STATE	STATE PROJECT REFERENCE NO.	SHEET NO.	TOTAL SHEETS
N.C.	33590.1.1 B-4247	1	10

STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
DIVISION OF HIGHWAYS
GEOTECHNICAL ENGINEERING UNIT

STRUCTURE SUBSURFACE INVESTIGATION

PROJ. REFERENCE NO. 33590.1.1 B-4247 F.A. PROJ. _____

COUNTY RICHMOND

PROJECT DESCRIPTION BRIDGE NO. 129 OVER BIG MOUNTAIN CREEK ON SR 1321 (CAPEL MILL ROAD)

SITE DESCRIPTION _____

CONTENTS

CAUTION NOTICE

THE SUBSURFACE INFORMATION AND THE SUBSURFACE INVESTIGATION ON WHICH IT IS BASED WERE MADE FOR THE PURPOSE OF STUDY, PLANNING, AND DESIGN, AND NOT FOR CONSTRUCTION OR PAY PURPOSES. THE VARIOUS FIELD BORING LOGS, ROCK CORES, AND SOIL TEST DATA AVAILABLE MAY BE REVIEWED OR INSPECTED IN RALEIGH BY CONTACTING THE N.C. DEPARTMENT OF TRANSPORTATION, GEOTECHNICAL ENGINEERING UNIT AT (919) 250-4088. NEITHER THE SUBSURFACE PLANS AND REPORTS, NOR THE FIELD BORING LOGS, ROCK CORES, OR SOIL TEST DATA ARE PART OF THE CONTRACT.

GENERAL SOIL AND ROCK STRATA DESCRIPTIONS AND INDICATED BOUNDARIES ARE BASED ON A GEOTECHNICAL INTERPRETATION OF ALL AVAILABLE SUBSURFACE DATA AND MAY NOT NECESSARILY REFLECT THE ACTUAL SUBSURFACE CONDITIONS BETWEEN BORINGS OR BETWEEN SAMPLED STRATA WITHIN THE BOREHOLE. THE LABORATORY SAMPLE DATA AND THE IN SITU (IN-PLACE) TEST DATA CAN BE RELIED ON ONLY TO THE DEGREE OF RELIABILITY INHERENT IN THE STANDARD TEST METHOD. THE OBSERVED WATER LEVELS OR SOIL MOISTURE CONDITIONS INDICATED IN THE SUBSURFACE INVESTIGATIONS ARE AS RECORDED AT THE TIME OF THE INVESTIGATION. THESE WATER LEVELS OR SOIL MOISTURE CONDITIONS MAY VARY CONSIDERABLY WITH TIME ACCORDING TO CLIMATIC CONDITIONS INCLUDING TEMPERATURES, PRECIPITATION, AND WIND, AS WELL AS OTHER NON-CLIMATIC FACTORS.

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PERSONNEL

R.W. TODD

M.L. SMITH

A.C. SMITH

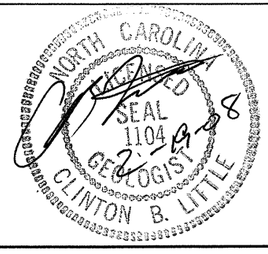
C.L. SMITH

INVESTIGATED BY J.P. ROGERS

CHECKED BY C.B. LITTLE

SUBMITTED BY C.B. LITTLE

DATE FEBRUARY 2008



PROJECT: 33590.1.1

ID: B-4247

DRAWN BY: J.K. McCLURE

NOTE - THE INFORMATION CONTAINED HEREIN IS NOT IMPLIED OR GUARANTEED BY THE N. C. DEPARTMENT OF TRANSPORTATION AS BEING ACCURATE NOR IT IS CONSIDERED TO BE PART OF THE PLANS, SPECIFICATIONS, OR CONTRACT FOR THE PROJECT.

NOTE - BY HAVING REQUESTED THIS INFORMATION THE CONTRACTOR SPECIFICALLY WAIVES ANY CLAIMS FOR INCREASED COMPENSATION OR EXTENSION OF TIME BASED ON DIFFERENCES BETWEEN THE CONDITIONS INDICATED HEREIN AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

PROJECT REFERENCE NO.	SHEET NO.
33590.1.1 B-4247	2

NORTH CAROLINA DEPARTMENT OF TRANSPORTATION
DIVISION OF HIGHWAYS
GEOTECHNICAL ENGINEERING UNIT

SUBSURFACE INVESTIGATION

SOIL AND ROCK LEGEND, TERMS, SYMBOLS, AND ABBREVIATIONS

SOIL DESCRIPTION

SOIL IS CONSIDERED TO BE THE UNCONSOLIDATED, SEMI-CONSOLIDATED, OR WEATHERED EARTH MATERIALS THAT CAN BE PENETRATED WITH A CONTINUOUS FLIGHT POWER AUGER, AND YIELD LESS THAN 100 BLOWS PER FOOT ACCORDING TO STANDARD PENETRATION TEST (AASHTO T206, ASTM D-1586). SOIL CLASSIFICATION IS BASED ON THE AASHTO SYSTEM. BASIC DESCRIPTIONS GENERALLY SHALL INCLUDE: CONSISTENCY, COLOR, TEXTURE, MOISTURE, AASHTO CLASSIFICATION, AND OTHER PERTINENT FACTORS SUCH AS MINERALOGICAL COMPOSITION, ANGULARITY, STRUCTURE, PLASTICITY, ETC. EXAMPLE:

VERY STIFF, GRAY, SILTY CLAY, MOIST WITH INTERBEDDED FINE SAND LAYERS, HIGHLY PLASTIC, A-7-6

SOIL LEGEND AND AASHTO CLASSIFICATION

GENERAL CLASS.	GRANULAR MATERIALS (≤ 35% PASSING #200)							SILT-CLAY MATERIALS (> 35% PASSING #200)				ORGANIC MATERIALS		
GROUP CLASS.	A-1		A-3	A-2				A-4	A-5	A-6	A-7	A-1, A-2 A-3	A-4, A-5 A-6, A-7	
	A-1-a	A-1-b		A-2-4	A-2-5	A-2-6	A-2-7				A-7-5 A-7-6			
SYMBOL														
% PASSING # 10	50 MX											GRANULAR SOILS	SILT-CLAY SOILS	MUCK, PEAT
# 40	30 MX	50 MX	51 MN											
# 200	15 MX	25 MX	10 MX	35 MX	35 MX	35 MX	35 MX	36 MN	36 MN	36 MN	36 MN			
LIQUID LIMIT	6 MX		NP	40 MX	41 MN	40 MX	41 MN	40 MX	41 MN	40 MX	41 MN	SOILS WITH LITTLE OR MODERATE AMOUNTS OF ORGANIC MATTER		HIGHLY ORGANIC SOILS
PLASTIC INDEX				10 MX	10 MX	11 MN	11 MN	10 MX	10 MX	11 MN	11 MN			
GROUP INDEX	0		0	0		4 MX		8 MX	12 MX	16 MX	No MX			
USUAL TYPES OF MAJOR MATERIALS	STONE FRAGS. GRAVEL, AND SAND		FINE SAND	SILTY OR CLAYEY GRAVEL AND SAND				SILTY SOILS		CLAYEY SOILS				
GEN. RATING AS A SUBGRADE	EXCELLENT TO GOOD							FAIR TO POOR				FAIR TO POOR	POOR	UNSUITABLE

PI OF A-7-5 SUBGROUP IS ≤ LL - 30 ; PI OF A-7-6 SUBGROUP IS > LL - 30

CONSISTENCY OR DENSENESS

PRIMARY SOIL TYPE	COMPACTNESS OR CONSISTENCY	RANGE OF STANDARD PENETRATION RESISTENCE (N-VALUE)	RANGE OF UNCONFINED COMPRESSIVE STRENGTH ($TONS/FT^2$)
GENERALLY GRANULAR MATERIAL (NON-COHESIVE)	VERY LOOSE	<4	N/A
	LOOSE	4 TO 10	
	MEDIUM DENSE	10 TO 30	
	DENSE	30 TO 50	
	VERY DENSE	>50	
GENERALLY SILT-CLAY MATERIAL (COHESIVE)	VERY SOFT	<2	<0.25
	SOFT	2 TO 4	0.25 TO 0.50
	MEDIUM STIFF	4 TO 8	0.5 TO 1.0
	STIFF	8 TO 15	1 TO 2
	VERY STIFF	15 TO 30	2 TO 4
	HARD	>30	>4

TEXTURE OR GRAIN SIZE

U.S. STD. SIEVE SIZE	4	10	40	60	200	270
OPENING (MM)	4.76	2.00	0.42	0.25	0.075	0.053

BOULDER (BLDR.)	COBBLE (COB.)	GRAVEL (GR.)	COARSE SAND (CSE. SD.)	FINE SAND (F SD.)	SILT (SL.)	CLAY (CL.)

GRAIN SIZE	MM	305	75	2.0	0.25	0.05	0.005
	IN.	12	3				

SOIL MOISTURE - CORRELATION OF TERMS

SOIL MOISTURE SCALE (ATTERBERG LIMITS)	FIELD MOISTURE DESCRIPTION	GUIDE FOR FIELD MOISTURE DESCRIPTION
	- SATURATED - (SAT.)	USUALLY LIQUID; VERY WET, USUALLY FROM BELOW THE GROUND WATER TABLE
LL - LIQUID LIMIT		
PLASTIC RANGE (PI)	- WET - (W)	SEMISOLID; REQUIRES DRYING TO ATTAIN OPTIMUM MOISTURE
PL - PLASTIC LIMIT		
OM - OPTIMUM MOISTURE	- MOIST - (M)	SOLID; AT OR NEAR OPTIMUM MOISTURE
SL - SHRINKAGE LIMIT		
	- DRY - (D)	REQUIRES ADDITIONAL WATER TO ATTAIN OPTIMUM MOISTURE

PLASTICITY

	PLASTICITY INDEX (PI)	DRY STRENGTH
NONPLASTIC	0-5	VERY LOW
LOW PLASTICITY	6-15	SLIGHT
MED. PLASTICITY	16-25	MEDIUM
HIGH PLASTICITY	26 OR MORE	HIGH

COLOR

DESCRIPTIONS MAY INCLUDE COLOR OR COLOR COMBINATIONS (TAN, RED, YELLOW-BROWN, BLUE-GRAY). MODIFIERS SUCH AS LIGHT, DARK, STREAKED, ETC. ARE USED TO DESCRIBE APPEARANCE.

GRADATION

WELL GRADED - INDICATES A GOOD REPRESENTATION OF PARTICLE SIZES FROM FINE TO COARSE.
UNIFORM - INDICATES THAT SOIL PARTICLES ARE ALL APPROXIMATELY THE SAME SIZE. (ALSO POORLY GRADED)
GAP-GRADED - INDICATES A MIXTURE OF UNIFORM PARTICLES OF TWO OR MORE SIZES.

ANGULARITY OF GRAINS

THE ANGULARITY OR ROUNDNESS OF SOIL GRAINS IS DESIGNATED BY THE TERMS: ANGULAR, SUBANGULAR, SUBROUNDED, OR ROUNDED.

MINERALOGICAL COMPOSITION

MINERAL NAMES SUCH AS QUARTZ, FELDSPAR, MICA, TALC, KAOLIN, ETC. ARE USED IN DESCRIPTIONS WHENEVER THEY ARE CONSIDERED OF SIGNIFICANCE.

COMPRESSIBILITY

SLIGHTLY COMPRESSIBLE	LIQUID LIMIT LESS THAN 31
MODERATELY COMPRESSIBLE	LIQUID LIMIT EQUAL TO 31-50
HIGHLY COMPRESSIBLE	LIQUID LIMIT GREATER THAN 50

PERCENTAGE OF MATERIAL

ORGANIC MATERIAL	GRANULAR SOILS	SILT - CLAY SOILS
TRACE OF ORGANIC MATTER	2 - 3%	3 - 5%
LITTLE ORGANIC MATTER	3 - 5%	5 - 12%
MODERATELY ORGANIC	5 - 10%	12 - 20%
HIGHLY ORGANIC	>10%	>20%

OTHER MATERIAL	
TRACE	1 - 10%
LITTLE	10 - 20%
SOME	20 - 35%
HIGHLY	35% AND ABOVE

GROUND WATER

- WATER LEVEL IN BORE HOLE IMMEDIATELY AFTER DRILLING
- STATIC WATER LEVEL AFTER 24 HOURS
- PW — PERCHED WATER, SATURATED ZONE, OR WATER BEARING STRATA
- SPRING OR SEEP

MISCELLANEOUS SYMBOLS

- ROADWAY EMBANKMENT (RE) WITH SOIL DESCRIPTION
- SOIL SYMBOL
- ARTIFICIAL FILL (AF) OTHER THAN ROADWAY EMBANKMENT
- INFERRED SOIL BOUNDARY
- INFERRED ROCK LINE
- ALLUVIAL SOIL BOUNDARY
- 25/025 — DIP & DIP DIRECTION OF ROCK STRUCTURES
- SOUNDING ROD
- SPT CPT / DPT DMT / VST PMT — TEST BORING
- AUGER BORING
- CORE BORING
- MW — MONITORING WELL
- PIEZOMETER INSTALLATION
- SLOPE INDICATOR INSTALLATION
- SPT N-VALUE
- REF — SPT REFUSAL

SAMPLE DESIGNATIONS

- S - BULK SAMPLE
- SS - SPLIT SPOON SAMPLE
- ST - SHELBY TUBE SAMPLE
- RS - ROCK SAMPLE
- RT - RECOMPACTED TRIAXIAL SAMPLE
- CBR - CALIFORNIA BEARING RATIO SAMPLE

ABBREVIATIONS

AR - AUGER REFUSAL
BT - BORING TERMINATED
CL. - CLAY
CPT - CONE PENETRATION TEST
CSE. - COARSE
DMT - DILATOMETER TEST
DPT - DYNAMIC PENETRATION TEST
e - VOID RATIO
F - FINE
FOSS. - FOSSILIFEROUS
FRAC. - FRACTURED, FRACTURES
FRAGS. - FRAGMENTS
HI. - HIGHLY
MED. - MEDIUM
MICA. - MICACEOUS
MOD. - MODERATELY
NP - NON PLASTIC
ORG. - ORGANIC
PMT - PRESSUREMETER TEST
SAP. - SAPROLITIC
SD. - SAND, SANDY
SL. - SILT, SILTY
SLI. - SLIGHTLY
TCR - TRICONE REFUSAL
w - MOISTURE CONTENT
V - VERY
VST - VANE SHEAR TEST
WEA. - WEATHERED
γ - UNIT WEIGHT
γ_d - DRY UNIT WEIGHT

EQUIPMENT USED ON SUBJECT PROJECT

DRILL UNITS:
- [ ] MOBILE B-___
- [ ] BK-51
- [ ] CME-45C
- [X] CME-550
- [ ] PORTABLE HOIST
- [ ] _____
- [ ] _____

ADVANCING TOOLS:
- [ ] CLAY BITS
- [ ] 6" CONTINUOUS FLIGHT AUGER
- [X] 8" HOLLOW AUGERS
- [ ] HARD FACED FINGER BITS
- [X] TUNG.-CARBIDE INSERTS
- [ ] CASING [ ] W/ ADVANCER
- [ ] TRICONE _____" STEEL TEETH
- [ ] TRICONE _____" TUNG.-CARB.
- [ ] CORE BIT
- [ ] _____

HAMMER TYPE:
- [X] AUTOMATIC
- [ ] MANUAL

CORE SIZE:
- [ ] -B_____
- [ ] -N_____
- [ ] -H_____

HAND TOOLS:
- [ ] POST HOLE DIGGER
- [ ] HAND AUGER
- [ ] SOUNDING ROD
- [ ] VANE SHEAR TEST
- [ ] _____

ROCK DESCRIPTION

HARD ROCK IS NON-COASTAL PLAIN MATERIAL THAT IF TESTED, WOULD YIELD SPT REFUSAL. AN INFERRED ROCK LINE INDICATES THE LEVEL AT WHICH NON-COASTAL PLAIN MATERIAL WOULD YIELD SPT REFUSAL. SPT REFUSAL IS PENETRATION BY A SPLIT SPOON SAMPLER EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS. IN NON-COASTAL PLAIN MATERIAL, THE TRANSITION BETWEEN SOIL AND ROCK IS OFTEN REPRESENTED BY A ZONE OF WEATHERED ROCK.
ROCK MATERIALS ARE TYPICALLY DIVIDED AS FOLLOWS:

WEATHERED ROCK (WR)	NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT N VALUES > 100 BLOWS PER FOOT IF TESTED.
CRYSTALLINE ROCK (CR)	FINE TO COARSE GRAIN IGNEOUS AND METAMORPHIC ROCK THAT WOULD YIELD SPT REFUSAL IF TESTED. ROCK TYPE INCLUDES GRANITE, GNEISS, GABBRO, SCHIST, ETC.
NON-CRYSTALLINE ROCK (NCR)	FINE TO COARSE GRAIN METAMORPHIC AND NON-COASTAL PLAIN SEDIMENTARY ROCK THAT WOULD YEILD SPT REFUSAL IF TESTED. ROCK TYPE INCLUDES PHYLLITE, SLATE, SANDSTONE, ETC.
COASTAL PLAIN SEDIMENTARY ROCK (CP)	COASTAL PLAIN SEDIMENTS CEMENTED INTO ROCK, BUT MAY NOT YIELD SPT REFUSAL. ROCK TYPE INCLUDES LIMESTONE, SANDSTONE, CEMENTED SHELL BEDS, ETC.

WEATHERING

FRESH	ROCK FRESH, CRYSTALS BRIGHT, FEW JOINTS MAY SHOW SLIGHT STAINING. ROCK RINGS UNDER HAMMER IF CRYSTALLINE.
VERY SLIGHT (V SLI.)	ROCK GENERALLY FRESH, JOINTS STAINED, SOME JOINTS MAY SHOW THIN CLAY COATINGS IF OPEN, CRYSTALS ON A BROKEN SPECIMEN FACE SHINE BRIGHTLY. ROCK RINGS UNDER HAMMER BLOWS IF OF A CRYSTALLINE NATURE.
SLIGHT (SLI.)	ROCK GENERALLY FRESH, JOINTS STAINED AND DISCOLORATION EXTENDS INTO ROCK UP TO 1 INCH. OPEN JOINTS MAY CONTAIN CLAY. IN GRANITOID ROCKS SOME OCCASIONAL FELDSPAR CRYSTALS ARE DULL AND DISCOLORED. CRYSTALLINE ROCKS RING UNDER HAMMER BLOWS.
MODERATE (MOD.)	SIGNIFICANT PORTIONS OF ROCK SHOW DISCOLORATION AND WEATHERING EFFECTS. IN GRANITOID ROCKS, MOST FELDSPARS ARE DULL AND DISCOLORED, SOME SHOW CLAY. ROCK HAS DULL SOUND UNDER HAMMER BLOWS AND SHOWS SIGNIFICANT LOSS OF STRENGTH AS COMPARED WITH FRESH ROCK.
MODERATELY SEVERE (MOD. SEV.)	ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. IN GRANITOID ROCKS, ALL FELDSPARS DULL AND DISCOLORED AND A MAJORITY SHOW KAOLINIZATION. ROCK SHOWS SEVERE LOSS OF STRENGTH AND CAN BE EXCAVATED WITH A GEOLOGIST'S PICK. ROCK GIVES "CLUNK" SOUND WHEN STRUCK. *IF TESTED, WOULD YIELD SPT REFUSAL*
SEVERE (SEV.)	ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. ROCK FABRIC CLEAR AND EVIDENT BUT REDUCED IN STRENGTH TO STRONG SOIL. IN GRANITOID ROCKS ALL FELDSPARS ARE KAOLINIZED TO SOME EXTENT. SOME FRAGMENTS OF STRONG ROCK USUALLY REMAIN. *IF TESTED, YIELDS SPT N VALUES > 100 BPF*
VERY SEVERE (V SEV.)	ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. ROCK FABRIC ELEMENTS ARE DISCERNIBLE BUT THE MASS IS EFFECTIVELY REDUCED TO SOIL STATUS, WITH ONLY FRAGMENTS OF STRONG ROCK REMAINING. SAPROLITE IS AN EXAMPLE OF ROCK WEATHERED TO A DEGREE SUCH THAT ONLY MINOR VESTIGES OF THE ORIGINAL ROCK FABRIC REMAIN. *IF TESTED, YIELDS SPT N VALUES < 100 BPF*
COMPLETE	ROCK REDUCED TO SOIL. ROCK FABRIC NOT DISCERNIBLE, OR DISCERNIBLE ONLY IN SMALL AND SCATTERED CONCENTRATIONS. QUARTZ MAY BE PRESENT AS DIKES OR STRINGERS. SAPROLITE IS ALSO AN EXAMPLE.

ROCK HARDNESS

VERY HARD	CANNOT BE SCRATCHED BY KNIFE OR SHARP PICK. BREAKING OF HAND SPECIMENS REQUIRES SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK.
HARD	CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY. HARD HAMMER BLOWS REQUIRED TO DETACH HAND SPECIMEN.
MODERATELY HARD	CAN BE SCRATCHED BY KNIFE OR PICK. GOUGES OR GROOVES TO 0.25 INCHES DEEP CAN BE EXCAVATED BY HARD BLOW OF A GEOLOGIST'S PICK. HAND SPECIMENS CAN BE DETACHED BY MODERATE BLOWS.
MEDIUM HARD	CAN BE GROOVED OR GOUGED 0.05 INCHES DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT. CAN BE EXCAVATED IN SMALL CHIPS TO PEICES 1 INCH MAXIMUM SIZE BY HARD BLOWS OF THE POINT OF A GEOLOGIST'S PICK.
SOFT	CAN BE GROVED OR GOUGED READILY BY KNIFE OR PICK. CAN BE EXCAVATED IN FRAGMENTS FROM CHIPS TO SEVERAL INCHES IN SIZE BY MODERATE BLOWS OF A PICK POINT. SMALL, THIN PIECES CAN BE BROKEN BY FINGER PRESSURE.
VERY SOFT	CAN BE CARVED WITH KNIFE. CAN BE EXCAVATED READILY WITH POINT OF PICK. PIECES 1 INCH OR MORE IN THICKNESS CAN BE BROKEN BY FINGER PRESSURE. CAN BE SCRATCHED READILY BY FINGERNAIL.

FRACTURE SPACING

TERM	SPACING
VERY WIDE	MORE THAN 10 FEET
WIDE	3 TO 10 FEET
MODERATELY CLOSE	1 TO 3 FEET
CLOSE	0.16 TO 1 FEET
VERY CLOSE	LESS THAN 0.16 FEET

BEDDING

TERM	THICKNESS
VERY THICKLY BEDDED	> 4 FEET
THICKLY BEDDED	1.5 - 4 FEET
THINLY BEDDED	0.16 - 1.5 FEET
VERY THINLY BEDDED	0.03 - 0.16 FEET
THICKLY LAMINATED	0.008 - 0.03 FEET
THINLY LAMINATED	< 0.008 FEET

INDURATION

FOR SEDIMENTARY ROCKS, INDURATION IS THE HARDENING OF THE MATERIAL BY CEMENTING, HEAT, PRESSURE, ETC.

FRIABLE	RUBBING WITH FINGER FREES NUMEROUS GRAINS; GENTLE BLOW BY HAMMER DISINTEGRATES SAMPLE.
MODERATELY INDURATED	GRAINS CAN BE SEPARATED FROM SAMPLE WITH STEEL PROBE; BREAKS EASILY WHEN HIT WITH HAMMER.
INDURATED	GRAINS ARE DIFFICULT TO SEPARATE WITH STEEL PROBE; DIFFICULT TO BREAK WITH HAMMER.
EXTREMELY INDURATED	SHARP HAMMER BLOWS REQUIRED TO BREAK SAMPLE; SAMPLE BREAKS ACROSS GRAINS.

TERMS AND DEFINITIONS

ALLUVIUM (ALLUV.) - SOILS THAT HAVE BEEN TRANSPORTED BY WATER.

AQUIFER - A WATER BEARING FORMATION OR STRATA.

ARENACEOUS - APPLIED TO ROCKS THAT HAVE BEEN DERIVED FROM SAND OR THAT CONTAIN SAND.

ARGILLACEOUS - APPLIED TO ALL ROCKS OR SUBSTANCES COMPOSED OF CLAY MINERALS, OR HAVING A NOTABLE PROPORTION OF CLAY IN THEIR COMPOSITION, AS SHALE, SLATE, ETC.

ARTESIAN - GROUND WATER THAT IS UNDER SUFFICIENT PRESSURE TO RISE ABOVE THE LEVEL AT WHICH IT IS ENCOUNTERED, BUT WHICH DOES NOT NECESSARILY RISE TO OR ABOVE THE GROUND SURFACE.

CALCAREOUS (CALC.) - SOILS THAT CONTAIN APPRECIABLE AMOUNTS OF CALCIUM CARBONATE.

COLLUVIUM - ROCK FRAGMENTS MIXED WITH SOIL DEPOSITED BY GRAVITY ON SLOPE OR AT BOTTOM OF SLOPE.

CORE RECOVERY (REC.) - TOTAL LENGTH OF ALL MATERIAL RECOVERED IN THE CORE BARREL DIVIDED BY TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE.

DIKE - A TABULAR BODY OF IGNEOUS ROCK THAT CUTS ACROSS THE STRUCTURE OF ADJACENT ROCKS OR CUTS MASSIVE ROCK.

DIP - THE ANGLE AT WHICH A STRATUM OR ANY PLANAR FEATURE IS INCLINED FROM THE HORIZONTAL.

DIP DIRECTION (DIP AZIMUTH) - THE DIRECTION OR BEARING OF THE HORIZONTAL TRACE OF THE LINE OF DIP, MEASURED CLOCKWISE FROM NORTH.

FAULT - A FRACTURE OR FRACTURE ZONE ALONG WHICH THERE HAS BEEN DISPLACEMENT OF THE SIDES RELATIVE TO ONE ANOTHER PARALLEL TO THE FRACTURE.

FISSILE - A PROPERTY OF SPLITTING ALONG CLOSELY SPACED PARALLEL PLANES.

FLOAT - ROCK FRAGMENTS ON SURFACE NEAR THEIR ORIGINAL POSITION AND DISLODGED FROM PARENT MATERIAL.

FLOOD PLAIN (FP) - LAND BORDERING A STREAM, BUILT OF SEDIMENTS DEPOSITED BY THE STREAM.

FORMATION (FM.) - A MAPPABLE GEOLOGIC UNIT THAT CAN BE RECOGNIZED AND TRACED IN THE FIELD.

JOINT - FRACTURE IN ROCK ALONG WHICH NO APPRECIABLE MOVEMENT HAS OCCURRED.

LEDGE - A SHELF-LIKE RIDGE OR PROJECTION OF ROCK WHOSE THICKNESS IS SMALL COMPARED TO ITS LATERAL EXTENT.

LENS - A BODY OF SOIL OR ROCK THAT THINS OUT IN ONE OR MORE DIRECTIONS.

MOTTLED (MOT.) - IRREGULARLY MARKED WITH SPOTS OF DIFFERENT COLORS. MOTTLING IN SOILS USUALLY INDICATES POOR AERATION AND LACK OF GOOD DRAINAGE.

PERCHED WATER - WATER MAINTAINED ABOVE THE NORMAL GROUND WATER LEVEL BY THE PRESENCE OF AN INTERVENING IMPERVIOUS STRATUM.

RESIDUAL (RES.) SOIL - SOIL FORMED IN PLACE BY THE WEATHERING OF ROCK.

ROCK QUALITY DESIGNATION (RQD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE.

SAPROLITE (SAP.) - RESIDUAL SOIL THAT RETAINS THE RELIC STRUCTURE OR FABRIC OF THE PARENT ROCK.

SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, THAT HAS BEEN EMPLACED PARALLEL TO THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS.

SLICKENSIDE - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT OR SLIP PLANE.

STANDARD PENETRATION TEST (PENETRATION RESISTANCE) (SPT) - NUMBER OF BLOWS (N OR BPF) OF A 140 LB. HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF 1 FOOT INTO SOIL WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT REFUSAL IS PENETRATION EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS.

STRATA CORE RECOVERY (SREC.) - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY TOTAL LENGTH OF STRATUM AND EXPRESSED AS A PERCENTAGE.

STRATA ROCK QUALITY DESIGNATION (SRQD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS WITHIN A STRATUM EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF STRATA AND EXPRESSED AS A PERCENTAGE.

TOPSOIL (TS.) - SURFACE SOILS USUALLY CONTAINING ORGANIC MATTER.

BENCH MARK: NCDOT BENCHMARK #2 (R.R. SPIKE IN BASE OF TREE)
STA. 16+23.22 -L-
93.57 LT. ELEVATION: 345.81 FT.

NOTES:

REVISED 02/23/06

PROJECT REFERENCE NO.	SHEET
33590.1.1 B-4247	3

SITE PLAN

0 50 100
FEET

BM #2
-L- STA. 16+23.22
OFFSET 93.57' LT.
-BL- STA 11+21
79.18' LT.
ELEV. 345.81'

-T- 7 5+00.00 POT
ELEV=344.26' REBAR SET

-L- Sta. 16+90.91 =
-BL- 4
PINC 11+90.10
OFFSET 13.12' LT.
-T- 5+92.53
ELEV=347.24'
REBAR CAP SET

CL STA. 16+57 -L-
1 @ 80'
OAL=80'
SKEW=90°

PC Sta. 13+78.98

PT Sta. 17+05.86

PC Sta. 18+72.66

NAD 83

15

20

BIG MOUNTAIN CREEK

S 23° 28' 20.10" E

S 23° 28' 35" W

S 87° 42' 22" E

S 86° 58' 56" E

S 89° 00' 27.6" E

80

40

+78.98
40.00'

+05.86
40.00'

+72.66
40.00'

EB1-A

EB2-A

EB1-B

EB2-B

ALLUVIUM

RESIDUAL

ROADWAY EMBANKMENT (FILL)

WOODS

-T- 8 6+79.34 POT
ELEV=343.72' REBAR SET

1
ARRON W.E. CAPEL II
DB 1117 PG 0051
DB 477 PG 414

5/28/99

0 50 100 FEET	PROJECT REFERENCE NO.	SHEET
	33590.1.1 B-4247	4
	PROFILE -L-	

-L-

PROFILE THROUGH EB1-B & EB2-B

BEGIN GRADE -L- STA. 12+50.00
ELEV. = 348.78'

33" BOX BEAM BRIDGE
SPAN = 1 @ 80'
SKEW = 90 DEGREES
℄ -L- STA. 16+57.00
℄ ELEV. = 348.86'

BEGIN BRIDGE -L- STA. 16+17.00

END BRIDGE -L- STA. 16+97.00

END GRADE -L- STA. 20+50.00
ELEV. = 359.60'

PI = 14+28.00
EL = 346.30'
VC = 350'
K = 139
V_d = 60 mph

PI = 18+75.00
EL = 351.30'
VC = 350'
*K = 97
V_d = 50 mph

SS-7
SS-8
EB1-B
16+17 -L-
14.00 RT.

SS-9
EB2-B
16+80 -L-
13.00 RT.

(-)1.3921%
(+)1.1186%
(+)1.1186%
(+)4.7417%

ROADWAY EMBANKMENT (FILL) BRN V. LOOSE/DRY SILTY CLAYEY COARSE SAND (A-2-4)

ALLUVIUM
BRN V. LOOSE DRY SILTY CLAYEY CSE. SAND (A-2-4)

RESIDUAL

ROADWAY EMBANKMENT (FILL)
BRN MED. STIFF MOIST LOW PLASTIC SANDY SILTY CLAY (A-6)

WEATHERED ROCK (SEV. WEATHERED META-MUDSTONE)
NON-CRYSTALLINE ROCK

WEATHERED ROCK (SEV. WEATHERED META-MUDSTONE)
NON-CRYSTALLINE ROCK

100/0.7
100/.5
100/.5
100/.3
II/07
II/07
AR
DRY (6/27/07)
AR

BRIDGE HYDRAULIC DATA

DESIGN DISCHARGE	= 3500	CFS
DESIGN FREQUENCY	= 25	YRS
DESIGN HW ELEVATION	= 348.34	FT
BASE DISCHARGE	= 5100	CFS
BASE FREQUENCY	= 100	YRS
BASE HW ELEVATION	= 349.72	FT
OVERTOPPING DISCHARGE	= < 3500	CFS
OVERTOPPING FREQUENCY	= < 25	YRS
OVERTOPPING ELEVATION	= 347.39	FT
NORMAL WATER SURFACE ELEVATION	= 338.62	FT
DATE OF SURVEY	= 7/26/2007	

BM #2 RAILROAD SPIKE SET IN 20" OAK
93.57' LEFT OF -L- STA. 16+23.22
ELEV. = 345.81' N 512312 E 1762597

NOTES:
1.) FOR -L- PLAN VIEW SEE SHEET 4

440 430 420 410 400 390 380 370 360 350 340 330 320 310 300 290 280 270 260 250

10 11 12 13 14 15 16 17 18 19 20 21 22 23

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johnrogers

5/28/99

PROJECT REFERENCE NO.	SHEET
33590.1.1 B-4247	5
SECTION THRU END BENT ONE	

0 5 10
FEET

SECTION THROUGH END BENT ONE

SS-6
EB1-A
16+17 -L-
46.00 LT.

SS-7
SS-8
EB1-B
16+17 -L-
14.00 RT.

GROUND SURFACE
ROADWAY EMBANKMENT (FILL)
BRN V. LOOSE DRY SILTY CLAYEY CSE SAND (A-2-4)

ALLUVIUM
BRN DENSE MOIST CLAYEY CSE. SAND & GRAVEL (A-1-a)

ALLUVIUM
BRN V. LOOSE DRY SILTY CLAYEY CSE. SAND (A-2-4)

48 11/07

100/.5

WEATHERED ROCK
(SEV. WEATHERED META-MUDSTONE)

AR
NON-CRYSTALLINE ROCK

3

100/0.7 11/07

100/.5

11/07

100/.5

WEATHERED ROCK
(SEV. WEATHERED META-MUDSTONE)

AR
NON-CRYSTALLINE ROCK

380 370 360 350 340 330 320 310 300

50 40 30 20 10 0 10 20 30 40 50

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dkinnaird
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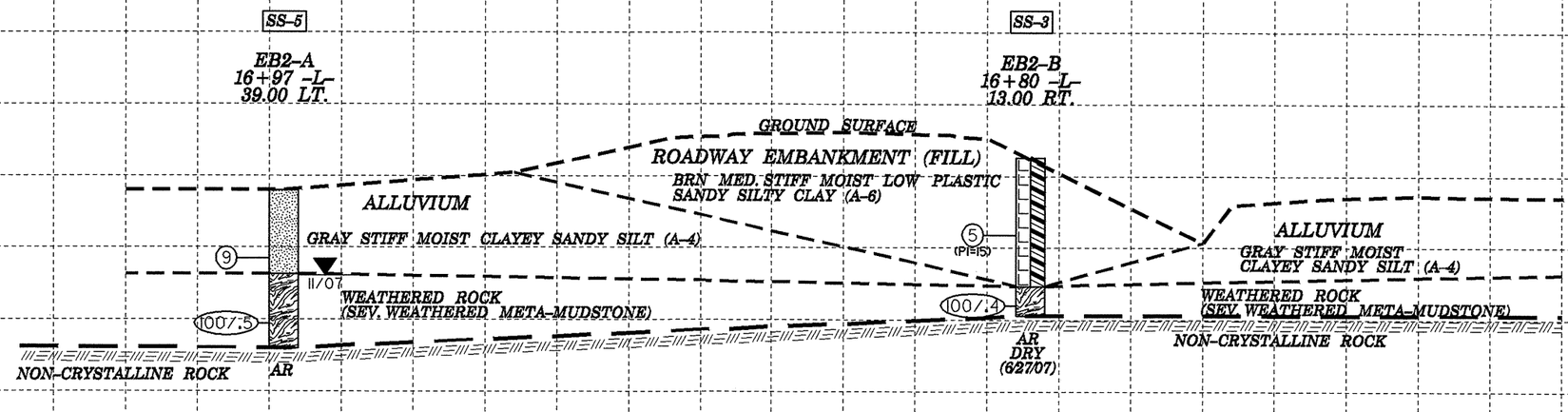
Layout: A1 (GEOTECH)

5/28/99

PROJECT REFERENCE NO.	SHEET
33590.1.1 B-4247	6
SECTION THRU END BENT TWO	

0 5 10
FEET

SECTION THROUGH END BENT TWO



NCDOT GEOTECHNICAL ENGINEERING UNIT
BORELOG REPORT

PROJECT NO. 33590.1.1	**ID.** B4247	**COUNTY** RICHMOND	**GEOLOGIST** Todd, R. W.
SITE DESCRIPTION BRIDGE NO. 129 OVER BIG MOUNTAIN CREEK ON SR 1321 (CAPEL MILL ROAD)			**GROUND WTR (ft)**
BORING NO. EB1-A	**STATION** 16+17	**OFFSET** 46ft LT	**ALIGNMENT** -L-	**0 HR.** Dry
COLLAR ELEV. 342.9 ft	**TOTAL DEPTH** 13.9 ft	**NORTHING** N/A	**EASTING** N/A	**24 HR.** 4.0
DRILL MACHINE CME-550X	**DRILL METHOD** H.S. Augers		**HAMMER TYPE** Automatic	
START DATE 11/27/07	**COMP. DATE** 11/27/07	**SURFACE WATER DEPTH** N/A	**DEPTH TO ROCK** 13.9 ft	

ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	BLOW COUNT 0.5ft	0.5ft	0.5ft	BLOWS PER FOOT 0 25 50 75 100	SAMP. NO.	MOI	LOG	SOIL AND ROCK DESCRIPTION (ELEV. (ft) / DEPTH (ft))
										342.9 / 0.0 GROUND SURFACE
	345									**ALLUVIAL** BRN. DENSE MOIST CLAYEY COARSE SAND & GRAVEL (A-1-a)
340	338.9	4.0	16	24	24	48	SS-6	M		
335	333.9	9.0	100/.5			100/.5				333.9 / 9.0 **WEATHERED ROCK** SEV. WEATHERED META-MUDSTONE
330										329.0 / 13.9 Boring Terminated BY AUGER REFUSAL at Elevation 329.0 ft ON NON-CRYSTALLINE ROCK
325										
320										
315										
310										
305										
300										
295										
290										
285										
280										
275										
270										
265										

PROJECT NO. 33590.1.1	**ID.** B4247	**COUNTY** RICHMOND	**GEOLOGIST** Todd, R. W.
SITE DESCRIPTION BRIDGE NO. 129 OVER BIG MOUNTAIN CREEK ON SR 1321 (CAPEL MILL ROAD)			**GROUND WTR (ft)**
BORING NO. EB1-B	**STATION** 16+17	**OFFSET** 14ft RT	**ALIGNMENT** -L-	**0 HR.** 15.0
COLLAR ELEV. 346.5 ft	**TOTAL DEPTH** 22.9 ft	**NORTHING** N/A	**EASTING** N/A	**24 HR.** 8.0
DRILL MACHINE CME-550X	**DRILL METHOD** H.S. Augers		**HAMMER TYPE** Automatic	
START DATE 11/27/07	**COMP. DATE** 11/27/07	**SURFACE WATER DEPTH** N/A	**DEPTH TO ROCK** 22.9 ft	

ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	BLOW COUNT 0.5ft	0.5ft	0.5ft	BLOWS PER FOOT 0 25 50 75 100	SAMP. NO.	MOI	LOG	SOIL AND ROCK DESCRIPTION (ELEV. (ft) / DEPTH (ft))
										346.5 / 0.0 GROUND SURFACE
	350									**ROADWAY EMBANKMENT** BRN V. LOOSE DRY SILTY CLAYEY COARSE SAND (A-2-4)
345	344.0	2.5								344.5 / 2.0
			0	1	2	3	SS-7	D		**ALLUVIAL** BRN V. LOOSE DRY SILTY CLAYEY COARSE SAND (A-2-4)
340	339.0	7.5								339.5 / 7.0
			14	73	27/0.2	100/0.7	SS-8	D		**WEATHERED ROCK** SEV. WEATHERED META-MUDSTONE
335	334.0	12.5	100/.5			100/.5		D		
330	329.0	17.5	100/.5			100/.5		D		
325										323.6 / 22.9 Boring Terminated BY AUGER REFUSAL at Elevation 323.6 ft ON NON-CRYSTALLINE ROCK
320										
315										
310										
305										
300										
295										
290										
285										
280										
275										
270										

NCDOT GEOTECHNICAL ENGINEERING UNIT
BORELOG REPORT

PROJECT NO. 33590.1.1	**ID.** B4247	**COUNTY** RICHMOND	**GEOLOGIST** Todd, R. W.
SITE DESCRIPTION BRIDGE NO. 129 OVER BIG MOUNTAIN CREEK ON SR 1321 (CAPEL MILL ROAD)			**GROUND WTR (ft)**
BORING NO. EB2-A	**STATION** 16+97	**OFFSET** 39ft LT	**ALIGNMENT** -L-	**0 HR.** Dry
COLLAR ELEV. 344.0 ft	**TOTAL DEPTH** 11.0 ft	**NORTHING** N/A	**EASTING** N/A	**24 HR.** 6.0
DRILL MACHINE CME-550X	**DRILL METHOD** H.S. Augers		**HAMMER TYPE** Automatic	
START DATE 11/27/07	**COMP. DATE** 11/27/07	**SURFACE WATER DEPTH** N/A	**DEPTH TO ROCK** 11.0 ft	

ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	BLOW COUNT 0.5ft	0.5ft	0.5ft	BLOWS PER FOOT 0 25 50 75 100	SAMP. NO.	MOI	LOG	SOIL AND ROCK DESCRIPTION
										ELEV. (ft) / DEPTH (ft)
	345									344.0 / 0.0 GROUND SURFACE
										ALLUVIAL
340	340.2	3.8								GRAY STIFF MOIST CLAYEY SANDY SILT (A-4)
			2	4	5	9	SS-5	M		
										338.1 / 5.9 **WEATHERED ROCK**
335	335.2	8.8								SEV. WEATHERED META-MUDSTONE
			100/.5			100/.5				
										333.0 / 11.0 Boring Terminated BY AUGER REFUSAL at Elevation 333.0 ft ON NON-CRYSTALLINE ROCK
330										
325										
320										
315										
310										
305										
300										
295										
290										
285										
280										
275										
270										
265										

PROJECT NO. 33590.1.1	**ID.** B4247	**COUNTY** RICHMOND	**GEOLOGIST** Todd, R. W.
SITE DESCRIPTION BRIDGE NO. 129 OVER BIG MOUNTAIN CREEK ON SR 1321 (CAPEL MILL ROAD)			**GROUND WTR (ft)**
BORING NO. EB2-B	**STATION** 16+80	**OFFSET** 13ft RT	**ALIGNMENT** -L-	**0 HR.** Dry
COLLAR ELEV. 346.3 ft	**TOTAL DEPTH** 11.0 ft	**NORTHING** N/A	**EASTING** N/A	**24 HR.** Dry
DRILL MACHINE CME-550X	**DRILL METHOD** H.S. Augers		**HAMMER TYPE** Automatic	
START DATE 06/27/07	**COMP. DATE** 06/27/07	**SURFACE WATER DEPTH** N/A	**DEPTH TO ROCK** 11.0 ft	

ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	BLOW COUNT 0.5ft	0.5ft	0.5ft	BLOWS PER FOOT 0 25 50 75 100	SAMP. NO.	MOI	LOG	SOIL AND ROCK DESCRIPTION
										ELEV. (ft) / DEPTH (ft)
	350									346.3 / 0.0 GROUND SURFACE
345										**ROADWAY EMBANKMENT**
										BRN MED. STIFF MOIST LOW (PI=15) PLASTIC SANDY SILTY CLAY (A-6)
	341.9	4.4								
340			2	2	3	5	SS-3	M		
	336.9	9.4								337.3 / 9.0 **WEATHERED ROCK**
335			6	100/.4		100/.4		D		SEV. WEATHERED META-MUDSTONE 335.3 / 11.0
										Boring Terminated BY AUGER REFUSAL at Elevation 335.3 ft ON NON-CRYSTALLINE ROCK
330										
325										
320										
315										
310										
305										
300										
295										
290										
285										
280										
275										
270										

TEST RESULTS

PROJECT: 33590.0.0 B-4247
COUNTY: RICHMOND
SITE DESCRIPTION: BRIDGE NO. 129 OVER BIG MOUNTAIN CREEK ON SR 1321 (CAPEL MILL ROAD)

SOIL SAMPLE RESULTS

SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS	N	L.L.	P.I.	% BY WEIGHT C. SAND	F. SAND	SILT	CLAY	% PASSING SIEVES 10	40	200	% MOISTURE	% ORGANIC	UNIT WT. (d)	VOID RATIO
		EB1-A																
SS-6	46.0 LT.	16+17 -L-	4.00-5.50	A-1-a(0)	48	18	2	67.2	13.3	7.3	12.3	40	20	8				
		EB1-B																
SS-7	14.0 RT.	16+17 -L-	2.50-4.00	A-2-4(0)	3	18	3	45.1	28.6	12.0	14.3	100	79	29				
SS-8			7.50-8.70	A-1-b(0)	100/.7	26	5	60.5	12.5	10.7	16.3	52	26	15				
		EB2-A																
SS-5	39.0 LT.	16+97 -L-	3.80-5.30	A-4(1)	9	23	6	20.2	29.0	22.2	28.6	99	88	55				
		EB2-B																
SS-3	13.0 RT.	16+80 -L-	4.40-5.90	A-6(10)	5	39	15	16.8	10.1	32.5	40.6	93	82	70				

ROCK SAMPLE RESULTS

SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	RQD	UNIT WT	Q(MPa) (MPsi)	E(MPa) (MPsi)

NORTH CAROLINA DEPARTMENT OF TRANSPORTATION
GEOTECHNICAL ENGINEERING UNIT

FIELD SCOUR REPORT

WBS: 33590.1.1 TIP: B-4247 COUNTY: RICHMOND

DESCRIPTION(1): BRIDGE NO. 129 OVER BIG MOUNTAIN CREEK ON SR 1321.

EXISTING BRIDGE

Information from: Field Inspection X Microfilm ____ (reel ____ pos: ____)
Other (explain) ____

Bridge No.: 129 Length: 40' Total Bents: 3 Bents in Channel: 3 Bents in Floodplain: 3
Foundation Type: END BENTS - ABUTMENTS. INTERIOR BENT - FOOTINGS.

EVIDENCE OF SCOUR(2)
Abutments or End Bent Slopes: NO

Interior Bents: NO

Channel Bed: NO

Channel Bank: NO

EXISTING SCOUR PROTECTION
Type(3): RIP-RAP

Extent(4): BOTH END BENTS

Effectiveness(5): OK

Obstructions(6): SAND BAR 15' UPSTREAM. OCCUPIES 1/2 OF CHANNEL. DIVERTS FLOW TO EAST.

DESIGN INFORMATION

Channel Bed Material(7): BOULDERS & COBBLES

Channel Bank Material(8): STIFF, SANDY SILT AS SS-5.

Channel Bank Cover(9): GRASS, SHRUBS, SMALL TREES.

Floodplain Width(10): APP. 300'.

Floodplain Cover(11): WOODS.

Stream is(12): Aggrading X Degrading ____ Static ____

Channel Migration Tendency(13): MODERATE TO HIGH TENDENCY FOR EASTWARD MIGRATION.

Observations and Other Comments: CLEAN, SWIFT WATER AT BRIDGE. DEBRIS IN BEAMS AND INTERIOR PIER. EVIDENCE OF RECENT HIGH WATER

DESIGN SCOUR ELEVATIONS(14) Feet X Meters ____

	100 YR.		500 YR.								
STREAM BED	336.0'		335.3'								
	NO	SCOUR	AT	END	BENTS						

Comparison of DSE to Hydraulics Unit theoretical scour:
THE ABOVE ELEVATIONS MATCH THOSE MADE BY HYDRAULICS.

SOIL ANALYSIS RESULTS FROM CHANNEL BED AND BANK MATERIAL

		SEE	SAMPLES	RESULTS			
Sample No.							
Retained #4							
Passed #10							
Passed #40							
Passed #200							
Coarse Sand							
Fine Sand							
Silt							
Clay							
LL							
PI							
AASHTO							
Station							
Offset							
Depth							

Template Revised 02/07/06

Reported by: RW TODD **Date:** 11/7/2008

INSTRUCTIONS

1 Describe the specific site's location, including route number and body of water crossed.
2 Note scour evidence at existing end bents or abutments (e.g. undermining, sloughing, degradations).
3 Note existing scour protection (e.g. rip rap).
4 Describe extent of existing scour protection.
5 Describe whether or not the scour protection appears to be working.
6 Note obstructions such as dams, fallen trees, debris at bents, etc.
7 Describe the channel bed material based on observation and/or samples. Include any lab results with report.
8 Describe the channel bank material based on observation and/or samples. Include any lab results with report.
9 Describe the material covering the banks (e.g. grass, trees, rip rap, none).
10 Determine the approximate floodplain width from field observation or a topographic map.
11 Describe the material covering the floodplain (e.g. grass, trees, crops).
12 Use professional judgement to specify if the stream is degrading, aggrading, or static.
13 Describe potential and direction of the stream to migrate laterally during the bridge's life (approx. 100 years).
14 Give the design scour elevation (DSE) expected over the life of the bridge (approx. 100 years). This elevation can be given as a range across the site, or for each bent. Discuss the relationship between the Hydraulics Unit theoritical scour and the DSE. If the DSE is dependent on scour counter measures, explain (e.g. rip rap armoring on slopes). The DSE is based on the erodability of materials, giving consideration to the influence of joints, foliation, bedding characteristics, % core recovery, % RQD, differential weathering, shear strength, observations at existing structures, other tests deemed appropriate, and overall geologic conditions at the site.