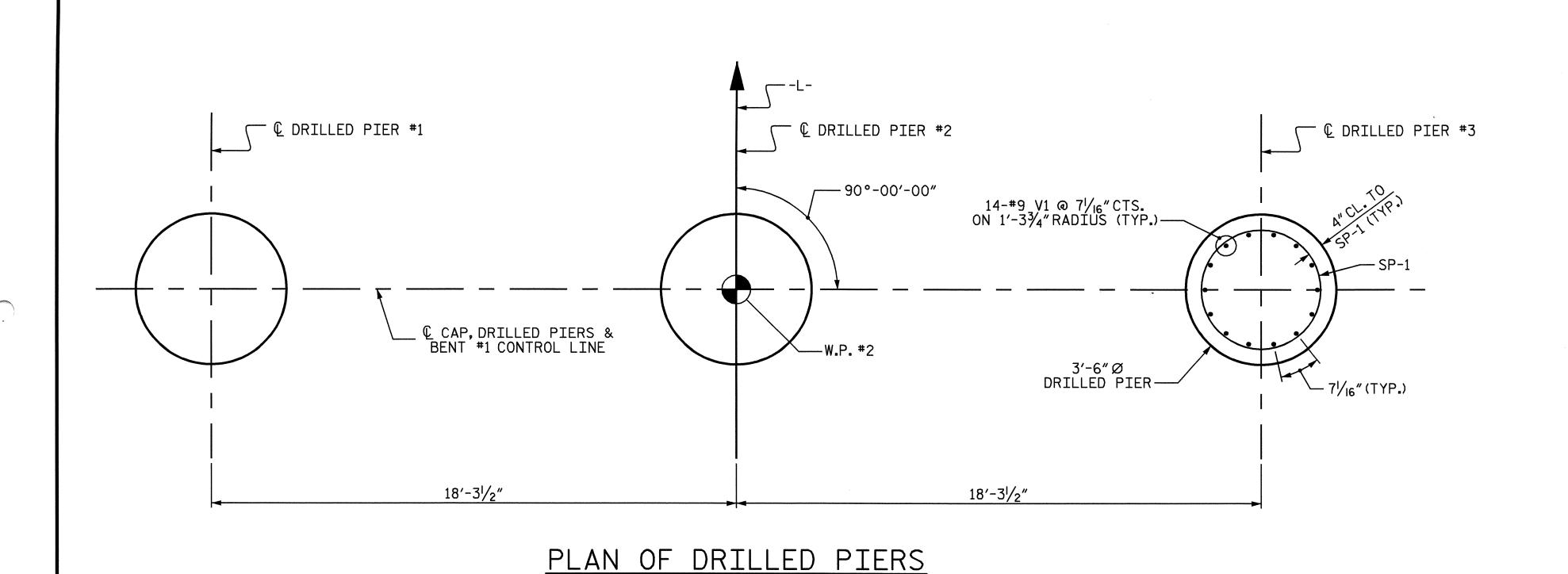
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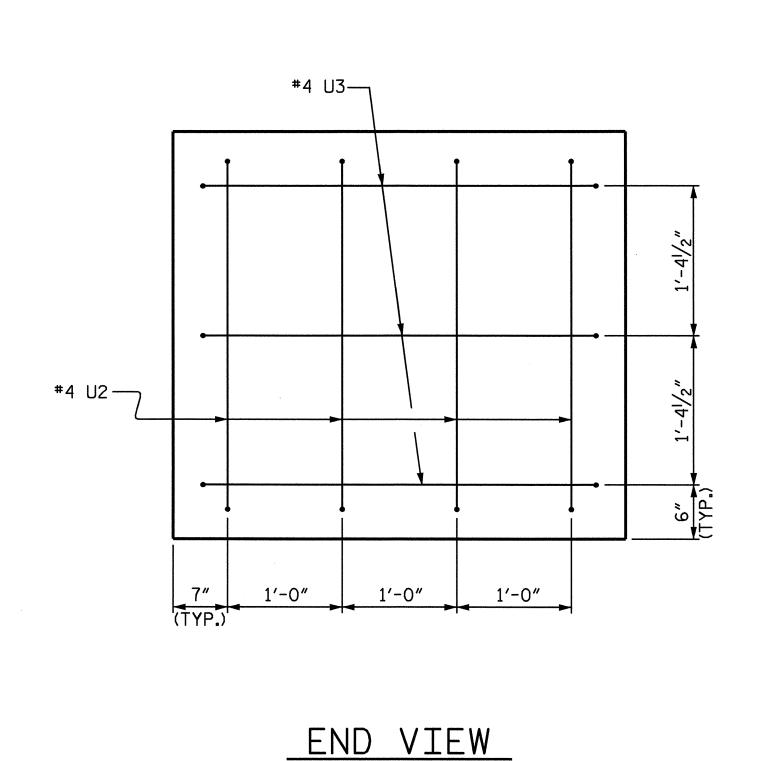
SHEET NO **REVISIONS** S-12 NO. BY: DATE: TOTAL SHEETS 21

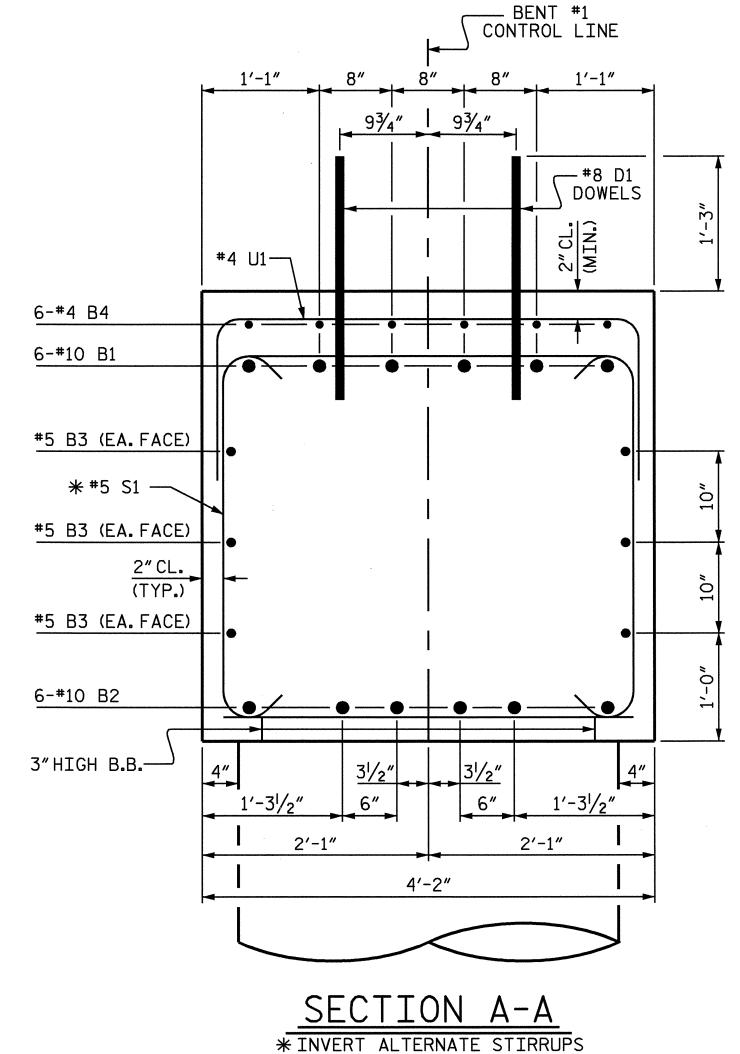
DRAWN BY: S. DOMBROWSKI DATE: 10/07 CHECKED BY: R.G. EMERSON DATE: 12/07

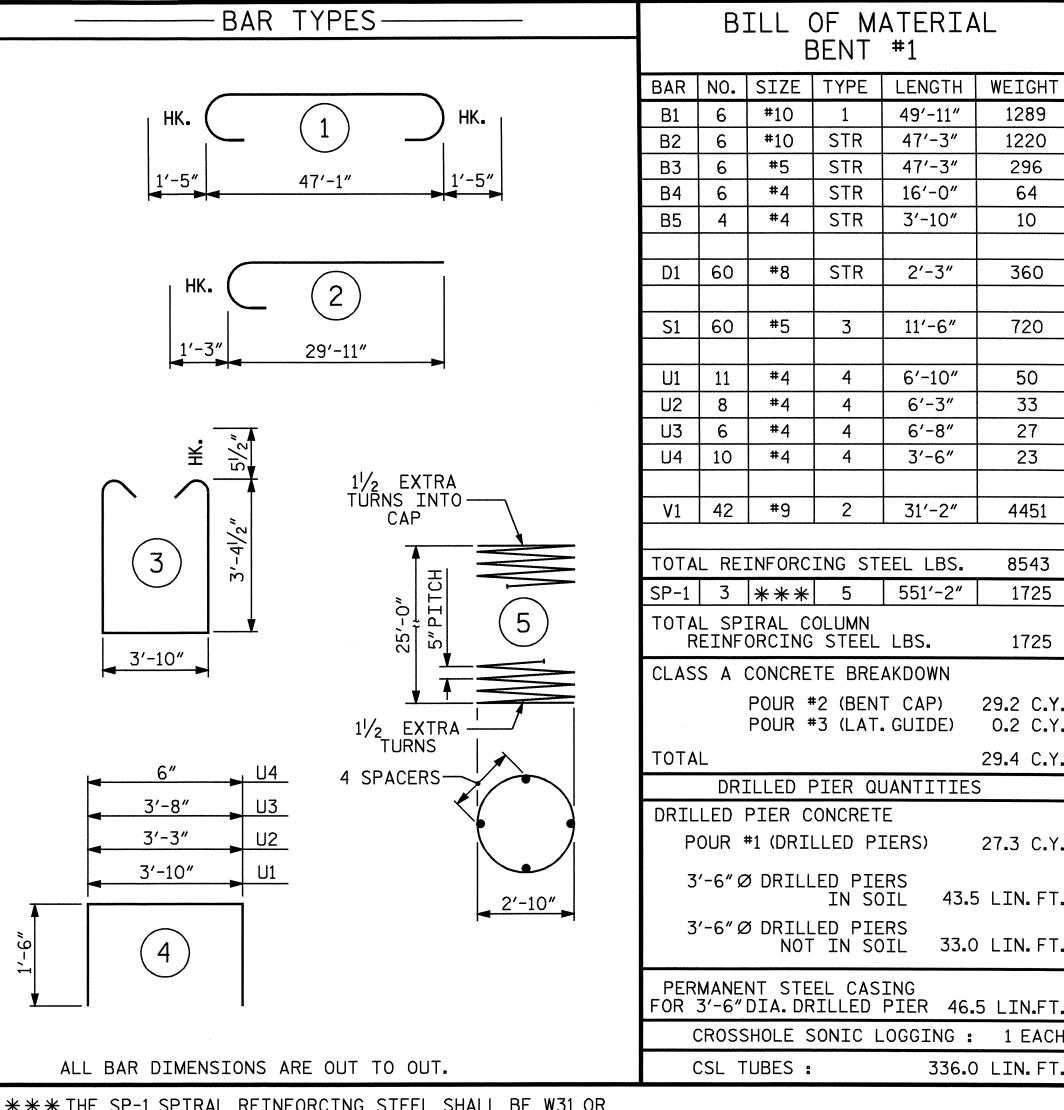


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*** THE SP-1 SPIRAL REINFORCING STEEL SHALL BE W31 OR D-31 COLD DRAWN WIRE OR #5 PLAIN OR DEFORMED BAR.

PROJECT NO. B-4276 STANLY COUNTY STATION: 19+96.00 -L-

1289

1220

296

64

10

360

720

50

33

27

23

4451

8543

1725

1725

29.2 C.Y.

0.2 C.Y.

29.4 C.Y.

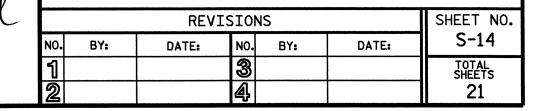
27.3 C.Y.

SHEET 2 OF 2

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH

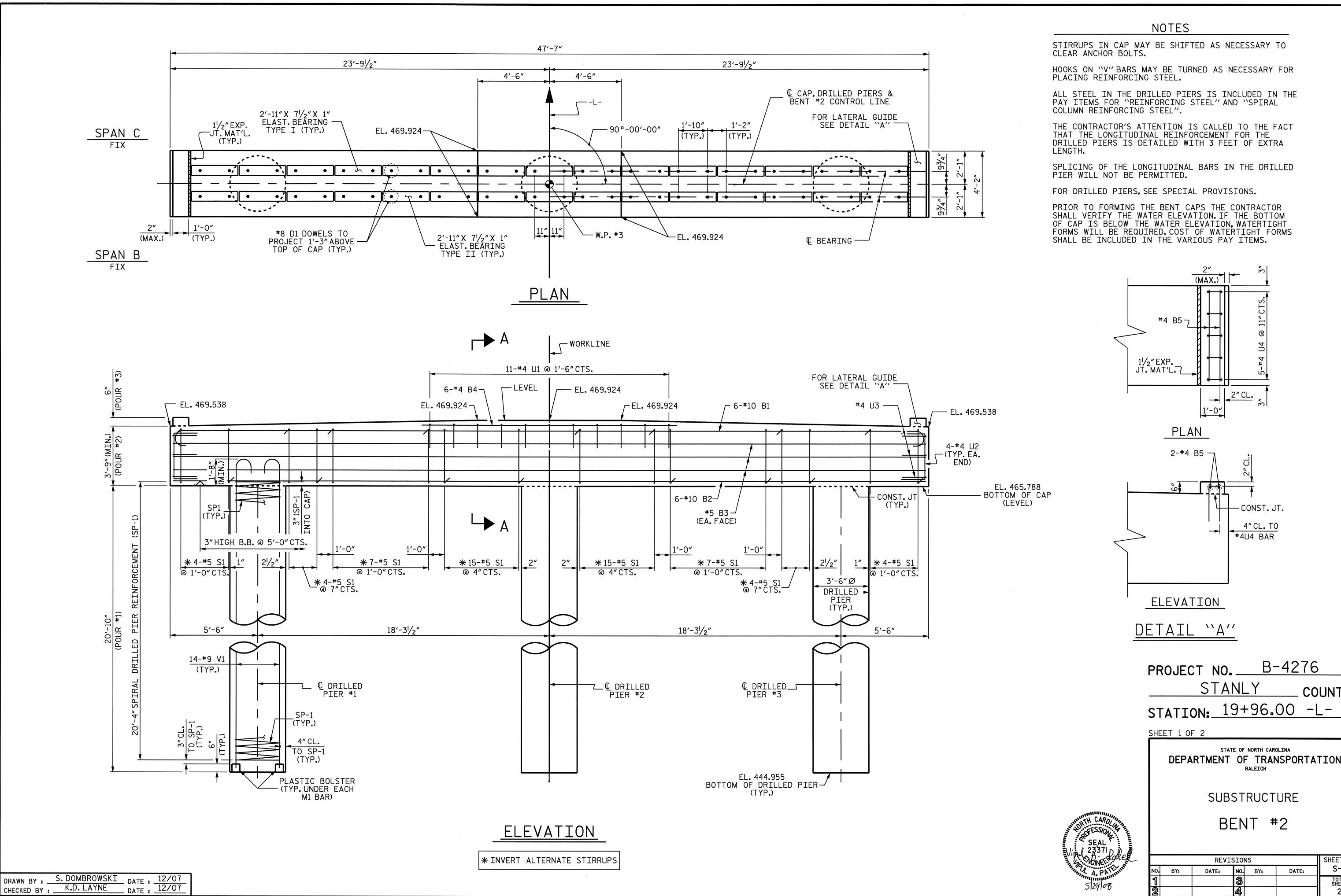
SUBSTRUCTURE

BENT #1



DRAWN BY: S. DOMBROWSKI DATE: 12/07 CHECKED BY: K.D. LAYNE DATE: 12/07

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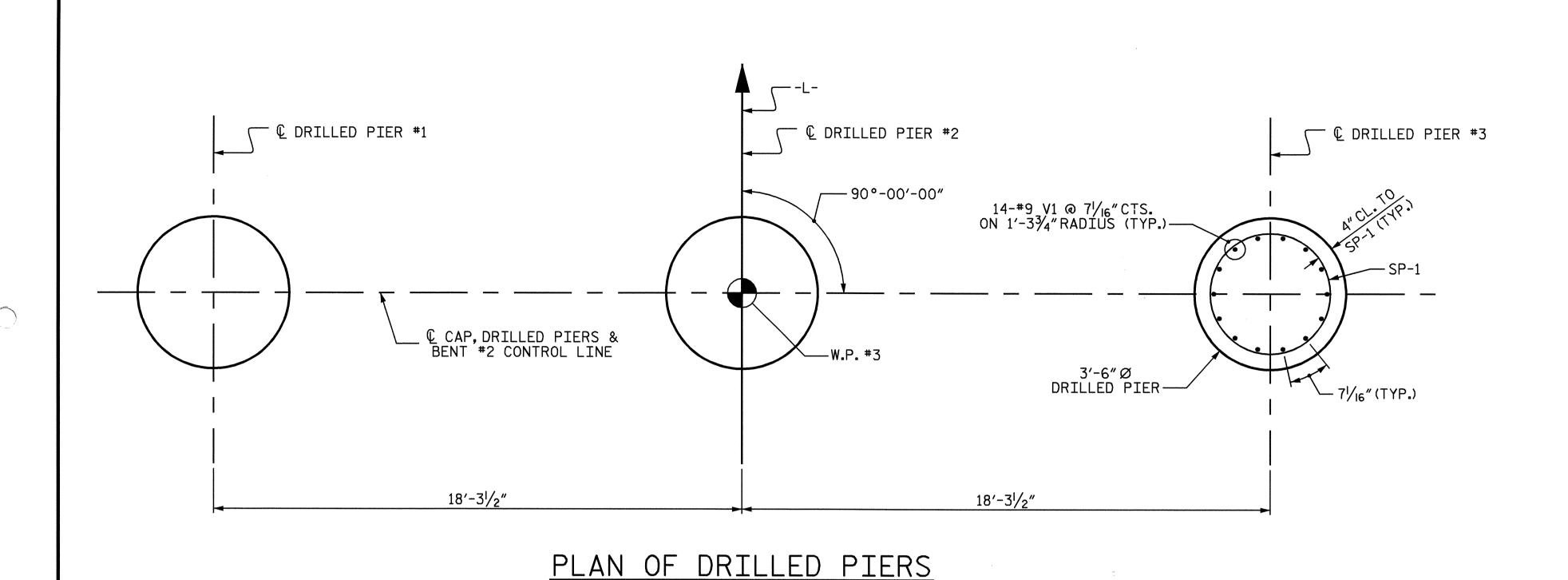
COUNTY

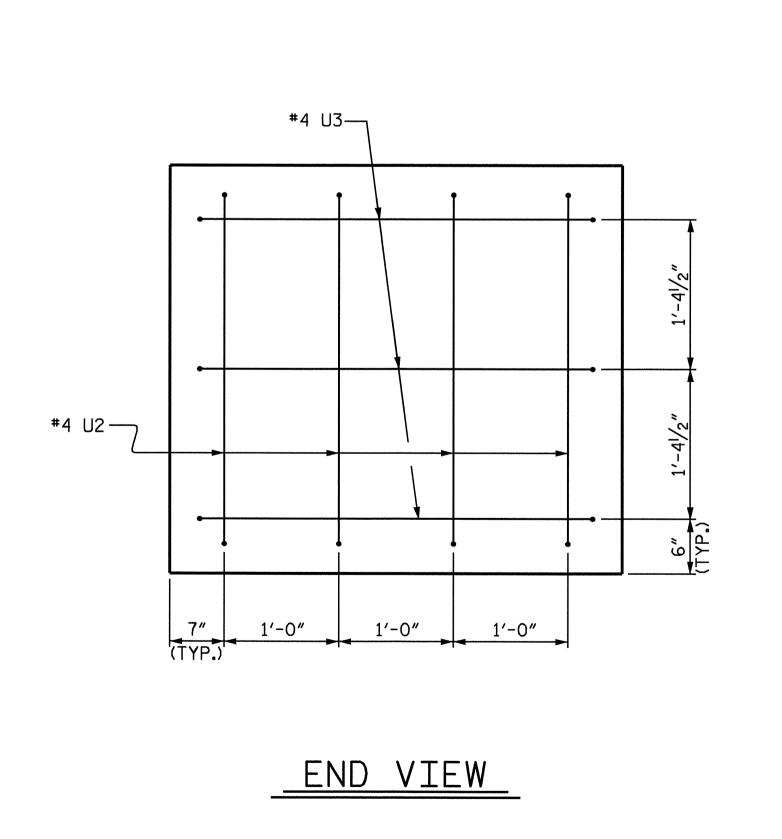
SHEET NO. S-15

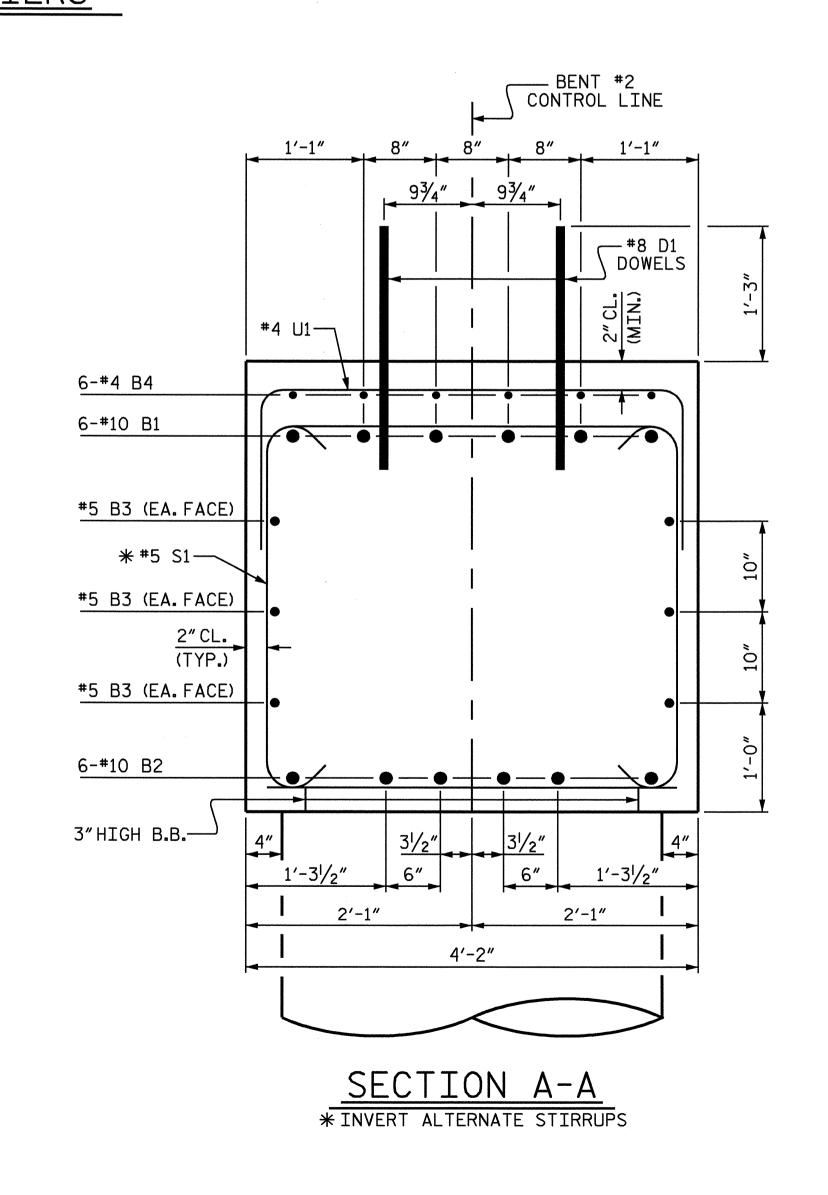
TOTAL SHEETS 21

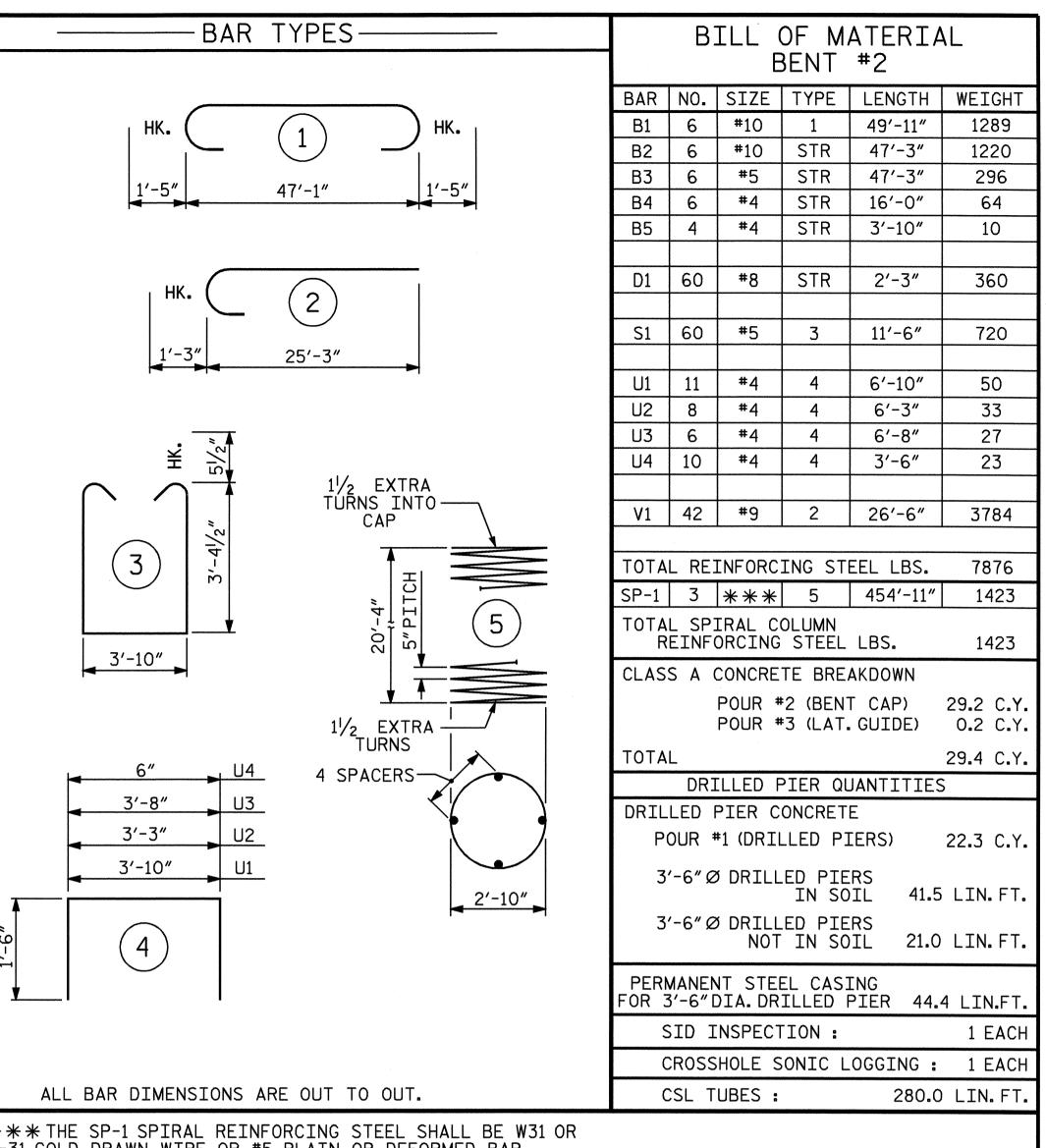
DATE:

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*** THE SP-1 SPIRAL REINFORCING STEEL SHALL BE W31 OR

PROJECT NO. B-4276 STANLY COUNTY STATION: 19+96.00 -L-

SHEET 2 OF 2

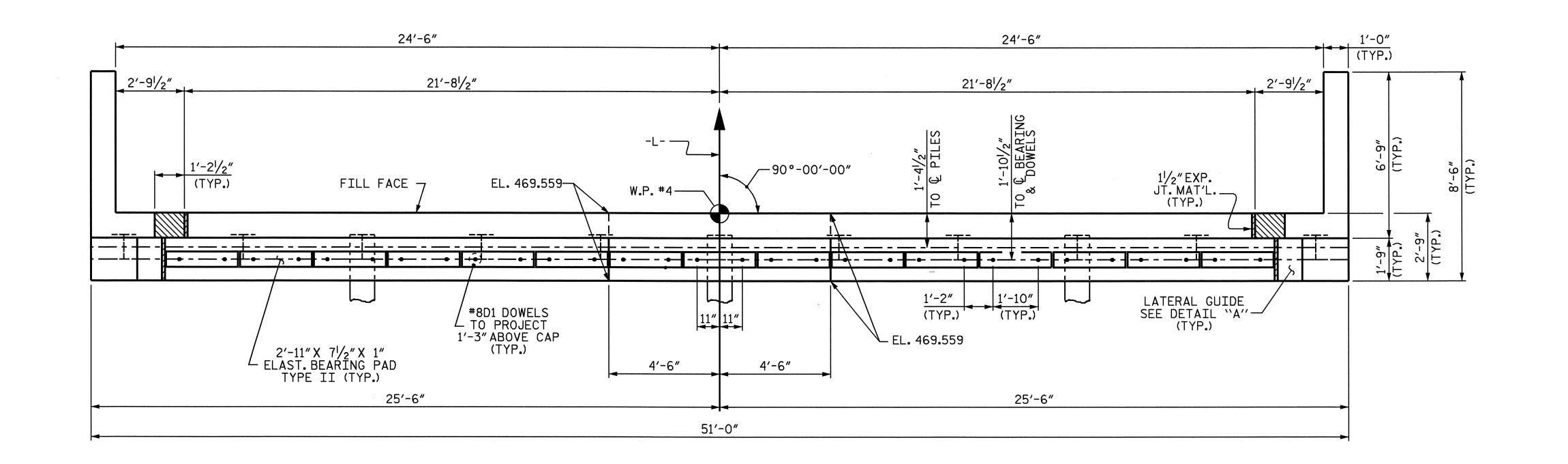
STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

SUBSTRUCTURE

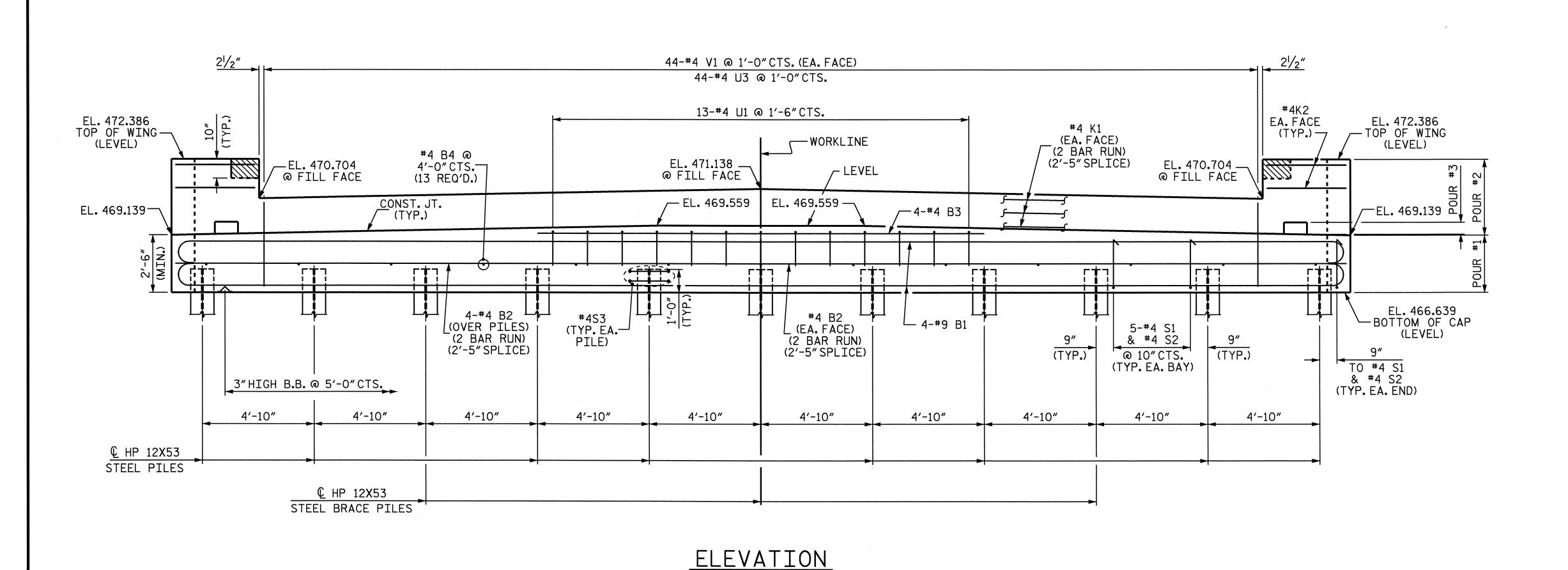
BENT #2

SHEET NO. **REVISIONS** S-16 NO. BY: DATE: TOTAL SHEETS 21

DRAWN BY: S. DOMBROWSKI DATE: 12/07
CHECKED BY: K.D. LAYNE DATE: 12/07



PLAN



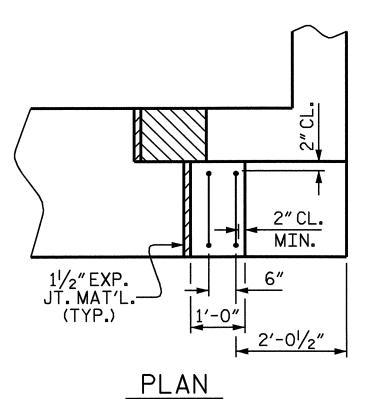
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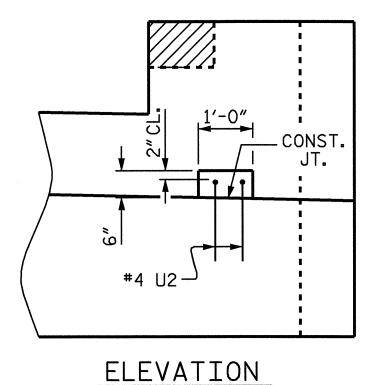
STIRRUPS IN CAP MAY BE SHIFTED AS NECESSARY TO CLEAR #8D1 DOWELS.

THE CONCRETE IN THE SHADED AREA OF THE WING SHALL BE POURED AFTER THE CONCRETE BARRIER RAIL IS CAST IF SLIP FORMING IS USED.

THE CONTRACTOR SHALL PROVIDE FOR INSTALLATION OF THE 4"DIAMETER DRAIN PIPE THROUGH THE WING WALL AS REQUIRED FOR REINFORCED BRIDGE APPROACH FILLS, SEE THE ROADWAY PLANS. REINFORCING STEEL IN THE WING WALL MAY BE SHIFTED AS NECESSARY TO CLEAR THE DRAIN PIPE.

THE LATERAL GUIDE AT EACH END OF THE CAP IS NOT TO BE POURED UNTIL AFTER CORED SLAB UNITS ARE IN PLACE.





DETAIL "A"

PROJECT NO. B-4276 STANLY COUNTY 19+96.00 -L-STATION:_

SHEET 1 OF 2

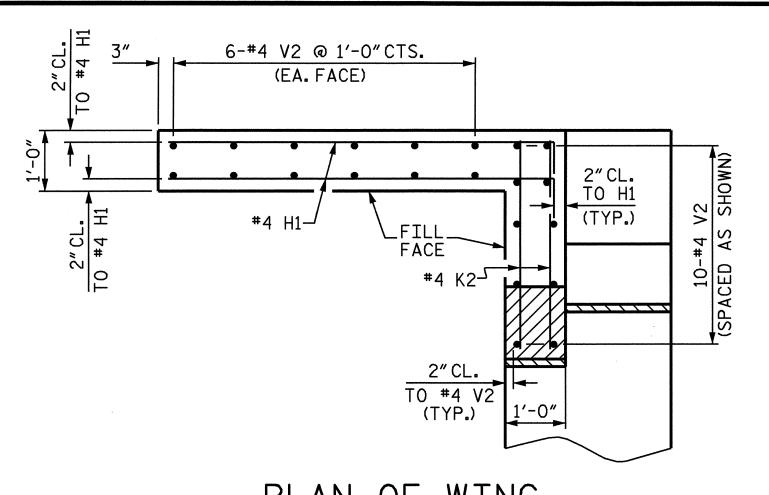
STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

SUBSTRUCTURE

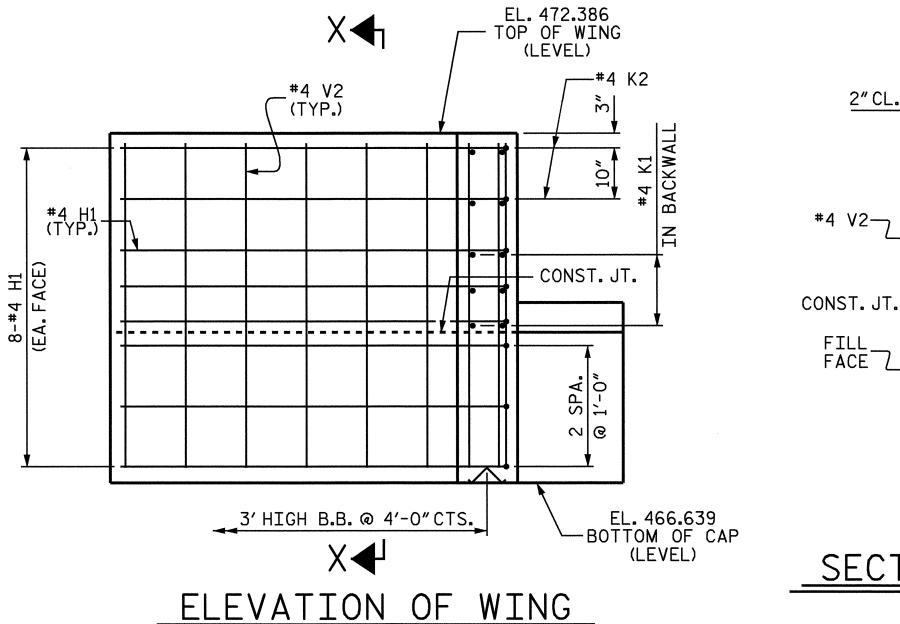


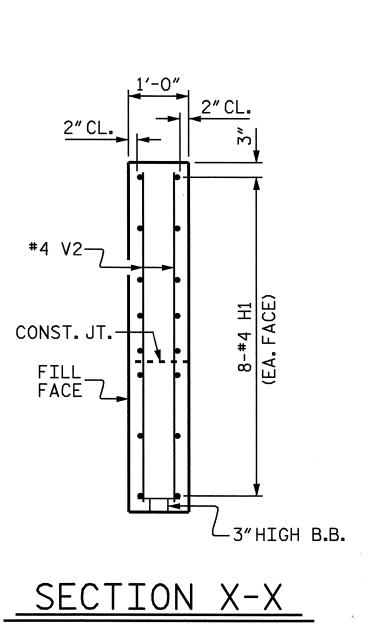
i.	END	BENT	#2
ALLINIUL S			

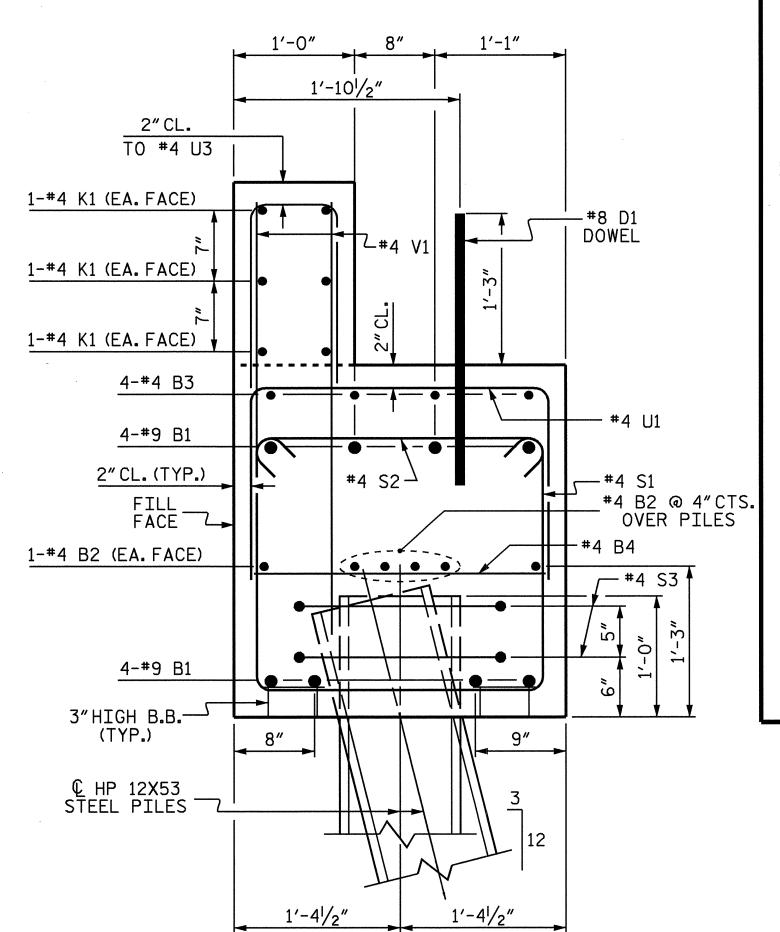
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BY:	DATE:	NO.	BY:	DATE:	S-17
		3			TOTAL SHEETS
		4			21





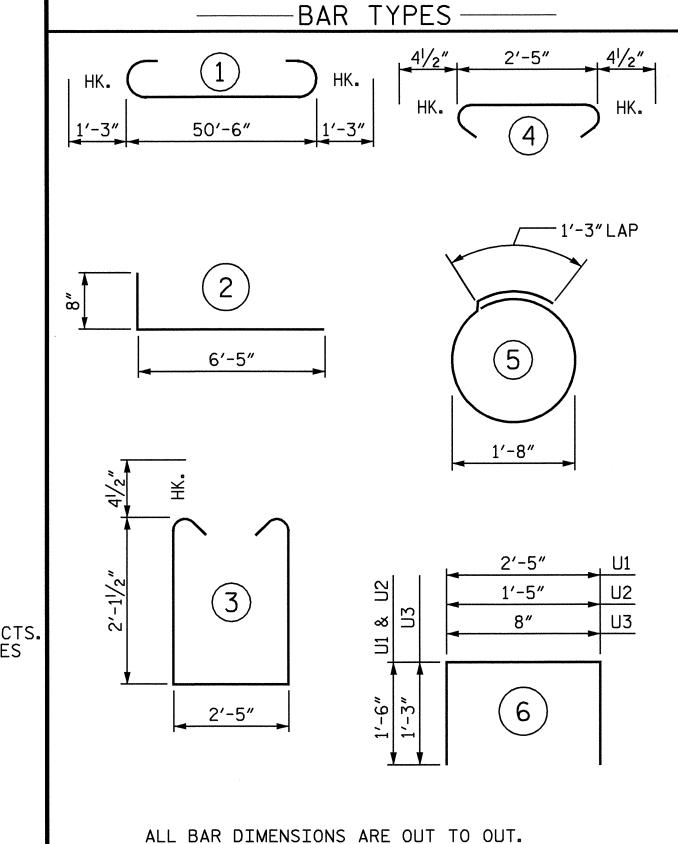






2'-9"

CAP SECTION



BACK GOUGE DETAIL B * PILE HORIZONTAL PILE VERTICAL

[Ο" ΤΟ ⅓" DETAIL A

OR VERTICAL 0" TO 1/8"

POSITION OF PILE DURING WELDING. DETAIL B

PILE SPLICE DETAILS

B-4276 PROJECT NO. COUNTY

STANLY

19+96.00 -L-STATION:_

BILL OF MATERIAL

END BENT #2

53′-0″ 26'-7"

19'-4"

7'-1"

213

258

216

157

3279

Cu. Yds. 4.8

Cu. Yds. 0.1

Cu. Yds. 20.2

Lin. Ft. 165

26′-7″

3'-5"

7′-5″

3'-2"

6'-6"

5′-5″

4'-5" 3'-2"

3′-8″

BAR NO. SIZE TYPE LENGTH WEIGHT

13 | #4 | STR. | 2'-5"

30 8 STR. 2'-3"

#4 STR.

4 | #4 | STR. |

32 #4 2

52 #4 3

52 #4 4

22 #4 5

13 #4 6

44 #4 6

88 #4 STR.

REINFORCING STEEL Lbs.

CLASS "A" CONCRETE

POUR #2 UPPER WINGS

HP 12×53 STEEL PILES

No. 11

POUR #3 LATERAL GUIDES

44 #4 STR. 5'-4"

POUR #1 CAP & LOWER WINGSCu. Yds. 15.3

S3

V1 |

V2

TOTAL

12 #4 STR.

8 #4 STR.

12

SHEET 2 OF 2

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

> SUBSTRUCTURE END BENT #2

SHEET NO. **REVISIONS** S-18 NO. BY: DATE: TOTAL SHEETS 21

MINIMUM OF 3- ONE CUBIC FOOT BAGS OF #78M STONE. BAGS SHALL BE OF POROUS FABRIC, SECURELY TIED. 6"(MIN.)PIPE FOR DRAINAGE 6"(MIN.)PIPE FOR DRAINAGE GRADE TO DRAIN GRADE TO DRAIN TOE OF SLOPE--TOE OF SLOPE

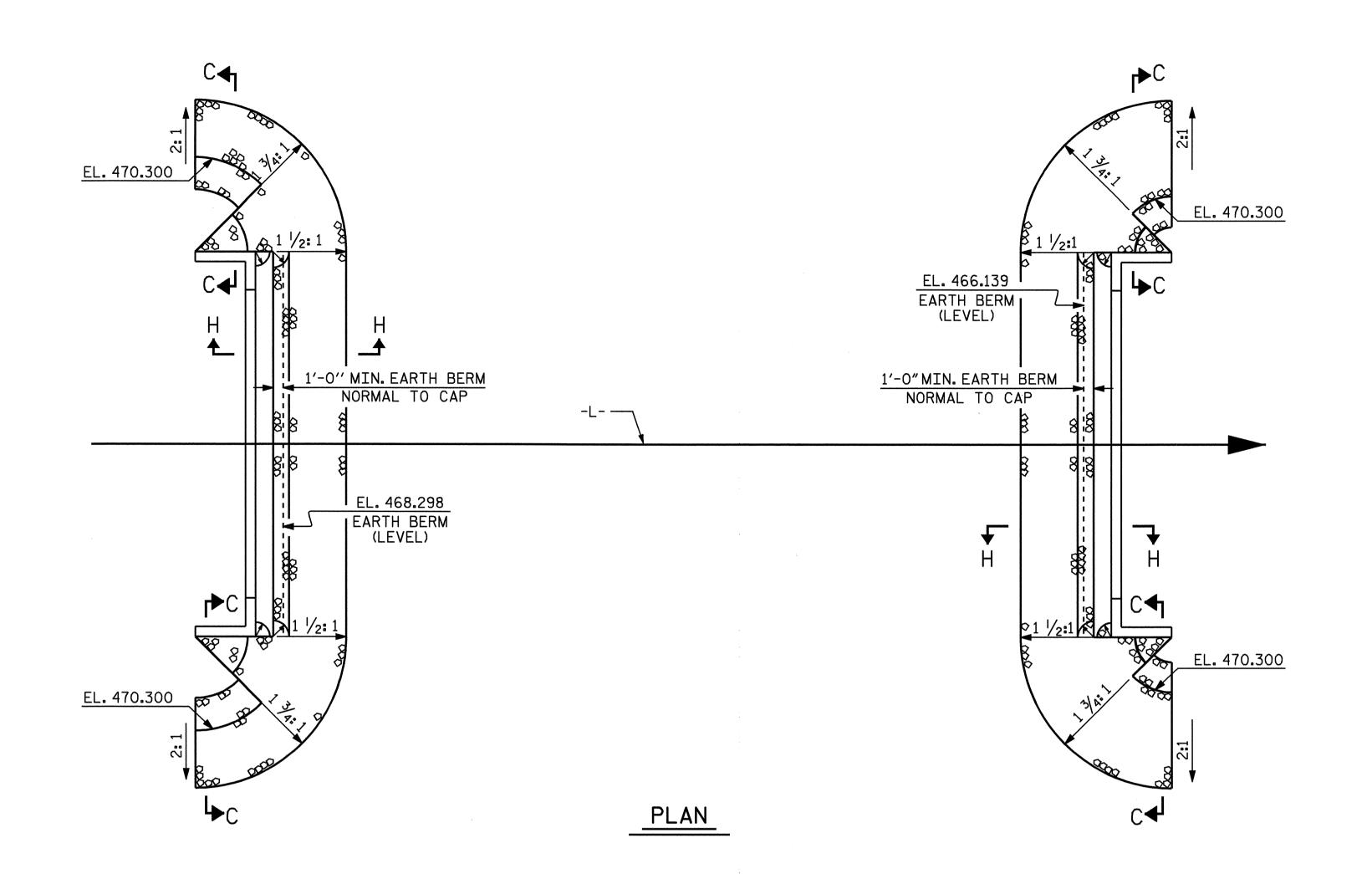
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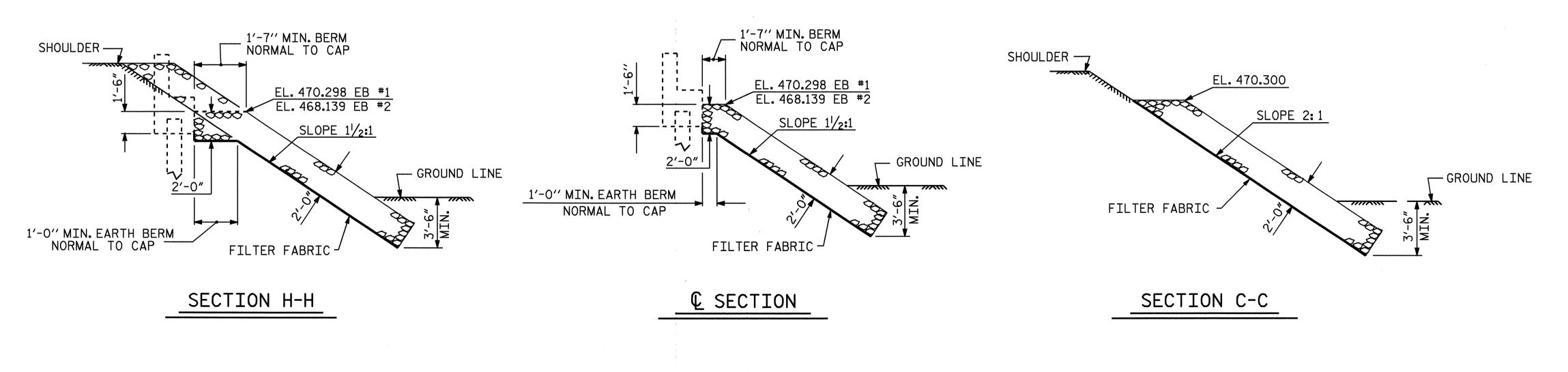
NO SEPARATE PAYMENT WILL BE MADE FOR THIS WORK AND THE ENTIRE COST OF THIS WORK SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR THE SEVERAL PAY ITEMS.

TEMPORARY DRAINAGE AT END BENT

DRAWN BY : S. DOMBROWSKI DATE : 10/07 CHECKED BY R.G. EMERSON DATE: 12/07



ESTIMATED QUANTITIES						
	RIP RAP CLASS II (2'-0"THICK)	FILTER FABRIC FOR DRAINAGE				
	TONS	SQUARE YARDS				
END BENT #1	160	180				
END BENT #2	110	120				



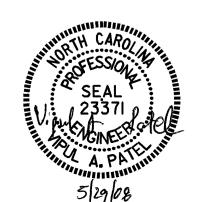
PROJECT NO. B-4276 STANLY COUNTY

STATION: 19+96.00 -L-

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

STANDARD

-RIP RAP DETAILS-

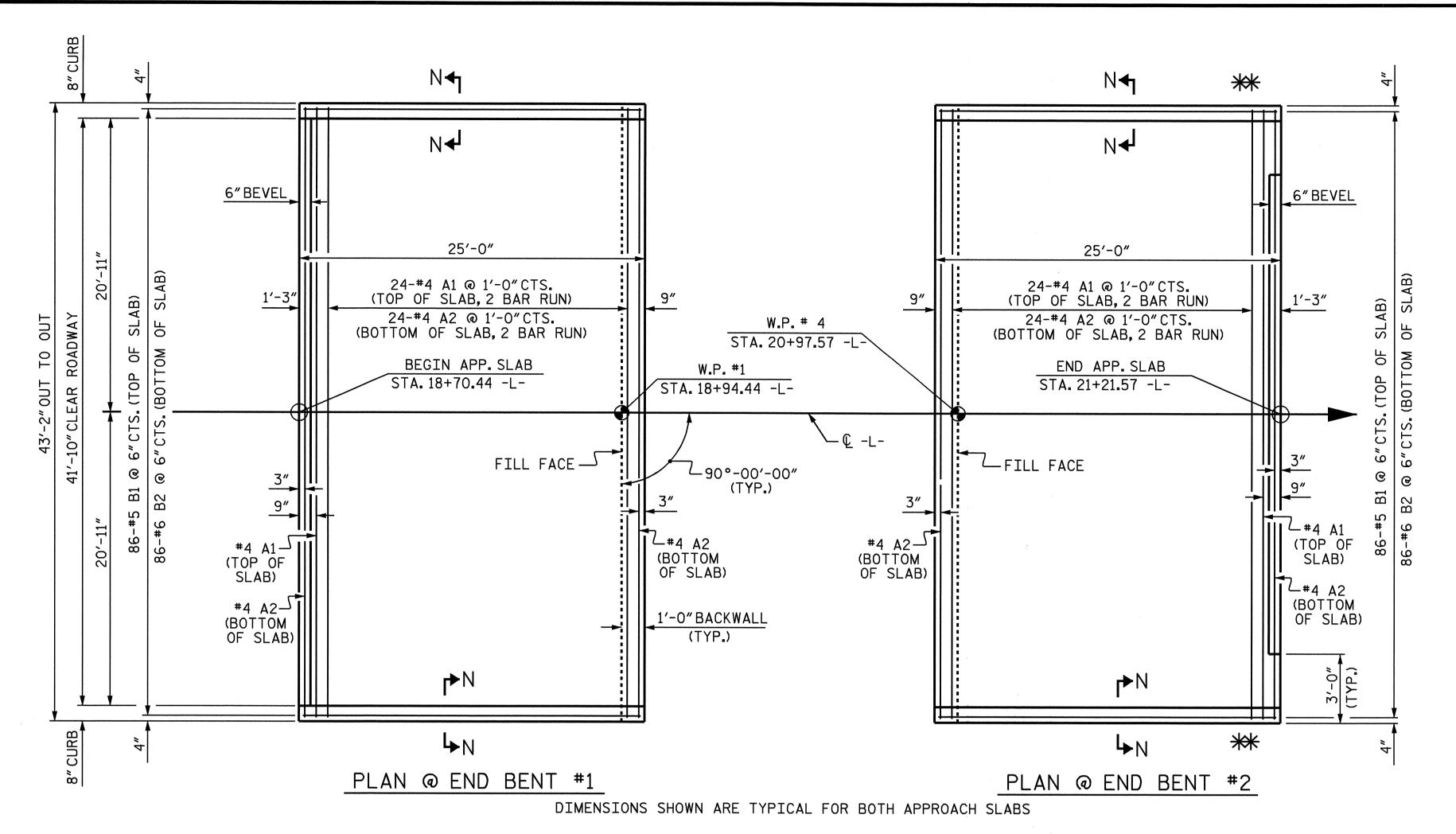


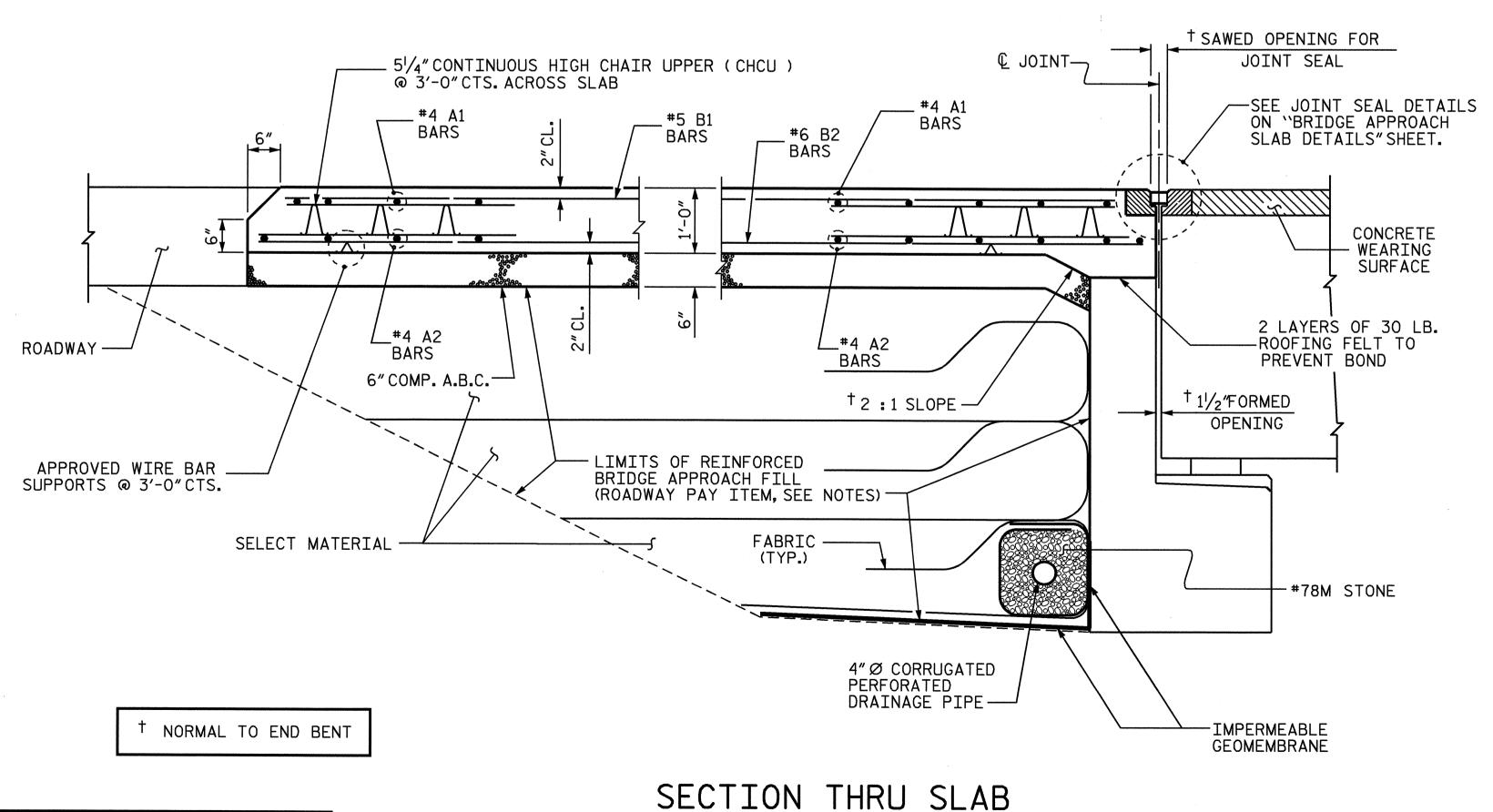
		`				
		SHEET NO.				
•	BY:	DATE:	NO.	BY:	DATE:	S-19
			3			TOTAL SHEETS
			4			21

ASSEMBLED BY : S. DOMBROWSKI CHECKED BY : K.D. LAYNE DATE : I/08 DATE : I/08 REV. 8/16/99 REV. 10/17/00 REV. 5/1/06 RWW/LES RWW/LES TLA/GM DRAWN BY: FCJ 2/88 CHECKED BY: ARB 8/88

SKEW 90°

STD. NO. RR2





SHOWING SECTION WITHOUT CONCRETE WEARING SURFACE

ASSEMBLED BY : T.R. PETERSON

CHECKED BY : K.D. LAYNE

DRAWN BY: EEM 3/95 CHECKED BY: VAP 3/95 DATE: 7/3/07

DATE: 7/13/07

REV. 7/IO/OI LES/RDR REV. 5/7/O3R RWW/JTE REV. 5/I/O6 TLA/GM

NOTES

APPROACH SLAB SHALL NOT BE CONSTRUCTED PRIOR TO COMPLETION OF THE BRIDGE DECK.

FOR REINFORCED BRIDGE APPROACH FILL INCLUDING FABRIC, IMPERMEABLE GEOMEMBRANE, 4" Ø DRAINAGE PIPE, #78M STONE, AND SELECT MATERIAL, SEE ROADWAY PLANS.

AREA BETWEEN THE WINGWALL AND APPROACH SLAB SHALL BE GRADED TO DRAIN THE WATER AWAY FROM THE FILL FACE OF THE BRIDGE AND SHALL BE PAVED. SEE ROADWAY PLANS.

THE 6"COMP. A.B.C. SHALL BE FLUSH WITH THE ROADWAY END OF THE APPROACH SLAB AND SHALL EXTEND 1'-0"OUTSIDE EACH EDGE OF THE APPROACH SLAB.

THE CONTRACTOR MAY USE 4"TYPE B-25.0B ASPHALT CONCRETE BASE COURSE IN LIEU OF 6"COMP. A.B.C. IF THIS OPTION IS USED, THE BASE COURSE SHALL BE FLUSH WITH THE ROADWAY END OF THE APPROACH SLAB, AND THE WIDTH SHALL BE THE SAME AS THAT OF THE APPROACH SLAB.

THE CONTRACTOR MAY USE 5"CLASS "A" CONCRETE BASE IN LIEU OF 6" COMP. A.B.C. IF THIS OPTION IS USED, THE CONCRETE BASE SHALL BE FLUSH WITH THE ROADWAY END OF THE APPROACH SLAB, AND THE WIDTH SHALL BE THE SAME AS THAT OF THE APPROACH SLAB. THE CONCRETE SHALL BE FINISHED TO A SMOOTH SURFACE AND A LAYER OF 30 LB ROOFING FELT SHALL BE PLACED BETWEEN THE CONCRETE BASE AND THE APPROACH SLAB TO PREVENT BOND. THE APPROACH SLAB SHALL NOT BE CAST UNTIL THE CONCRETE BASE HAS REACHED AN AGE OF THREE CURING DAYS.

WITH EVAZOTE JOINT SEAL

FOR EVAZOTE JOINT SEALS, SEE SPECIAL PROVISIONS.

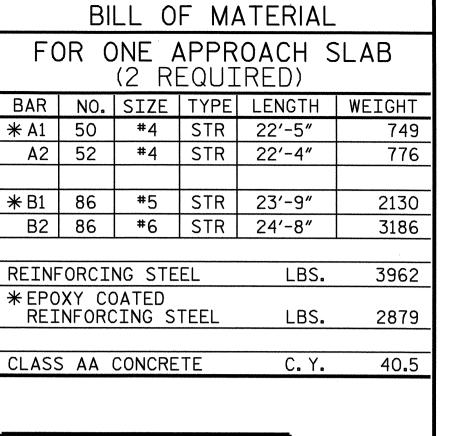
THE NOMINAL UNCOMPRESSED SEAL WIDTH OF THE EVAZOTE JOINT SEAL SHALL BE $2\frac{1}{2}$.

FOR ELASTOMERIC CONCRETE, SEE SPECIAL PROVISIONS.

WITH CONCRETE WEARING SURFACE

APPROACH SLABS SHALL BE POURED AFTER CONCRETE OVERLAY IS POURED.

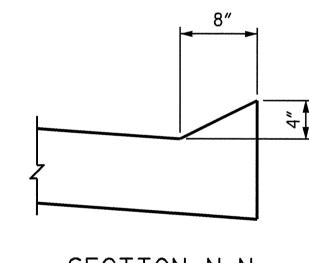
THE JOINT SHALL BE SAWED AFTER THE CASTING OF THE CONCRETE BARRIER



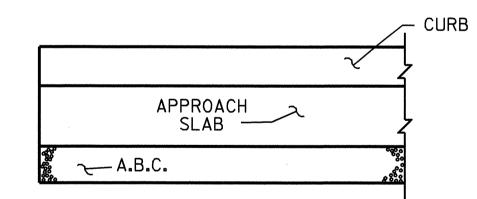
 SPLICE CHART

 * #4 A1
 2'-0"

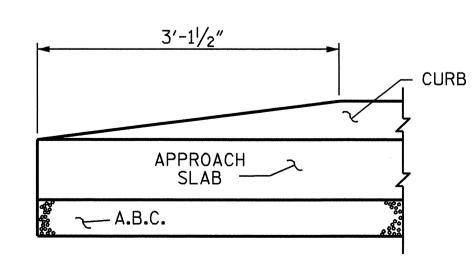
 #4 A2
 1'-9"



SECTION N-N



** END OF CURB WITH SHOULDER BERM GUTTER



END OF CURB WITHOUT SHOULDER BERM GUTTER

CURB DETAILS

PROJECT NO. B-4276

STANLY COUNTY

STATION: 19+96.00 -L

SHEET 1 OF 2

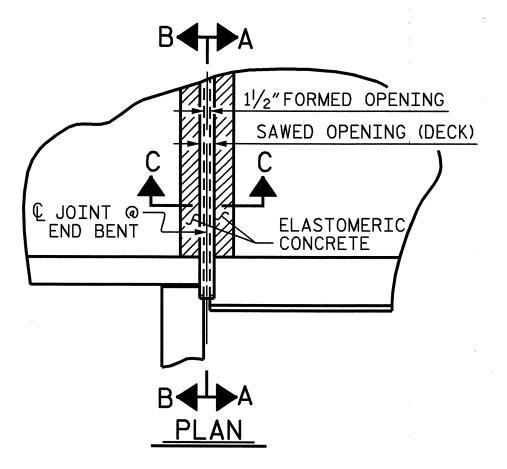
DEPARTMENT OF TRANSPORTATION
RALEIGH
STANDARD

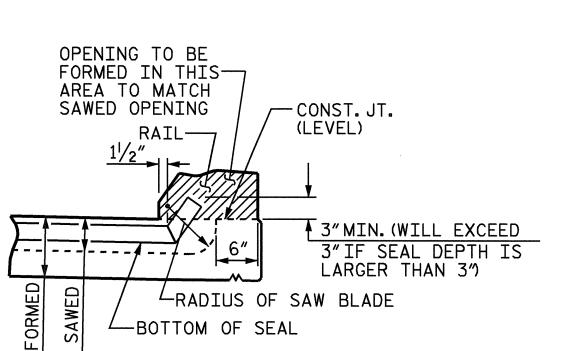
STANDARD

BRIDGE APPROACH SLAB FOR FLEXIBLE PAVEMENT

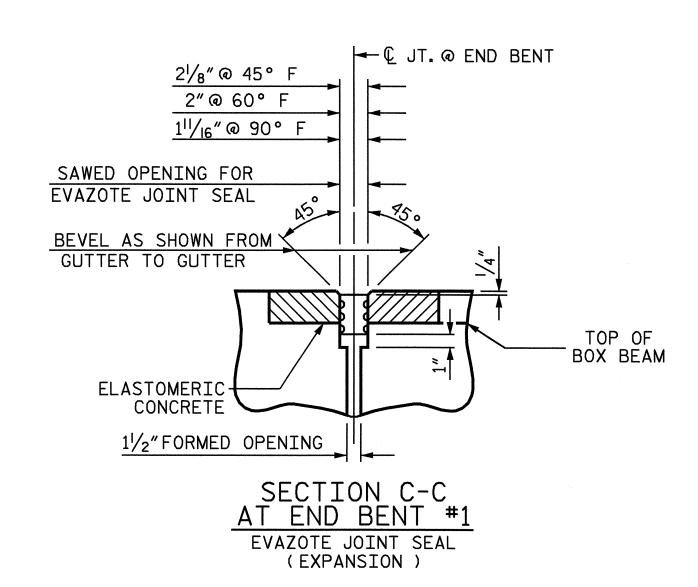
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NO.	BY:	DATE:	NO.	BY:	DATE:	S-20
1			3			TOTAL SHEETS
2			4			21

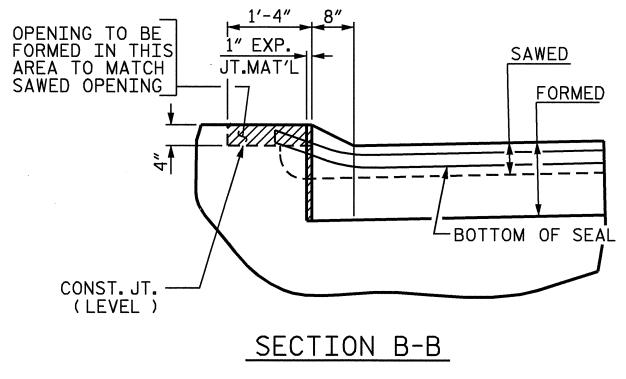
28-MAY-2008 08:49 I:\Structures\B4276\plans\B4276_sd_AS.dgn

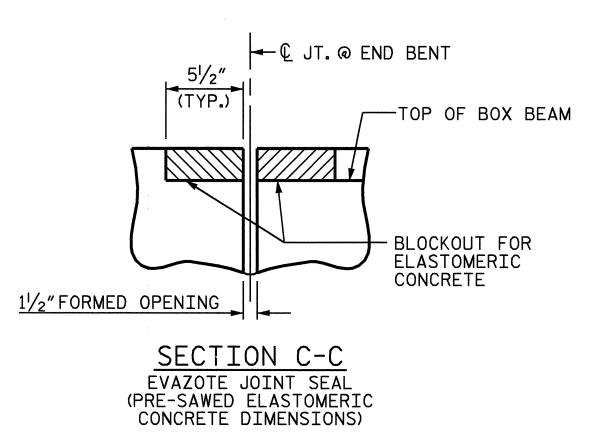




SECTION A-A

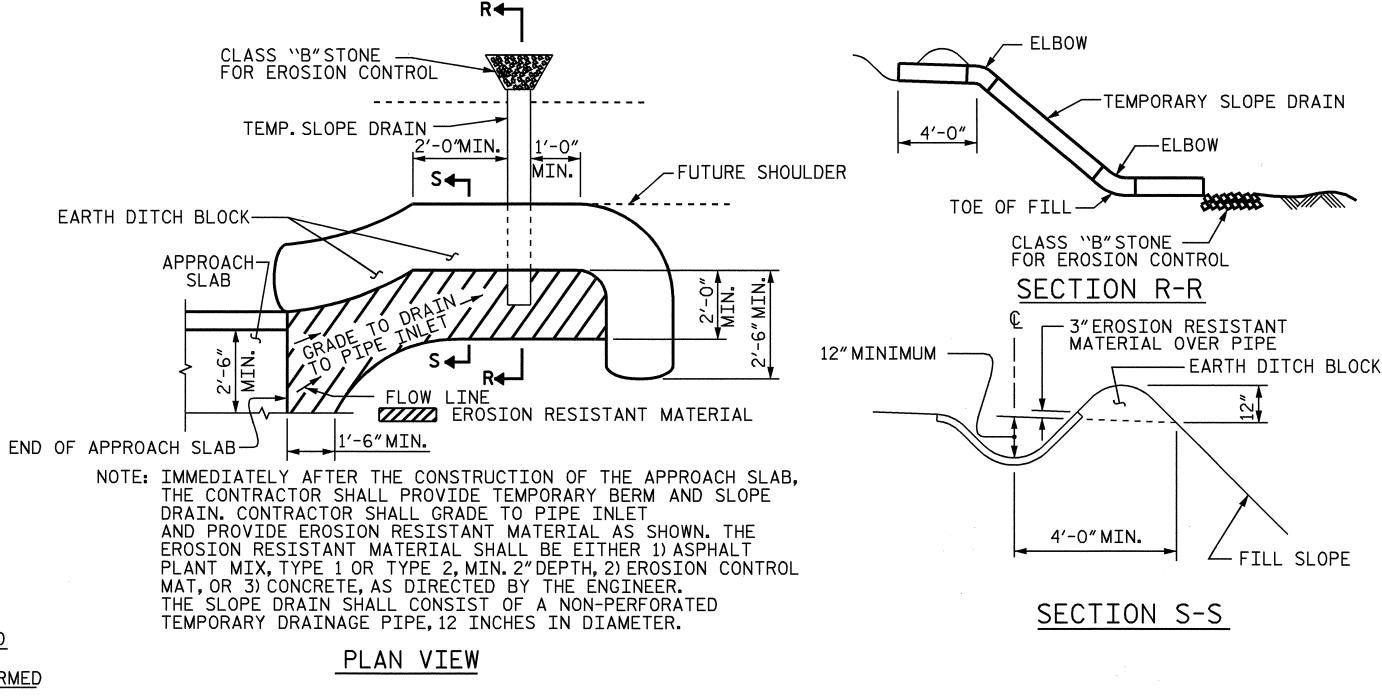






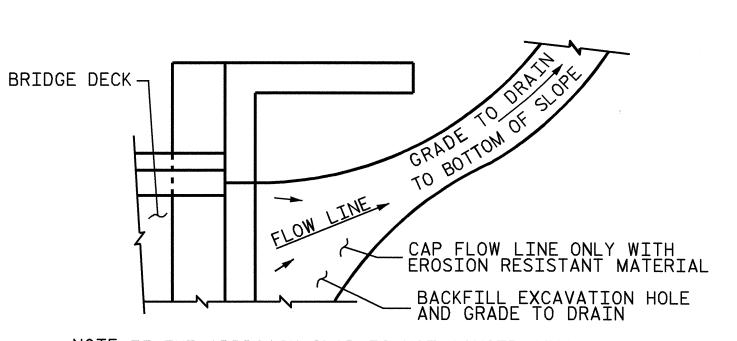
JOINT SEAL DETAILS @ END BENT

EVAZOTE JOINT SEAL TO BE CUT, HEAT WELDED AND TURNED UP PARALLEL TO SLOPED FACE OF THE BARRIER RAIL.



TEMPORARY BERM AND SLOPE DRAIN DETAILS

(TO BE USED WHEN SHOULDER BERM GUTTER IS REQUIRED)



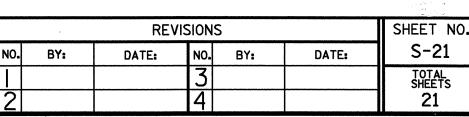
NOTE: IF THE APPROACH SLAB IS NOT CONSTRUCTED IMMEDIATELY AFTER THE BACKFILLING OF THE END BENT EXCAVATION, GRADE TO DRAIN TO THE BOTTOM OF THE SLOPE AND PROVIDE EROSION RESISTANT MATERIAL, SUCH AS FIBERGLASS ROVING OR AS DIRECTED BY THE ENGINEER TO PREVENT SOIL EROSION AND TO PROTECT THE AREA ADJACENT TO THE STRUCTURE. THE CONTRACTOR WILL BE REQUIRED TO REMOVE THESE MATERIALS PRIOR TO CONSTRUCTION OF THE APPROACH SLAB. TEMPORARY DRAINAGE DETAIL

> PROJECT NO. B-4276 STANLY _ COUNTY 19+96.00-L-

SHEET 2 OF 2

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH STANDARD

> BRIDGE APPROACH SLAB DETAILS



ELASTOMERIC CONCRETE					
END BENT #	ELASTOMERIC CONCRETE (CU.FT.)				
1	19.2				
2	19.2				
TOTAL	38.4				

ASSEMBLED BY: T.R. PETERSON DATE: 7/3/07 CHECKED BY : K.D. LAYNE DATE: 7/13/07 DRAWN BY: FCJ 11/88 REV. 10/17/00 REV. 5/7/03 REV. 5/1/06 RWW/LES RWW/JTE TLA/GM

STANDARD NOTES

DESIGN DATA:

SPECIFICATIONS ---- A.A.S.H.T.O. (CURRENT) LIVE LOAD IMPACT ALLOWANCE ---- SEE A.A.S.H.T.O. STRESS IN EXTREME FIBER OF STRUCTURAL STEEL - AASHTO M270 GRADE 36 - 20,000 LBS. PER SQ. IN. - AASHTO M270 GRADE 50W - 27,000 LBS. PER SQ. IN. - AASHTO M270 GRADE 50 - 27,000 LBS. PER SQ. IN. REINFORCING STEEL IN TENSION GRADE 60 - - 24,000 LBS. PER SQ. IN. CONCRETE IN COMPRESSION ---- 1,200 LBS. PER SQ. IN. CONCRETE IN SHEAR ---- SEE A.A.S.H.T.O. STRUCTURAL TIMBER - TREATED OR

---- 1,800 LBS. PER SQ. IN.

375 LBS. PER SQ. IN.

(MINIMUM)

30 LBS. PER CU. FT.

MATERIAL AND WORKMANSHIP:

EQUIVALENT FLUID PRESSURE OF EARTH ----

UNTREATED - EXTREME FIBER STRESS

COMPRESSION PERPENDICULAR TO GRAIN

EXCEPT AS MAY OTHERWISE BE SPECIFIED ON PLANS OR IN THE SPECIAL PROVISIONS, ALL MATERIAL AND WORKMANSHIP SHALL BE IN ACCORDANCE WITH THE 2006 "STANDARD SPECIFICATIONS FOR ROADS AND STRUCTURES" OF THE N. C. DEPARTMENT OF TRANSPORTATION.

OF TIMBER ----

STEEL SHEET PILING FOR PERMANENT OR TEMPORARY APPLICATIONS SHALL BE HOT ROLLED.

CONCRETE:

UNLESS OTHERWISE REQUIRED ON PLANS, CLASS A CONCRETE SHALL BE USED FOR ALL PORTIONS OF ALL STRUCTURES WITH THE EXCEPTION THAT: CLASS AA CONCRETE SHALL BE USED IN BRIDGE SUPERSTRUCTURES, ABUTMENT BACKWALLS, AND APPROACH SLABS; AND CLASS B CONCRETE SHALL BE USED FOR SLOPE PROTECTION AND RIP RAP.

CONCRETE CHAMFERS:

UNLESS OTHERWISE NOTED ON THE PLANS, ALL EXPOSED CORNERS ON STRUCTURES SHALL BE CHAMFERED 3/4" WITH THE FOLLOWING EXCEPTIONS: TOP CORNERS OF CURBS MAY BE ROUNDED TO 1-1/2" RADIUS WHICH IS BUILT INTO CURB FORMS; CORNERS OF TRANSVERSE FLOOR EXPANSION JOINTS SHALL BE ROUNDED WITH A 1/4" FINISHING TOOL UNLESS OTHERWISE REQUIRED ON PLANS; AND CORNERS OF EXPANSION JOINTS IN THE ROADWAY FACES AND TOPS OF CURBS AND SIDEWALKS SHALL BE ROUNDED TO A 1/4" RADIUS WITH A FINISHING STONE OR TOOL UNLESS OTHERWISE REQUIRED ON PLANS.

DOWELS:

DOWELS WHEN INDICATED ON PLANS AS FOR CULVERT EXTENSIONS, SHALL BE EMBEDDED AT LEAST 12"INTO THE OLD CONCRETE AND GROUTED INTO PLACE WITH 1:2 CEMENT MORTAR.

<u>ALLOWANCE FOR DEAD LOAD DEFLECTION, SETTLEMENT, ETC. IN CASTING SUPERSTRUCTURES:</u>

BRIDGES SHALL BE BUILT ON THE GRADE OR VERTICAL CURVE SHOWN ON PLANS. SLABS, CURBS AND PARAPETS SHALL CONFORM TO THE GRADE OR CURVE.

ALL DIMENSIONS WHICH ARE GIVEN IN SECTION AND ARE AFFECTED BY DEAD LOAD DEFLECTIONS ARE DIMENSIONS AT CENTER LINE OF BEARING UNLESS OTHERWISE NOTED ON PLANS. IN SETTING FORMS FOR STEEL BEAM BRIDGES AND PRESTRESSED CONCRETE GIRDER BRIDGES, ADJUSTMENTS SHALL BE MADE DUE TO THE DEAD LOAD DEFLECTIONS FOR THE ELEVATIONS SHOWN. WHERE BLOCKS ARE SHOWN OVER BEAMS FOR BUILDING UP TO THE SLAB, THE VERTICAL DIMENSIONS OF THE BLOCKS SHALL BE ADJUSTED BETWEEN BEARINGS TO COMPENSATE FOR DEAD LOAD DEFLECTIONS, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER. WHERE BOTTOM OF SLAB IS IN LINE WITH BOTTOM OF TOP FLANGES, DEPTH OF SLAB BETWEEN BEARINGS SHALL BE ADJUSTED TO COMPENSATE FOR DEAD LOAD DEFLECTION, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER.

IN SETTING FALSEWORK AND FORMS FOR REINFORCED CONCRETE SPANS, AN ALLOWANCE SHALL BE MADE FOR DEAD LOAD DEFLECTIONS, SETTLEMENT OF FALSEWORK, AND PERMANENT CAMBER WHICH SHALL BE PROVIDED FOR IN ADDITION TO THE ELEVATIONS SHOWN. AFTER REMOVAL OF THE FALSEWORK, THE FINISHED STRUCTURES SHALL CONFORM TO THE PROFILE AND ELEVATIONS SHOWN ON THE PLANS AND CONSTRUCTION ELEVATIONS FURNISHED BY THE ENGINEER.

DETAILED DRAWINGS FOR FALSEWORK OR FORMS FOR BRIDGE SUPERSTRUCTURE AND ANY STRUCTURE OR PARTS OF A STRUCTURE AS NOTED ON THE PLANS SHALL

BE SUBMITTED TO THE ENGINEER FOR APPROVAL BEFORE CONSTRUCTION OF THE

REINFORCING STEEL:

FALSEWORK OR FORMS IS STARTED.

ALL REINFORCING STEEL SHALL BE DEFORMED. DIMENSIONS RELATIVE TO PLACEMENT OF REINFORCING ARE TO CENTERS OF BARS UNLESS OTHERWISE INDICATED IN THE PLANS. DIMENSIONS ON BAR DETAILS ARE TO CENTERS OF BARS OR ARE OUT TO OUT AS INDICATED ON PLANS.

WIRE BAR SUPPORTS SHALL BE PROVIDED FOR REINFORCING STEEL WHERE INDICATED ON THE PLANS. WHEN BAR SUPPORT PIECES ARE PLACED IN CONTINUOUS LINES, THEY SHALL BE SO PLACED THAT THE ENDS OF THE SUPPORTING WIRES SHALL BE LAPPED TO LOCK LEGS ON ADJOINING PIECES.

STRUCTURAL STEEL:

AT THE CONTRACTOR'S OPTION, HE MAY SUBSTITUTE 7/8" Ø SHEAR STUDS FOR THE 3/4" Ø STUDS SPECIFIED ON THE PLANS. THIS SUBSTITUTION SHALL BE MADE AT THE RATE OF 3 - 7/8" Ø STUDS FOR 4 - 3/4" Ø STUDS, AND STUD SPACING CHANGES SHALL BE MADE AS NECESSARY TO PROVIDE THE SAME EQUIVALENT NUMBER OF 7/8" Ø STUDS ALONG THE BEAM AS SHOWN FOR 3/4" Ø STUDS BASED ON THE RATIO OF 3 - 7/8" Ø STUDS FOR 4 - 3/4" Ø STUDS. STUDS OF THE LENGTH SPECIFIED ON THE PLANS MUST BE PROVIDED. THE MAXIMUM SPACING SHALL BE 2'-0".

EXCEPT AT THE INTERIOR SUPPORTS OF CONTINUOUS BEAMS WHERE THE COVER PLATE IS IN CONTACT WITH BEARING PLATE, THE CONTRACTOR MAY, AT HIS OPTION, SUBSTITUTE FOR THE COVER PLATES DESIGNATED ON THE PLANS COVER PLATES OF THE EQUIVALENT AREA PROVIDED THESE PLATES ARE AT LEAST 5/16"IN THICKNESS AND DO NOT EXCEED A WIDTH EQUAL TO THE FLANGE WIDTH LESS 2"OR A THICKNESS EQUAL TO 2 TIMES THE FLANGE THICKNESS. THE SIZE OF FILLET WELDS SHALL CONFORM TO THE REQUIREMENTS OF THE CURRENT ANSI/AASHTO/AWS "BRIDGE WELDING CODE". ELECTROSLAG WELDING WILL NOT BE PERMITTED.

ELECTROSLAG WELDING WILL NOT BE PERMITTED.

WITH THE SOLE EXCEPTION OF EDGES AT SURFACES WHICH BEAR ON OTHER SURFACES, ALL SHARP EDGES AND ENDS OF SHAPES AND PLATES SHALL BE SLIGHTLY ROUNDED BY SUITABLE MEANS TO A RADIUS OF APPROXIMATELY 1/16 INCH OR EQUIVALENT FLAT SURFACE AT A SUITABLE ANGLE PRIOR TO PAINTING, GALVANIZING, OR METALLIZING.

HANDRAILS AND POSTS:

METAL STANDARDS AND FACES OF THE CONCRETE END POSTS FOR THE METAL RAIL SHALL BE SET NORMAL TO THE GRADE OF THE CURB, UNLESS OTHERWISE SHOWN ON PLANS. THE METAL RAIL AND TOPS OF CONCRETE POSTS USED WITH THE ALUMINUM RAIL SHALL BE BUILT PARALLEL TO THE GRADE OF THE CURB.

METAL HANDRAILS SHALL BE IN ACCORDANCE WITH THE PLANS. RAILS SHALL BE AS MANUFACTURED FOR BRIDGE RAILING. CASTINGS SHALL BE OF A UNIFORM APPEARA

METAL HANDRAILS SHALL BE IN ACCORDANCE WITH THE PLANS. RAILS SHALL BE AS MANUFACTURED FOR BRIDGE RAILING. CASTINGS SHALL BE OF A UNIFORM APPEARANCE. FINS AND OTHER DEFORMATIONS RESULTING FROM CASTING OR OTHERWISE SHALL BE REMOVED IN A MANNER SO THAT A UNIFORM COLORING OF THE COMPLETED CASTING SHALL BE OBTAINED. CASTINGS WITH DISCOLORATIONS OR OF NON-UNIFORM COLORING WILL NOT BE ACCEPTED. CERTIFIED MILL REPORTS ARE REQUIRED FOR METAL RAILS AND POSTS.

SPECIAL NOTES:

GENERALLY, IN CASE OF DISCREPANCY, THIS STANDARD SHEET OF NOTES SHALL GOVERN OVER THE SPECIFICATIONS, BUT THE REMAINDER OF THE PLANS SHALL GOVERN OVER NOTES HEREON, AND SPECIAL PROVISIONS SHALL GOVERN OVER ALL. SEE SPECIFICATIONS ARTICLE 105-4.

ENGLISH