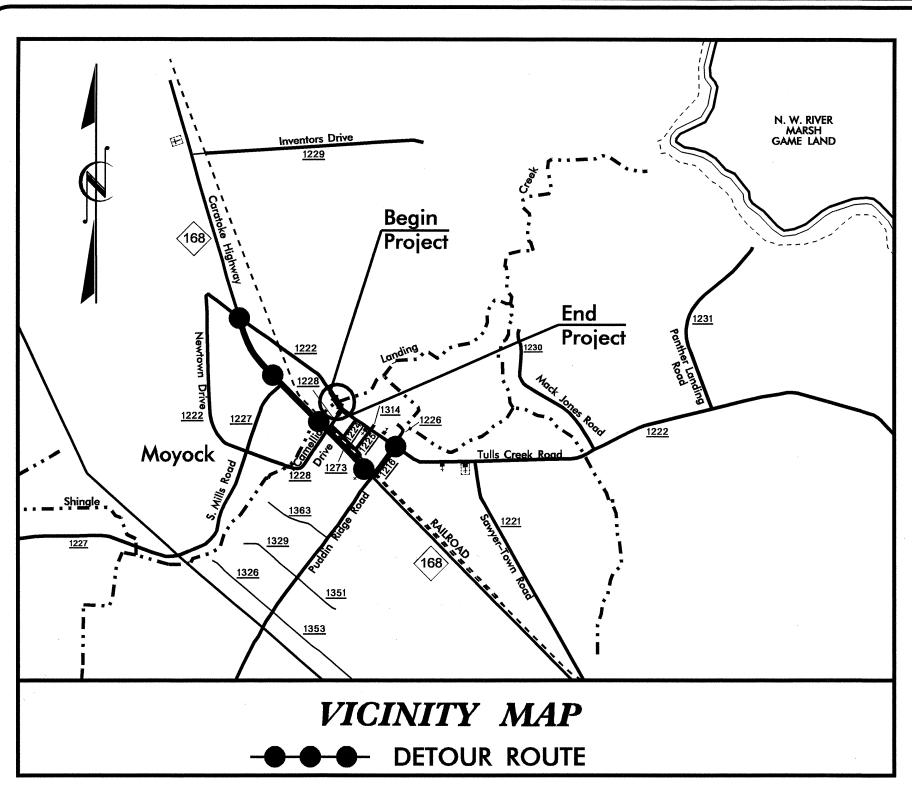
M

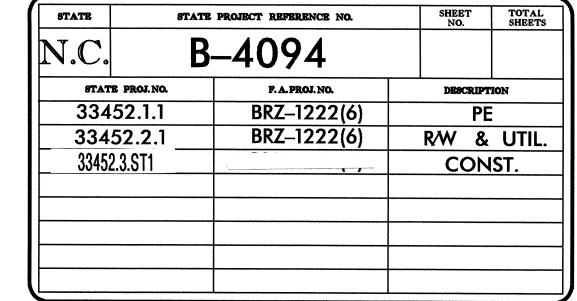


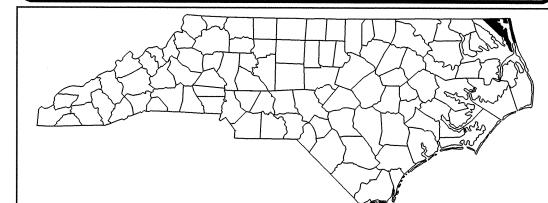
NEAREST SHIPPING POINT: MOYOCK ON NORFOLK RR APPROX. 0.12 MILES FROM PROJECT

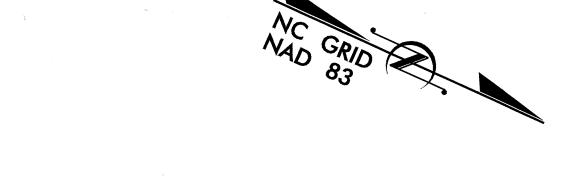
STATE OF NORTH CAROLINA DIVISION OF HIGHWAYS

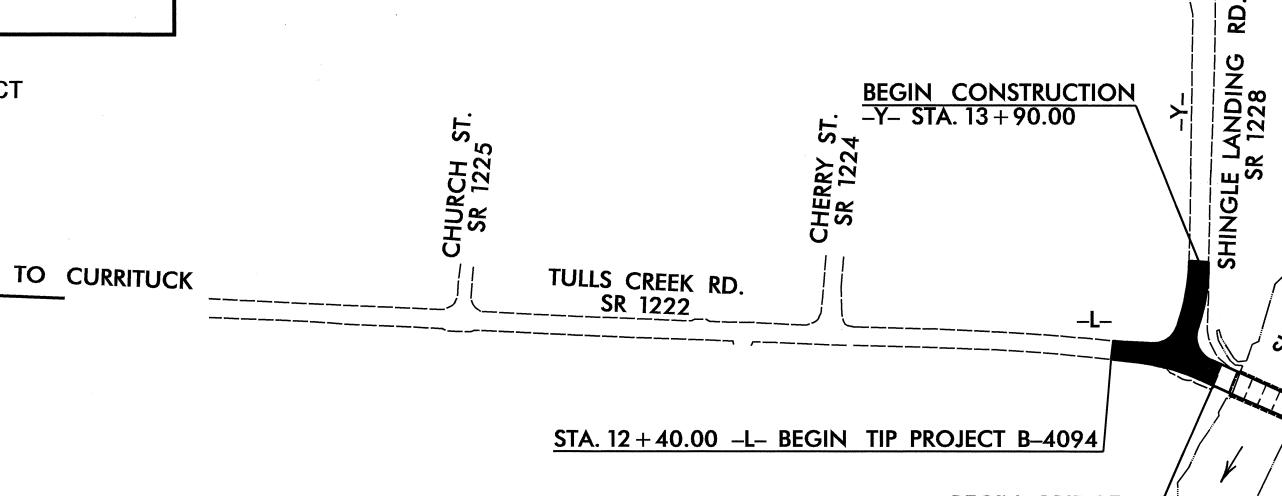
CURRITUCK COUNTY

LOCATION: BRIDGE NO. 28 OVER SHINGLE LANDING CREEK ON SR 1222 TYPE OF WORK: GRADING, PAVING, DRAINAGE AND STRUCTURE





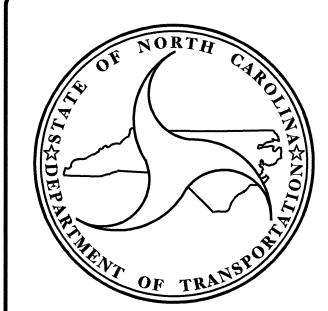




BEGIN BRIDGE -L- STA. 13 + 48.12 END BRIDGE -L- STA. 14+56.12

STRUCTURE

** Design Exception – Lane Width, Shoulder Width, Bridge Width, Sag Vertical Curve K, Crest Vertical Curve K, Vertical SSD and Horizontal Clearance.



DESIGN DATA

ADT 2009 = 2,700ADT 2030 = 5,600

DHV = 12%D = 70%

= 35 mph

Func Class = Local Rural

* Duals 3% TTST 1%

PROJECT LENGTH

LENGTH OF ROADWAY TIP PROJECT B-4094 = 0.040 MI.

LENGTH OF STRUCTURE TIP PROJECT B-4094 = 0.021 MI.

TOTAL LENGTH OF TIP PROJECT B-4094 = 0.061 MI.

Prepared in the Office of:

DIVISION OF HIGHWAYS 1000 BIRCH RIDGE DR. RALEIGH, NC 27610

STA. 15 + 60.00 -L- END TIP PROJECT B-4094

2006 STANDARD SPECIFICATIONS

LETTING DATE:

MARCH 17, 2009

N. N. BULLOCK, PE PROJECT ENGINEER

D. R. CALHOUN, PE PROJECT DESIGN ENGINEER

STRUCTURE DESIGN UNIT

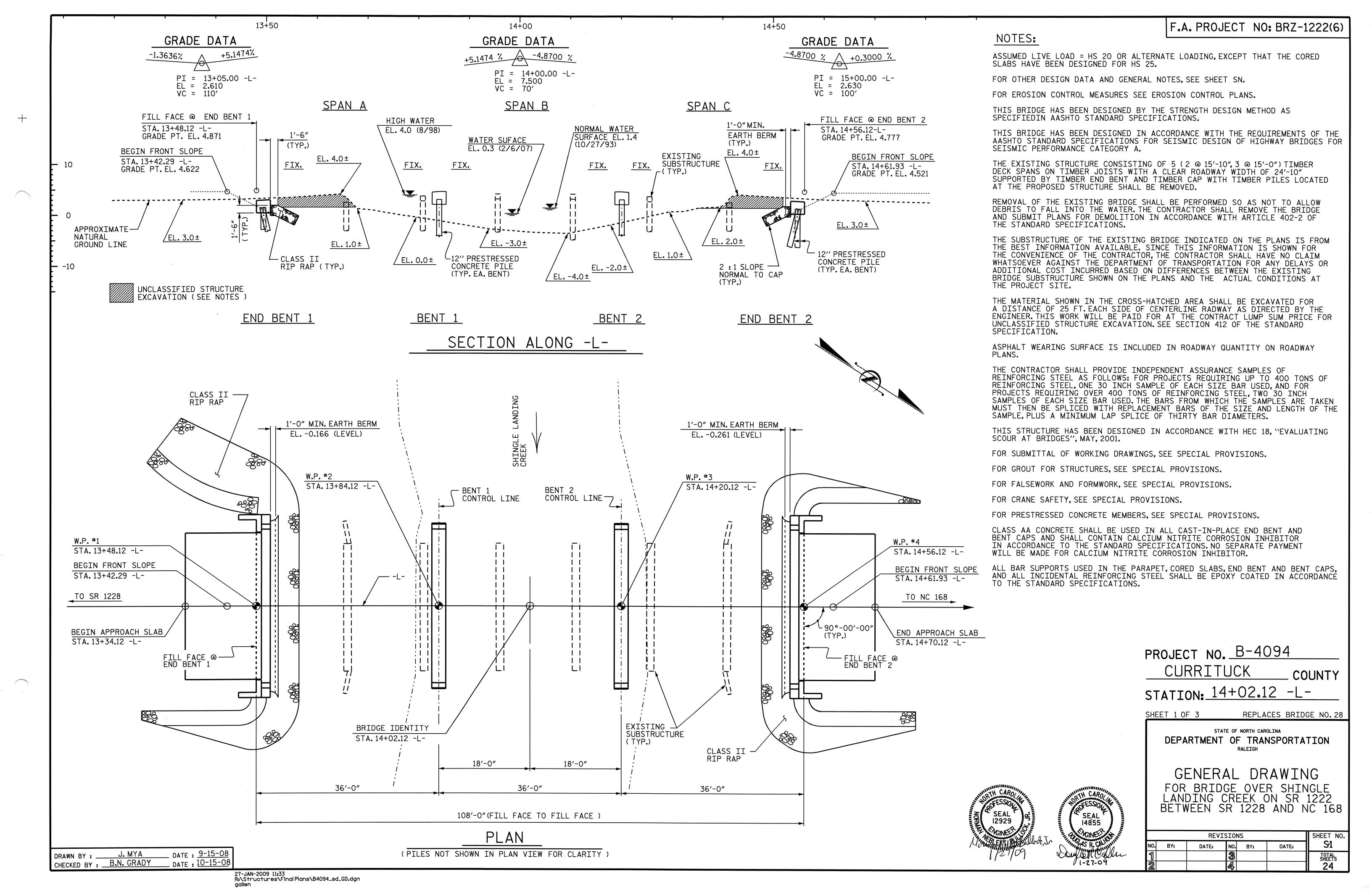
DIVISION OF HIGHWAYS STATE OF NORTH CAROLINA

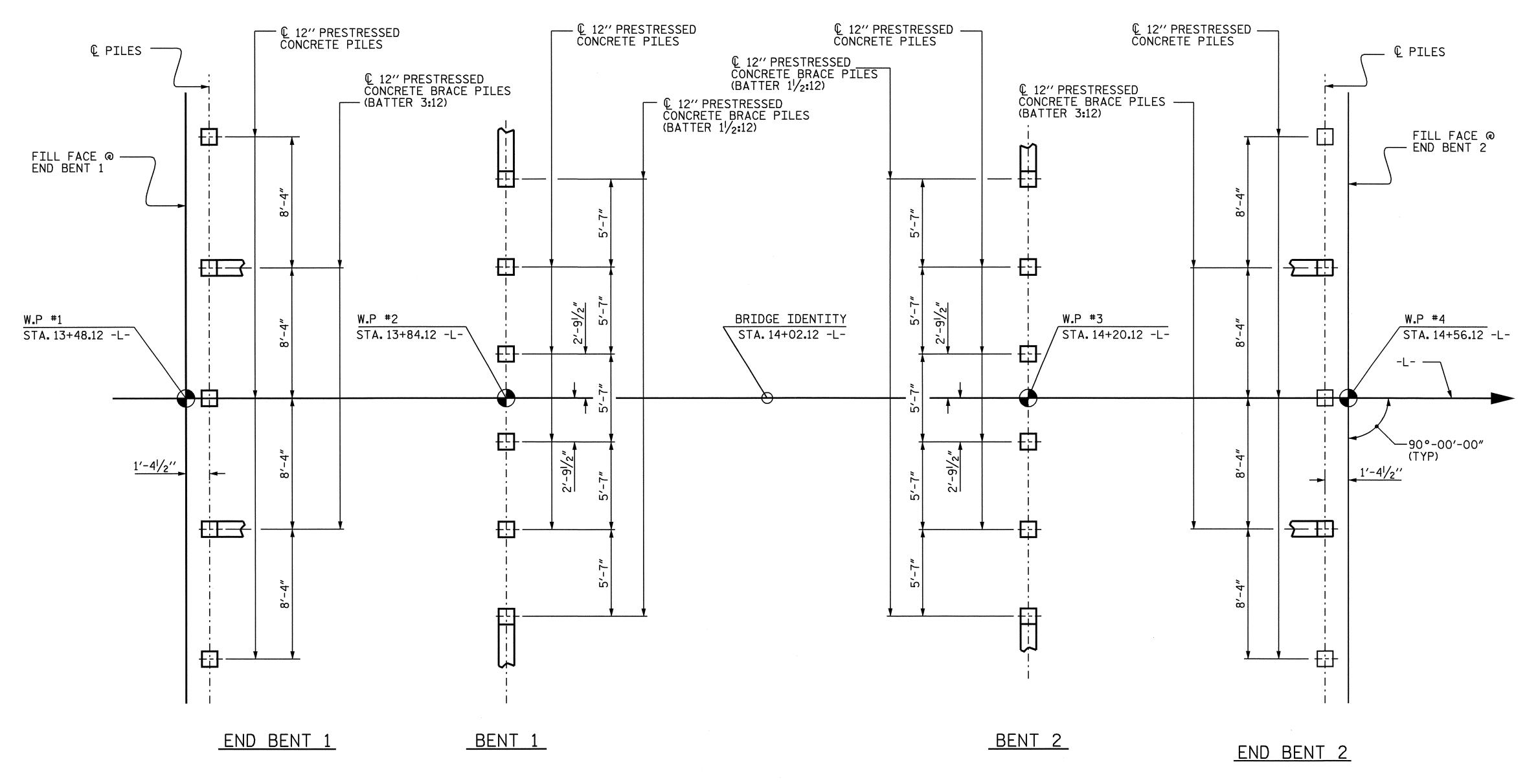
STATE DESIGN ENGINEER

DEPARTMENT OF TRANSPORTATION FEDERAL HIGHWAY ADMINISTRATION

DATE

APPROVED
DIVISION ADMINISTRATOR





FOUNDATION LAYOUT

(DIMENSIONS LOCATING PILES ARE SHOWN TO CENTERLINE OF PILES)

FOUNDATION NOTES:

PILE RESTRIKES FOR LRFD ARE REQUIRED FOR THE FIRST PRODUCTION PILE TESTED WITH THE PILE DRIVING ANALYZER (PDA). SEE PILE RESTRIKES FOR LRFD SPECIAL PROVISION.

TESTING THE FIRST PRODUCTION PILE WITH THE PILE DRIVING ANALYZER DURING DRIVING, RESTRIKING OR REDRIVING IS REQUIRED. SEE PILE DRIVING ANALYZER SPECIAL PROVISION.

DRIVE PILES AT END BENT 1 AND 2 TO A REQUIRED BEARING CAPACITY OF 100 TONS PER PILE. THE REQUIRED BEARING CAPACITY IS EQUAL TO THE ALLOWABLE BEARING CAPACITY WITH A MINIMUM FACTOR OF SAFETY OF TWO.

THE ALLOWABLE BEARING CAPACITY FOR PILES AT END BENT 1 AND 2 IS 50 TONS PER PILE.

DRIVE PILES AT BENT 1 AND 2 TO A REQUIRED BEARING CAPACITY OF 115 TONS PER PILE. THE REQUIRED BEARING CAPACITY IS EQUAL TO THE ALLOWABLE BEARING CAPACITY WITH A MINIMUM FACTOR OF SAFETY OF TWO PLUS ANY ADDITIONAL CAPACITY TO ACCOUNT FOR DOWN DRAG OR NEGATIVE SKIN FRICTION AND SCOUR.

THE ALLOWABLE BEARING CAPACITY FOR PILES AT BENT 1 AND 2 IS 50 TONS PER PILE.

DRIVE PILES AT BENT 1 AND 2 TO A TIP ELEVATION NO HIGHER THAN -22.0 FEET.

IT HAS BEEN ESTIMATED THAT A HAMMER WITH AN EQUIVALENT RATED ENERGY IN THE RANGE OF 20,000 TO 40,000 FT-LBS PER BLOW WILL BE REQUIRED TO DRIVE PILES AT BENT 1 AND 2. THIS ESTIMATED ENERGY RANGE DOES NOT RELEASE THE CONTRACTOR FROM ARTICLE 450-5 OF THE STANDARD SPECIFICATIONS.

THE SCOUR CRITICAL ELEVATION FOR BENT 1 AND BENT 2 IS ELEVATION -11.0 FEET. SCOUR CRITICAL ELEVATIONS ARE USED TO MONITOR POSSIBLE SCOUR PROBLEMS DURING THE LIFE OF THE STRUCTURE.



PROJECT NO. B-4094

CURRITUCK COUNTY

STATION: 14+02.12 -L-

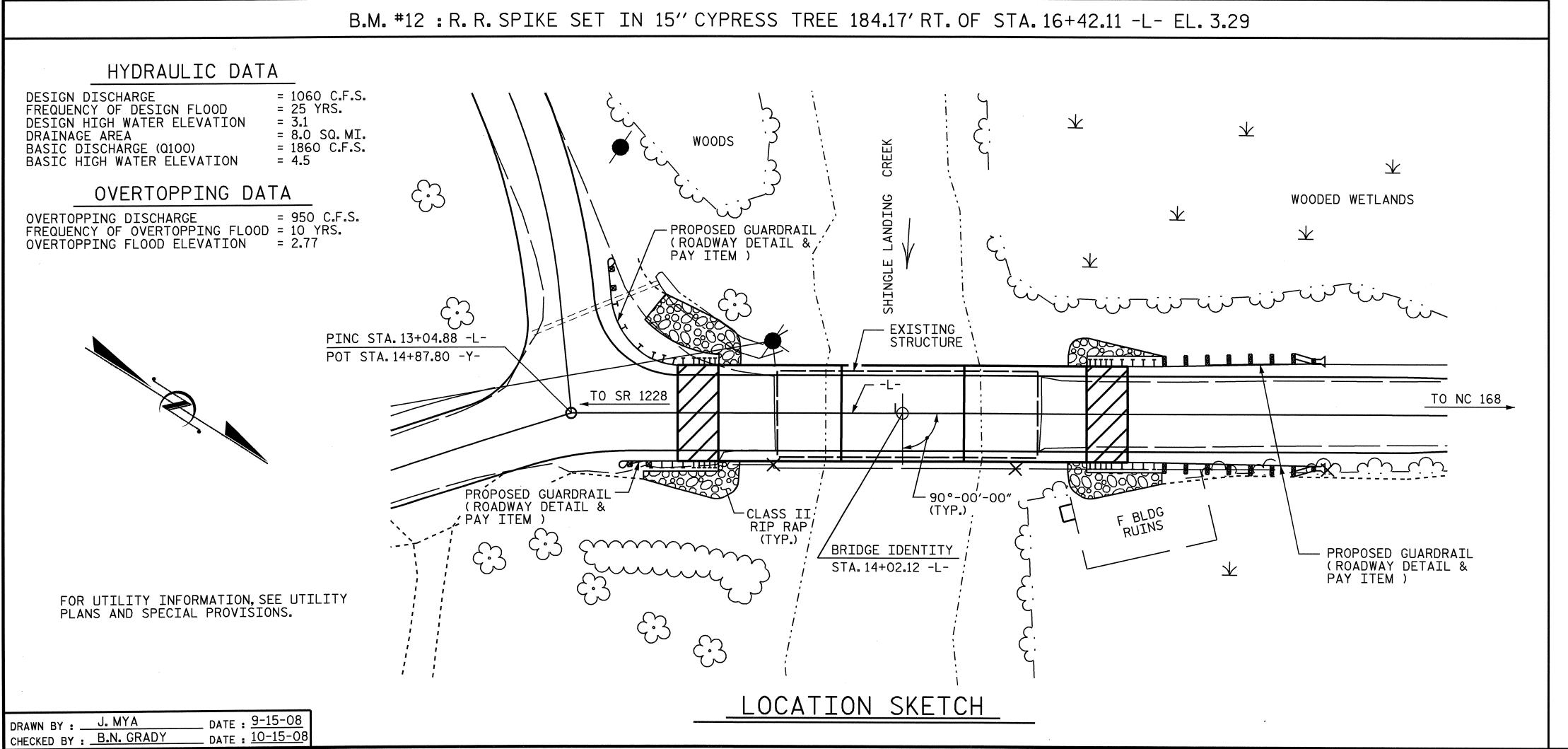
SHEET 2 OF 3

DEPARTMENT OF TRANSPORTATION

GENERAL DRAWING FOR BRIDGE OVER SHINGLE LANDING CREEK ON SR 1222 BETWEEN SR 1228 AND NC 168

		SHEET NO.				
NO.	BY:	DATE:	NO.	BY:	DATE:	S-2
1			3			TOTAL SHEETS
2			4			24

					— то	TAL B	ILL OF	MAT	ERIAL	_							
	REMOVAL OF EXISTING STRUCTURE	PDA TESTING	PDA ASSISTANCE	UNCLASSIFIED STRUCTURE EXCAVATION	CLASS AA CONCRETE	BRIDGE APPROACH SLABS	EPOXY COATED REINFORCING STEEL	12" PF CONCF	RESTRESSED RETE PILES	PILE REDRIVES	TWO BAR METAL RAIL	1'-2''X2'-8'' CONCRETE PARAPET	RIP RAP CLASS II (2'-0" THICK)	FILTER FABRIC FOR DRAINAGE	ELASTOMERIC BEARINGS	3'-0" PRES CON CORE	"X 1'-6" TRESSED NCRETE D SLABS
	LUMP SUM	EACH	EACH	LUMP SUM	CU. YDS.	LUMP SUM	LBS.	NO.	LIN.FT.	EACH	LIN.FT.	LIN.FT.	TONS	SQ. YDS.	LUMP SUM	NO.	LIN.FT.
SUPERSTRUCTURE						LUMP SUM					196.50	211.50			LUMP SUM	30	1055.00
END BENT 1				LUMP SUM	11.8		1739	5	250	4			71	79			
BENT 1					8.5		1678	6	300	4							
BENT 2					8.5		1678	6	300	4							
END BENT 2				LUMP SUM	11.8		1739	5	250	4			61	67			
TOTAL	LUMP SUM	2	2	LUMP SUM	40.6	LUMP SUM	6834	22	1100	16	196.50	211.50	132	146	LUMP SUM	30	1055.00



PROJECT NO. B-4094

CURRITUCK COUNTY

STATION: 14+02.12 -L-

SHEET 3 OF 3

STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH

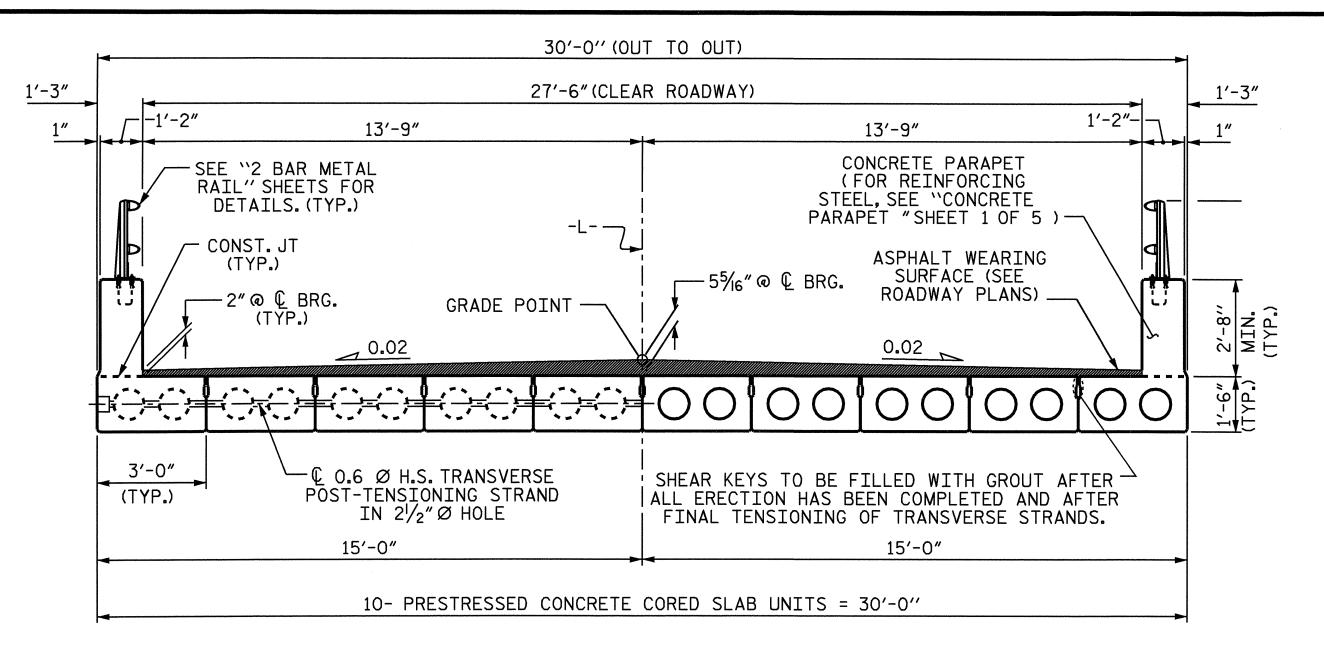
GENERAL DRAWING
FOR BRIDGE OVER SHINGLE
LANDING CREEK ON SR 1222
BETWEEN SR 1228 AND NC 168

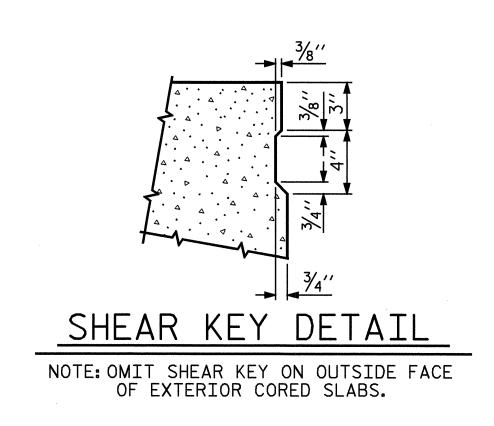
REVISIONS

BY: DATE: NO. BY: DATE:

TOTAL SHEETS
24

27-JAN-2009 12:46 R:\Structures\FinalPlans\B4094_sd_GD.dgn gallen





∕− © 2¹/₂′′ Ø DOWEL HOLES DIM. A SPAN A AND C = $8\frac{3}{8}$ " 10" Ø VOIDS-SPAN B = $8\frac{3}{4}$ " DIM. A 1'-0'' 1'-3'' 90° SKEW

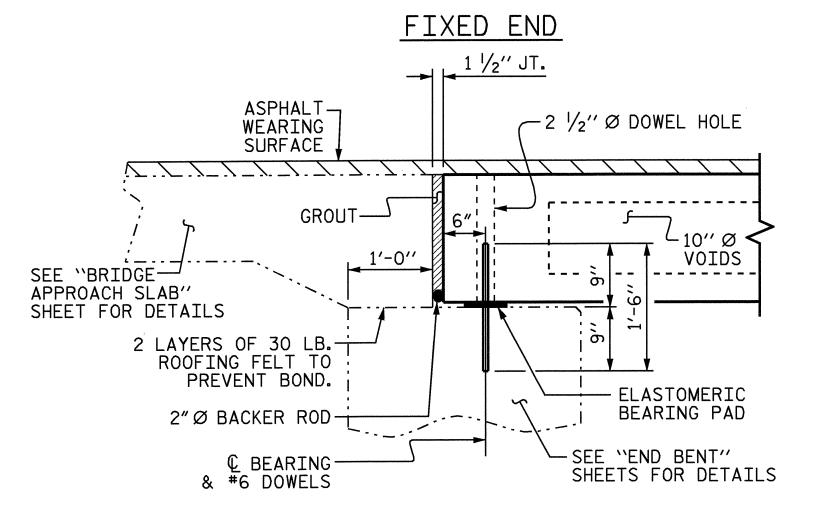
PART PLAN-EXTERIOR SECTION

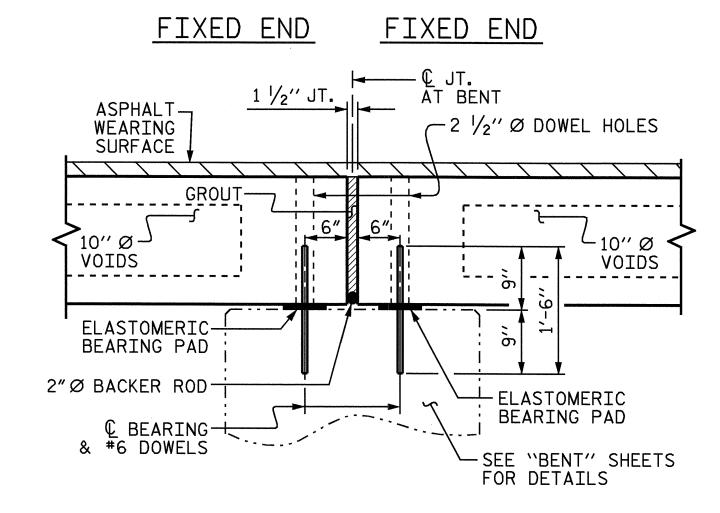
NOTE: EXTERIOR SECTION SHOWN-INTERIOR SECTION SIMILAR EXCEPT OMIT S3 BARS.

HALF SECTION @ INTERMEDIATE DIAPHRAGM

HALF SECTION @ 10" Ø VOIDS

TYPICAL SECTION



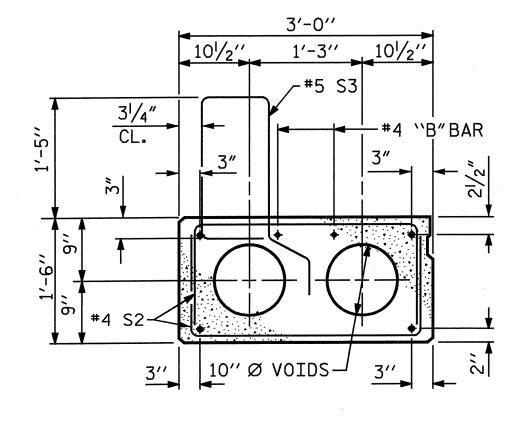


© 0.6 Ø H.S. TRANSVERSE POST-TENSIONING STRAND SHEATHED WITH A NON-CORROSIVE PIPE. --HOLE FOR TRANSVERSE STRAND -5/8" X 5" X 5" ₽ ______ -STRAND VISE -FILL RECESS WITH GROUT 4'' **ELEVATION VIEW** SECTION B-B

GROUTED RECESS AT END OF POST-TENSIONED STRAND CORED SLABS

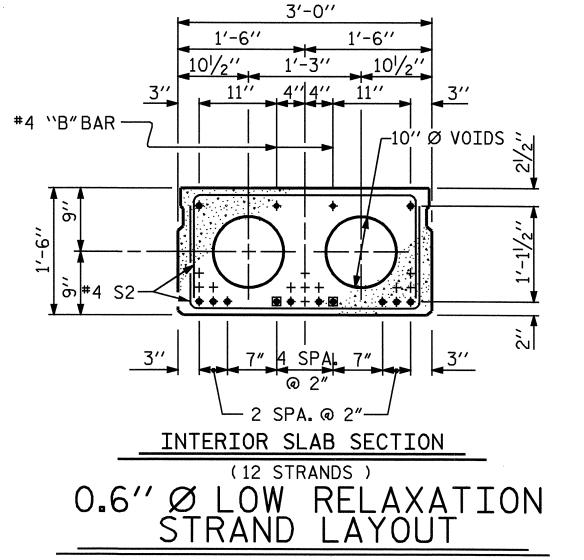
SECTION AT BENT

SECTION AT END BENT





(FOR PRESTRESSED STRAND LAYOUT, SEE INTERIOR SLAB SECTION)



SHOWING PLACEMENT OF DOUBLE STIRRUPS
AND LOCATION OF DOWEL HOLES
(STRAND LAYOUT NOT SHOWN)
INTERIOR SLAB SECTION SHOWN-EXTERIOR SLAB
SECTION SIMILAR EXCEPT SHEAR KEY LOCATION.

3'-0'' 81/2" 91/2" 91/2" 81/2" _ © 2½" Ø - DOWEL HOLES END ELEVATION

B-4094 PROJECT NO. CURRITUCK COUNTY STATION: 14+02.12 -L-

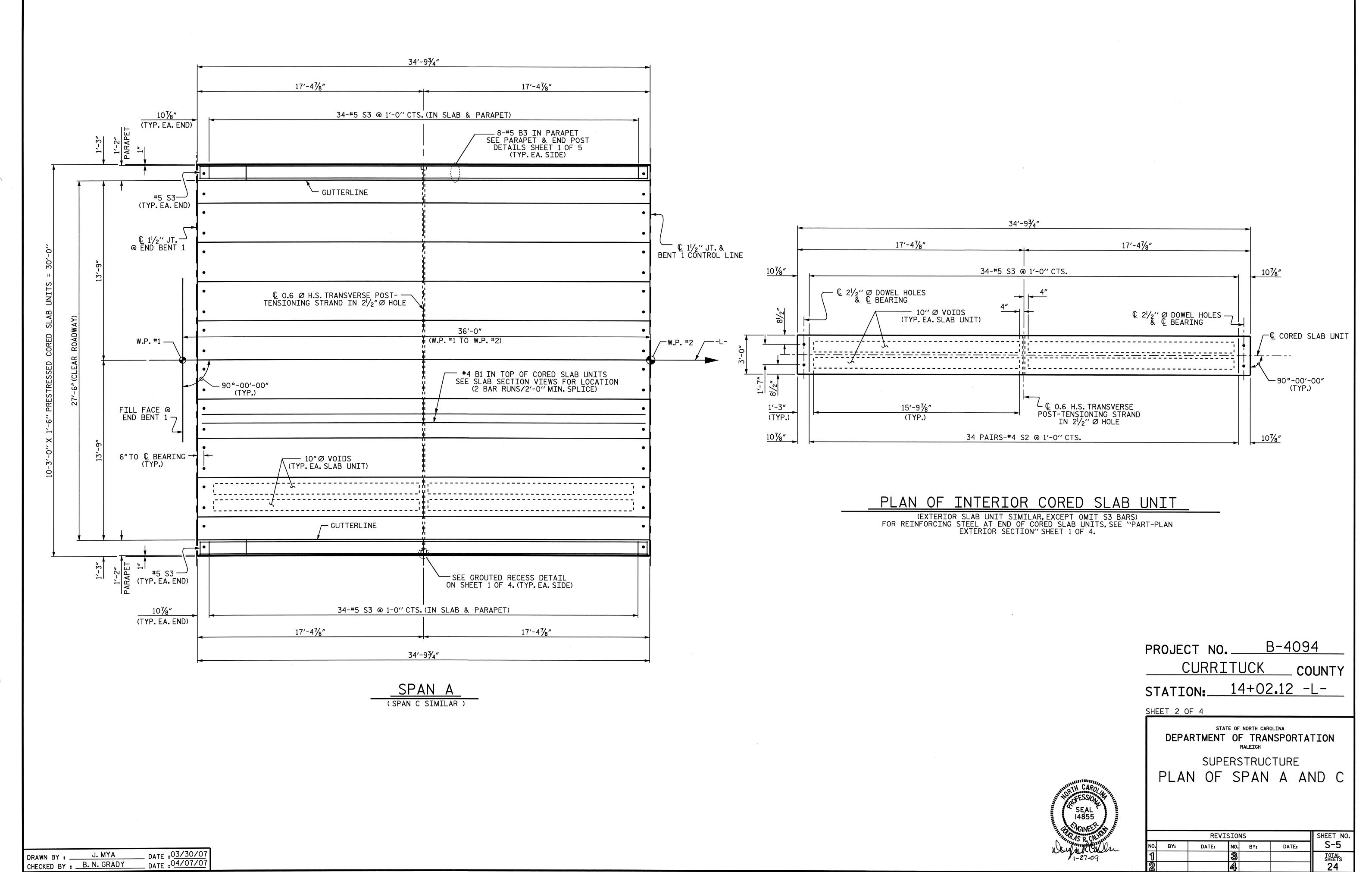
SHEET 1 OF 4

DEPARTMENT OF TRANSPORTATION STANDARD 3'-0" X 1'-6" PRESTRESSED CONCRETE CORED SLAB UNIT

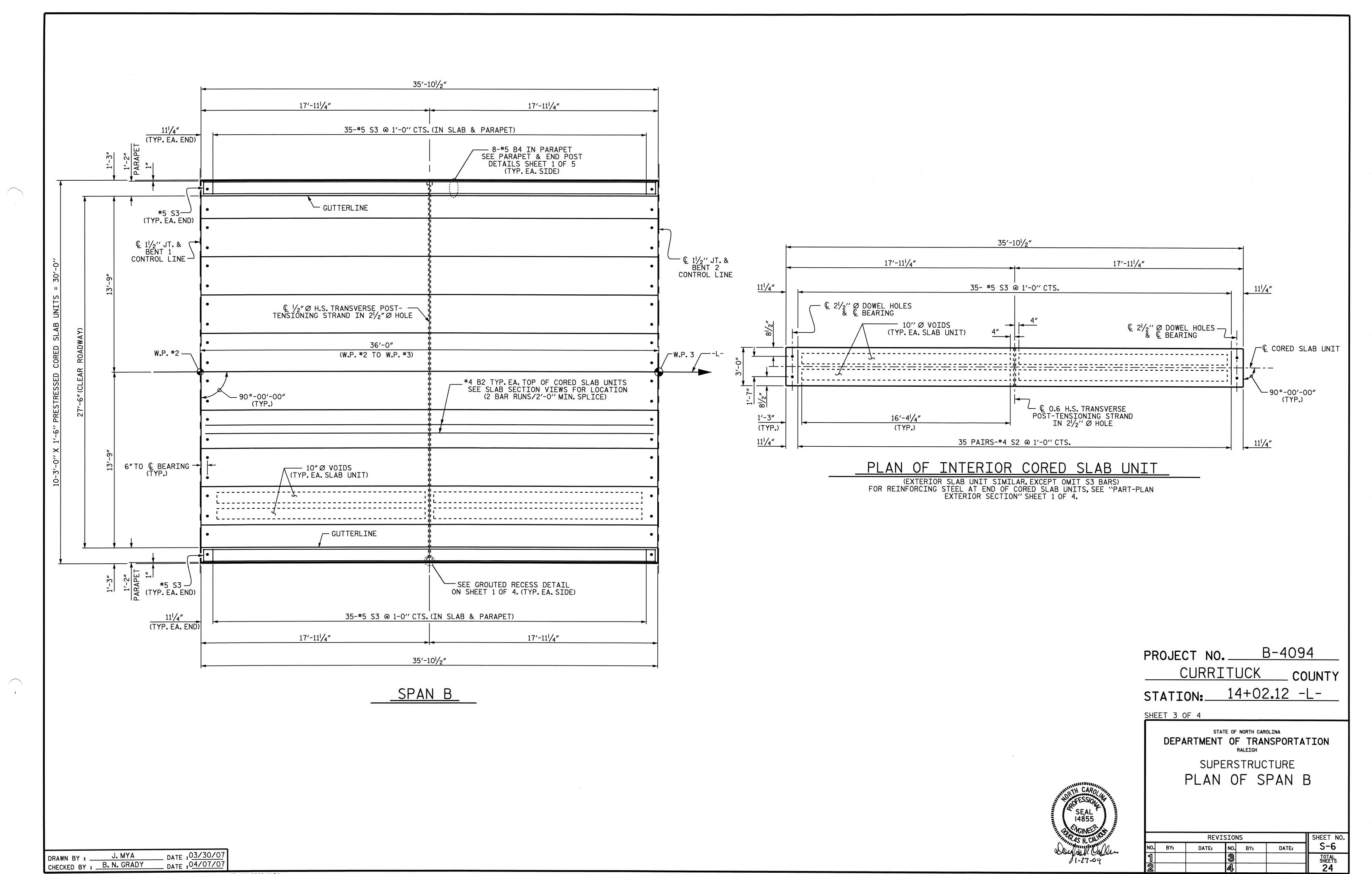
STATE OF NORTH CAROLINA

REVISIONS SHEET NO S-4 DATE: BY: TOTAL SHEETS 24

DATE : 3-30-07 DATE : 4-07-07 ASSEMBLED BY : J. MYA CHECKED BY : B.N. GRADY DRAWN BY: WJH 4/89 REV. 10/17/00 RWW/LES REV. 7/10/01RR RWW/LES REV. 5/1/06 TLA/GM BOND SHALL BE BROKEN ON THESE STRANDS FOR A DISTANCE OF 4'-O" FROM END OF CORED SLAB UNIT, SEE STANDARD SPECIFICATIONS ARTICLE 1078-7.



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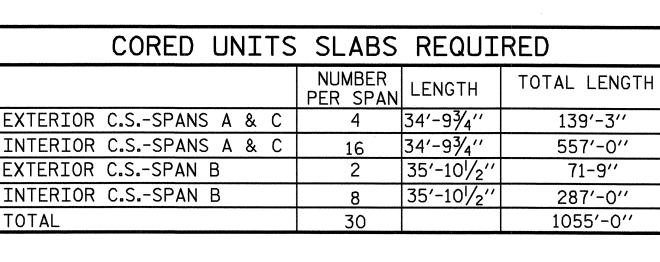


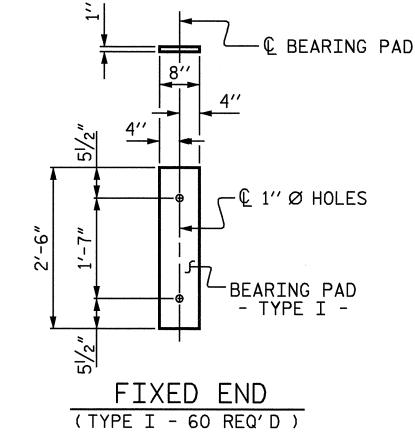
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	BILL OF MATERIAL FOR							ONE C	OKED S	PLAB	SECIT	UN	······································		
	SPAN A AND C											SPAN B			
				EXTERIO	OR UNIT	INTERI	OR UNIT					EXTERI	OR UNIT	INTERI	OR UNIT
BAR	NUMBER	SIZE	TYPE	LENGTH	WEIGHT	LENGTH	WEIGHT	BAR	NUMBER	SIZE	TYPE	LENGTH	WEIGHT	LENGTH	WEIGHT
B1	4	#4	STR	18′-3′′	49	18′-3′′	49	B2	4	#4	STR	18'-10''	50	18'-10''	50
S1	8	#4	1	3′-9′′	20	3'-9''	20	S1	8	#4	1	3'-9''	20	3'-9''	20
S2	68	#4	1	4'-10''	220	4'-10''	220	S2	70	#4	1	4'-10''	226	4'-10''	226
* S3	36	#5	2	5′-9″	216			* S3	37	#5	2	5′-9″	222		
DETLIE	DOTAL C	`TCC!			280 LBC		289 LBS.	DETNE	ODCING S	`TCCI			206 LB6		206 D6
REINFORCING STEEL 289 LBS. ** EPOXY COATED REINFORCING STEEL 216 LBS.			216 LBS.			REINFORCING STEEL 296 LBS. 296 LBS. 296 LBS.									
5000 P.S.I. CONCRETE 4.5 CU. YDS.			4.5	CU. YDS.	5000	P.S.I. CO	NCRETE		4.	7 CU. YDS.	4.	7 CU. YDS			
0.6" 2	L.R. STR	ANDS	No.		12		12	0.6″Ø	L.R. STR	ANDS	No.		12		12

DEAD LOAD DEFL	ECTION AN	ND CAMBER	
	SPAN A	SPAN B	SPAN C
CAMBER (SLAB ALONE IN PLACE)	11/8′′	11/8′′	11/8′′
DEFLECTION DUE TO SUPERIMPOSED DEAD LOAD	¹ /8′′	1/8′′	1/8′′
FINAL CAMBER	1''	1''	1′′
** INCLUDES FUTURE WEARING SURFA	CE		

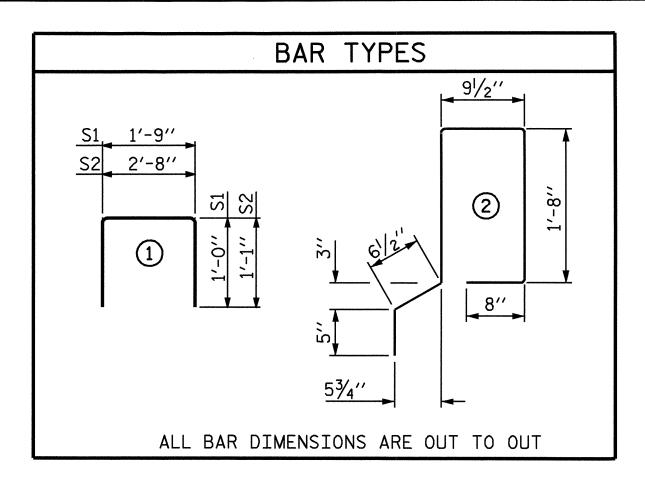
CORED UNITS	SLABS	REQUI	RED
	NUMBER PER SPAN	LENGTH	TOTAL LENGTH
EXTERIOR C.SSPANS A & C		34'-93/4''	139'-3''
INTERIOR C.SSPANS A & C	16	34'-93/4''	557′-0′′
EXTERIOR C.SSPAN B	2	35′-101/2′′	71-9''
INTERIOR C.SSPAN B	8	35′-101/2′′	287′-0′′
TOTAL	30		1055′-0′′





ELASTOMERIC BEARING DETAILS

(50 DUROMETER HARDNESS)



″Ø L.R.
C Laita
0.217
3,600
, 950

NOTES

ALL PRESTRESSING STRANDS SHALL BE 7-WIRE LOW RELAXATION GRADE 270 STRANDS AND SHALL CONFORM TO AASHTO M203 EXCEPT FOR SAMPLING REQUIREMENTS WHICH SHALL BE IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

ALL REINFORCING STEEL CAST WITH THE CORED SLAB SECTIONS SHALL BE GRADE 60 AND SHALL BE INCLUDED IN THE UNIT PRICE BID FOR PRESTRESSED CONCRETE CORED SLABS.

RECESSES FOR TRANSVERSE STRANDS SHALL BE GROUTED AFTER THE TENSIONING OF THE STRANDS.

THE 21/2" Ø DOWEL HOLES AT FIXED ENDS OF SLAB SECTIONS SHALL BE FILLED WITH GROUT.

THE 2"Ø BACKER ROD SHALL CONFORM TO THE REQUIREMENTS OF TYPE M BOND BREAKER. SEE SECTION 1028 OF THE STANDARD SPECIFICATIONS.

WHEN CORED SLABS ARE CAST, A POSITIVE HOLD-DOWN SYSTEM SHALL BE EMPLOYED TO PREVENT VOIDS FROM RISING OR MOVING SIDEWAYS. THIS SYSTEM SHALL BE DESIGNED TO BE LEFT IN PLACE UNTIL THE CONCRETE HAS REACHED RELEASE STRENGTH. AT LEAST THREE WEEKS PRIOR TO CASTING CORED SLABS, THE CONTRACTOR SHALL SUBMIT TO THE ENGINEER FOR REVIEW AND COMMENT, DETAILED DRAWINGS OF THE PROPOSED HOLD-DOWN SYSTEM. IN ADDITION TO STRUCTURAL DETAILS, LOCATION AND SPACING OF THE HOLD-DOWNS SHALL BE INDICATED.

THE TRANSFER OF LOAD FROM THE ANCHORAGES TO THE CORED SLAB UNIT SHALL BE DONE WHEN THE CONCRETE HAS REACHED A COMPRESSIVE STRENGTH OF NOT LESS THAN 4000 PSI.

ALL REINFORCING STEEL IN PARAPETS AND END POSTS SHALL BE EPOXY COATED.

PRESTRESSING STRANDS SHALL BE CUT FLUSH WITH THE CORED SLAB UNIT

APPLY EPOXY PROTECTIVE COATING TO CORED SLAB UNIT ENDS.

VERTICAL GROOVED CONTRACTION JOINTS, 1/2" IN DEPTH, SHALL BE TOOLED IN ALL EXPOSED FACES OF THE PARAPET AND IN ACCORDANCE WITH ARTICLE 825-10(B) OF THE STANDARD SPECIFICATIONS. A VERTICAL CONTRACTION JOINT SHALL BE LOCATED AT EACH THIRD POINT BETWEEN BARRIER RAIL EXPANSION JOINTS. ONLY ONE CONTRACTION JOINT IS REQUIRED AT MIDPOINT OF BARRIER RAIL SEGMENTS LESS THAN 20 FEET IN LENGTH AND NO CONTRACTION JOINTS ARE REQUIRED FOR THOSE SEGMENTS LESS THAN 10 FEET IN LENGTH.

PRESTRESSED CONCRETE CORED SLAB UNITS ARE DESIGNED FOR O PSI TENSION IN THE PRECOMPRESSED TENSILE ZONE UNDER ALL LOADING CONDITIONS.

PRESTRESSED CONCRETE CORED SLAB UNITS SHALL CONTAIN CALCIUM NITRITE CORROSION INHIBITOR IN ACCORDANCE TO THE STANDARD SPECIFICATIONS. NO SEPARATE PAYMENT WILL BE MADE FOR CALCIUM NITRITE CORROSION INHIBITOR.

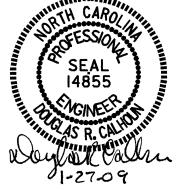
FOR PRESTRESSED CONCRETE MEMBERS, SEE SPECIAL PROVISIONS.

PROJECT NO. B-4094 CURRITUCK COUNTY STATION: 14+02.12 -L-

SHEET 4 OF 4

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION STANDARD

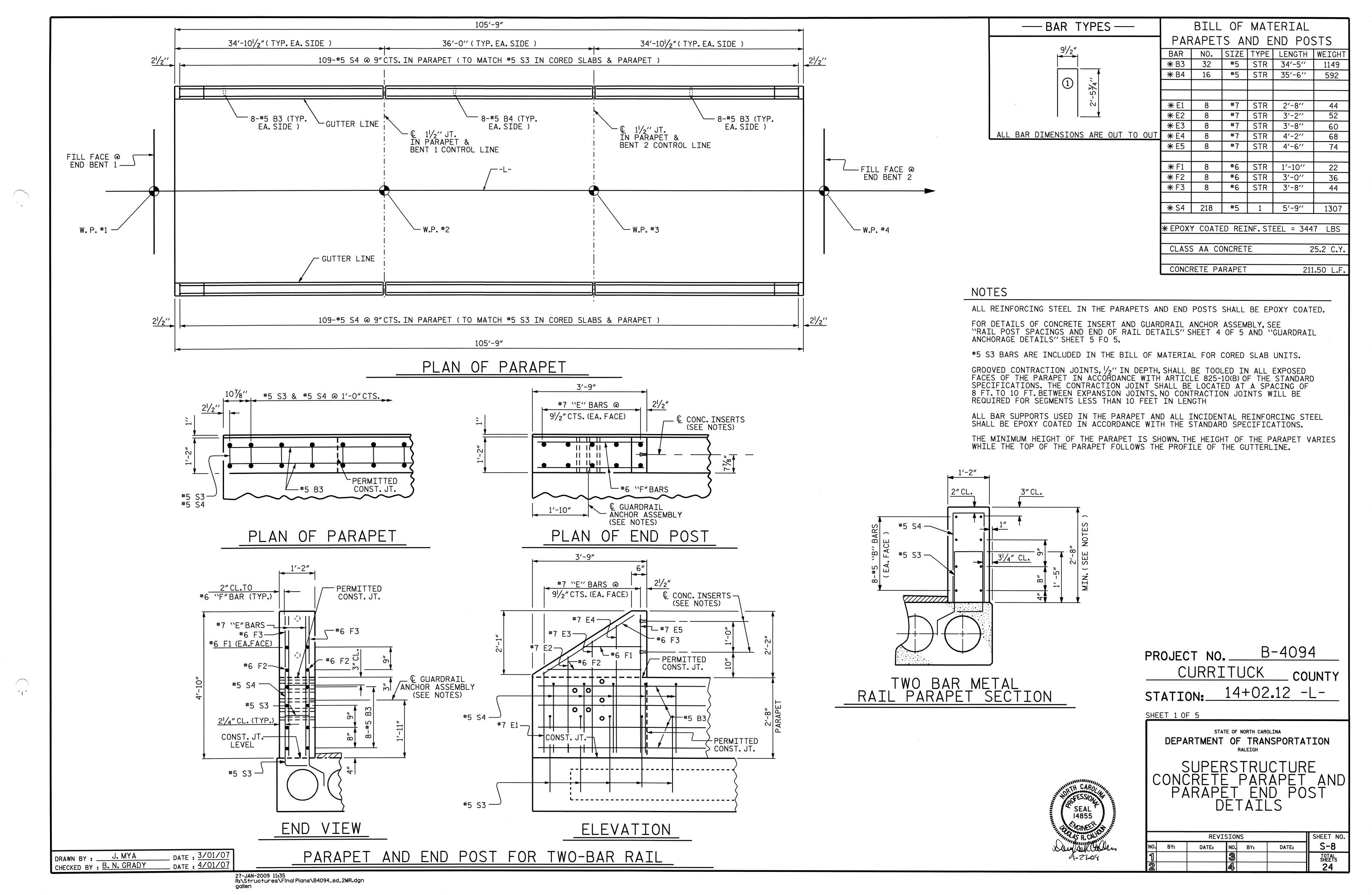
3'-0" X 1'-6"
PRESTRESSED CONCRETE
CORED SLAB UNIT

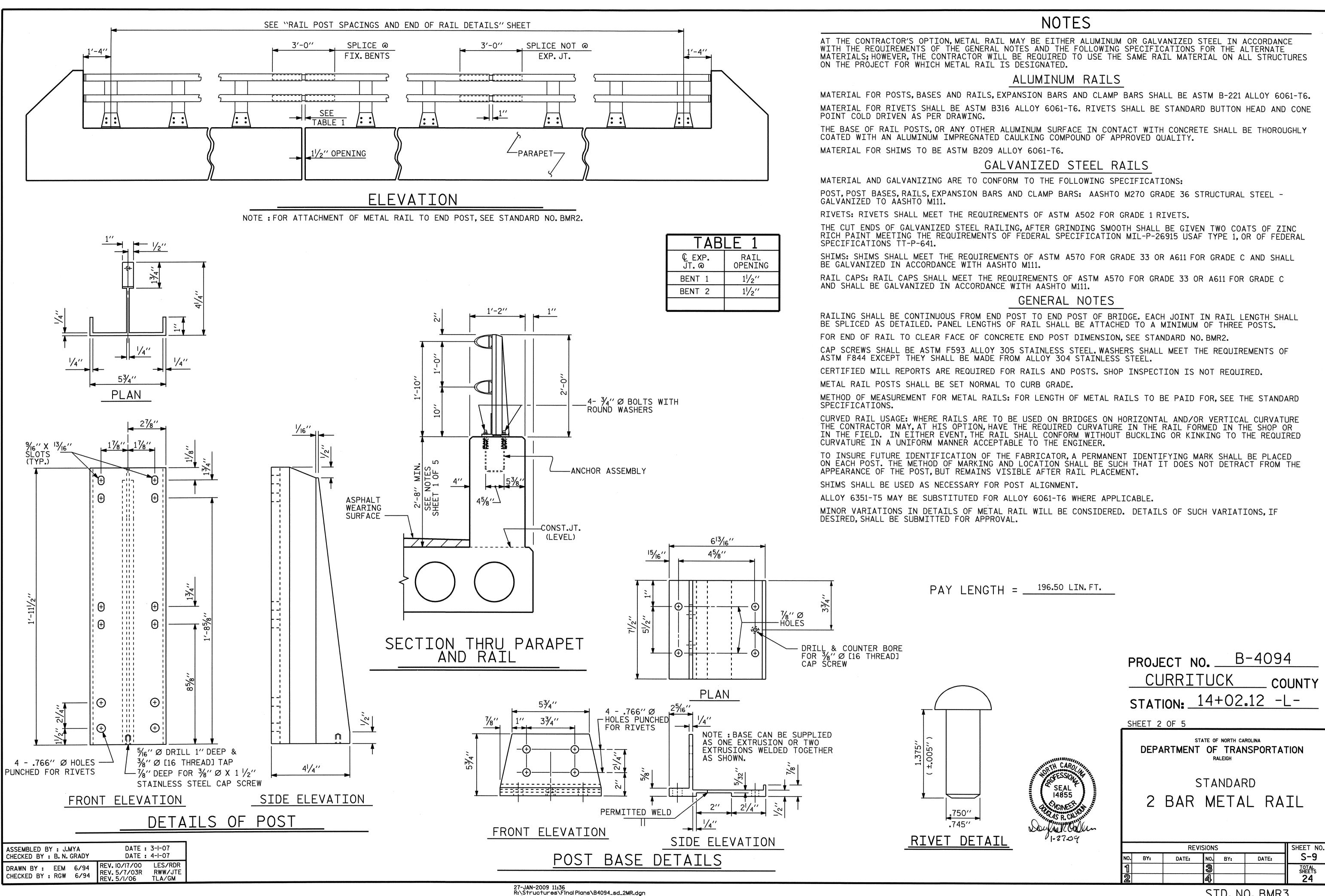


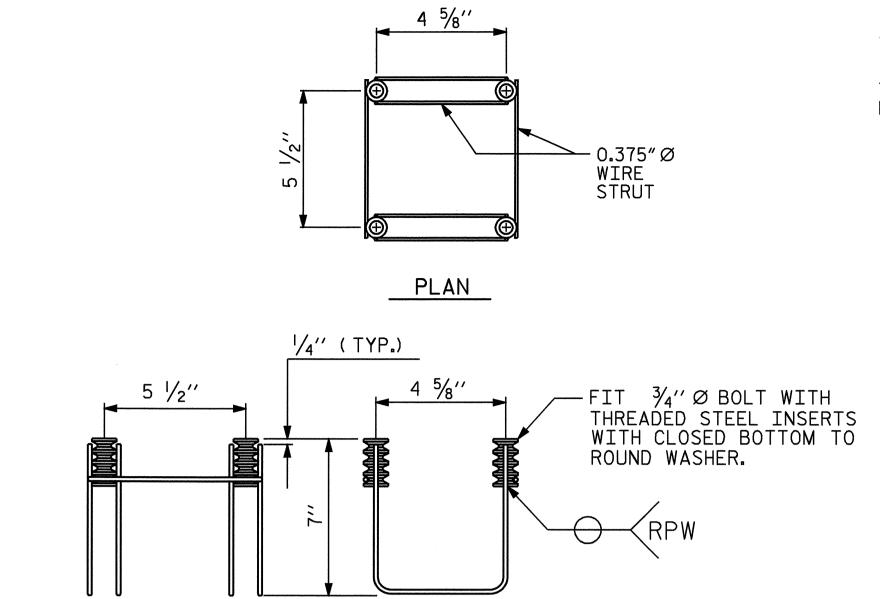


DATE : 03/30/07 DATE : 04/07/07 ASSEMBLED BY : J.MYA CHECKED BY : B. N. GRADY REV. 7/10/01 RWW/LES REV. 5/7/03RRR RWW/JTI REV. 5/1/06 TLA/GM DRAWN BY: WJH 4/89 CHECKED BY: FCJ 5/89

SHEET NO. REVISIONS S-7 BY: DATE:







ELEVATION

MINIMUM LENGTH OF THREADS IN INSERT (FERRULE): $1\frac{3}{4}$ "

4-BOLT METAL RAIL ANCHOR ASSEMBLY

SIDE VIEW

(36 ASSEMBLIES REQUIRED



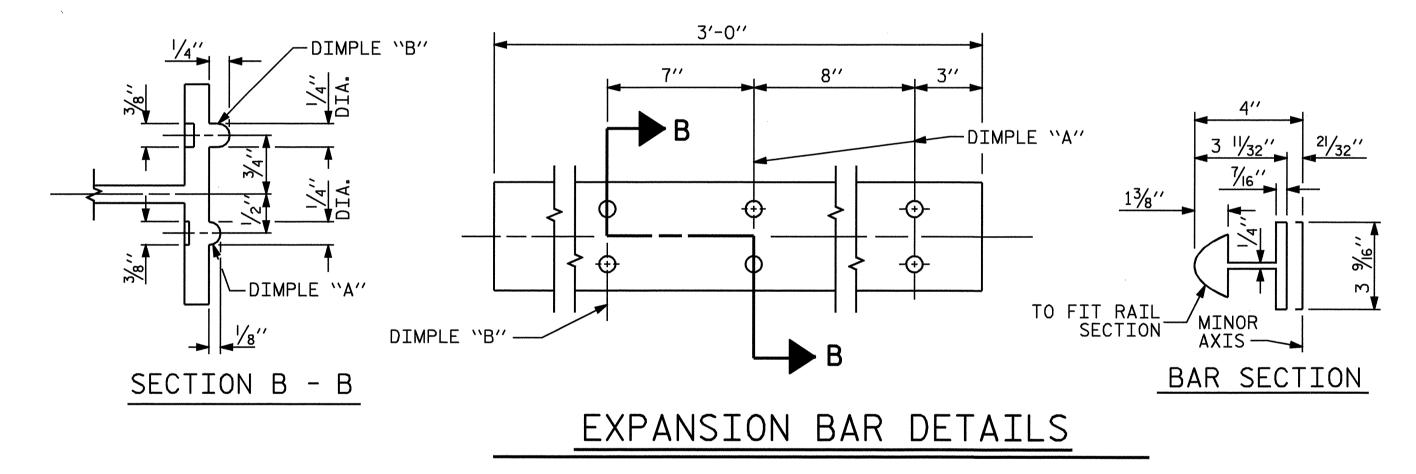
STRUCTURAL CONCRETE ANCHOR ASSEMBLY

THE STRUCTURAL CONCRETE ANCHOR ASSEMBLY SHALL CONSIST OF THE FOLLOWING COMPONENTS:

- A. FERRULES SHALL BE MADE FROM STEEL MEETING THE REQUIREMENTS OF AASHTO M169, GRADE 12L14 AND SHALL HAVE A MINIMUM LENGTH OF THREADS OF 2" FOR 3/4" FERRULES.
- B. 4 $\frac{3}{4}$ " Ø X 2 $\frac{1}{2}$ " BOLTS WITH WASHERS. BOLTS SHALL CONFORM TO THE REQUIREMENTS OF ASTM A307. BOLTS AND WASHERS SHALL BE GALVANIZED. AT THE CONTRACTOR'S OPTION, STAINLESS STEEL BOLTS AND WASHERS MAY BE USED AS AN ALTERNATE FOR THE 1/4" Ø X 21/2" GALVANIZED BOLTS AND WASHERS. THEY SHALL CONFORM TO OR EXCEED THE MECHANICAL REQUIREMENTS OF ASTM A307. THE USE OF THIS ALTERNATE SHALL BE APPROVED BY THE ENGINEER.
- C. WIRE STRUT SHOWN IN THE CONCRETE ANCHOR ASSEMBLY DETAIL IS THE MINIMUM ALLOWABLE SIZE AND SHALL HAVE A MINIMUM TENSILE STRENGTH OF 100,000 PSI. AS AN OPTION, A $7_{16}^{\prime\prime}$ Ø WIRE STRUT WITH A MINIMUM TENSILE STRENGTH OF 90,000 PSI IS ACCEPTABLE.
- D. THE METAL RAIL ANCHOR ASSEMBLIES TO BE HOT DIPPED GALVANIZED TO CONFORM TO REQUIREMENTS OF AASHTO M111.
- E. THE COST OF THE METAL RAIL ANCHOR ASSEMBLY WITH BOLTS AND WASHERS COMPLETE IN PLACE SHALL BE INCLUDED IN THE PRICE BID FOR LINEAR FEET OF METAL RAIL.
- F. BOLTS TO BE TIGHTENED ONE-HALF TURN WITH A WRENCH FROM A FINGER-TIGHT POSITION.

THE CONTRACTOR MAY USE ADHESIVELY ANCHORED ANCHOR BOLTS IN PLACE OF THE METAL RAIL ANCHOR ASSEMBLY. LEVEL ONE FIELD TESTING IS REQUIRED, AND THE YIELD LOAD OF THE 3/4" Ø BOLT IS 10 KIPS. FOR ADHESIVELY ANCHORED ANCHOR BOLTS OR DOWELS, SEE SPECIAL PROVISIONS.

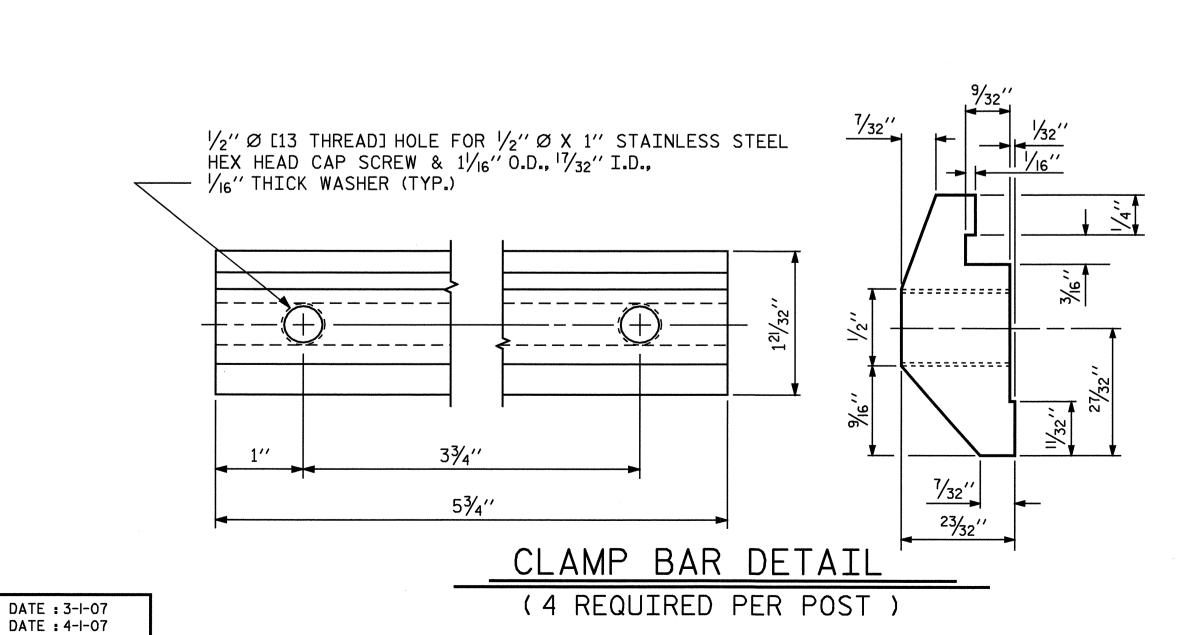
WHEN ADHESIVELY ANCHORED ANCHOR BOLTS ARE USED, BOLTS SHALL MEET THE REQUIREMENTS OF ASTM F593 ALLOY 304 STAINLESS STEEL WITH MINIMUM 75,000 PSI ULTIMATE STRENGTH. NUTS SHALL MEET THE REQUIREMENTS OF ASTM F594 ALLOY 304 STAINLESS STEEL AND WASHERS SHALL MEET THE REQUIREMENTS OF ASTM F844 EXCEPT THEY SHALL BE MADE FROM ALLOY 304 STAINLESS STEEL.

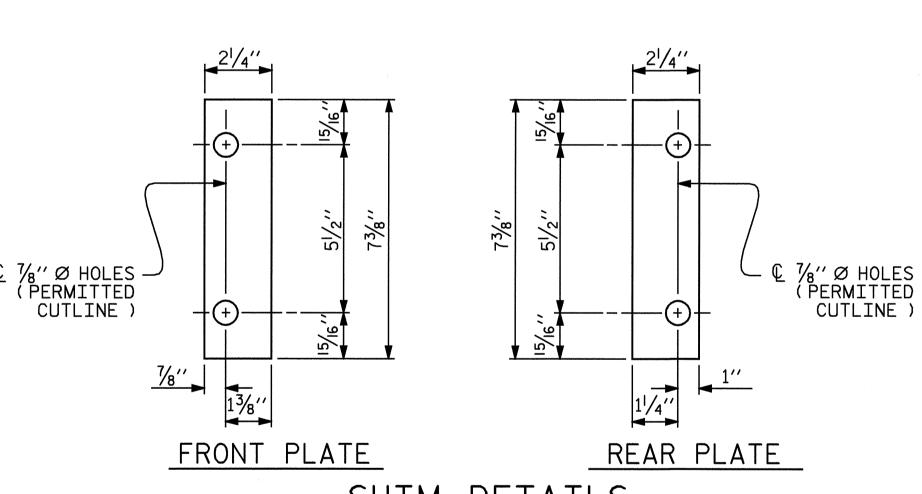


ASSEMBLED BY : J. MYA CHECKED BY : B.N. GRADY

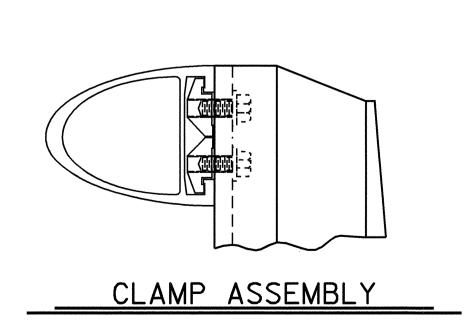
DRAWN BY: EEM 6/94 CHECKED BY : RGW 6/94

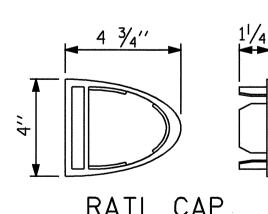
REV. 2/6/97 EEM/RGW REV. 8/16/99 MAB/LES REV. 5/1/06R KMM/GM



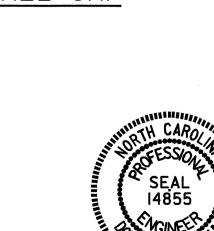


SHIM DETAILS NOTE:
SHIMS MAY BE CUT ALONG PERMITTED CUTLINE OR
SLOTTED TO EDGE OF PLATE TO FACILITATE PLACEMENT.





RAIL CAP



PROJECT NO. B-4094 CURRITUCK ___ COUNTY STATION: 14+02.12-L-

MINOR AXIS

RAIL SECTION

- SEMI-ELLIPSE

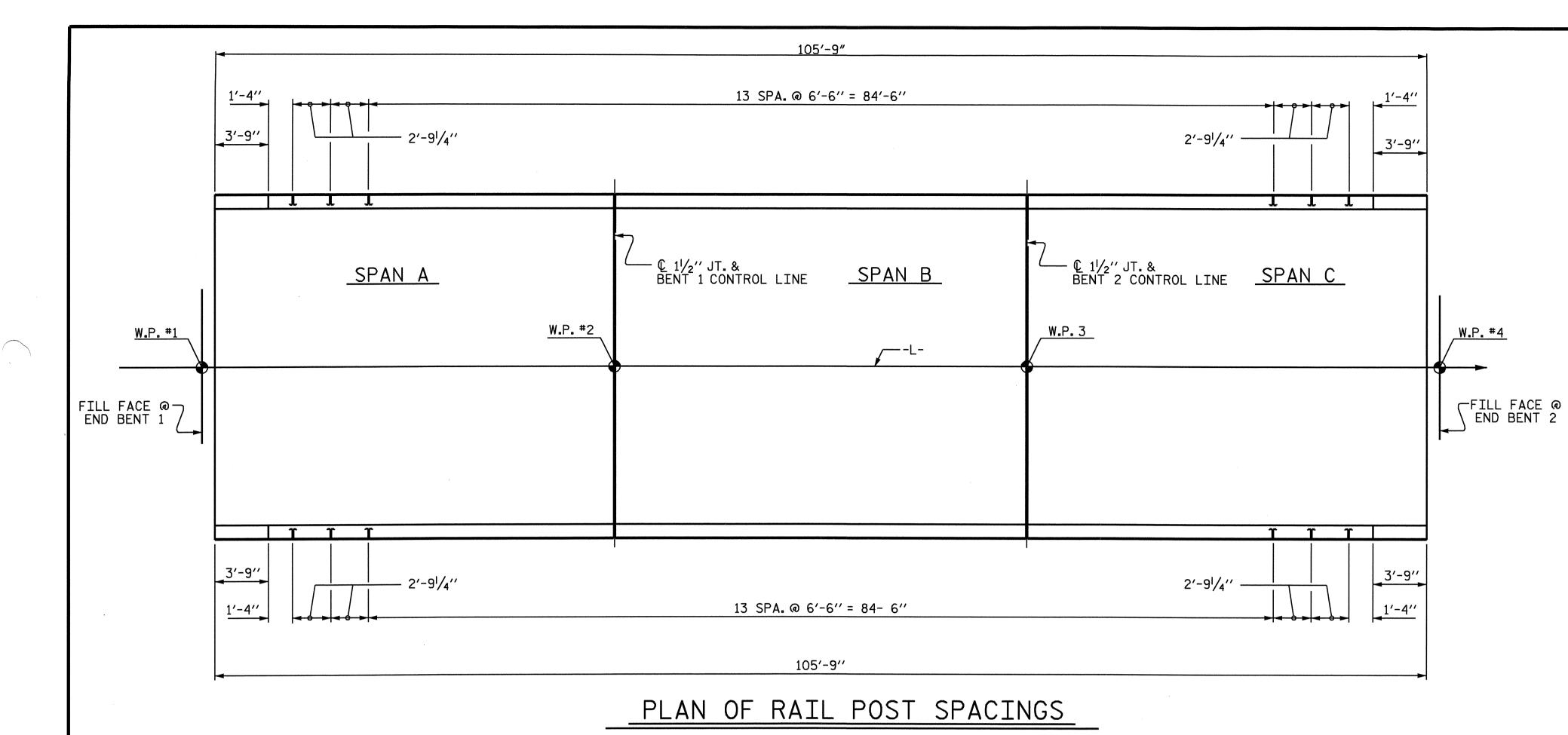
MAJOR AXIS

SHEET 3 OF 5

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION STANDARD

2 BAR METAL RAIL

		SHEET NO.				
NO.	BY:	DATE:	NO.	BY:	DATE:	S-I0
1			3			TOTAL SHEETS
2			4			24





STRUCTURAL CONCRETE INSERT

THE STRUCTURAL CONCRETE INSERT ASSEMBLY SHALL CONSIST OF THE FOLLOWING COMPONENTS:

- A. FERRULES SHALL BE MADE FROM STEEL MEETING THE REQUIREMENTS OF AASHTO M169, GRADE 12L14 AND SHALL HAVE A MINIMUM LENGTH OF THREADS OF $1\frac{1}{2}$ ".
- B. 1 $\frac{3}{4}$ " Ø X 1 $\frac{5}{8}$ " BOLT WITH WASHER. BOLT SHALL CONFORM TO THE REQUIREMENTS OF ASTM A307. BOLT AND WASHER SHALL BE GALVANIZED. (AT THE CONTRACTOR'S OPTION, STAINLESS STEEL BOLT AND WASHER MAY BE USED AS AN ALTERNATE FOR THE 3/4" Ø X 15/8" GALVANIZED BOLT AND WASHER. THEY SHALL CONFORM TO OR EXCEED THE MECHANICAL REQUIREMENTS OF ASTM A307. THE USE OF THIS ALTERNATE SHALL BE APPROVED BY THE ENGINEER.)
- C. WIRE STRUT SHOWN IN THE CONCRETE INSERT ASSEMBLY DETAIL IS THE MINIMUM ALLOWABLE SIZE AND SHALL HAVE A MINIMUM TENSILE STRENGTH OF 100,000 PSI. AS AN OPTION, A 7/6" Ø WIRE STRUT WITH A MINIMUM TENSILE STRENGTH OF 90,000 PSI IS ACCEPTABLE.

NOTES

METAL RAIL TO END POST CONNECTION

THE METAL RAIL TO END POST CONNECTION SHALL CONSIST OF THE FOLLOWING COMPONENTS:

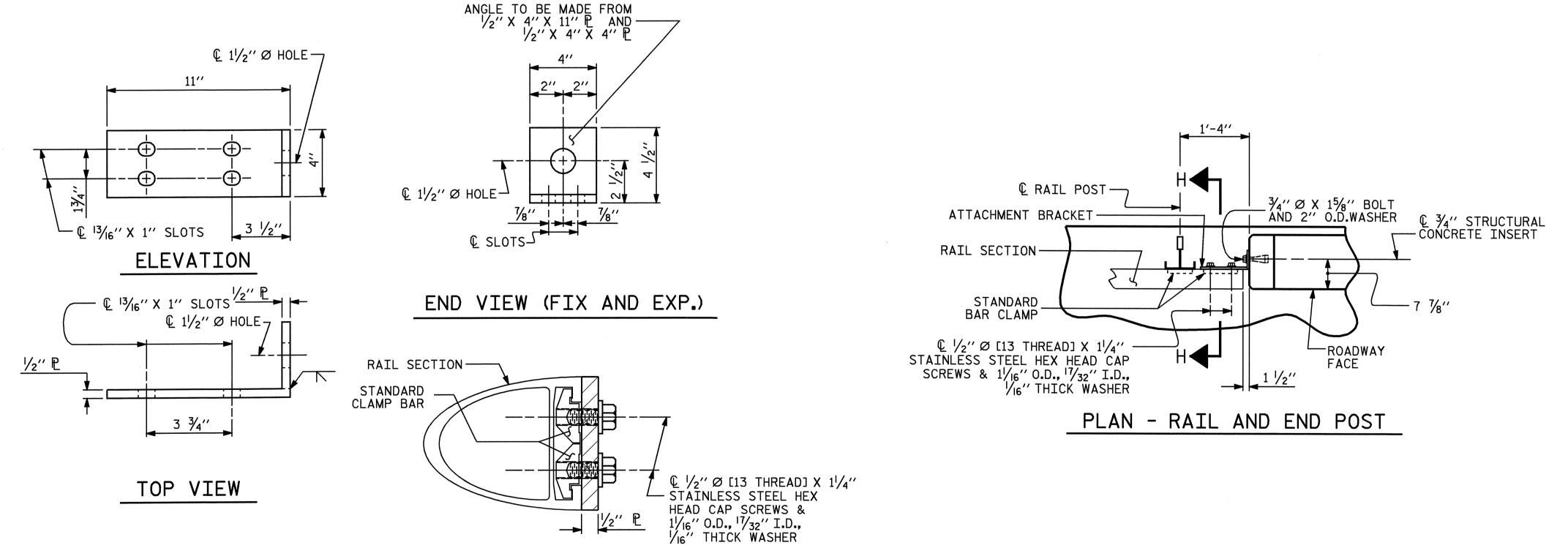
- A. $\frac{1}{2}$ " PLATES SHALL CONFORM TO AASHTO M270 GRADE 36 AND SHALL BE GALVANIZED AFTER FABRICATION.
- B. 3/4" STRUCTURAL CONCRETE INSERT SHALL HAVE A WORKING LOAD SHEAR CAPACITY OF 4800 LBS. THE FERRULES SHALL ENGAGE A 34" X 158" BOLT WITH 2" O.D. WASHER IN PLACE. THE 34" X 158" BOLT SHALL HAVE N. C. THREADS.
- C. CAP SCREWS FOR RAIL ATTACHMENT TO ANGLE SHALL CONFORM TO THE REQUIREMENTS OF ASTM F593 ALLOY 305 STAINLESS STEEL. CAP SCREWS TO BE CENTERED IN SLOTS AT 60°F.
- D. STANDARD CLAMP BARS (SEE METAL RAIL SHEET).
- E. $\frac{1}{2}$ " Ø PIPE SLEEVES (IF REQUIRED) TO BE GALVANIZED.

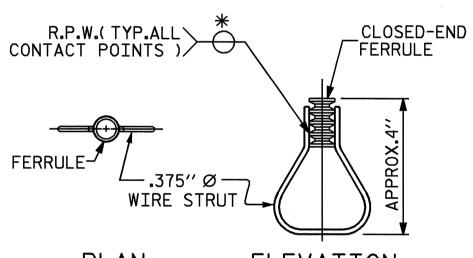
THE COST OF THE STANDARD CLAMP BARS AND CAP SCREWS USED IN THE METAL RAIL TO END POST CONNECTION SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR LINEAR FEET OF 1 OR 2 BAR METAL RAILS.

THE $\frac{3}{4}$ " STRUCTURAL CONCRETE INSERT WITH BOLT SHALL BE ASSEMBLED IN THE SHOP.

THE COST OF THE $\frac{3}{4}$ " STRUCTURAL CONCRETE INSERT ASSEMBLY, AND THE $\frac{1}{2}$ " PLATES COMPLETE IN PLACE SHALL BE INCLUDED IN THE VARIOUS PAY ITEMS.

THE CONTRACTOR, AT HIS OPTION, MAY USE AN ADHESIVE BONDING SYSTEM IN LIEU OF THE STRUCTURAL CONCRETE INSERT EMBEDDED IN THE END POST. IF THE ADHESIVE BONDING SYSTEM IS USED, THE $\frac{3}{4}$ " $\frac{9}{9}$ X $\frac{15}{8}$ " BOLT WITH WASHER SHALL BE REPLACED WITH A $\frac{3}{4}$ " $\frac{9}{9}$ X $\frac{6}{2}$ " BOLT AND 2" O.D. WASHER. ALL SPECIFICATIONS THAT APPLY TO THE 34" Ø X 158" BOLT SHALL APPLY TO THE 34" Ø X 6 1/2" BOLT. FIELD TESTING OF THE ADHESIVE BONDING SYSTEM IS NOT REQUIRED.





PLAN

ELEVATION

STRUCTURAL CONCRETE

*EACH WELDED ATTACHMENT OF WIRE TO FERRULE SHALL DEVELOP THE TENSILE STRENGTH OF THE WIRE.

B-4094 PROJECT NO. _ CURRITUCK

STATION: 14+02.12 -L-

SHEET 4 OF 5

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION STANDARD

RAIL POST SPACINGS

END OF RAIL DETAILS FOR ONE OR TWO BAR METAL RAILS

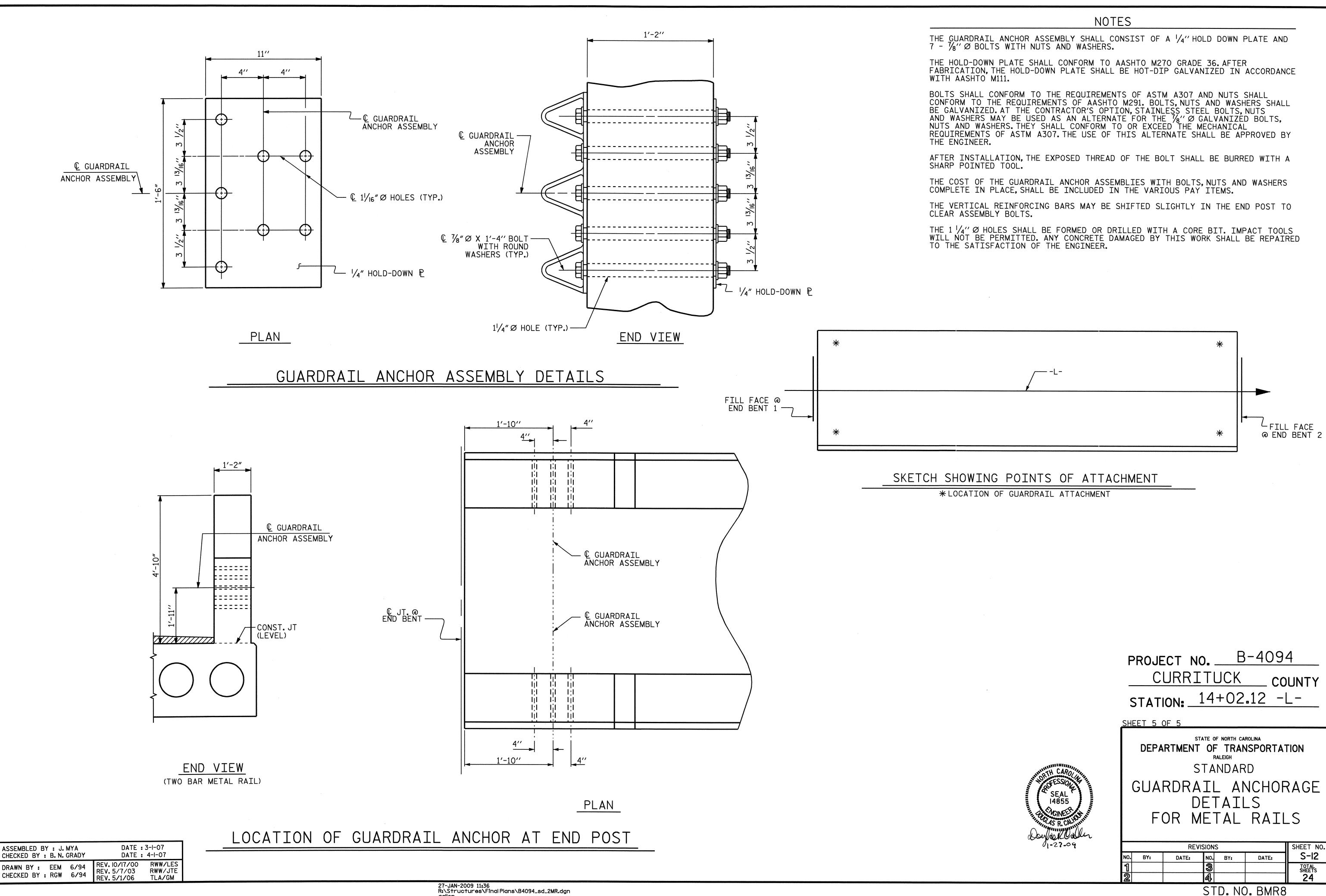
SHEET NO. **REVISIONS** DATE: DATE: BY: TOTAL SHEETS 24

DETAILS FOR ATTACHING METAL RAIL TO END POST

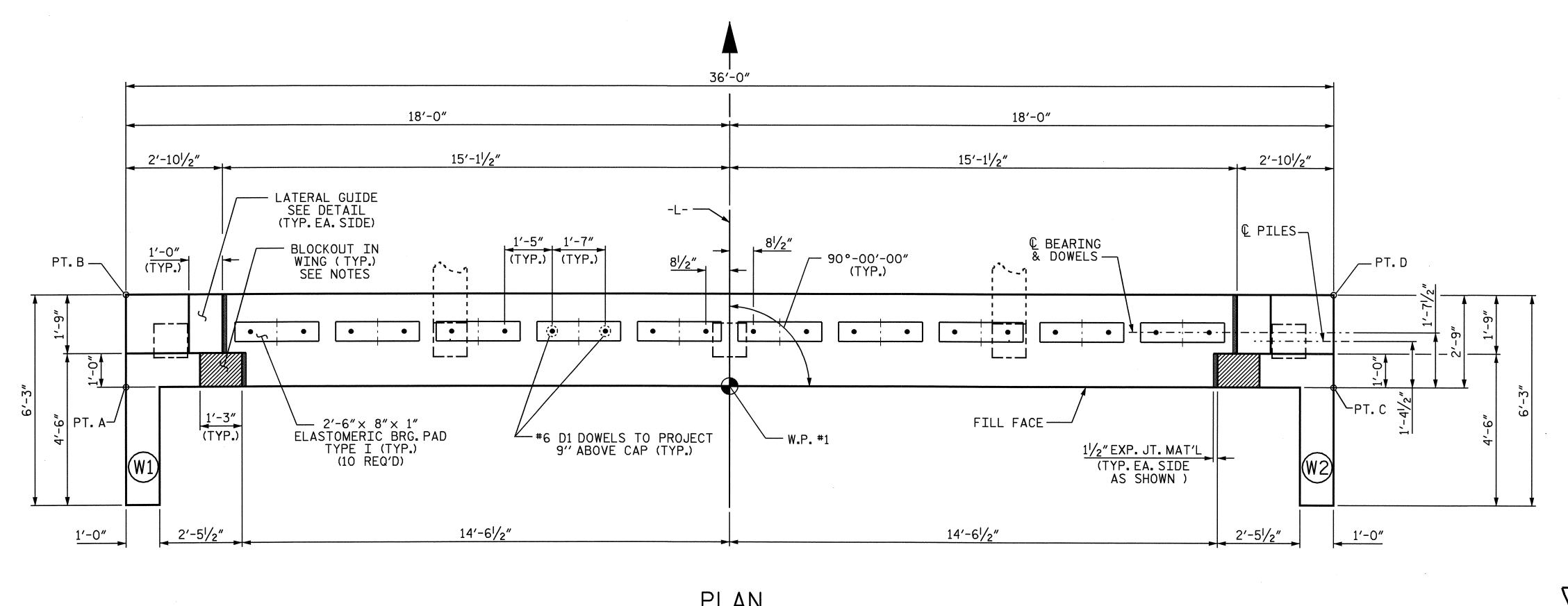
ASSEMBLED BY : J. MYA CHECKED BY : B. N. GRADY DATE : 3-1-07 DATE: 4-1-07 REV. IO/I7/00 REV. 5/7/03 REV. 5/I/06 LES/RDR DRAWN BY: FCJ 1/88 RWW/JTE CHECKED BY : CRK 3/89 TLA/GM

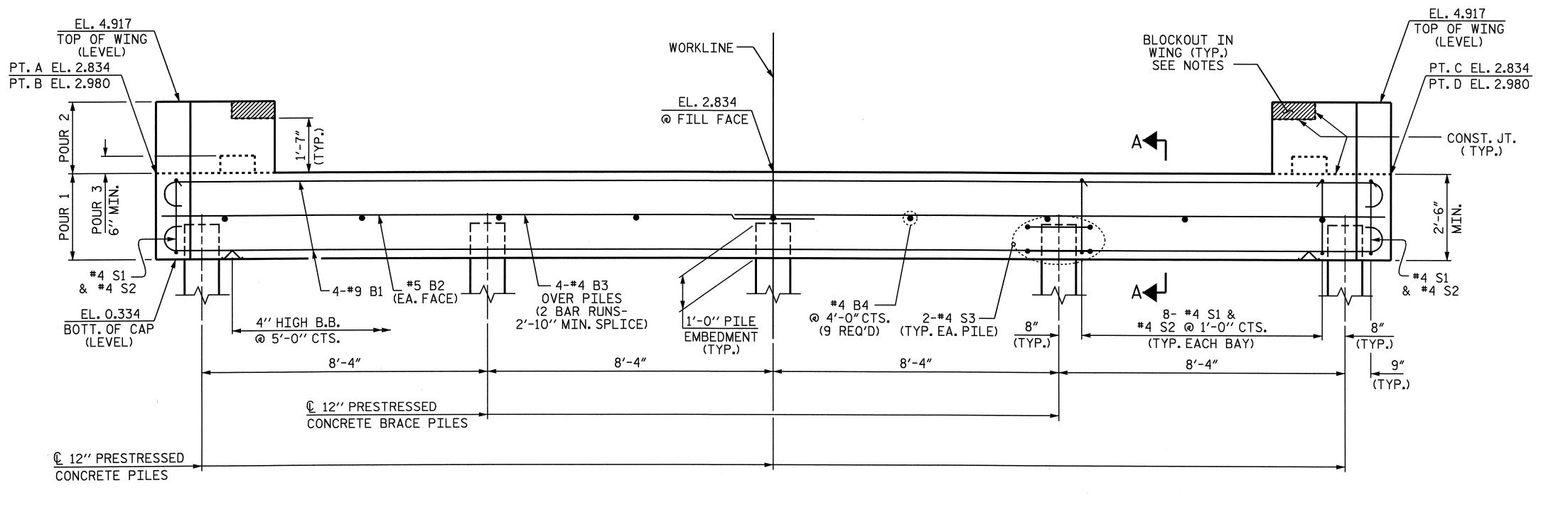
SECTION H-H (FIX)

FIXED



STD. NO. BMR8





ELEVATION

__ DATE : 4-1-07 __ DATE : 5-1-07 DRAWN BY : _____J. MYA CHECKED BY : A. K. PATEL

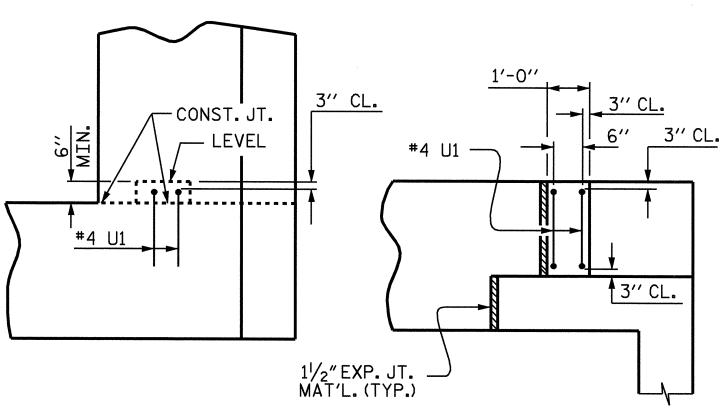
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STIRRUPS IN CAP MAY BE SHIFTED AS NECESSARY TO CLEAR DOWELS.

THE LATERAL GUIDE AT EACH END OF THE CAP IS NOT TO BE POURED UNTIL AFTER THE CORED SLAB UNITS ARE IN PLACE.

THE CONCRETE IN THE SHADED AREA OF THE WING SHALL BE POURED AFTER THE PARAPET IS CAST IF SLIP FORMING IS USED.

THE CONTRACTOR SHALL PROVIDE FOR INSTALLATION OF THE 4" DIAMETER DRAIN PIPE THROUGH THE WING WALL AS REQUIRED FOR REINFORCED BRIDGE APPROACH FILLS, SEE THE ROADWAY PLANS. REINFORCING STEEL IN THE WING WALL MAY BE SHIFTED AS NECESSARY TO CLEAR THE DRAIN PIPE.



ELEVATION

<u>PLAN</u>

LATERAL GUIDE (RIGHT SIDE SHOWN, LEFT SIDE SIMILAR)

> B-4094 PROJECT NO. ____ CURRITUCK _ COUNTY 14+02.12 -L-STATION:_

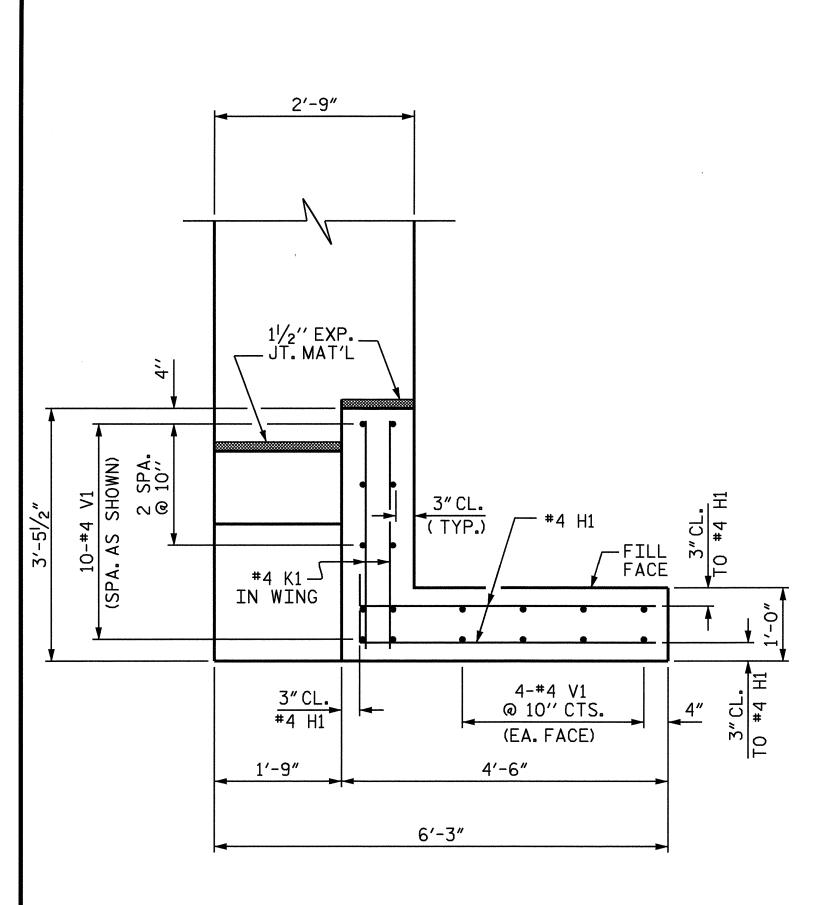
STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

> SUBSTRUCTURE END BENT 1

SHEET NO. **REVISIONS** S-13 DATE: BY: DATE: BY: TOTAL SHEETS 24

SHEET 1 OF 2

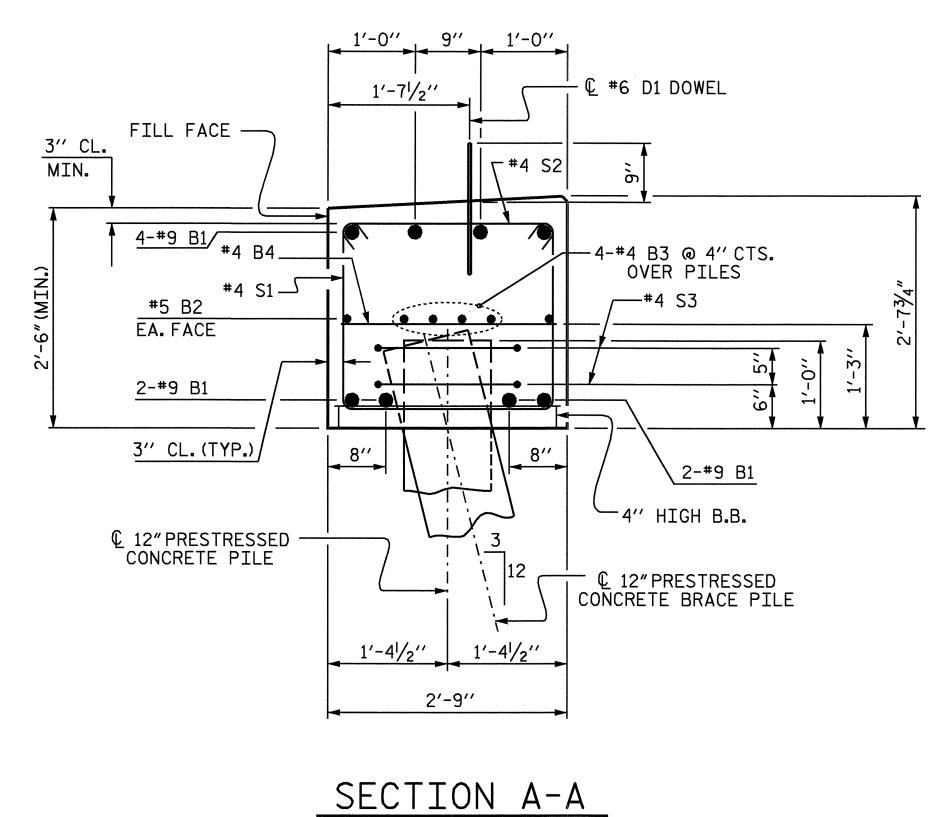


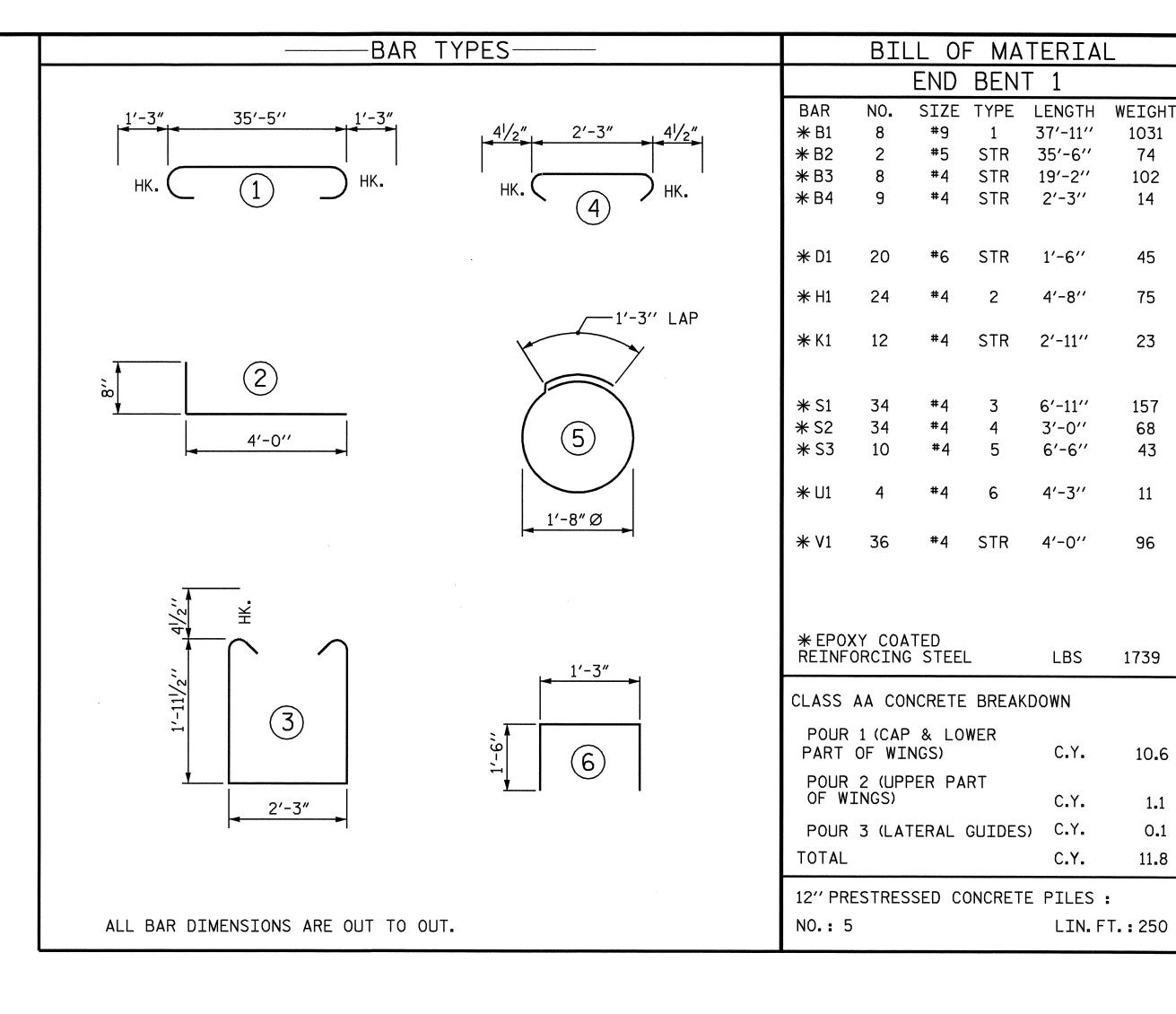


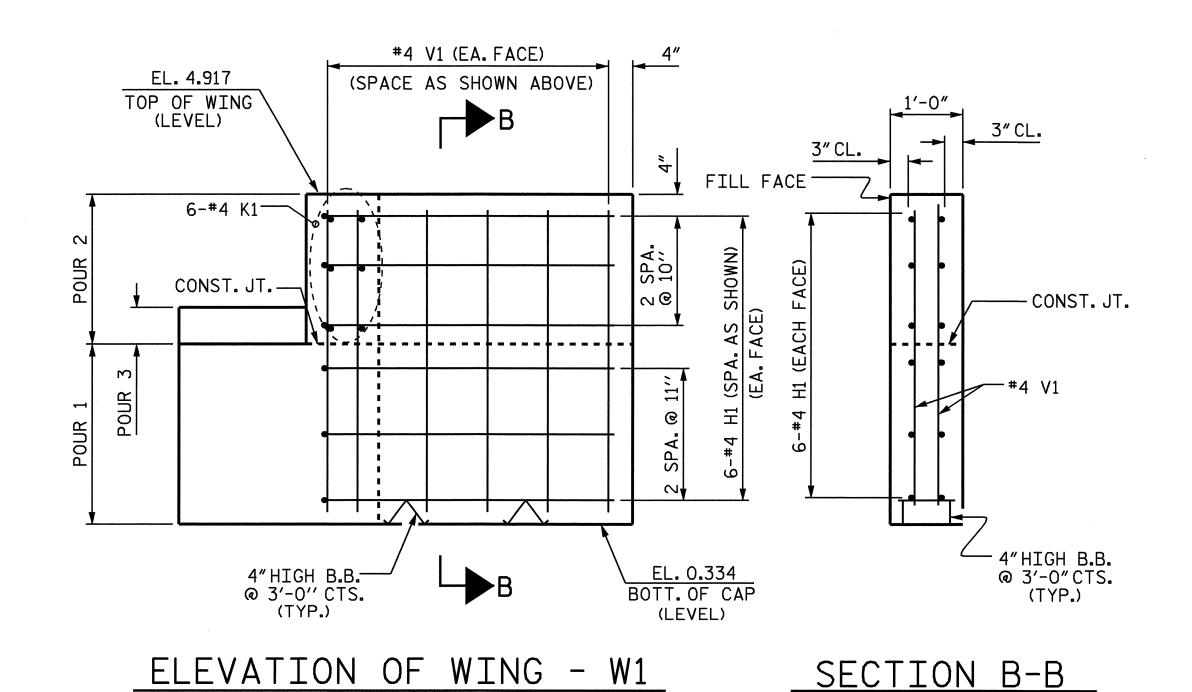
PLAN OF WING - W1

(WING 2 SIMILAR)

(WING 2 SIMILAR)







MINIMUM OF 3- ONE CUBIC FOOT BAGS OF #78M STONE. BAGS SHALL BE OF POROUS FABRIC, SECURELY TIED. 6"(MIN.)PIPE 6"(MIN.)PIPE FOR DRAINAGE FOR DRAINAGE GRADE TO DRAIN GRADE TO DRAIN TOE OF SLOPE--TOE OF SLOPE

BAGGED STONE AND PIPE SHALL BE PLACED IMMEDIATELY AFTER COMPLETION OF END BENT EXCAVATION. PIPE MAY BE EITHER CONCRETE, CORRUGATED STEEL, CORRUGATED ALUMINUM ALLOY, OR CORRUGATED PLASTIC. PERFORATED PIPE WILL NOT BE ALLOWED.

BAGGED STONE SHALL REMAIN IN PLACE UNTIL THE ENGINEER DIRECTS THAT IT BE REMOVED. THE CONTRACTOR SHALL REMOVE AND DISPOSE OF SILT ACCUMULATIONS AT BAGGED STONE WHEN SO DIRECTED BY THE ENGINEER. BAGS SHALL BE REMOVED AND REPLACED WHENEVER THE ENGINEER DETER-MINES THAT THEY HAVE DETERIORATED AND LOST THEIR EFFECTIVENESS.

NO SEPARATE PAYMENT WILL BE MADE FOR THIS WORK AND THE ENTIRE COST OF THIS WORK SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR THE SEVERAL PAY ITEMS.

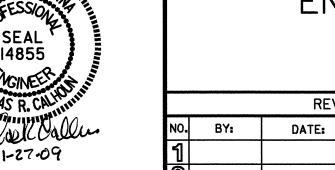
TEMPORARY DRAINAGE AT END BENT

B-4094 PROJECT NO. CURRITUCK COUNTY 14+02.12 -L-

SHEET 2 OF 2

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

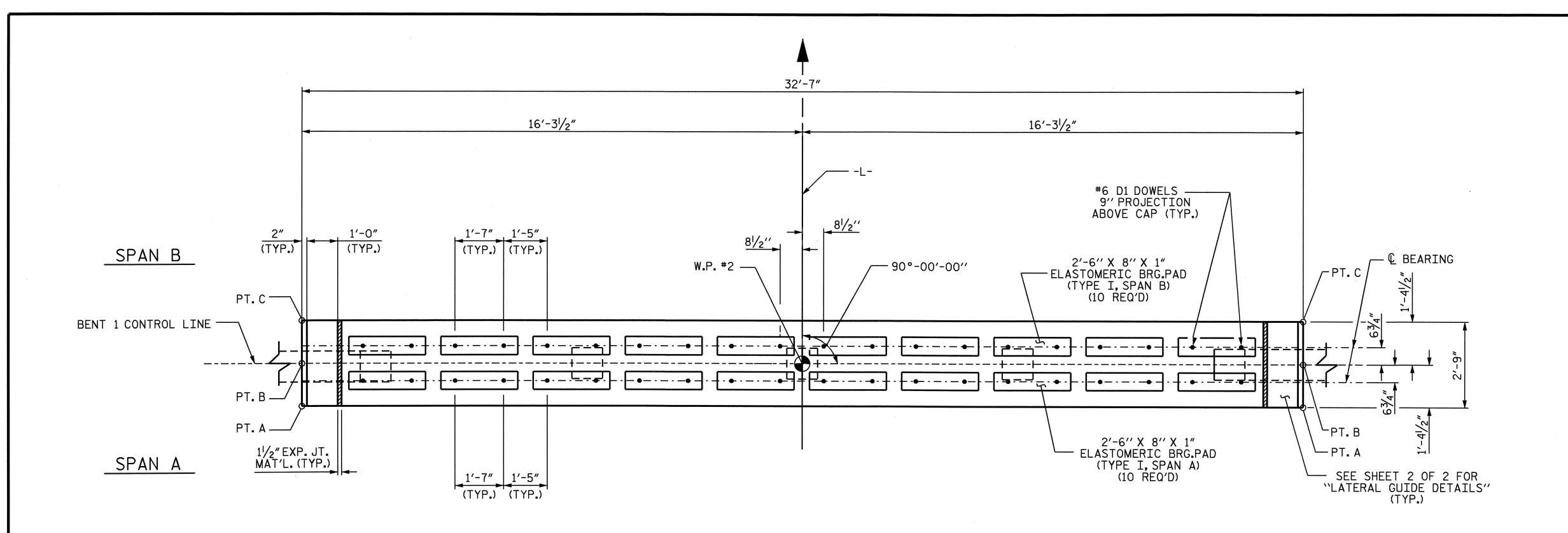
> SUBSTRUCTURE END BENT 1



SEAL 14855

SHEET NO. **REVISIONS** S-14 DATE: DATE: BY: TOTAL SHEETS

J. MYA ___ DATE : <u>4-1-07</u> ___ DATE : <u>5-1-07</u> CHECKED BY : A.K. PATEL

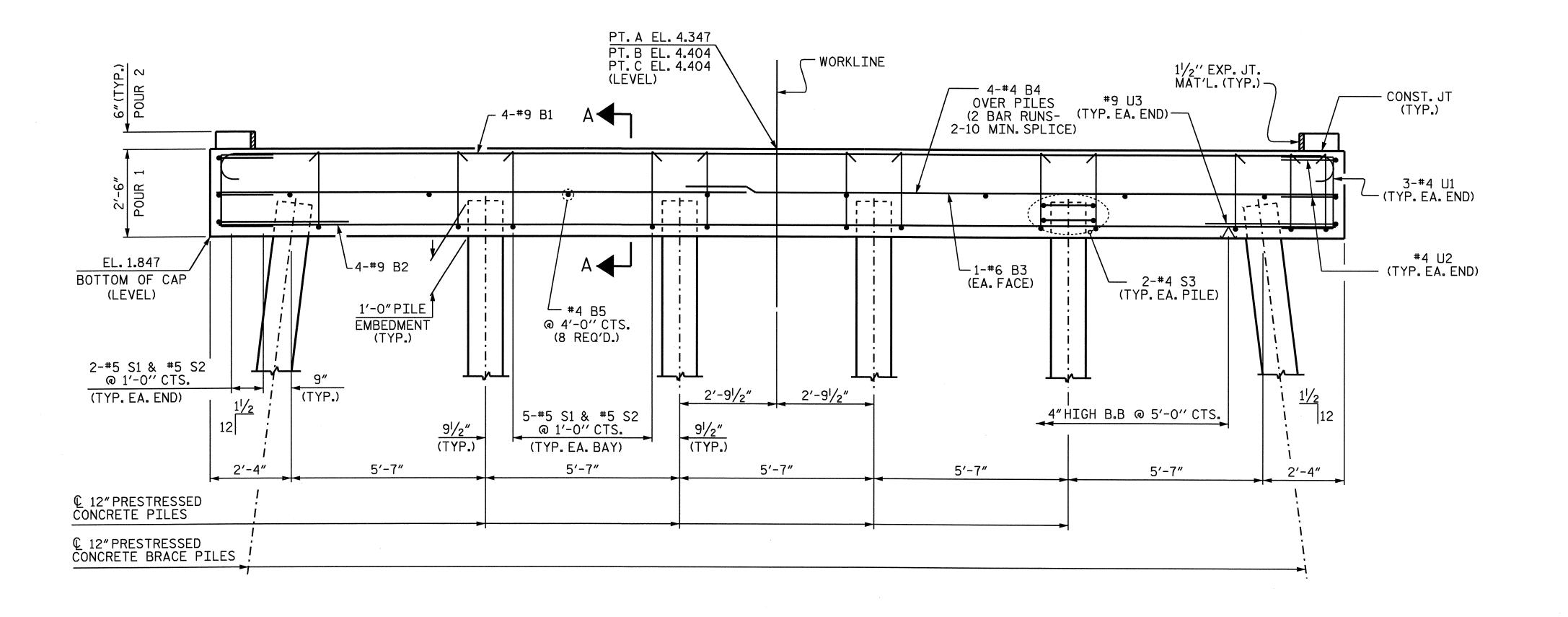


<u>PLAN</u>

NOTES

STIRRUPS IN CAP MY BE SHIFTED AS NECESSARY TO CLEAR DOWELS.

THE LATERAL GUIDE AT EACH END OF THE CAP IS NOT TO BE POURED UNTIL AFTER THE CORED SLAB UNITS ARE IN PLACE.



ELEVATION

PROJECT NO. B-4094 CURRITUCK COUNTY STATION: 14+02.12 -L-

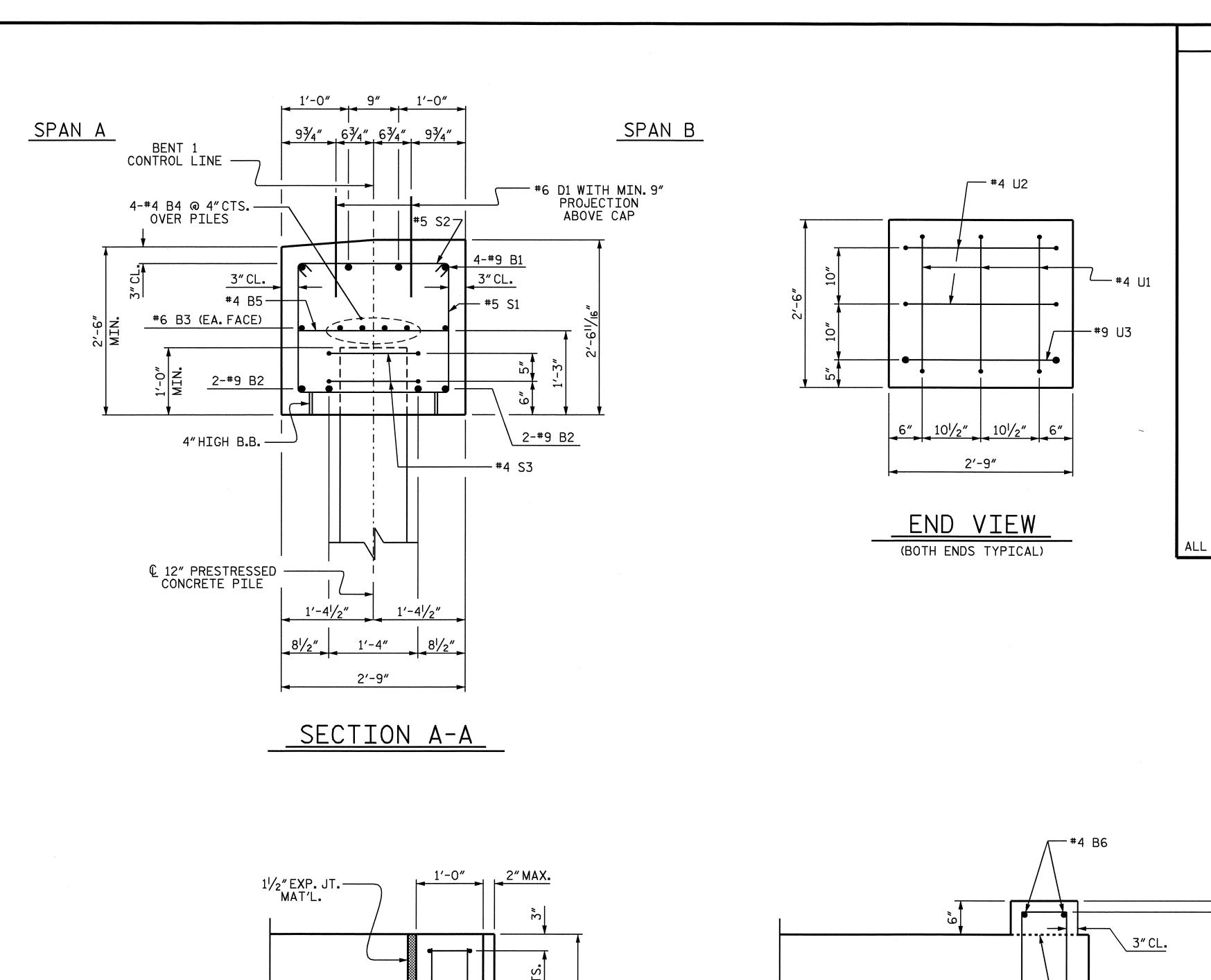
SHEET 1 OF 2

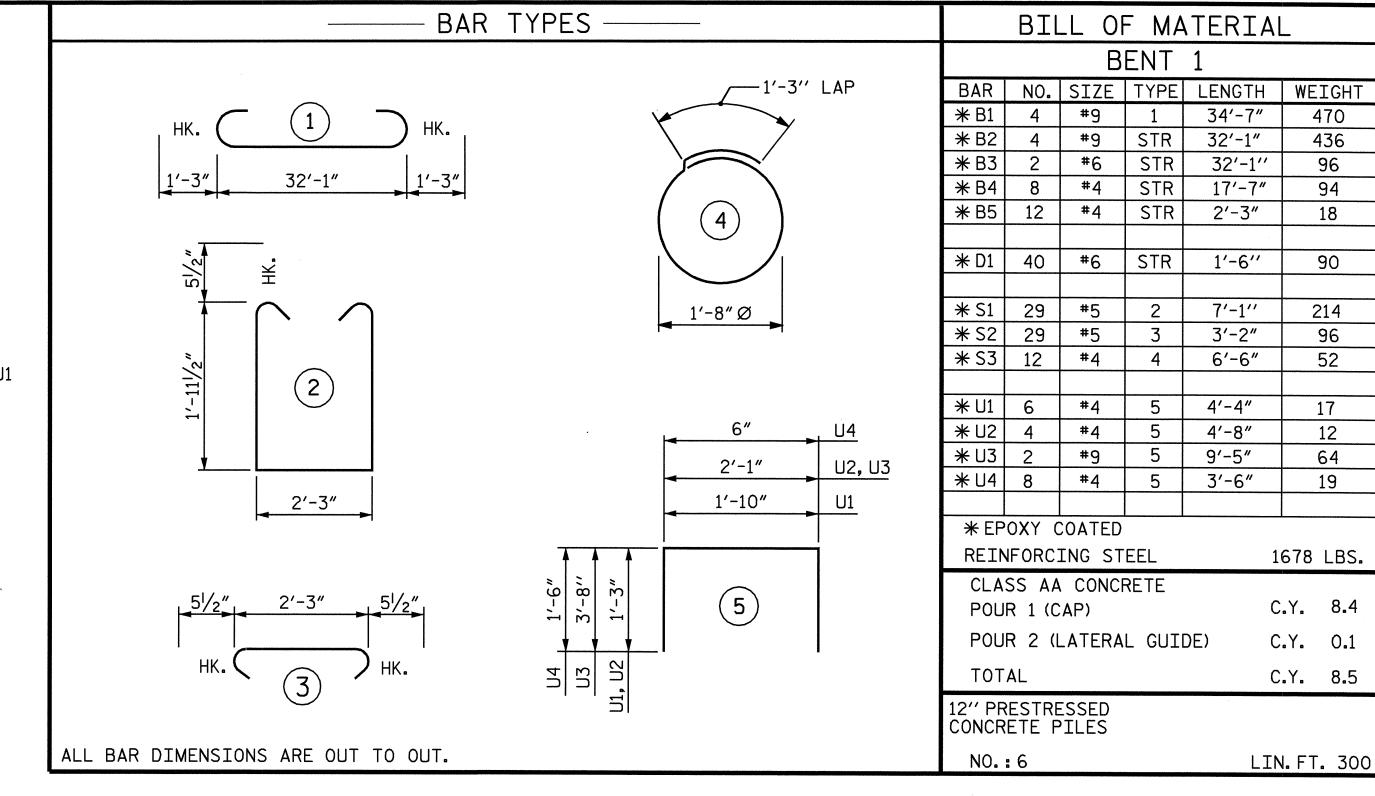
STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

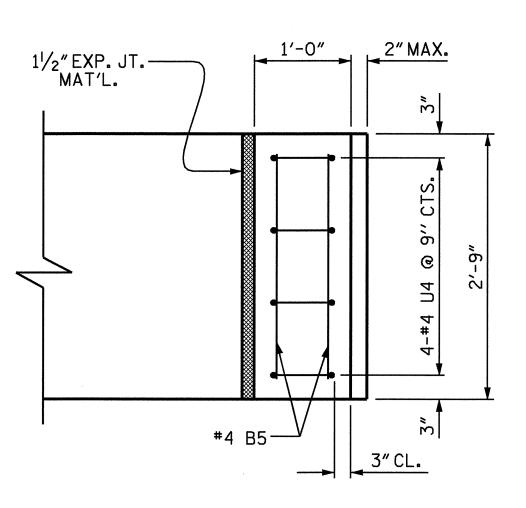
> SUBSTRUCTURE BENT 1

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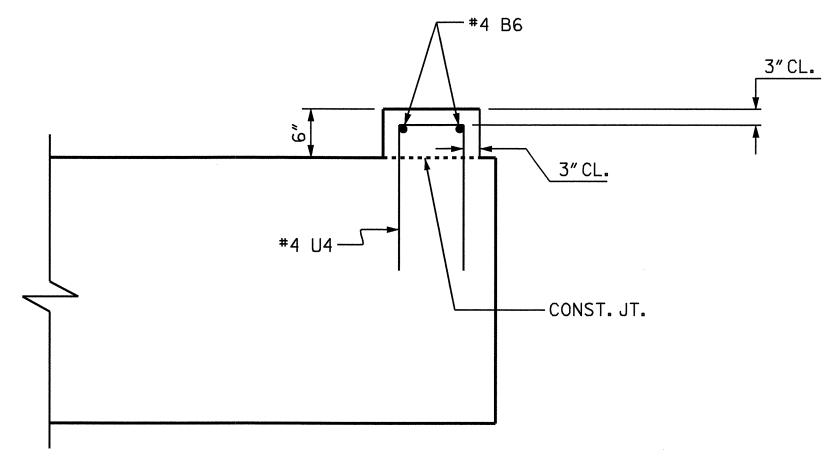
DRAWN BY : M. FOWLER CHECKED BY : K.P. SEDAI __ DATE : <u>2/19/08</u> __ DATE : <u>3/4/08</u>







<u>PLAN</u>



ELEVATION

LATERAL GUIDE REINFORCING DETAIL

(RIGHT END OF THE CAP SHOWN, LEFT END SIMILAR)

SEAL 14855

SEAL 14855

OUT OF EXCENSION AND THE PROPERTY OF T

PROJECT NO. B-4094

CURRITUCK COUNTY

STATION: 14+02.12 -L-

SHEET 2 OF 2

STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION
RALEIGH

SUBSTRUCTURE BENT 1

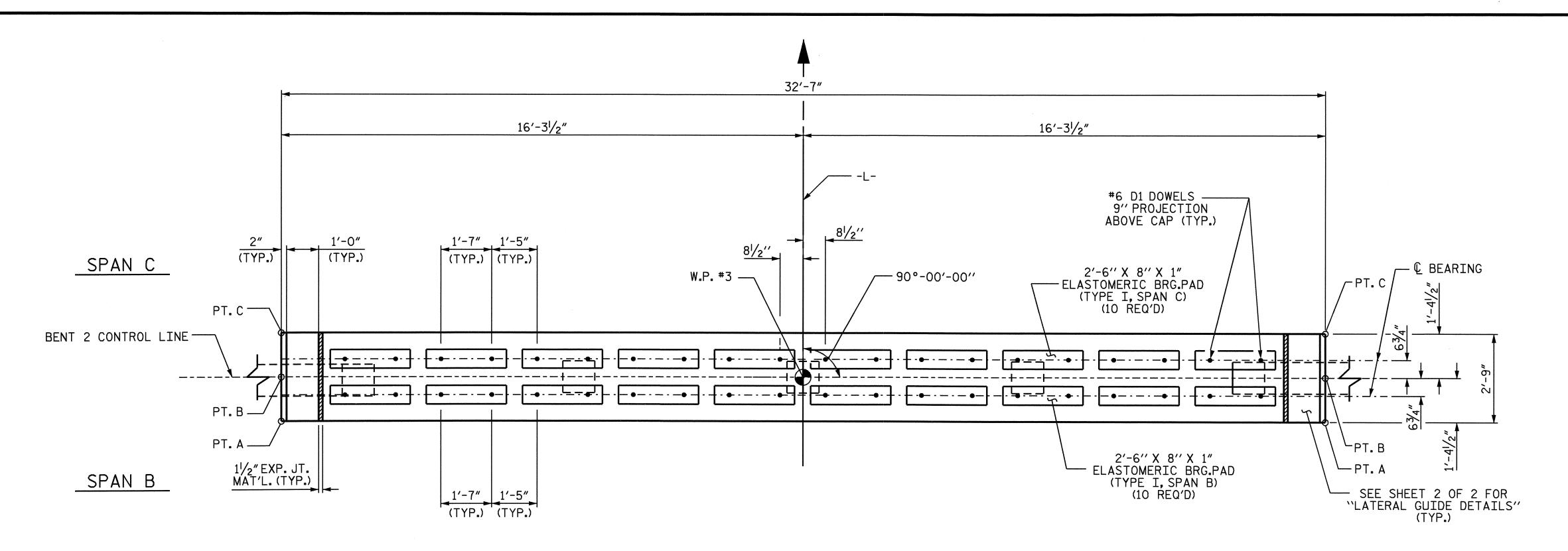
REVISIONS

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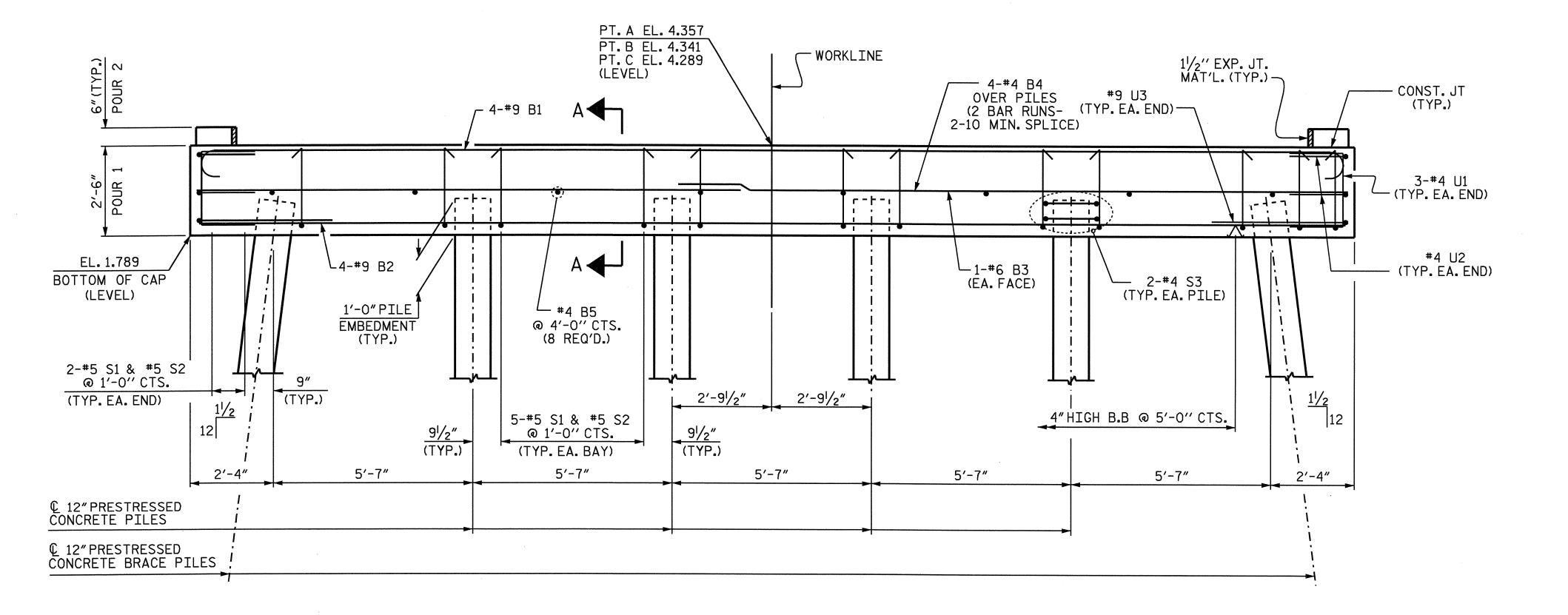
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DRAWN BY: M. FOWLER DATE: 2/19/08
CHECKED BY: K.P. SEDAI DATE: 3/4/08

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<u>PLAN</u>



_ELEVATION

DRAWN BY: M. FOWLER DATE: 2/19/08
CHECKED BY: K.P. SEDAI DATE: 3/4/08

SEAL 14855

SEAL 14855

SUNCERTOR

1-27-09

NOTES

STIRRUPS IN CAP MY BE SHIFTED AS NECESSARY TO CLEAR DOWELS.

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PROJECT NO. B-4094

CURRITUCK COUNTY

STATION: 14+02.12 -L-

SHEET 1 OF 2

STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION

RALEIGH

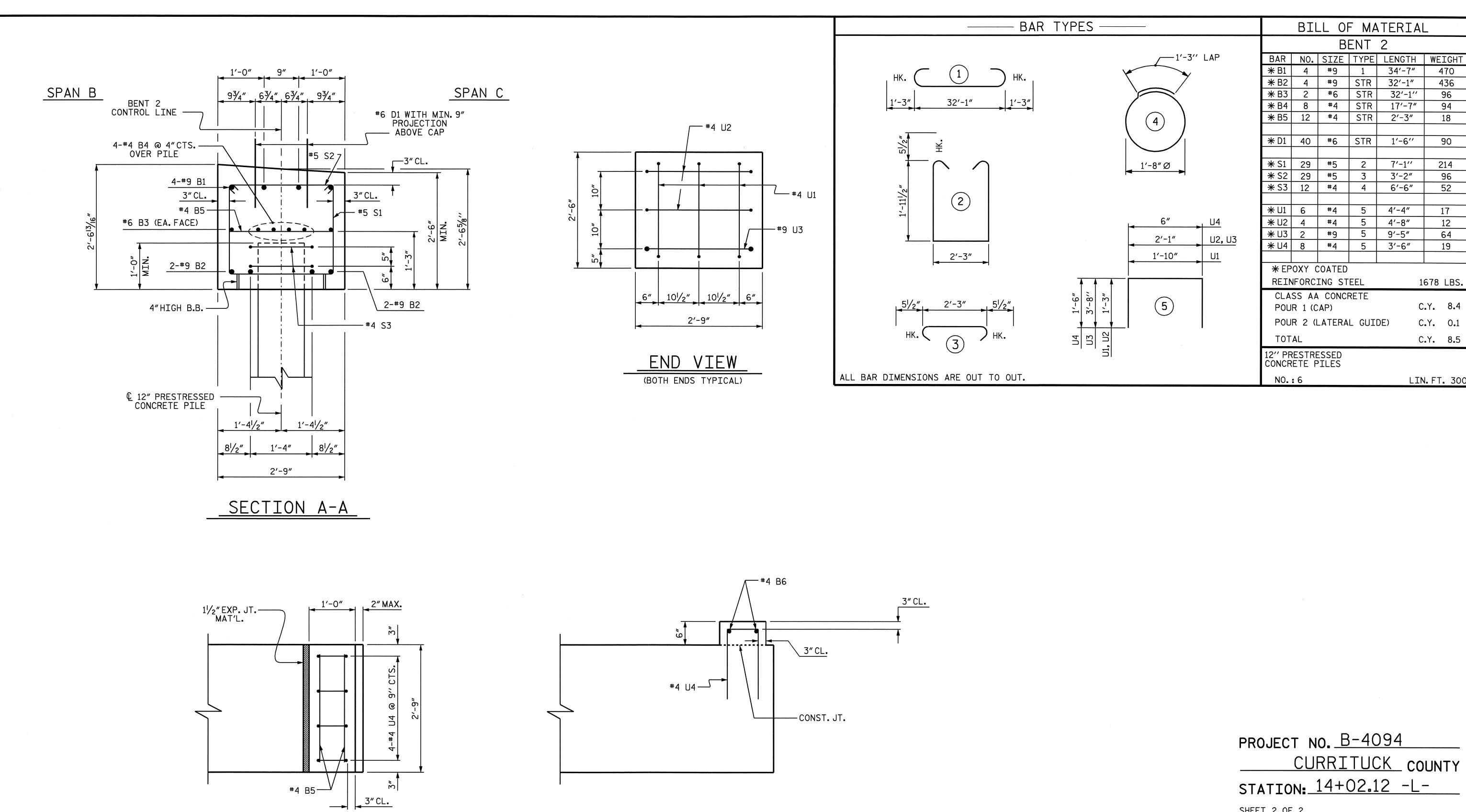
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ELEVATION

PROJECT NO. B-4094 CURRITUCK COUNTY STATION: 14+02.12 -L-

BILL OF MATERIAL

BENT 2

STR

STR

STR

4

#4 5 4'-8"

#9 5 9'-5"

#4 5

#9 STR

34'-7"

32'-1"

32'-1"

17'-7"

2'-3"

7'-1''

3'-2"

6'-6"

3'-6"

470

436

96

94

18

90

214

96

52

12

64

19

1678 LBS.

C.Y. 8.4

C.Y. 0.1

C.Y. 8.5

LIN.FT. 300

#9

#6

#4

#4

#5

SHEET 2 OF 2

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

RUCTURE

ALL WALLES.	SUBSTF
CAROLANTIA	BEN
SEAL 1	
GINEE	

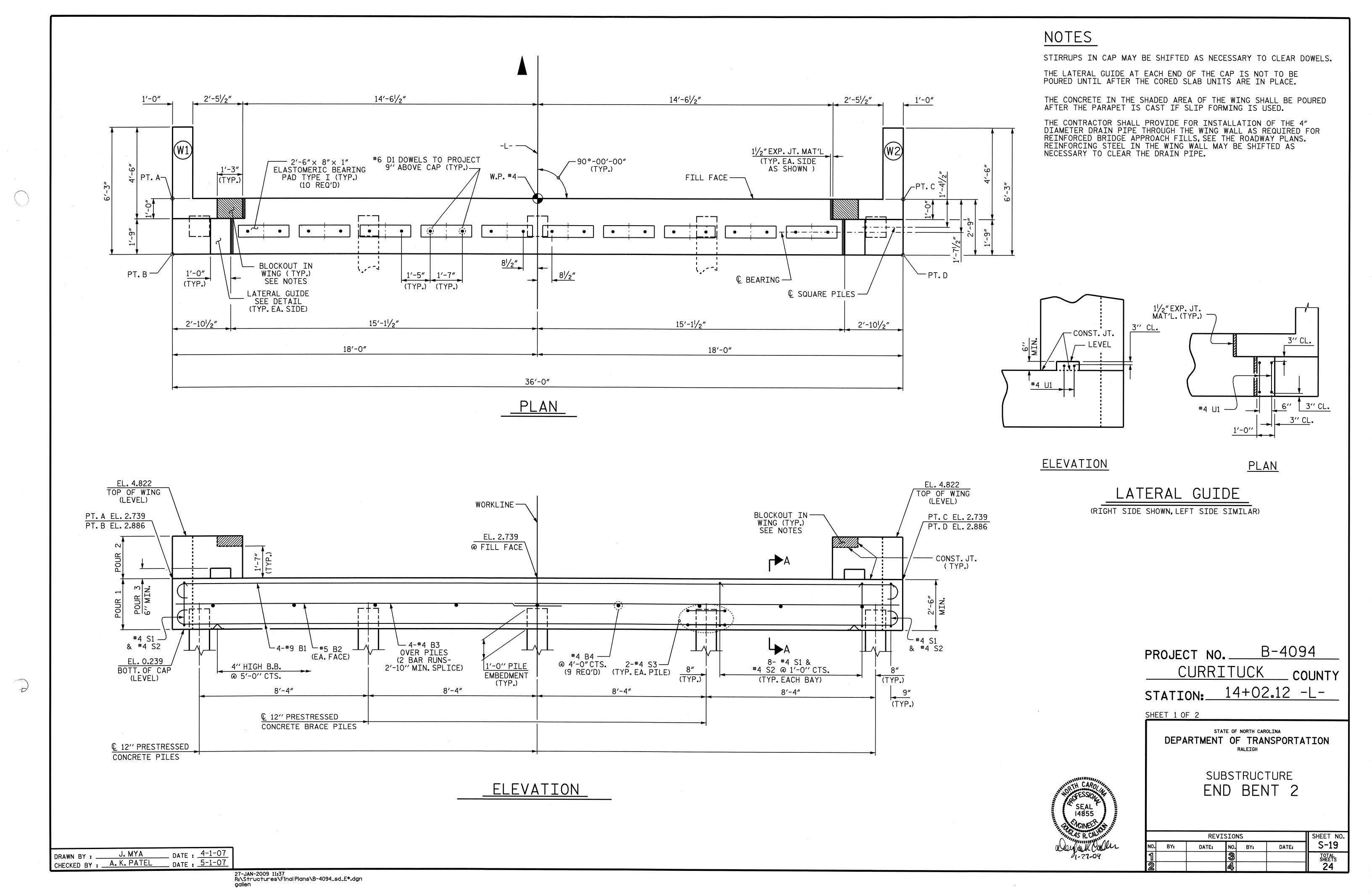
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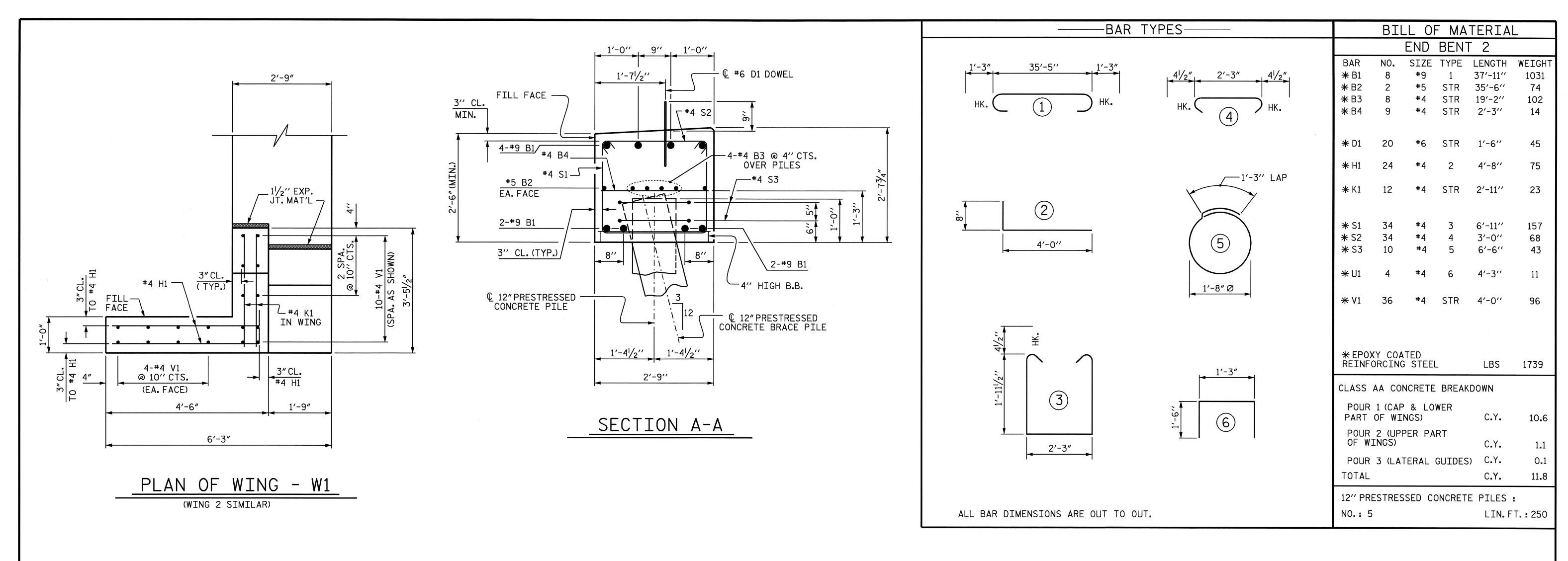
LATERAL GUIDE REINFORCING DETAIL

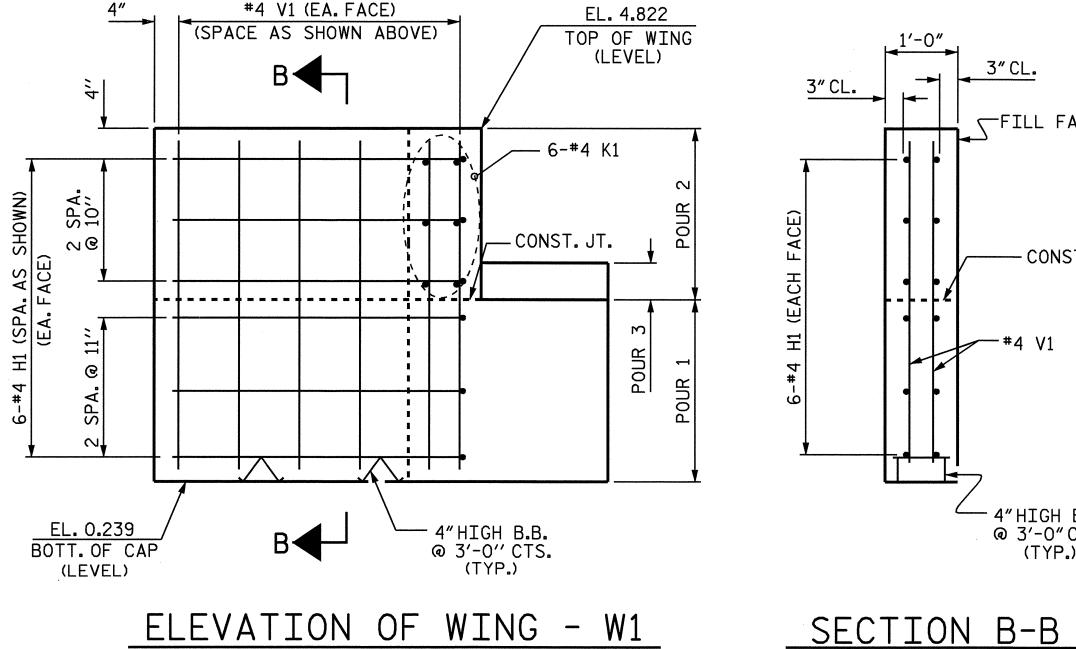
(RIGHT END OF THE CAP SHOWN, LEFT END SIMILAR)

__ DATE : 2/19/08 __ DATE : 3/4/08 DRAWN BY : M. FOWLER
CHECKED BY : K.P. SEDAI

<u>PLAN</u>



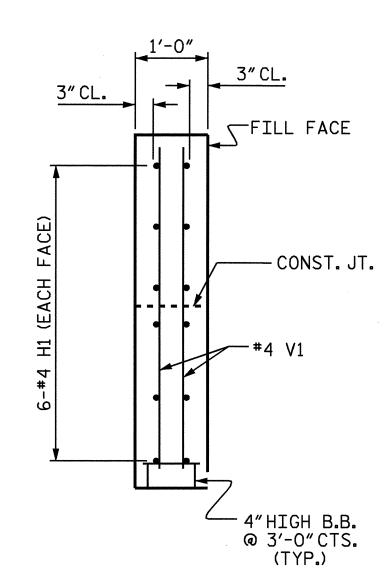




(WING 2 SIMILAR)

DRAWN BY : J. MYA

CHECKED BY : A.K. PATEL



6"(MIN.)PIPE FOR DRAINAGE

BAGGED STONE AND PIPE SHALL BE PLACED IMMEDIATELY AFTER COMPLETION OF END BENT EXCAVATION. PIPE MAY BE EITHER CONCRETE, CORRUGATED STEEL, CORRUGATED ALUMINUM ALLOY, OR CORRUGATED PLASTIC. PERFORATED PIPE WILL NOT BE ALLOWED.

GRADE TO DRAIN GRADE TO DRAIN

6"(MIN.)PIPE FOR DRAINAGE

TOE OF SLOPE-

MINIMUM OF 3- ONE CUBIC FOOT BAGS OF #78M STONE. BAGS SHALL BE OF POROUS FABRIC, SECURELY TIED.

BAGGED STONE SHALL REMAIN IN PLACE UNTIL THE ENGINEER DIRECTS THAT IT BE REMOVED. THE CONTRACTOR SHALL REMOVE AND DISPOSE OF SILT ACCUMULATIONS AT BAGGED STONE WHEN SO DIRECTED BY THE ENGINEER. BAGS SHALL BE REMOVED AND REPLACED WHENEVER THE ENGINEER DETER-MINES THAT THEY HAVE DETERIORATED AND LOST THEIR EFFECTIVENESS.

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TEMPORARY DRAINAGE AT END BENT

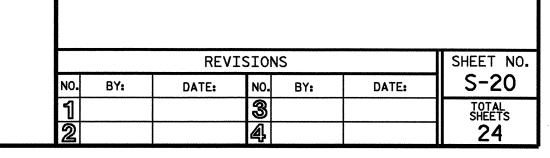
B-4094 PROJECT NO. __ CURRITUCK _ COUNTY 14+02.12 -L-STATION:_

SHEET 2 OF 2

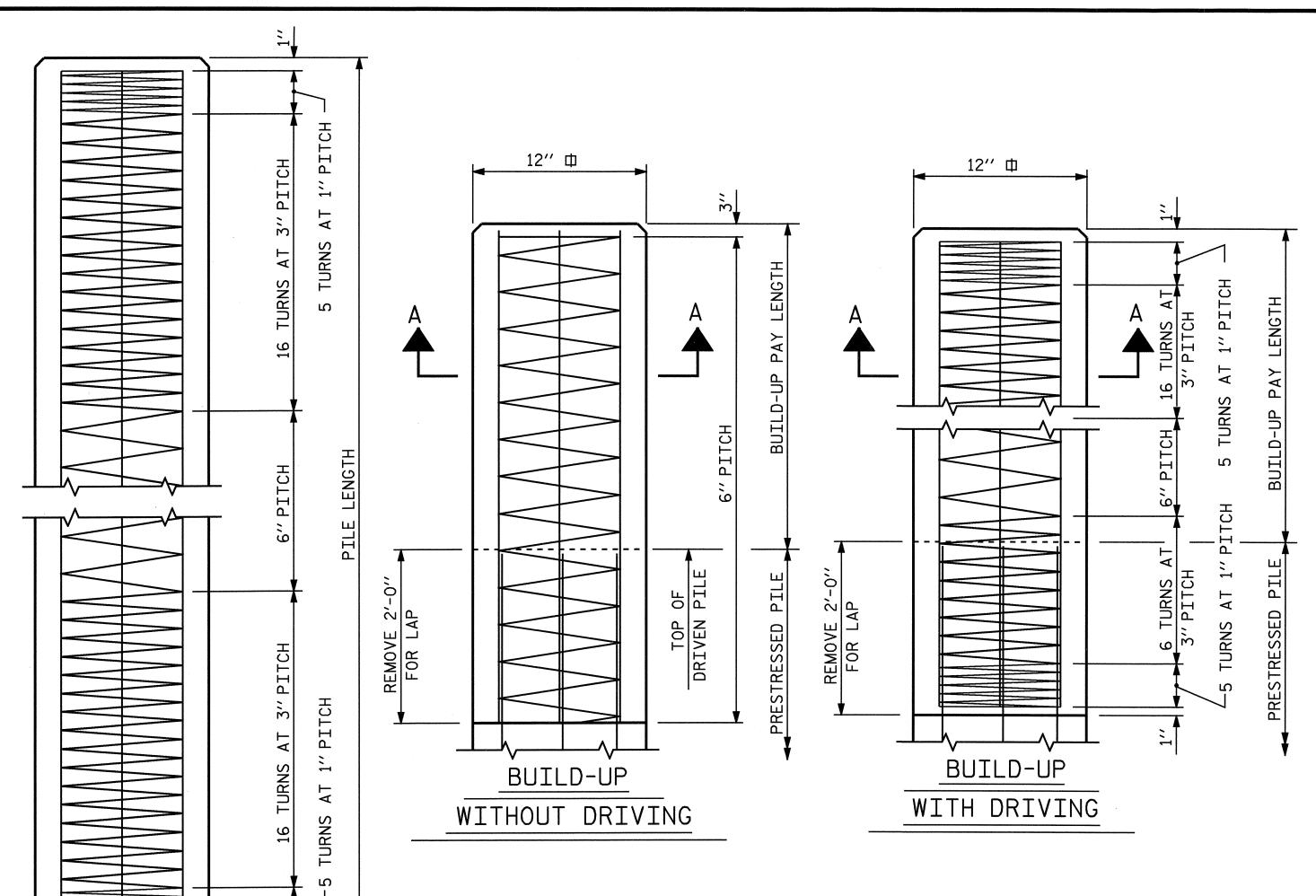
SEAL 14855

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH

> SUBSTRUCTURE END BENT 2



-TOE OF SLOPE



5 PRESTRESS STRANDS

∠ W3.5 COLD DRAWN STEEL WIRE SPIRAL

∠ W3.5 COLD DRAWN STEEL WIRE SPIRAL

√

TYPICAL SECTION

TYPICAL SECTION

TYP.

PRESTRESS STRANDS

TYP.

1/2" Ø GRADE 270 L.R. PRESTRESS STRANDS

1/2" Ø GRADE 270 L.R. PRESTRESS STRANDS

TYP.

DATE : 4-1-07 DATE : 12-5-08

RWW/LES RWW/LES TLA/GM

--- 5-#5 BARS

SECTION "A-A" FOR BURNING STRANDS

-4- #5 BARS

SECTION "A-A" FOR BURNING STRANDS

TYPICAL PATTERN

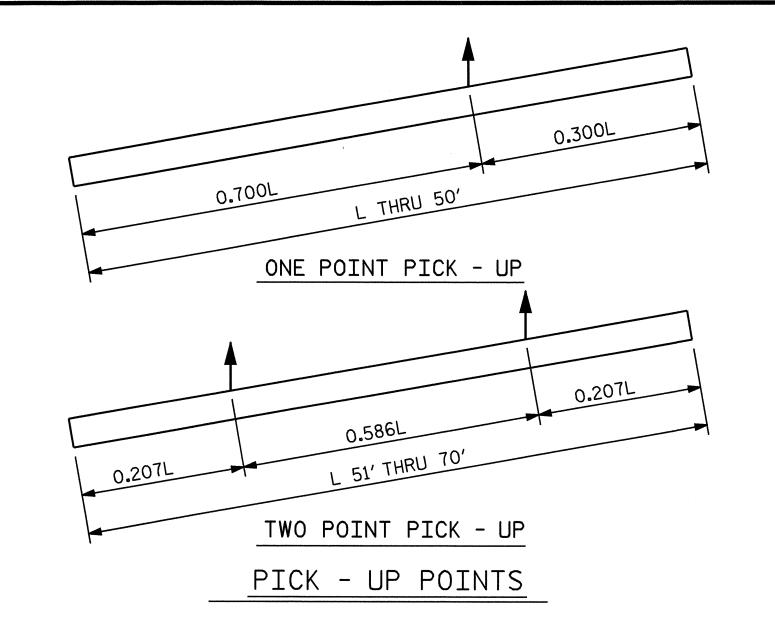
TYPICAL PATTERN

12′′ 巾

ASSEMBLED BY : J. MYA
CHECKED BY : E.G. ALLEN

DRAWN BY: FCJ 7/88 CHECKED BY: CRK 3/89

ELEVATION



QUAI	QUANTITIES FOR ONE 12" PRESTRESSED PILE									
	CONCRETE	PILE WT.	ONE PICK	-UP POINT	TWO PIC	K-UP POINT				
LENGTH	CU. YDS.	TONS	0.300L	0.700L	0.207L	0.586L				
25′-0′′	0.91	1.85	7′-6′′	17′-6′′						
30′-0′′	1.10	2.22	9'-0''	21′-0′′						
35′-0′′	1.28	2.59	10′-6′′	24'-6''						
40′-0′′	1.46	2.96	12'-0''	28'-0''		-				
45′-0′′	1.64	3.33	13′-6′′	31′-6′′						
50′-0′′	1.83	3.70	15′-0′′	35′-0′′						
55′-0′′	2.01	4.07			11'-4 ¹ /2''	32′-3′′				
60′-0′′	2.19	4.44			12'-5''	35′-2′′				
65′-0′′	2.37	4.81			13'-51/2''	38′-1′′				
70′-0′′	2.56	5.18			14'-6''	41'-0''				

NOTES

CONCRETE DESIGN DATA : f'c = 5,000 PSI ;fc = 2,000 PSI

IMPACT IN HANDLING = 50%

THE TRANSFER OF LOAD FROM THE ANCHORAGES TO THE PILE SHALL BE DONE WHEN THE CONCRETE HAS REACHED A COMPRESSIVE STRENGTH OF NOT LESS THAN 3,500 PSI.

IN DRIVING PILES, A METHOD APPROVED BY THE ENGINEER SHALL BE USED, WHEREBY THE HEAD OF THE PILE IS NOT DAMAGED.

PROPOSED DEVICES FOR LIFTING PILES, RECESS DETAILS, AND PATCHING MATERIAL SHALL BE DETAILED IN SHOP DRAWINGS. AFTER ATTACHMENTS HAVE BEEN REMOVED, OPENINGS SHALL BE REPAIRED SUCH THAT THE APPEARANCE OF THE PILE IS UNIFORM.

WHERE CAST-IN-PLACE LIFTING DEVICES ARE NOT USED, PICK-UP POINTS TO BE INDICATED WITH A BLACK MARK 2" WIDE.

ALL PRESTRESSING STRANDS SHALL BE 7-WIRE LOW-RELAXATION GRADE 270 STRANDS AND SHALL CONFORM TO AASHTO M203 EXCEPT FOR SAMPLING REQUIREMENTS WHICH SHALL BE IN ACCORDANCE WITH STANDARD SPECIFICATIONS.

THE CONTRACTOR MAY USE EITHER OF THE FOLLOWING STRAND CONFIGURATIONS:

SIZE	GRADE	NUMBER OF STRANDS	AREA	ULTIMATE STRENGTH	APPLIED PRESTRESS FORCE
1/2′′	270 L.R.	4	0.153	41,300# PER STRAND	30,980# PER STRAND
1/2′′	270 L.R.	5	0.153	41,300# PER STRAND	30,980# PER STRAND

THE SLIP-FORM METHOD OF CASTING PILES WILL NOT BE PERMITTED.

IF STRAND STRESS IS RELIEVED BY BURNING, THE STRANDS SHALL BE BURNED IN PAIRS, EXCEPT WHERE 5 STRANDS ARE USED THE LAST STRAND MAY BE BURNED SINGLY, ACCORDING TO BURNING PATTERNS SHOWN. NOT MORE THAN 4 STRANDS MAY BE BURNED AT ANY ONE SECTION BEFORE THE SAME STRANDS ARE BURNED AT BOTH ENDS OF THE BED AND BETWEEN EACH PAIR OF PILES IN THE BED.

BUILD-UPS SHALL BE 'CLASS A' CONCRETE WITH 20% ADDITIONAL CEMENT. NO DRIVING OF THE BUILT-UP PILE WILL BE PERMITTED UNTIL THE CONCRETE HAS REACHED A COMPRESSIVE STRENGTH OF 3,000 PSI AND UNTIL A PERIOD OF SEVEN DAYS HAS ELAPSED SINCE CASTING OF THE BUILD-UP.

ALL CORNERS TO BE CHAMFERED 3/4".

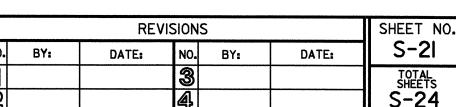
PRESTRESSED CONCRETE PILES SHALL CONTAIN CALCIUM NITRITE CORROSION INHIBITOR IN ACCORDANCE TO THE STANDARD SPECIFICATIONS. NO SEPARATE PAYMENT WILL BE MADE FOR CALCIUM NITRITE CORROSION INHIBITOR.

THE WATER/CEMENT RATIO FOR CONCRETE PILES SHALL NOT EXCEED 0.40.

PROJECT NO. B-4094 CURRITUCK COUNTY STATION: 14+02.12 -L-

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION STANDARD

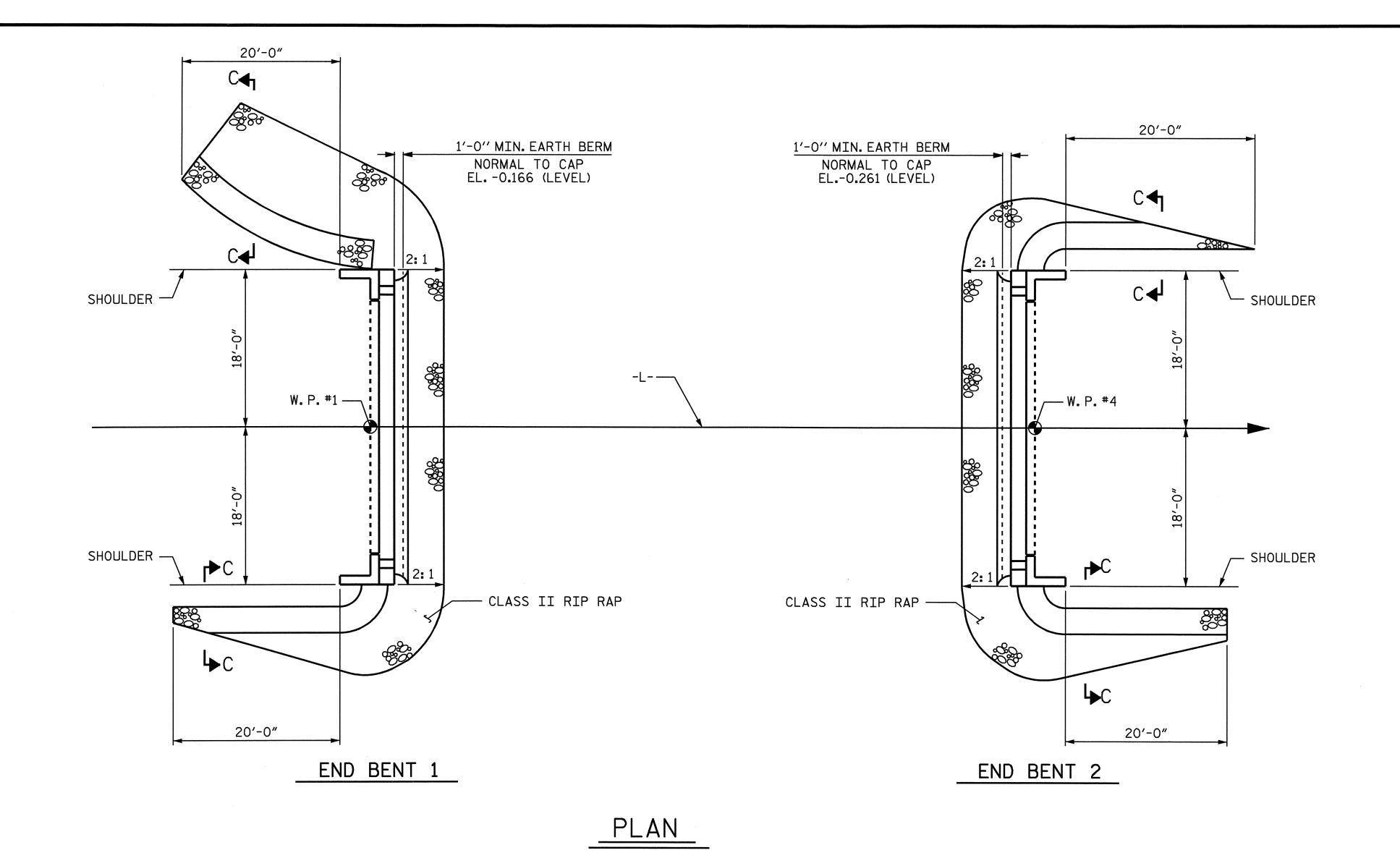
12" PRESTRESSED CONCRETE PILE



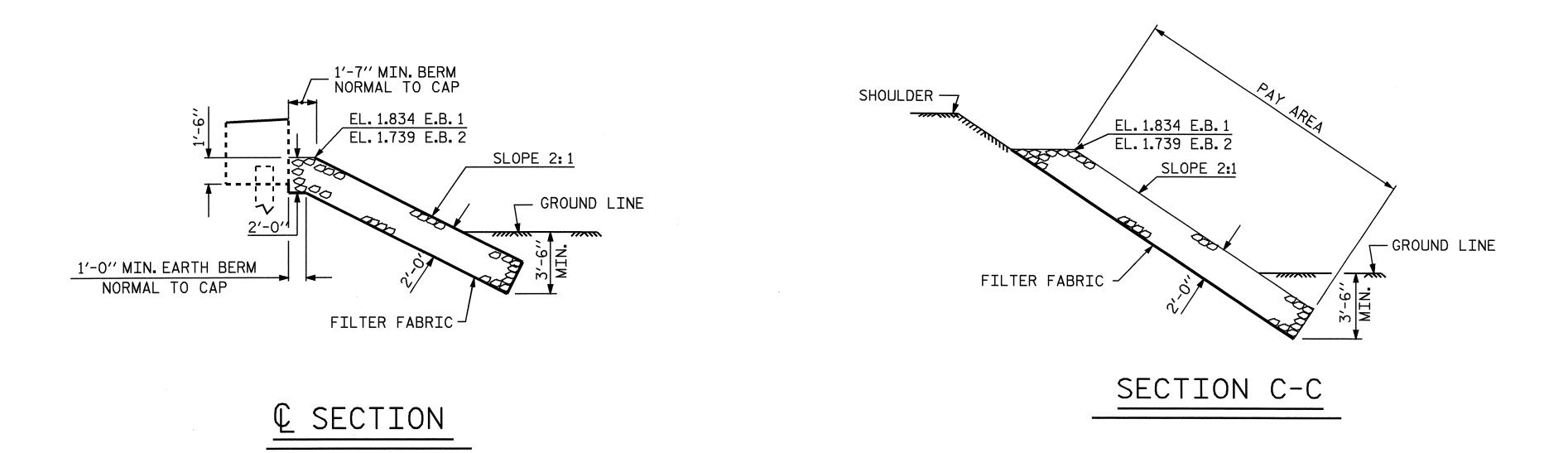
TOTAL SHEETS S-24

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STD. NO. PCP1



FST	IMATED QUANTITIE	-ς
BRIDGE @ STA. 14+02.12 -L-	RIP RAP CLASS II (2'-0"THICK)	FILTER FABRIC FOR DRAINAGE
	TONS	SQUARE YARDS
END BENT 1	71	79
END BENT 2	61	67



PROJECT NO. B-4094

CURRITUCK COUNTY

STATION: 14+02.12 -L-

STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION
RALEIGH

STANDARD

-RIP RAP DETAILS-

	REV:	ISION	S		SHEET NO.
BY:	DATE:	NO.	BY:	DATE:	S-22
		3			TOTAL SHEETS
		4			∥ 24

ASSEMBLED BY : J. MYA
CHECKED BY : B.N. GRADY

DATE : 10-10-08
DRAWN BY : FCJ 2/88
CHECKED BY : ARB 8/88

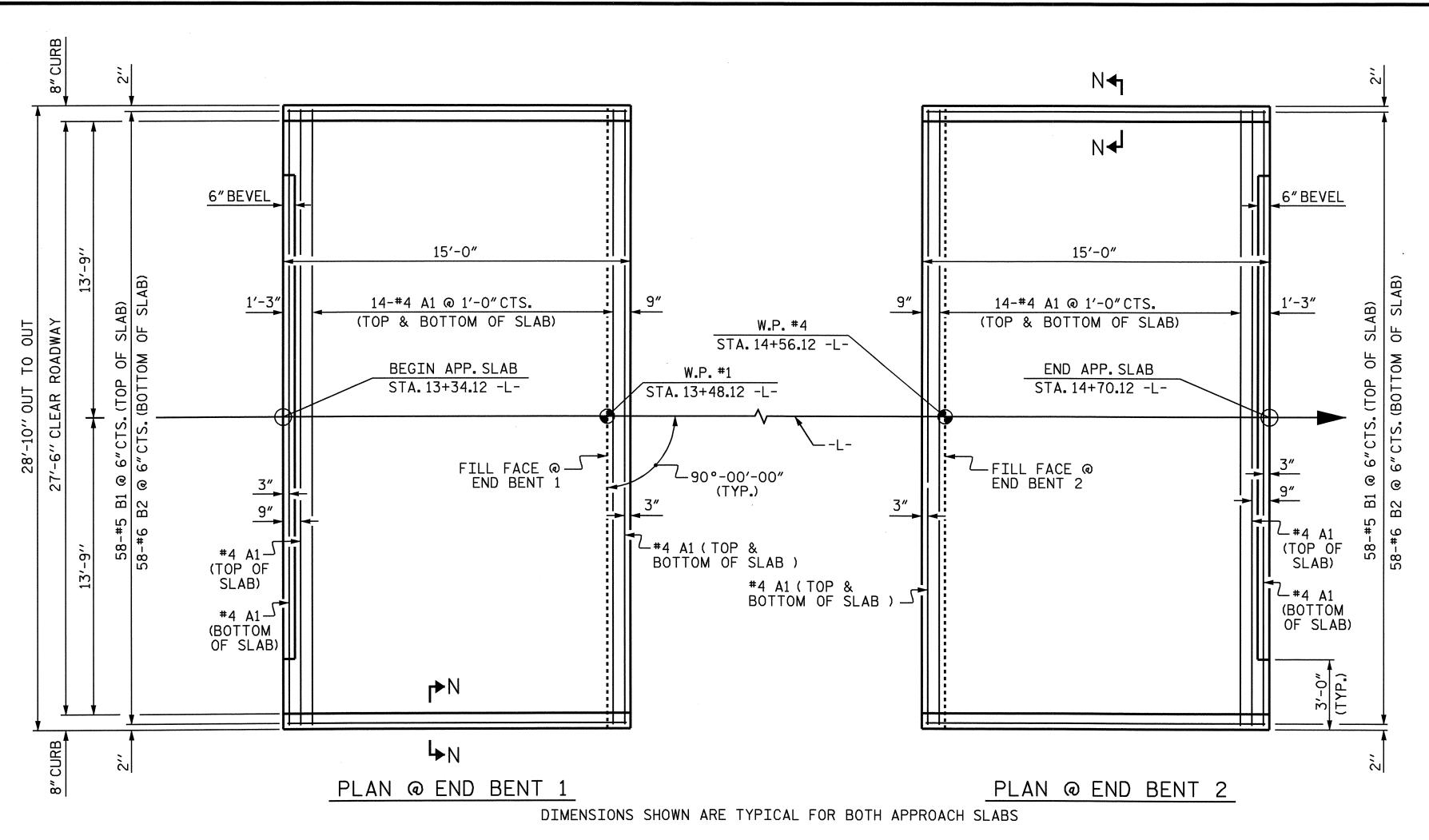
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BERM RIP RAPPED

STD. NO. RR2



FOR REINFORCED BRIDGE APPROACH FILL INCLUDING FABRIC, IMPERMEABLE GEOMEMBRANE, 4" Ø DRAINAGE PIPE, #78M STONE, AND SELECT MATERIAL, SEE ROADWAY PLANS.

AREA BETWEEN THE WINGWALL AND APPROACH SLAB SHALL BE GRADED TO DRAIN THE WATER AWAY FROM THE FILL FACE OF THE BRIDGE AND SHALL BE PAVED. SEE ROADWAY PLANS.

THE 6"COMP. A.B.C. SHALL BE FLUSH WITH THE ROADWAY END OF THE APPROACH SLAB AND SHALL EXTEND 1'-0"OUTSIDE OF EACH EDGE OF THE APPROACH SLAB.

THE CONTRACTOR MAY USE 4"TYPE B-25.0B ASPHALT CONCRETE BASE COURSE IN LIEU OF 6"COMP. A.B.C. IF THIS OPTION IS USED, THE BASE COURSE SHALL BE FLUSH WITH THE ROADWAY END OF THE APPROACH SLAB, AND THE WIDTH SHALL BE THE SAME AS THAT OF THE APPROACH SLAB.

THE CONTRACTOR MAY USE 5"CLASS "A" CONCRETE BASE IN LIEU OF 6" COMP. A.B.C. IF THIS OPTION IS USED, THE CONCRETE BASE SHALL BE FLUSH WITH THE ROADWAY END OF THE APPROACH SLAB, AND THE WIDTH SHALL BE THE SAME AS THAT OF THE APPROACH SLAB. THE CONCRETE SHALL BE FINISHED TO A SMOOTH SURFACE AND A LAYER OF 30 LB ROOFING FELT SHALL BE PLACED BETWEEN THE CONCRETE BASE AND THE APPROACH SLAB TO PREVENT BOND. THE APPROACH SLAB SHALL NOT BE CAST UNTIL THE CONCRETE BASE HAS REACHED AN AGE OF THREE CURING DAYS.

FOR JOINT DETAILS, SEE "PRESTRESSED CONCRETE CORED SLAB UNIT" SHEETS.

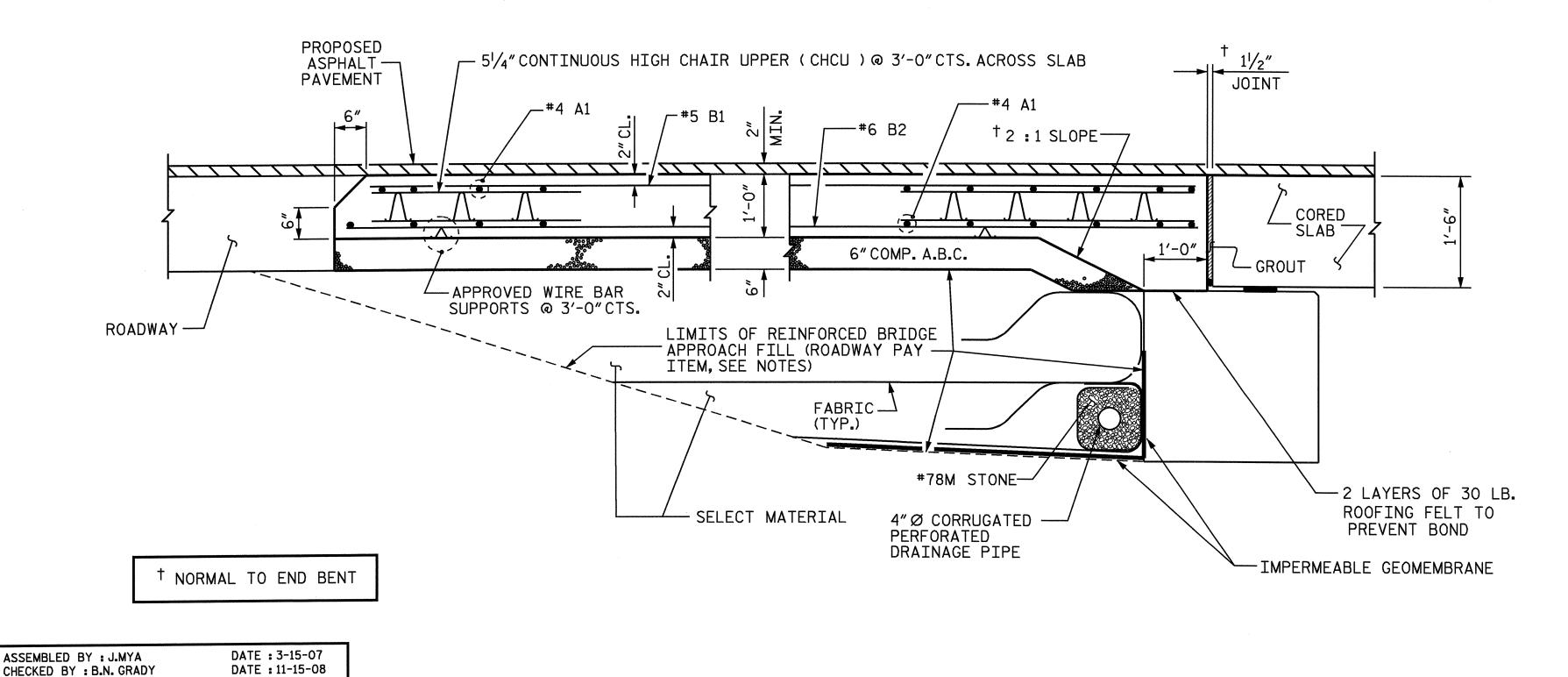
THE JOINT AT THE END BENT SHALL BE GROUTED AS SOON AS PRACTICAL AFTER THE CONSTRUCTION OF THE APPROACH SLABS.

APPROACH SLAB GROOVING IS NOT REQUIRED.

	BILL OF MATERIAL					
F(FOR ONE APPROACH SLAB (2 REQ'D)					
BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT	
* A1	32	#4	STR	28'-6''	609	
	1					
★ B1	58	#5	STR	14'-2''	857	
★ B2	58	#6	STR	14'-8''	1278	
<pre>* EPOXY COATED REINFORCING STEEL LBS. 2744</pre>						

C. Y. 16.8

CLASS AA CONCRETE



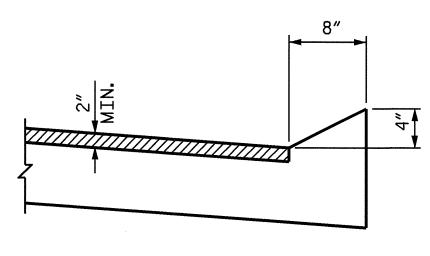
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LES/RDR RWW/JTE

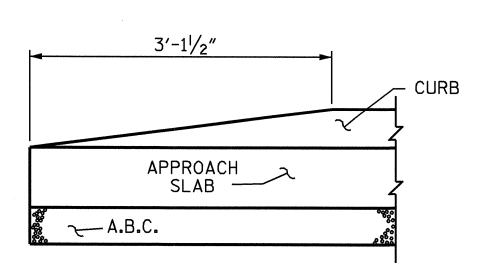
KMM/GM

REV. 7/10/01 REV. 5/7/03R REV. 5/1/06R

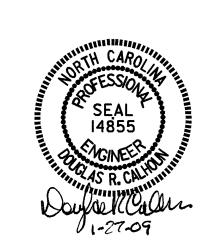
DRAWN BY: FCJ 6/87 CHECKED BY: EGA 6/87



SECTION N-N



END OF CURB WITHOUT SHOULDER BERM GUTTER SHOULDER BERM GUTTER IS REQUIRED)



PROJECT NO. B-4094

CURRITUCK COUNTY

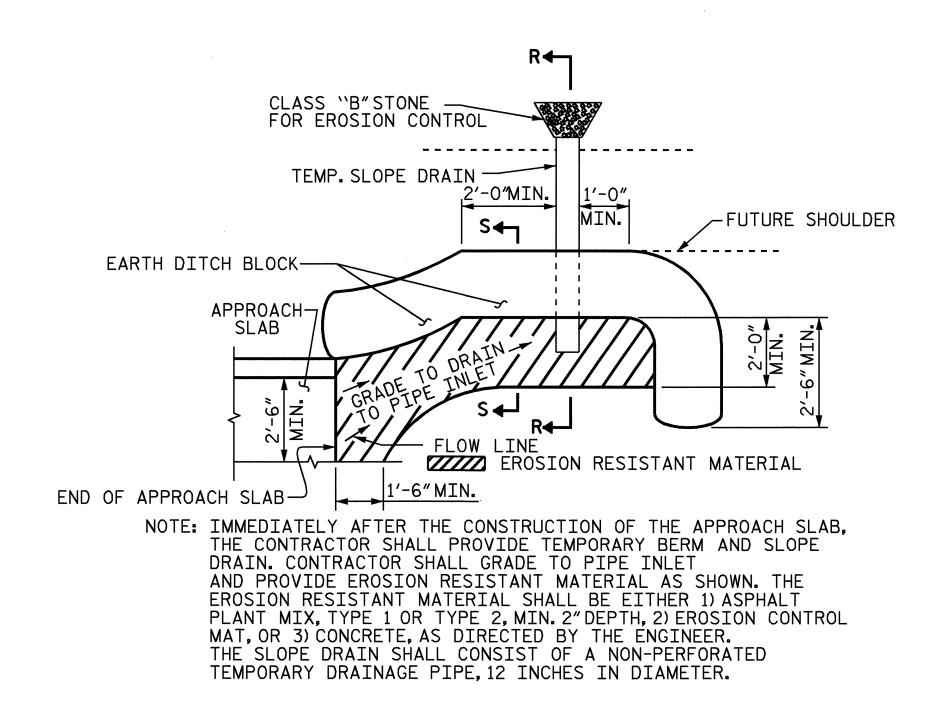
STATION: 14+02.12 -L-

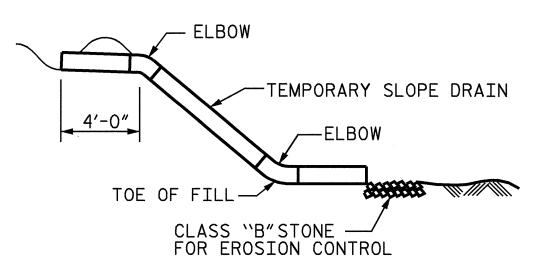
SHEET 1 OF 2

DEPARTMENT OF TRANSPORTATION
RALEIGH
STANDARD

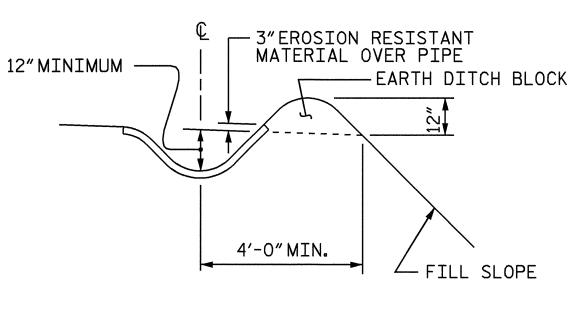
BRIDGE APPROACH SLAB FOR PRESTRESSED CONCRETE CORED SLAB

		RE\	/ISIONS)		SHEET NO.
0.	BY:	DATE:	NO.	BY:	DATE:	S-23
			3			TOTAL SHEETS
2			4			24





SECTION R-R

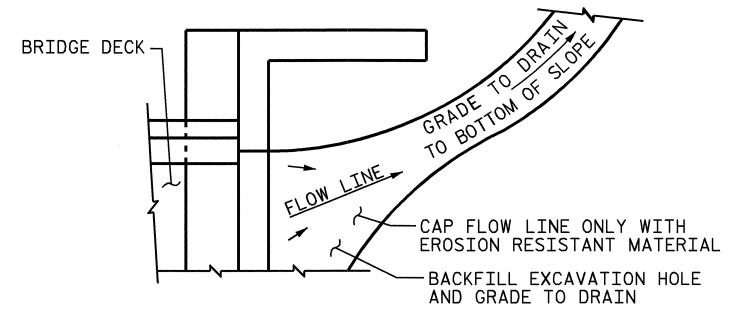


SECTION S-S

PLAN VIEW

TEMPORARY BERM AND SLOPE DRAIN DETAILS

(TO BE USED WHEN SHOULDER BERM GUTTER IS REQUIRED)



NOTE: IF THE APPROACH SLAB IS NOT CONSTRUCTED IMMEDIATELY AFTER THE BACKFILLING OF THE END BENT EXCAVATION, GRADE TO DRAIN TO THE BOTTOM OF THE SLOPE AND PROVIDE EROSION RESISTANT MATERIAL, SUCH AS FIBERGLASS ROVING OR AS DIRECTED BY THE ENGINEER TO PREVENT SOIL EROSION AND TO PROTECT THE AREA ADJACENT TO THE STRUCTURE. THE CONTRACTOR WILL BE REQUIRED TO REMOVE THESE MATERIALS PRIOR TO CONSTRUCTION OF THE APPROACH SLAB.

TEMPORARY DRAINAGE DETAIL

PROJECT NO. B-4094 CURRITUCK 14+02.12 -L-SHEET 2 OF 2

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH

STANDARD BRIDGE APPROACH SLAB DETAILS

					1988
	REVISIONS				
BY:	DATE:	NO.	BY:	DATE:	S-24
		3			TOTAL SHEETS
		4			24

STD. NO. BAS10

ASSEMBLED BY : J. MYA	DATE : 03-15-07		
CHECKED BY : B.N. GRADY	DATE : II-15-08		
DRAWN BY: FCJ 11/88 CHECKED BY: ARB 11/88	REV.10/17/00 RWW/LES REV.5/7/03 RWW/JTE REV.5/1/06 TLA/GM		

STANDARD NOTES

DESIGN DATA:

---- A.A.S.H.T.O. (CURRENT) SPECIFICATIONS LIVE LOAD ---- SEE PLANS IMPACT ALLOWANCE ---- SEE A.A.S.H.T.O. STRESS IN EXTREME FIBER OF STRUCTURAL STEEL - AASHTO M270 GRADE 36 - 20,000 LBS. PER SQ. IN. - AASHTO M270 GRADE 50W - 27,000 LBS. PER SQ. IN. - AASHTO M270 GRADE 50 - 27,000 LBS. PER SQ. IN. REINFORCING STEEL IN TENSION GRADE 60 -- 24,000 LBS. PER SQ. IN. CONCRETE IN COMPRESSION ---- 1,200 LBS. PER SQ. IN. CONCRETE IN SHEAR ---- SEE A.A.S.H.T.O. STRUCTURAL TIMBER - TREATED OR UNTREATED - EXTREME FIBER STRESS - - - - - 1,800 LBS. PER SQ. IN.

MATERIAL AND WORKMANSHIP:

EQUIVALENT FLUID PRESSURE OF EARTH ----

COMPRESSION PERPENDICULAR TO GRAIN

EXCEPT AS MAY OTHERWISE BE SPECIFIED ON PLANS OR IN THE SPECIAL PROVISIONS, ALL MATERIAL AND WORKMANSHIP SHALL BE IN ACCORDANCE WITH THE 2006 "STANDARD SPECIFICATIONS FOR ROADS AND STRUCTURES" OF THE N. C. DEPARTMENT OF TRANSPORTATION.

OF TIMBER ----

375 LBS. PER SQ. IN.

(MINIMUM)

30 LBS. PER CU. FT.

STEEL SHEET PILING FOR PERMANENT OR TEMPORARY APPLICATIONS SHALL BE HOT ROLLED.

CONCRETE:

UNLESS OTHERWISE REQUIRED ON PLANS, CLASS A CONCRETE SHALL BE USED FOR ALL PORTIONS OF ALL STRUCTURES WITH THE EXCEPTION THAT: CLASS AA CONCRETE SHALL BE USED IN BRIDGE SUPERSTRUCTURES, ABUTMENT BACKWALLS, AND APPROACH SLABS; AND CLASS B CONCRETE SHALL BE USED FOR SLOPE PROTECTION AND RIP RAP.

CONCRETE CHAMFERS:

UNLESS OTHERWISE NOTED ON THE PLANS, ALL EXPOSED CORNERS ON STRUCTURES SHALL BE CHAMFERED 3/4" WITH THE FOLLOWING EXCEPTIONS: TOP CORNERS OF CURBS MAY BE ROUNDED TO 1-1/2" RADIUS WHICH IS BUILT INTO CURB FORMS; CORNERS OF TRANSVERSE FLOOR EXPANSION JOINTS SHALL BE ROUNDED WITH A 1/4" FINISHING TOOL UNLESS OTHERWISE REQUIRED ON PLANS; AND CORNERS OF EXPANSION JOINTS IN THE ROADWAY FACES AND TOPS OF CURBS AND SIDEWALKS SHALL BE ROUNDED TO A 1/4" RADIUS WITH A FINISHING STONE OR TOOL UNLESS OTHERWISE REQUIRED ON PLANS.

DOWELS:

DOWELS WHEN INDICATED ON PLANS AS FOR CULVERT EXTENSIONS, SHALL BE EMBEDDED AT LEAST 12"INTO THE OLD CONCRETE AND GROUTED INTO PLACE WITH 1:2 CEMENT MORTAR.

ALLOWANCE FOR DEAD LOAD DEFLECTION, SETTLEMENT, ETC. IN CASTING SUPERSTRUCTURES:

BRIDGES SHALL BE BUILT ON THE GRADE OR VERTICAL CURVE SHOWN ON PLANS. SLABS, CURBS AND PARAPETS SHALL CONFORM TO THE GRADE OR CURVE.

ALL DIMENSIONS WHICH ARE GIVEN IN SECTION AND ARE AFFECTED BY DEAD LOAD DEFLECTIONS ARE DIMENSIONS AT CENTER LINE OF BEARING UNLESS OTHERWISE NOTED ON PLANS. IN SETTING FORMS FOR STEEL BEAM BRIDGES AND PRESTRESSED CONCRETE GIRDER BRIDGES, ADJUSTMENTS SHALL BE MADE DUE TO THE DEAD LOAD DEFLECTIONS FOR THE ELEVATIONS SHOWN. WHERE BLOCKS ARE SHOWN OVER BEAMS FOR BUILDING UP TO THE SLAB, THE VERTICAL DIMENSIONS OF THE BLOCKS SHALL BE ADJUSTED BETWEEN BEARINGS TO COMPENSATE FOR DEAD LOAD DEFLECTIONS, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER. WHERE BOTTOM OF SLAB IS IN LINE WITH BOTTOM OF TOP FLANGES, DEPTH OF SLAB BETWEEN BEARINGS SHALL BE ADJUSTED TO COMPENSATE FOR DEAD LOAD DEFLECTION, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER.

IN SETTING FALSEWORK AND FORMS FOR REINFORCED CONCRETE SPANS, AN ALLOWANCE SHALL BE MADE FOR DEAD LOAD DEFLECTIONS, SETTLEMENT OF FALSEWORK, AND PERMANENT CAMBER WHICH SHALL BE PROVIDED FOR IN ADDITION TO THE ELEVATIONS SHOWN. AFTER REMOVAL OF THE FALSEWORK, THE FINISHED STRUCTURES SHALL CONFORM TO THE PROFILE AND ELEVATIONS SHOWN ON THE PLANS AND CONSTRUCTION ELEVATIONS FURNISHED BY THE ENGINEER.

DETAILED DRAWINGS FOR FALSEWORK OR FORMS FOR BRIDGE SUPERSTRUCTURE AND ANY STRUCTURE OR PARTS OF A STRUCTURE AS NOTED ON THE PLANS SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL BEFORE CONSTRUCTION OF THE FALSEWORK OR FORMS IS STARTED.

REINFORCING STEEL:

ALL REINFORCING STEEL SHALL BE DEFORMED. DIMENSIONS RELATIVE TO PLACEMENT OF REINFORCING ARE TO CENTERS OF BARS UNLESS OTHERWISE INDICATED IN THE PLANS. DIMENSIONS ON BAR DETAILS ARE TO CENTERS OF BARS OR ARE OUT TO OUT AS INDICATED ON PLANS.

WIRE BAR SUPPORTS SHALL BE PROVIDED FOR REINFORCING STEEL WHERE INDICATED ON THE PLANS. WHEN BAR SUPPORT PIECES ARE PLACED IN CONTINUOUS LINES, THEY SHALL BE SO PLACED THAT THE ENDS OF THE SUPPORTING WIRES SHALL BE LAPPED TO LOCK LEGS ON ADJOINING PIECES.

STRUCTURAL STEEL:

AT THE CONTRACTOR'S OPTION, HE MAY SUBSTITUTE 7/8" Ø SHEAR STUDS FOR THE 3/4" Ø STUDS SPECIFIED ON THE PLANS. THIS SUBSTITUTION SHALL BE MADE AT THE RATE OF 3 - 7/8" Ø STUDS FOR 4 - 3/4" Ø STUDS, AND STUD SPACING CHANGES SHALL BE MADE AS NECESSARY TO PROVIDE THE SAME EQUIVALENT NUMBER OF 7/8" Ø STUDS ALONG THE BEAM AS SHOWN FOR 3/4" Ø STUDS BASED ON THE RATIO OF 3 - 7/8" Ø STUDS FOR 4 - 3/4" Ø STUDS. STUDS OF THE LENGTH SPECIFIED ON THE PLANS MUST BE PROVIDED. THE MAXIMUM SPACING SHALL BE 2'-0".

EXCEPT AT THE INTERIOR SUPPORTS OF CONTINUOUS BEAMS WHERE THE COVER PLATE IS IN CONTACT WITH BEARING PLATE, THE CONTRACTOR MAY, AT HIS OPTION, SUBSTITUTE FOR THE COVER PLATES DESIGNATED ON THE PLANS COVER PLATES OF THE EQUIVALENT AREA PROVIDED THESE PLATES ARE AT LEAST 5/16"IN THICKNESS AND DO NOT EXCEED A WIDTH EQUAL TO THE FLANGE WIDTH LESS 2"OR A THICKNESS EQUAL TO 2 TIMES THE FLANGE THICKNESS. THE SIZE OF FILLET WELDS SHALL CONFORM TO THE REQUIREMENTS OF THE CURRENT ANSI/AASHTO/AWS "BRIDGE WELDING CODE". ELECTROSLAG WELDING WILL NOT BE PERMITTED.

WITH THE SOLE EXCEPTION OF EDGES AT SURFACES WHICH BEAR ON OTHER SURFACES, ALL SHARP EDGES AND ENDS OF SHAPES AND PLATES SHALL BE SLIGHTLY ROUNDED BY SUITABLE MEANS TO A RADIUS OF APPROXIMATELY 1/16 INCH OR EQUIVALENT FLAT SURFACE AT A SUITABLE ANGLE PRIOR TO PAINTING, GALVANIZING, OR METALLIZING.

HANDRAILS AND POSTS:

METAL STANDARDS AND FACES OF THE CONCRETE END POSTS FOR THE METAL RAIL SHALL BE SET NORMAL TO THE GRADE OF THE CURB, UNLESS OTHERWISE SHOWN ON PLANS. THE METAL RAIL AND TOPS OF CONCRETE POSTS USED WITH THE ALUMINUM RAIL SHALL BE BUILT PARALLEL TO THE GRADE OF THE CURB.

METAL HANDRAILS SHALL BE IN ACCORDANCE WITH THE PLANS. RAILS SHALL BE AS MANUFACTURED FOR BRIDGE RAILING. CASTINGS SHALL BE OF A UNIFORM APPEARANCE. FINS AND OTHER DEFORMATIONS RESULTING FROM CASTING OR OTHERWISE SHALL BE REMOVED IN A MANNER SO THAT A UNIFORM COLORING OF THE COMPLETED CASTING SHALL BE OBTAINED. CASTINGS WITH DISCOLORATIONS OR OF NON-UNIFORM COLORING WILL NOT BE ACCEPTED. CERTIFIED MILL REPORTS ARE REQUIRED FOR METAL RAILS AND POSTS.

SPECIAL NOTES:

GENERALLY, IN CASE OF DISCREPANCY, THIS STANDARD SHEET OF NOTES SHALL GOVERN OVER THE SPECIFICATIONS, BUT THE REMAINDER OF THE PLANS SHALL GOVERN OVER NOTES HEREON, AND SPECIAL PROVISIONS SHALL GOVERN OVER ALL. SEE SPECIFICATIONS ARTICLE 105-4.

ENGLISH

JANUARY, 1990