

TOTAL LENGTH OF TIP PROJECT U-3462 = 1.225 MILES

LETTING DATE:

March 17, 2009

PROJECT ENGINEER

TIMOTHY L. COGGINS, PE PROJECT DESIGN ENGINEER

STATE HIGHWAY DESIGN ENGINEER

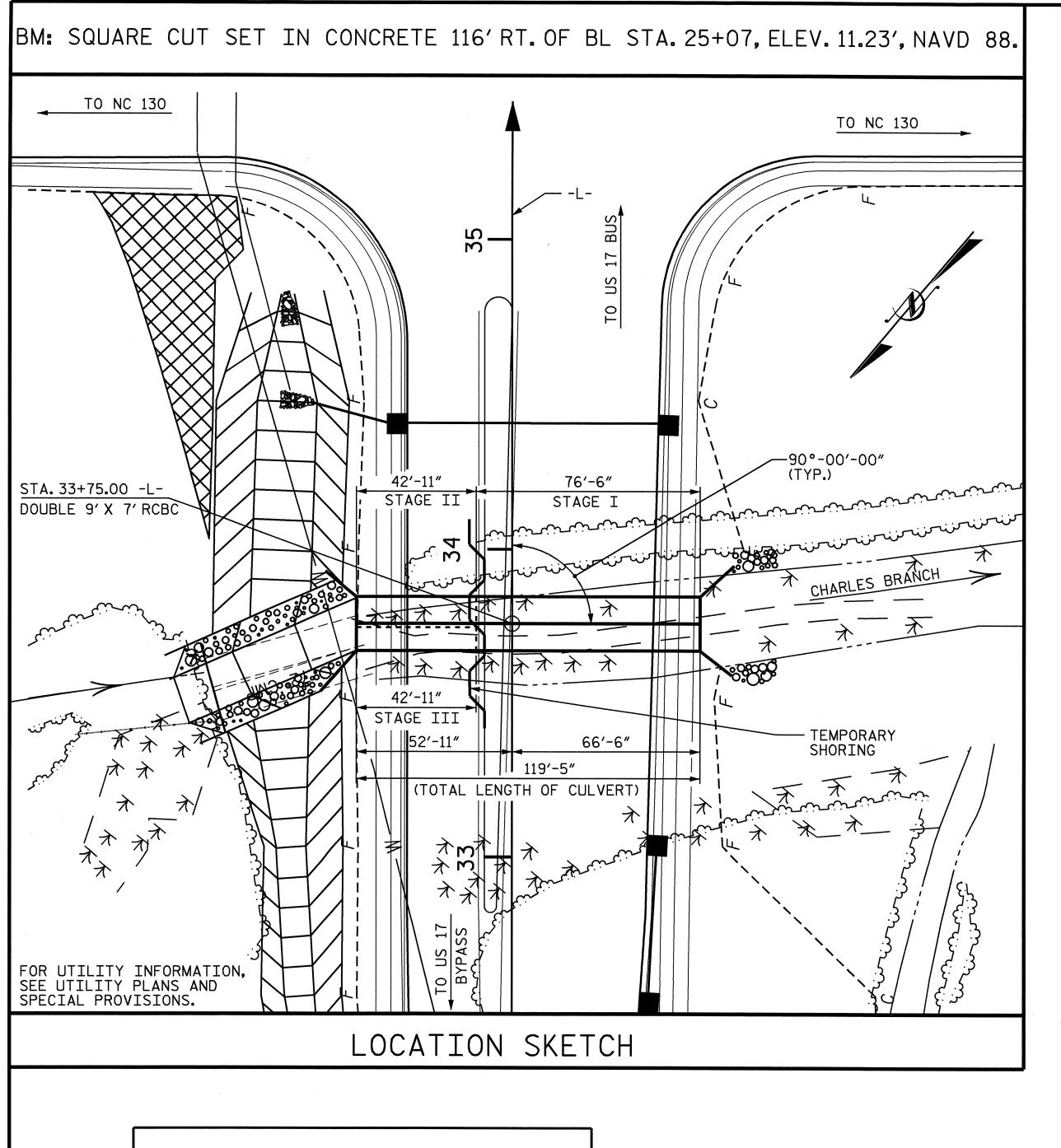
T = 3 % *

V = 40 MPH

FUNCTIONAL CLASSICATION MAJOR COLLECTOR

* TTST 1%

DUAL 2%



STRUCTURE QUANTITIES STAGE I CLASS A CONCRETE BARREL @ 1.79 CY/FT 137.0 C.Y. 14.8 C.Y. WINGS ETC. _____ 151.8 C.Y. TOTAL _____ EPOXY COATED REINFORCING STEEL 26901 LBS. WINGS ETC. 828 LBS. TOTAL 27729 LBS. CULVERT EXCAVATION LUMP SUM FOUNDATION COND. MAT'L. 108 TONS STRUCTURE QUANTITIES STAGE II CLASS A CONCRETE BARREL @ 1.07 CY/FT 45.9 C.Y. 7.5 C.Y. WING ETC. _____ 53.4 C.Y. TOTAL _____ EPOXY COATED REINFORCING STEEL 10946 LBS. WING ETC. _____ 414 LBS. 11360 LBS. TOTAL CULVERT EXCAVATION LUMP SUM FOUNDATION COND. MAT'L. 34 TONS STRUCTURE QUANTITIES STAGE III CLASS A CONCRETE BARREL @ ___O.72 CY/FT 30.9 C.Y. 7.3 C.Y. WING ETC. _____ 38.2_c.y. TOTAL _____ EPOXY COATED REINFORCING STEEL 6417 LBS. WING ETC. ____ 414 LBS. 6831 LBS. TOTAL CULVERT EXCAVATION LUMP SUM FOUNDATION COND. MAT'L 26 TONS

TOTAL STRUCTURE QUANTITIES

CLASS A CONCRETE <u>213.8</u> C.Y. BARREL @ __1.79 CY/FT 29.6 C.Y. WINGS ETC. 243.4 _c.Y. TOTAL _____ EPOXY COATED REINFORCING STEEL 44264 LBS.

NOTES

ASSUMED LIVE LOAD -----HS20-44 OR ALTERNATE LOADING.

DESIGN FILL----- MAX, 3,28' ----- MTN, 2,18'

FOR OTHER DESIGN DATA AND NOTES SEE STANDARD NOTE SHEET.

3"Ø WEEP HOLES INDICATED TO BE IN ACCORDANCE WITH THE SPECIFICATIONS.

CONCRETE IN CULVERTS TO BE POURED IN THE FOLLOWING ORDER:

STAGE I CONSTRUCTION

- 1. STAGE I WING FOOTINGS AND FLOOR SLAB INCLUDING 4" OF ALL VERTICAL WALLS.
- 2. THE REMAINING PORTIONS OF THE STAGE I WALLS AND WINGS FULL HEIGHT FOLLOWED BY ROOF SLAB AND HEADWALL. STAGE II CONSTRUCTION

F. A. PROJECT NO. STP-1357(4)

- 1. STAGE II WING FOOTING AND FLOOR SLAB INCLUDING 4" OF ALL VERTICAL WALLS.
- 2. THE REMAINING PORTIONS OF THE STAGE II WALLS AND WING FULL HEIGHT FOLLOWED BY ROOF SLAB AND HEADWALL.

STAGE III CONSTRUCTION

- 1. STAGE III WING FOOTING AND FLOOR SLAB INCLUDING 4" OF VERTICAL WALL.
- 2. THE REMAINING PORTION OF THE STAGE III WALL AND WING FULL HEIGHT FOLLOWED BY ROOF SLAB AND HEADWALL.
- THE RESIDENT ENGINEER SHALL CHECK THE LENGTH OF CULVERT BEFORE STAKING IT OUT TO MAKE CERTAIN THAT IT WILL PROPERLY TAKE CARE OF THE FILL.

DIMENSIONS FOR WING LAYOUT AS WELL AS ADDITIONAL REINFORCING STEEL EMBEDDED IN BARREL ARE SHOWN ON WING SHEET.

TRANSVERSE CONSTRUCTION JOINTS SHALL BE USED IN THE BARREL, SPACED TO LIMIT THE POURS TO A MAXIMUM OF 70 FT.LOCATION OF JOINTS SHALL BE SUBJECT TO APPROVAL OF THE ENGINEER.

STEEL IN THE BOTTOM SLAB MAY BE SPLICED AT THE PERMITTED CONSTRUCTION JOINT AT THE CONTRACTOR'S OPTION. EXTRA WEIGHT OF STEEL DUE TO THE SPLICES SHALL BE PAID FOR BY THE CONTRACTOR.

THE SPLICE LENGTH SHALL BE AS PROVIDED IN THE SPLICE LENGTH CHART SHOWN ON THE PLANS. EXTRA WEIGHT OF STEEL DUE TO THE SPLICES AHLL BE PAID FOR BY THE CONTRACTOR.

FOR CULVERT DIVERSION DETAILS AND PAY ITEM, SEE EROSION CONTROL PLANS.

A 3 FOOT STRIP OF FILTER FABRIC SHALL BE ATTACHED TO THE FILL FACE OF THE WING COVERING THE ENTIRE LENGTH OF THE EXPANSION JOINT.

FOR SUBMITTAL OF WORKING DRAWINGS, SEE SPECIAL PROVISIONS.

FOR FALSEWORK AND FORMWORK, SEE SPECIAL PROVISIONS.

NO PRECAST REINFORCED BOX CULVERT OPTION WILL BE ALLOWED.

FOR GROUT FOR STRUCTURES. SEE SPECIAL PROVISIONS.

FOR CRANE SAFETY, SEE SPECIAL PROVISIONS.

THE CONTRACTOR SHALL PROVIDE INDEPENDENT ASSURANCE SAMPLES OF REINFORCING STEEL AS FOLLOWS: FOR PROJECTS REQUIRING UP TO 400 TONS OF REINFORCING STEEL, ONE 30 INCH SAMPLE OF EACH SIZE BAR USED, AND FOR PROJECTS REQUIRING OVER 400 TONS OF REINFORCING STEEL, TWO 30 INCH SAMPLES OF EACH SIZE BAR USED. THE BARS FROM WHICH THE SAMPLES ARE TAKEN MUST THEN BE SPLICED WITH REPLACEMENT BARS OF THE SIZE AND LENGTH OF THE SAMPLE, PLUS A MINIMUM LAP SPLICE OF THIRTY BAR DIAMETERS.

ALL BAR SUPPORTS USED IN THE CULVERT AND ALL INCIDENTAL REINFORCING STEEL SHALL BE EPOXY COATED IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

FOR MAINTENANCE OF TRAFFIC, SEE TRAFFIC CONTROL PLANS.

FOR LIMITS OF TEMPORARY SHORING, SEE TRAFFIC CONTROL PLANS. FOR PAY ITEM FOR TEMPORARY SHORING, SEE ROADWAY PLANS.

GRADE DATA

GRADE POINT ELEVATION @ STA. 33+75.00 -L-

STA. 33+75.00 -L-

10.257 FT. 0.0 FT.

3:1

ROADWAY SLOPES

DRAWN BY : PEGGY ADKINS DATE: 6-05 CHECKED BY : B.N. BARODAWALA DATE : 10-08

BED ELEVATION @

HYDRAULIC DATA

= 390 c.f.s. DESIGN DISCHARGE = 25 YR. FREQUENCY OF DESIGN FLOOD DESIGN HIGH WATER ELEVATION = 7.2 FT. DRAINAGE AREA $= 1.46 \text{ MI}^2$

BASIC DISCHARGE (Q100) = 710 c.f.s.BASIC HIGH WATER ELEVATION = 8.8 FT.

OVERTOPPING FLOOD DATA

OVERTOPPING DISCHARGE = 950 c.f.s. FREQUENCY OF OVERTOPPING FLOOD = 100 YR.+ = 9.6 FT. OVERTOPPING FLOOD ELEVATION

35'-0" 53'-0" 65'-0" 45'-0" 60'-0" ,∕-- (£ -L-

WINGS ETC.

TOTAL _____

CULVERT EXCAVATION

FOUNDATION COND. MAT'L.

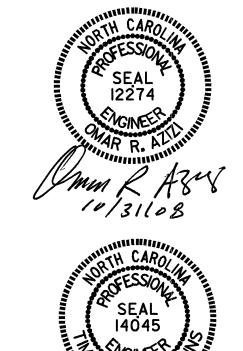
1656 LBS.

45920 LBS.

LUMP SUM

168 TONS

PROFILE ALONG & CULVERT



PROJECT NO. U-3462 BRUNSWICK COUNTY STATION: 33+75.00 -L-

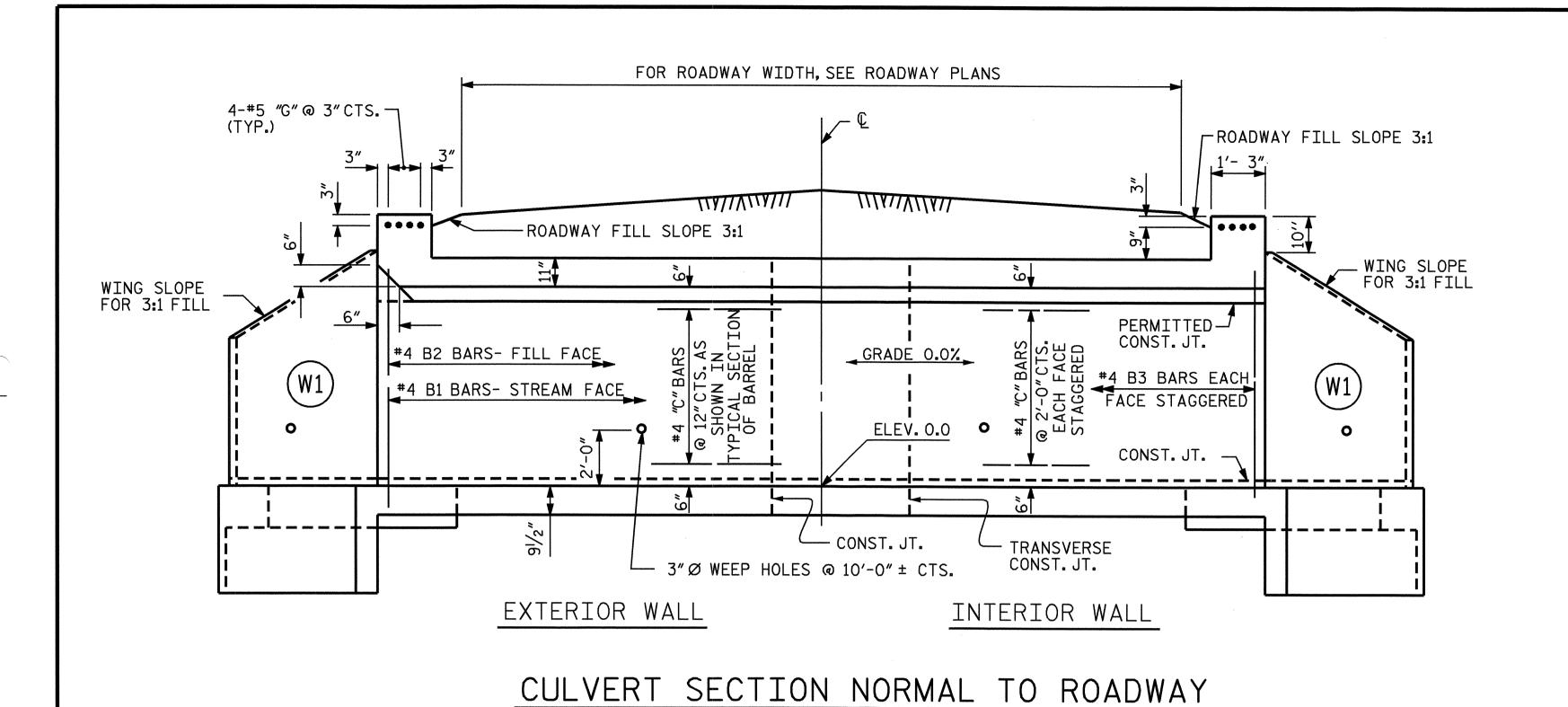
SHEET 1 OF 5

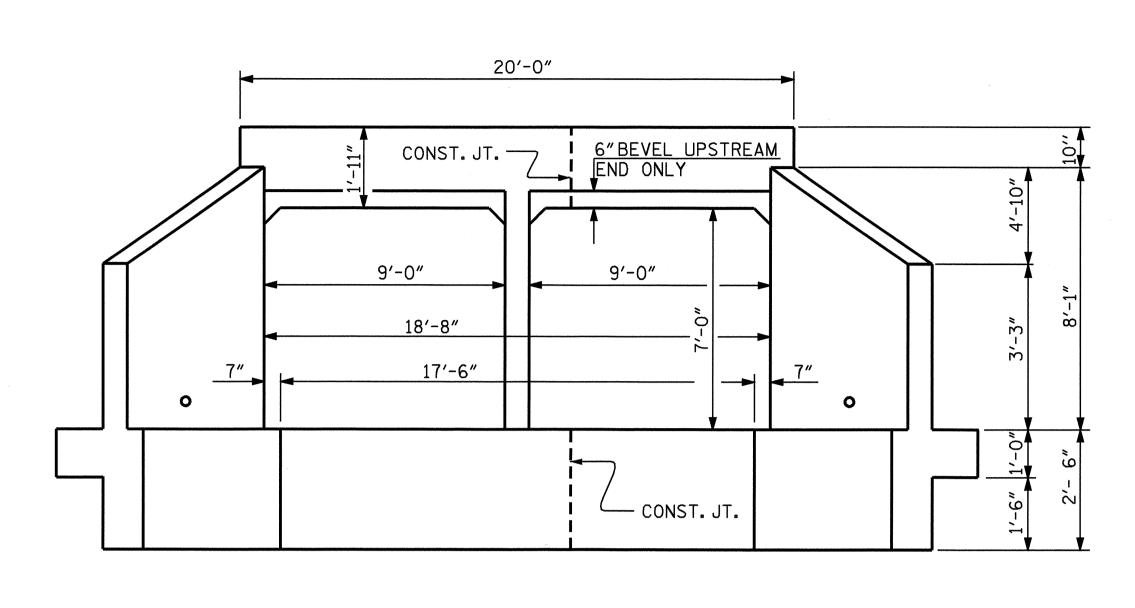
STR. #1

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH

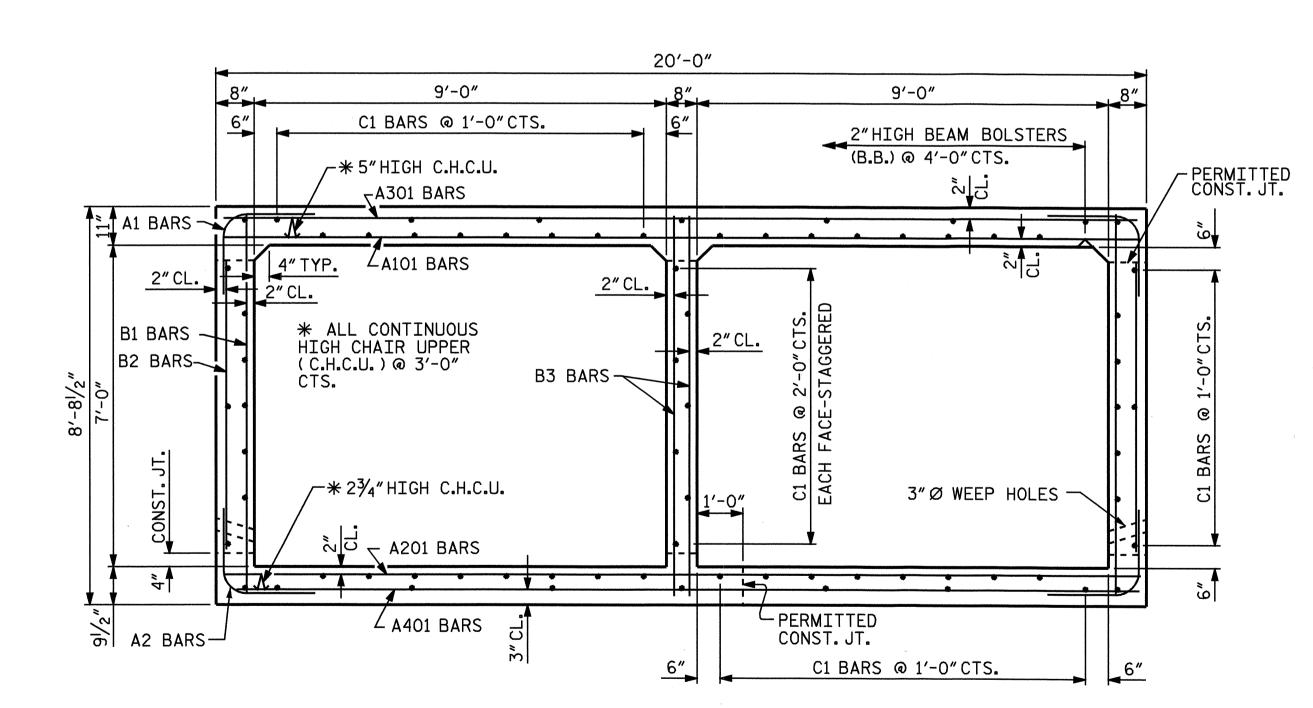
DOUBLE 9 FT. X 7 FT. CONCRETE BOX CULVERT 90° SKEW

	SHEET NO.			
NO.	BY:	C−1		
1		3		TOTAL SHEETS
2		4		10





END ELEVATION

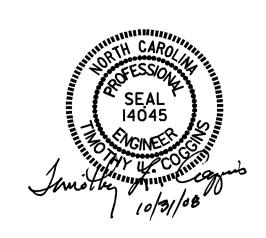




20'-0" 11'-4" (STAGE II) 8'-8" (STAGE III) 9'-0" 9'-0" C2 BARS @ 1'-0"CTS. 2"HIGH BEAM BOLSTERS
(B.B.) @ 4'-0"CTS. /÷ ★ 5″HIGH C.H.C.U. CONST. JT. NO -A303 BARS -A302 BARS A1 BARS -4" TYP. 2" CL. 2"CL. *5"HIGH C.H.C.U. * ALL CONTINUOUS
HIGH CHAIR UPPER
(C.H.C.U.)@ 3'-0"
CTS. B1 BARS B2 BARS B3 BARS — /─* 2¾″HIGH C.H.C.U. 3"Ø WEEP HOLES -* 23/4" HIGH C.H.C.U. 7 ─ A202 BARS A203 BARS CONST. JT. ∠ A402 BARS ∠ A403 BARS A2 BARS-C2 BARS @ 1'-0"CTS.

RIGHT ANGLE SECTION OF BARREL -- STAGE II & III

THERE ARE 73 "C" BARS IN SECTION OF BARREL. (THIS VIEW IS LOOKING DOWNSTREAM)



U-3462 PROJECT NO. ___ BRUNSWICK __ COUNTY STATION: 33+75.00 -L-

SHEET 2 OF 5

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

DOUBLE 9 FT. X 7 FT. CONCRETE BOX CULVERT 90° SKEW

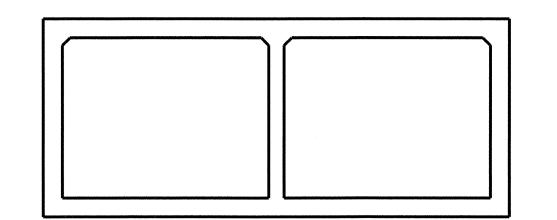
	SHEET NO.									
3Y:	DATE:	NO.	BY:	DATE:	C-2					
		3			TOTAL SHEETS					
		4			10					
- N.	NO 0040									

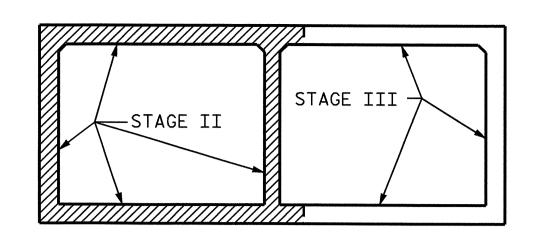
ASSEMBLED BY : PEGGY ADKINS DATE : 6-05 CHECKED BY : B.N. BARODAWALA DATE : 10-08 **SPECIAL** DRAWN BY : RALPH D. UNDERWOOD
CHECKED BY : JOEL A. JOHNSON __ DATE : MAY 1971 __ DATE : JULY 1971 STANDARD

31-0CT-2008 12:26

STD. NO. CB12

CTD #1

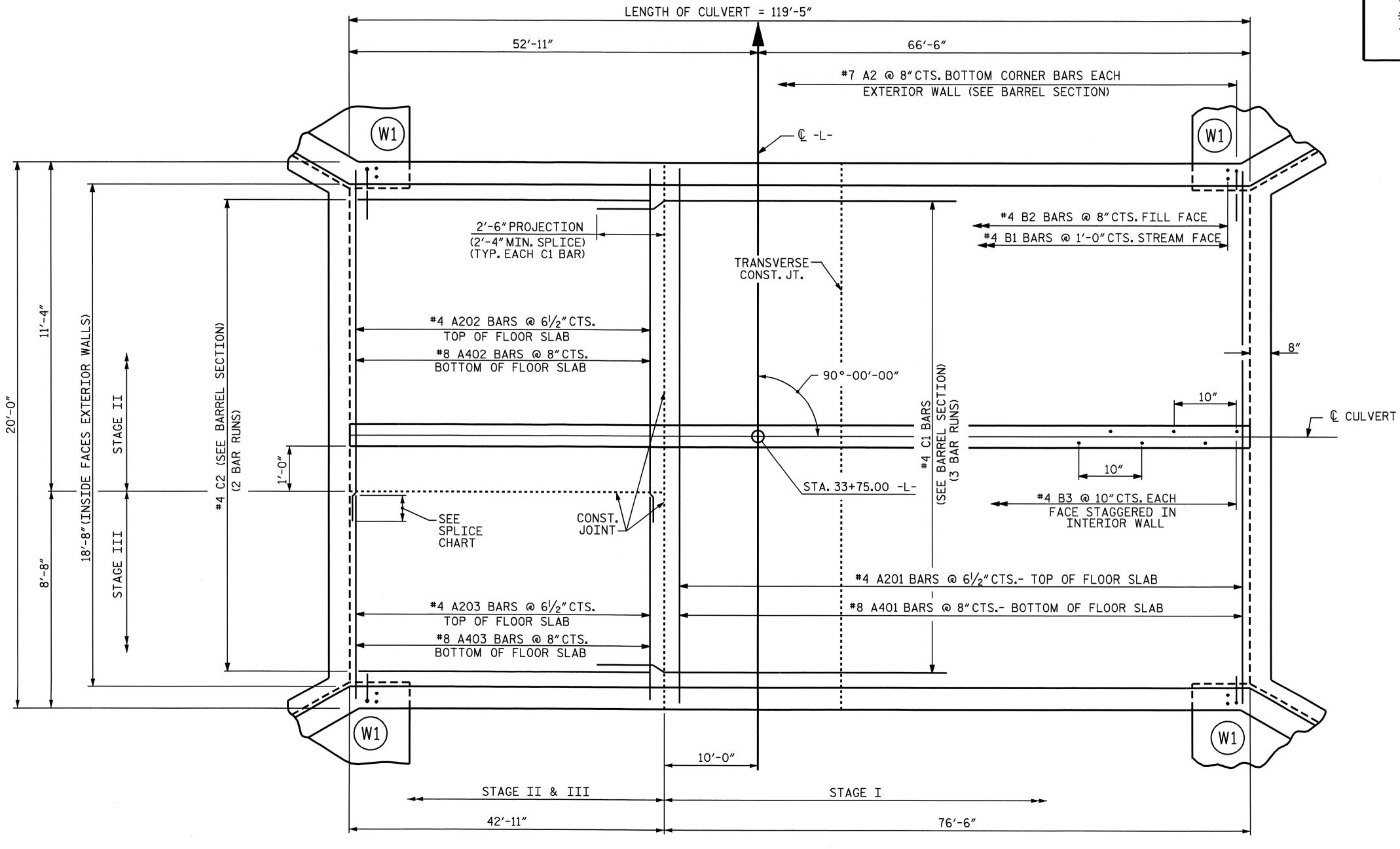




STAGE I

STAGE II AND III (LOOKING DOWN STREAM)

CONSTRUCTION SEQUENCE



BAR TYPE VERTICAL LEG-6"R.¬ BAR DIMENSIONS ARE OUT TO OUT

BAR TYPE

SIZE SPLICE LENGTH * A"100" 4'-3" * A"200" 2'-0" * A"300" 4'-9" 6'-2" * A"400" **∗** B1 2'-0" **₩** B3 2'-0" * C1 2'-4"

BAR | NO. | SIZE | TYPE | LENGTH | WEIGHT 5 | 6 | 4'-8" 1119 * A2 230 7 6 6′-9″ 3173 * A101 108 STR 19'-7" 3177 * A201 141 STR 19'-7" 1845 STR 19'-7" * A301 115 4603 * A401 115 8 STR 19'-7" 6013 * B1 154 4 STR 8'-2" 840 STR 6'-4" * B2 230 973 4 4 STR 8'-2" * B3 184 1004 * C1 219 4 STR 27'-10" 4072 * G1 4 5 STR 19'-8" 82 * EPOXY COATED REINFORCING STEEL 26901 LBS. STAGE II BAR | NO. | SIZE | TYPE | LENGTH | WEIGHT 4'-8" * A1 65 316

6'-9"

16'-1"

8'-2"

6'-4"

8'-2"

4'-8"

6'-9"

8'-4"

8'-4"

6 STR 15′-7″

4 STR 13'-4"

8 STR 17'-6"

STR

4 STR

STAGE III

BAR | NO. | SIZE | TYPE | LENGTH | WEIGHT

4 STR 8'-4"

STR

8 STR

4 STR

* C2 86 4 STR 22'-6"

* G2 4 5 STR 14'-10"

* EPOXY COATED

* A2 65 7 6

REINFORCING STEEL

* A103 61 6 STR 8'-4"

* B1 43 4 STR 8'-2"

* B2 65 4 STR 6'-4"

* C2 60 4 STR 22'-6"

* G3 4 5 STR 8'-4"

* EPOXY COATED REINFORCING STEEL

897

1428

704

2137

3037

275

562

1293

62

316

897

440

1107

1446

235

275

902

35

6417 LBS.

10946 LBS.

* A2 65

* A102 61

* A202 79

★ A302 65

* A402 65

* B1 | 43 |

* B2 65

* A1 65

* A203 79

★ A303 65

* A403 65

* B3 103 4 STR

REINFORCING STEEL

BAR SCHEDULE

STAGE I

U-3462 PROJECT NO. ____ BRUNSWICK _ COUNTY STATION: 33+75.00 -L-

SHEET 3 OF 5

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

DOUBLE 9 FT. X 7 FT. CONCRETE BOX CULVERT 90° SKEW

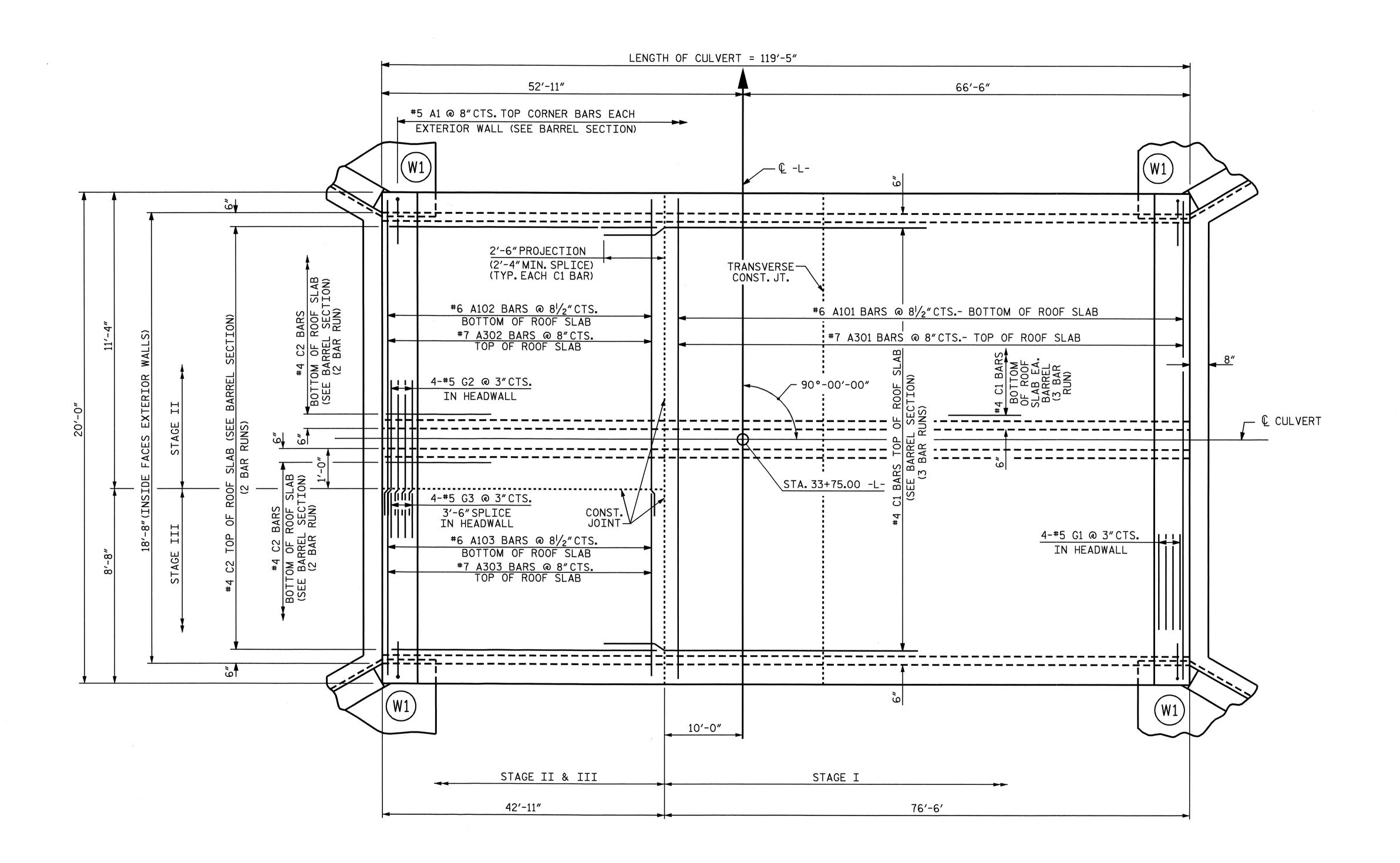
SHEET NO. REVISIONS C-3 NO. BY: DATE: TOTAL SHEETS 10

PLAN OF FLOOR SLAB

ASSEMBLED BY : PEGGY ADKINS DATE : 6-05 CHECKED BY : B.N. BARODAWALA DATE : 10-08

STD. NO. CR12A CTD #1

31-0CT-2008 12:25



PLAN OF ROOF SLAB



PROJECT NO. U-3462

BRUNSWICK COUNTY

STATION: 33+75.00 -L-

SHEET 4 OF 5

STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION
RALEIGH

DOUBLE 9 FT.X 7 FT. CONCRETE BOX CULVERT 90° SKEW

REVISIONS

SHEET NO.

C-4

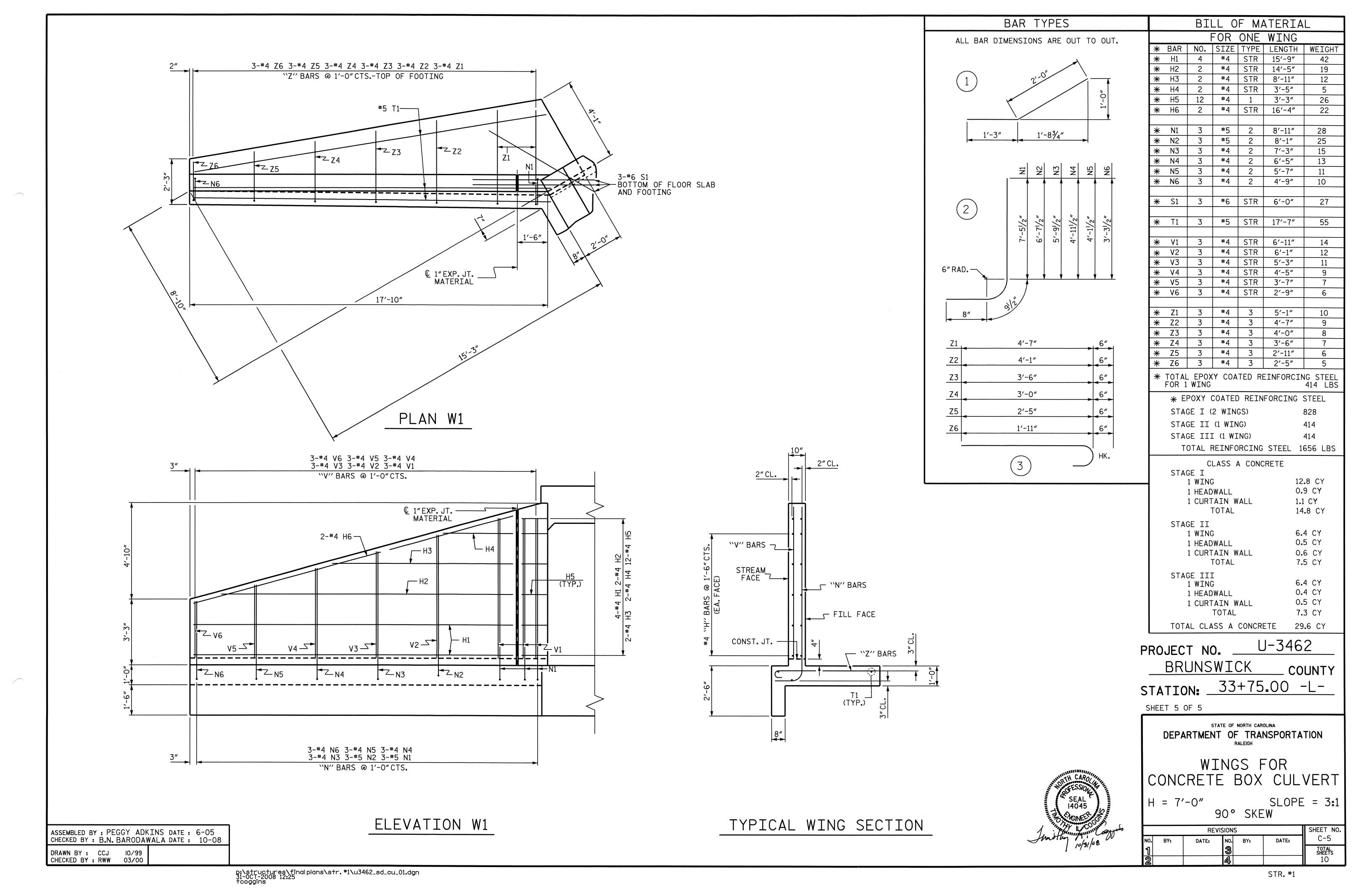
TOTAL SHEETS

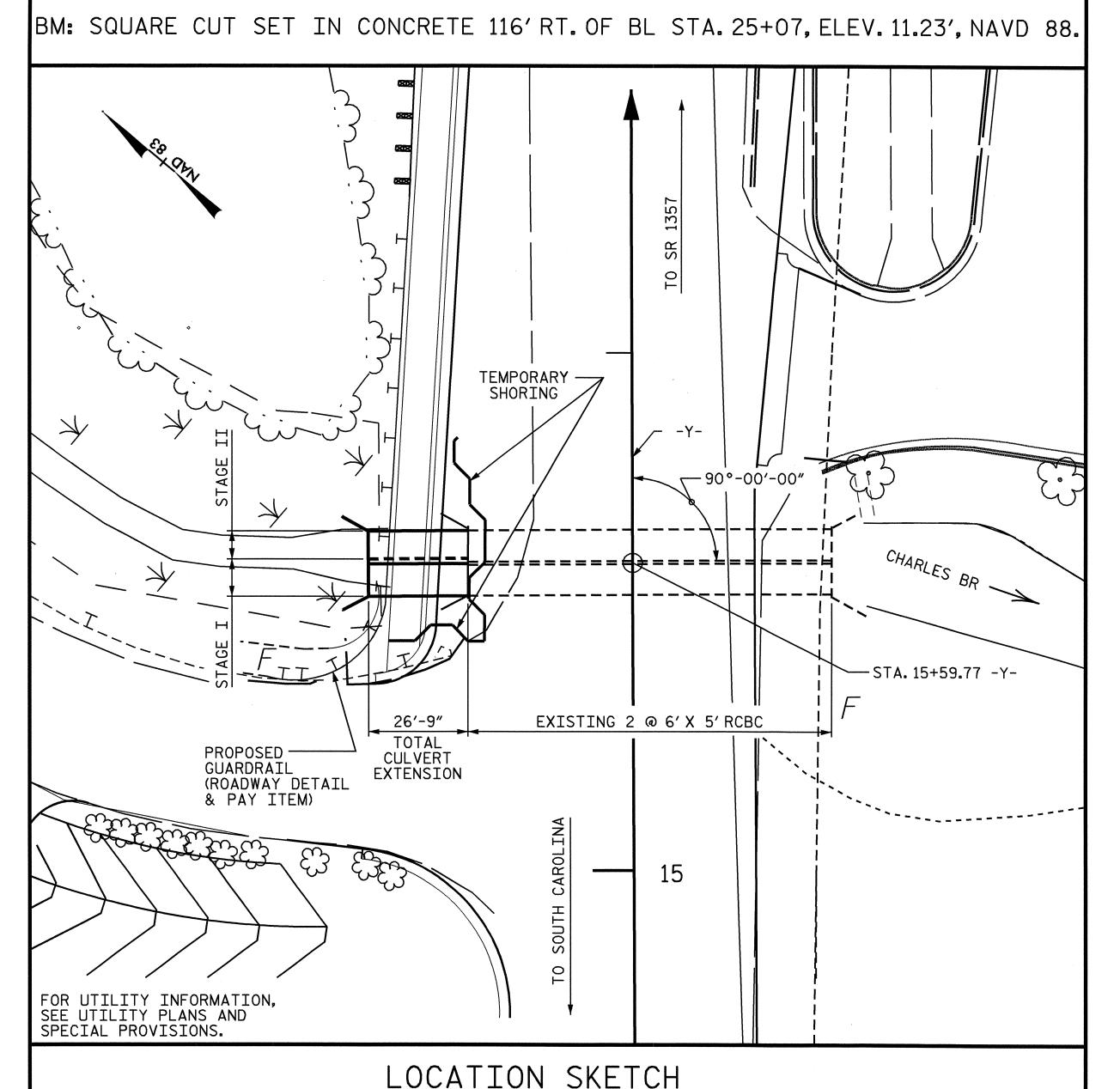
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STD. NO. CB12A CTD #1

ASSEMBLED BY : PEGGY ADKINS DATE : 6-05 CHECKED BY : B.N. BARODAWALA DATE : 10-08

31-0CT-2008 12:25 ni structures final plane str #1\13462 ed all 01 dan





STRUCTURE QUANTITIES STAGE I CLASS A CONCRETE BARREL @ <u>0.729</u> CY/FT <u>19.5</u> C.Y. WINGS ETC. 4.9 C.Y. 24.4 ____C.Y. EPOXY COATED REINFORCING STEEL 3490 LBS. WINGS ETC. 261 LBS. 3751 LBS. TOTAL _____ CULVERT EXCAVATION LUMP SUM FOUNDATION COND. MAT'L. 16 TONS STRUCTURE QUANTITIES STAGE II CLASS A CONCRETE BARREL @ ____0.449 __CY/FT_____12.0 __C.Y. WINGS ETC. ______ 4.7 C.Y. 16.7 ___C.Y. TOTAL _____ EPOXY COATED REINFORCING STEEL 1955 LBS. WINGS ETC. 261 LBS. 2216 LBS. TOTAL CULVERT EXCAVATION LUMP SUM FOUNDATION COND. MAT'L. 11 TONS TOTAL STRUCTURE QUANTITIES CLASS A CONCRETE BARREL @ <u>1.178</u> CY/FT<u> 31.5</u> C.Y. WINGS ETC. 9.6 C.Y. 41.1 ____C.Y. TOTAL _____ EPOXY COATED REINFORCING STEEL 5445___LBS. WINGS ETC. ____ 5967 _LBS. TOTAL CULVERT EXCAVATION LUMP SUM FOUNDATION COND. MAT'L. 27 TONS

NOTES

F. A. PROJECT NO. STP-1357(2)

ASSUMED LIVE LOAD -----HS20-44 OR ALTERNATE LOADING.

DESIGN FILL------2.58 MIN.----3.32 MAX.

FOR OTHER DESIGN DATA AND NOTES SEE STANDARD NOTE SHEET.

3"Ø WEEP HOLES INDICATED TO BE IN ACCORDANCE WITH THE SPECIFICATIONS.

CONCRETE IN CULVERTS TO BE POURED IN THE FOLLOWING ORDER:

STAGE I CONSTRUCTION

- 1. WING FOOTING AND FLOOR SLAB INCLUDING 4" OF ALL VERTICAL WALLS.
- 2. THE REMAINING PORTIONS OF THE WALLS AND WING FULL HEIGHT FOLLOWED BY ROOF SLAB AND HEADWALL.

STAGE II CONSTRUCTION

- 1. WING FOOTING AND FLOOR SLAB INCLUDING 4" OF VERTICAL WALL.
- 2. THE REMAINING PORTION OF THE WALL AND WING FULL HEIGHT FOLLOWED BY ROOF SLAB AND HEADWALL.

THE RESIDENT ENGINEER SHALL CHECK THE LENGTH OF CULVERT BEFORE STAKING IT OUT TO MAKE CERTAIN THAT IT WILL PROPERLY TAKE CARE OF THE FILL.

DOWELS SHALL BE USED TO CONNECT THE CULVERT EXTENSION TO THE EXISTING CULVERT AS SHOWN. FOR NOTE REGARDING SETTING OF DOWELS. SEE SHEET SN.

DIMENSIONS FOR WING LAYOUT AS WELL AS ADDITIONAL REINFORCING STEEL EMBEDDED IN BARREL ARE SHOWN ON WING SHEET.

AT THE CONTRACTOR'S OPTION, HE MAY SPLICE THE VERTICAL REINFORCING STEEL IN THE INTERIOR FACE OF EXTERIOR WALL AND BOTH FACES OF INTERIOR WALLS ABOVE LOWER WALL CONSTRUCTION JOINT. THE SPLICE LENGTH SHALL BE AS PROVIDED IN THE SPLICE LENGTH CHART SHOWN ON THE PLANS. EXTRA WEIGHT OF STEEL DUE TO THE SPLICES AHLL BE PAID FOR BY THE CONTRACTOR.

FOR CULVERT DIVERSION DETAILS AND PAY ITEM. SEE EROSION CONTROL PLANS.

A 3 FOOT STRIP OF FILTER FABRIC SHALL BE ATTACHED TO THE FILL FACE OF THE WING COVERING THE ENTIRE LENGTH OF THE EXPANSION JOINT.

FOR SUBMITTAL OF WORKING DRAWINGS, SEE SPECIAL PROVISIONS.

FOR FALSEWORK AND FORMWORK, SEE SPECIAL PROVISIONS.

NO PRECAST REINFORCED BOX CULVERT OPTION WILL BE ALLOWED.

FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.

FOR CRANE SAFETY, SEE SPECIAL PROVISIONS.

IF APPROVED BY THE ENGINEER, THE CONTRACTOR MAY USE THE EXISTING WINGS AS TEMPORARY SHORING FOR THE CONSTRUCTION OF THE CULVERT EXTENSIONS. IN THIS CASE, THE BOTTOM SLAB OF THE EXTENSION SHALL BE POURED AT LEAST 72 HOURS PRIOR TO CUTTING THE WINGS. THE WINGS MAY BE CUT EARLIER PROVIDED THE SLAB CONCRETE STRENGTH HAS REACHED A MINIMUM COMPRESSIVE STRENGTH OF 1500 PSI.

THE CONTRACTOR SHALL PROVIDE INDEPENDENT ASSURANCE SAMPLES OF REINFORCING STEEL AS FOLLOWS: FOR PROJECTS REQUIRING UP TO 400 TONS OF REINFORCING STEEL, ONE 30 INCH SAMPLE OF EACH SIZE BAR USED, AND FOR PROJECTS REQUIRING OVER 400 TONS OF REINFORCING STEEL, TWO 30 INCH SAMPLES OF EACH SIZE BAR USED. THE BARS FROM WHICH THE SAMPLES ARE TAKEN MUST THEN BE SPLICED WITH REPLACEMENT BARS OF THE SIZE AND LENGTH OF THE SAMPLE, PLUS A MINIMUM LAP SPLICE OF THIRTY BAR DIAMETERS.

ALL BAR SUPPORTS USED IN THE CULVERT AND ALL INCIDENTAL REINFORCING STEEL SHALL BE EPOXY COATED IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

FOR MAINTENANCE OF TRAFFIC. SEE TRAFFIC CONTROL PLANS.

7.535 FT.

3:1

= 450 c.f.s.

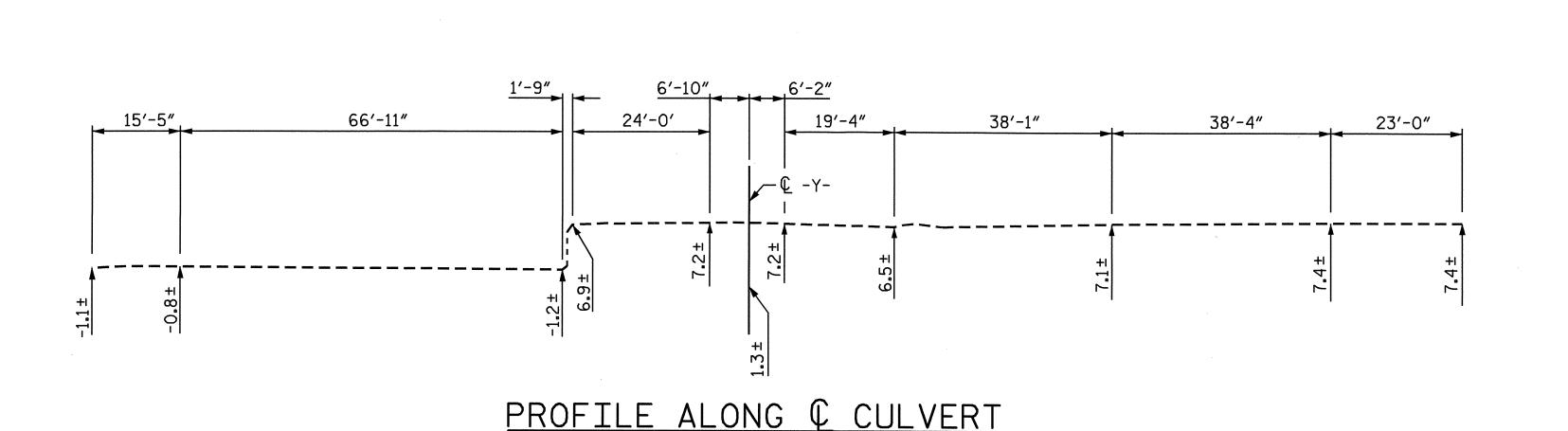
= 25 YR.**

 $= 1.63 \text{ MI}^2$

= 810 c.f.s.

-0.651 ± FT.

FOR LIMITS OF TEMPORARY SHORING, SEE TRAFFIC CONTROL PLANS. FOR PAY ITEM FOR TEMPORARY SHORING, SEE ROADWAY PLANS.



OVERTOPPING FLOOD DATA

GRADE DATA

HYDRAULIC DATA

DESIGN HIGH WATER ELEVATION = 6.7 FT.**

BASIC HIGH WATER ELEVATION = 7.8 FT.

GRADE POINT ELEVATION @

STA. 15+59.77-Y-

STA. 15+59.77 -Y-

FREQUENCY OF DESIGN FLOOD

BASIC DISCHARGE (Q100)

BED ELEVATION @

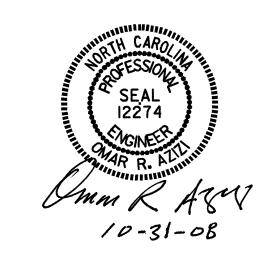
ROADWAY SLOPES

DESIGN DISCHARGE

DRAINAGE AREA

OVERTOPPING DISCHARGE = 550 c.f.s. FREQUENCY OF OVERTOPPING FLOOD = 25 YR.+ = 7.2 FT.OVERTOPPING FLOOD ELEVATION

** THE DESIGN MAINTAINS THE EXISTING LEVEL OF SERVICE, BUT DOES NOT ACHIEVE THE DESIRED (50 YR) LEVEL OF SERVICE. TO DO SO, THE GRADE WOULD NEED TO BE RAISED AND THE CULVERT REPLACED.



PROJECT NO. U-3462 BRUNSWICK 15+59.77 -Y-STATION: SHEET 1 OF 5

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH

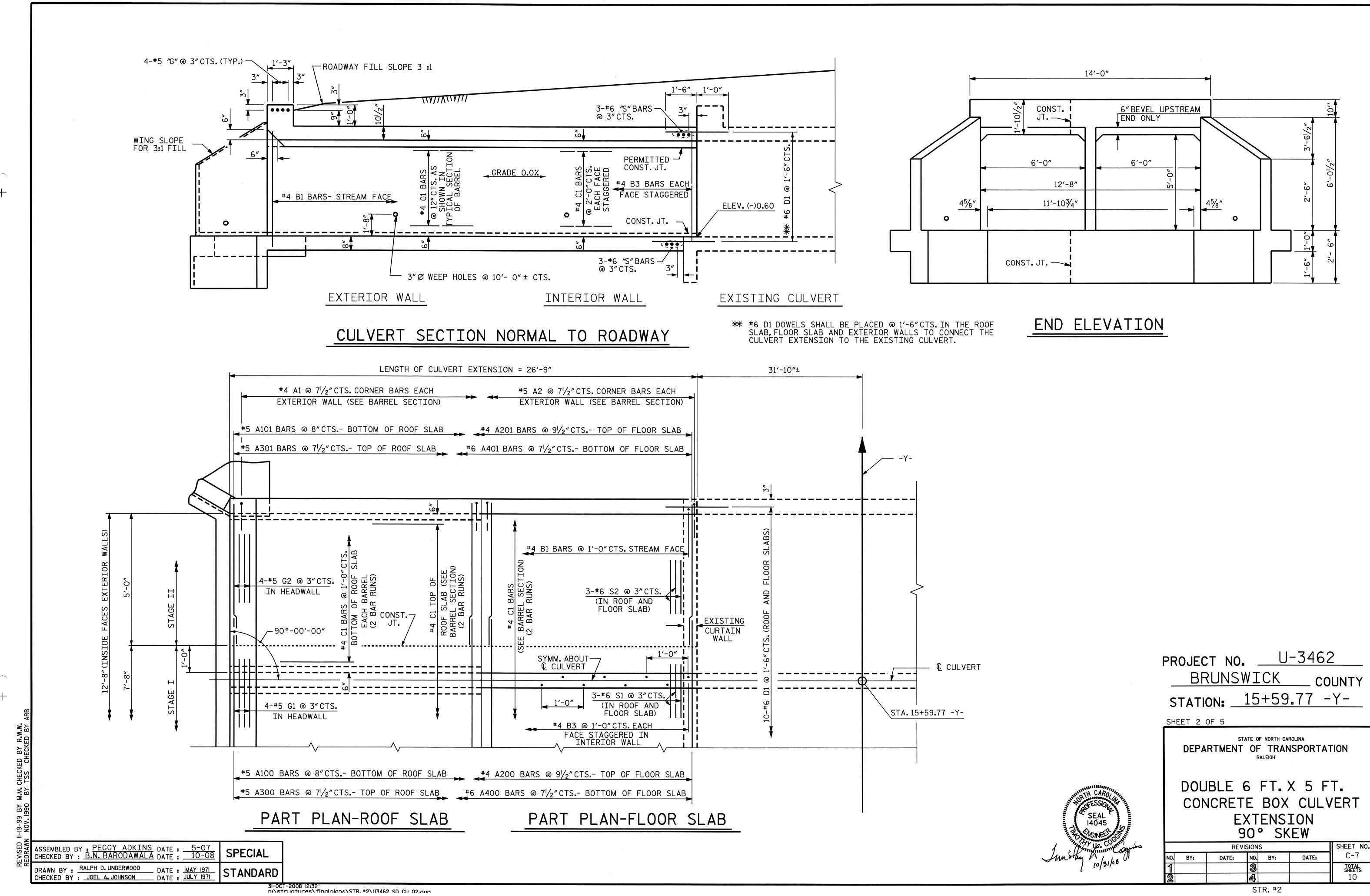
COUNTY

DOUBLE 6 FT. X 5 FT. CONCRETE BOX CULVERT EXTENSION 90° SKEW

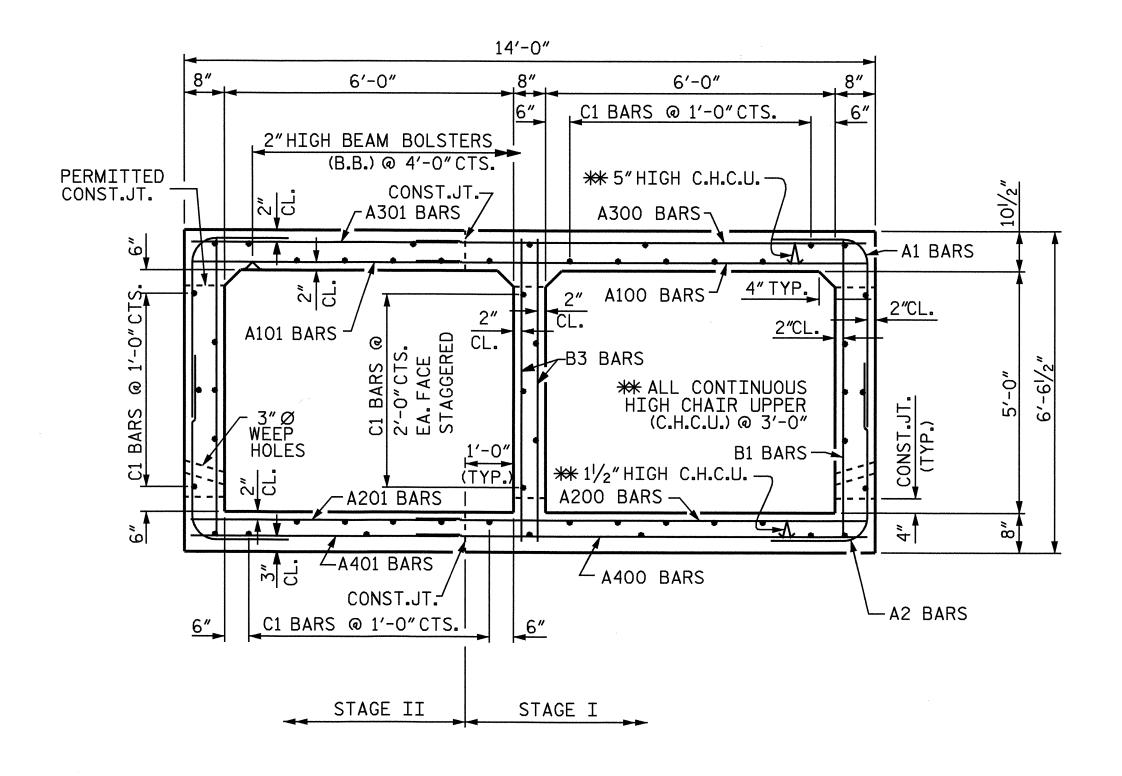
	REVISIONS							
NO.	BY:	DATE:	NO.	BY:	DATE:	C-6		
1			3			TOTAL SHEETS		
2			4			10		

STD. NO. CB12A

DRAWN BY : PEGGY ADKINS DATE : 5-07 CHECKED BY : B.N. BARODAWALA DATE : 10-08

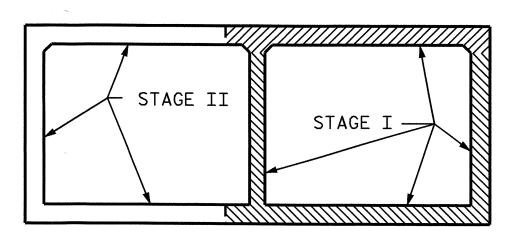


STR.#2



RIGHT ANGLE SECTION OF BARREL

THERE ARE 51 "C"BARS IN SECTION OF BARREL. (LOOKING DOWNSTREAM)



STAGE I AND II

CONSTRUCTION SEQUENCE

BAR TYPE	REINFORCING STEEL BAR SCHEDULE					
	STAGE I					
A2	BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT
VERTICAL LEG ─	* A1	43	4	6	5′-9″	165
3′-1″	∗ A2	43	5	6	5′-8″	254
6"R. — m m						
0 1/1.	*A100	41	5	STR	10'-11"	467
	*A200	34	4	STR	10'-4"	235
A1 $1'-7^{1/2}$ $3^{1/2}$	<u></u> * A300 * A400	43	<u>5</u>	STR STR	10′-6″ 11′-9″	471 759
- 1/2	*************************************	73	0	SIK	11 -9	103
$\frac{A2}{4} \frac{1'-9^{1/2}}{2}$	★ B1	27	4	STR	6'-0"	108
	* B3	54	4	STR	6'-0"	216
BAR DIMENSIONS ARE OUT TO OUT				·		
BAR TYPE	* C1	62	4	STR	14′-5″	597
BAR SIZE SPLICE LENGTH	* D1	15	6	STR	2′-6″	56
A"100" 5 2'-7" A"200" 4 2'-0"	* G1	4		STR	11/ 10//	40
A"300" 5 2'-2"	<u> </u>	7	5	SIK	11'-10"	49
A"400" 6 3'-5"	* S1	6	6	STR	12'-7"	113
B1 4 2'-0" B3 4 2'-0"				<u> </u>		110
C1 4 2'-4"			XY CO. ING ST			3490 LBS
			ING ST			3490 LBS
C1 4 2'-4" G1 5 3'-6"			ING ST	EEL		3490 LBS WEIGHT
C1 4 2'-4" G1 5 3'-6"	BAR * A1	NFORC NO. 43	ST. SIZE 4	AGE TYPE 6	II LENGTH 5'-9"	
C1 4 2'-4" G1 5 3'-6"	REII BAR	NFORC	ING ST ST. SIZE	AGE TYPE	II LENGTH	WEIGHT
C1 4 2'-4" G1 5 3'-6"	BAR * A1 * A2	NFORC NO. 43 43	ST SIZE 4 5	AGE TYPE 6 6	II LENGTH 5'-9" 5'-8"	WEIGHT 165 254
C1 4 2'-4" G1 5 3'-6"	BAR * A1 * A2 *A101	NFORC NO. 43 43	ST SIZE 4 5	AGE TYPE 6 6 STR	II LENGTH 5'-9" 5'-8"	WEIGHT 165 254 228
C1 4 2'-4" G1 5 3'-6"	BAR * A1 * A2 *A101 *A201	NFORC NO. 43 43	ST SIZE 4 5	AGE TYPE 6 6	II LENGTH 5'-9" 5'-8"	WEIGHT 165 254 228 121
C1 4 2'-4" G1 5 3'-6"	BAR * A1 * A2 *A101	NO. 43 43 41 34	ST SIZE 4 5	AGE TYPE 6 6 STR STR	II LENGTH 5'-9" 5'-8" 5'-4" 5'-4"	WEIGHT 165 254 228
C1 4 2'-4" G1 5 3'-6"	BAR * A1 * A2 *A101 *A201 *A301	NFORC NO. 43 43 41 34 43 43	ST SIZE 4 5 5 4 5	AGE TYPE 6 6 STR STR STR	II LENGTH 5'-9" 5'-8" 5'-4" 5'-4" 5'-4"	WEIGHT 165 254 228 121 239
C1 4 2'-4" G1 5 3'-6"	BAR * A1 * A2 *A101 *A201 *A301	NFORC NO. 43 43 41 34 43	ST SIZE 4 5 5 4 5	AGE TYPE 6 6 STR STR STR	II LENGTH 5'-9" 5'-8" 5'-4" 5'-4" 5'-4"	WEIGHT 165 254 228 121 239
C1 4 2'-4" G1 5 3'-6"	BAR * A1 * A2 *A101 *A201 *A301 *A401	NFORC NO. 43 43 41 34 43 43	ST SIZE 4 5 5 4 5	TYPE 6 6 STR STR STR STR	II LENGTH 5'-9" 5'-8" 5'-4" 5'-4" 5'-4" 5'-4"	WEIGHT 165 254 228 121 239 344
C1 4 2'-4" G1 5 3'-6"	BAR * A1 * A2 *A101 *A201 *A301 *A401	NFORC NO. 43 43 41 34 43 43	ST SIZE 4 5 5 4 5	TYPE 6 6 STR STR STR STR	II LENGTH 5'-9" 5'-8" 5'-4" 5'-4" 5'-4" 5'-4"	WEIGHT 165 254 228 121 239 344
C1 4 2'-4" G1 5 3'-6"	BAR * A1 * A2 *A101 *A201 *A301 *A401 * B1 * C1	NO. 43 43 41 34 43 43 47 40	ST. SIZE 4 5 6 4	AGE TYPE 6 6 STR STR STR STR STR STR	II LENGTH 5'-9" 5'-8" 5'-4" 5'-4" 5'-4" 6'-0"	WEIGHT 165 254 228 121 239 344 108
C1 4 2'-4" G1 5 3'-6"	BAR * A1 * A2 *A101 *A201 *A301 *A401 * B1	NFORC NO. 43 43 41 34 43 43	ST. SIZE 4 5 6 4	AGE TYPE 6 6 STR STR STR STR STR	II LENGTH 5'-9" 5'-4" 5'-4" 5'-4" 5'-4" 6'-0"	WEIGHT 165 254 228 121 239 344
C1 4 2'-4" G1 5 3'-6"	BAR * A1 * A2 *A101 *A201 *A301 *A401 * B1 * C1	NO. 43 43 41 34 43 43 47 40	ST. SIZE 4 5 6 4	AGE TYPE 6 6 STR STR STR STR STR STR	II LENGTH 5'-9" 5'-8" 5'-4" 5'-4" 5'-4" 6'-0"	WEIGHT 165 254 228 121 239 344 108
C1 4 2'-4" G1 5 3'-6"	BAR * A1 * A2 *A101 *A201 *A301 *A401 * B1 * C1 * D1 * G2	NO. 43 41 34 43 43 27 40	ST SIZE 4 5 5 6 4 4 4 6	TYPE 6 6 STR STR STR STR STR STR	II LENGTH 5'-9" 5'-4" 5'-4" 5'-4" 5'-4" 6'-0" 14'-5"	WEIGHT 165 254 228 121 239 344 108 385
C1 4 2'-4" G1 5 3'-6"	BAR * A1 * A2 *A101 *A201 *A301 *A401 * B1 * C1 * D1	NO. 43 43 41 34 43 43 47 40 11 4 6	ST SIZE 4 5 5 6 4 5 6 5 6	TYPE 6 6 STR STR STR STR STR STR STR	II LENGTH 5'-9" 5'-4" 5'-4" 5'-4" 5'-4" 6'-0" 14'-5" 2'-6"	WEIGHT 165 254 228 121 239 344 108 385 41 22

PROJECT NO. U-3462 BRUNSWICK COUNTY STATION: 15+59.77 -Y-

SHEET 3 OF 5

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH

DOUBLE 6 FT. X 5 FT. CONCRETE BOX CULVERT EXTENSION 90° SKEW

STD. NO. CB12 STR. #2

REVISIONS SHEET NO. C-8 NO. BY:

DRAWN BY: RALPH D. UNDERWOOD DATE: MAY 1971 CHECKED BY: JOEL A. JOHNSON DATE: JULY 1971 STANDARD



THE GUARDRAIL ANCHOR ASSEMBLY FOR CULVERTS SHALL CONSIST OF THE FOLLOWING COMPONENTS:

- A. FERRULES SHALL BE MADE FROM STEEL MEETING THE REQUIREMENTS OF AASHTO M169, GRADE 12L14 AND SHALL HAVE A MINIMUM LENGTH OF THREADS OF $2\frac{1}{2}$.
- B. 4 1" Ø X 2 1/4" BOLTS WITH WASHERS, BOLTS SHALL CONFORM TO THE REQUIREMENTS OF ASTM A307. BOLTS AND WASHERS SHALL BE GALVANIZED. (AT THE CONTRACTOR'S OPTION, STAINLESS STEEL BOLTS AND WASHERS MAY BE USED AS AN ALTERNATE FOR THE 1" \varnothing X $2^{1}/4$ " GALVANIZED BOLTS AND WASHERS. THEY SHALL CONFORM TO OR EXCEED THE MECHANICAL REQUIREMENTS OF ASTM A307. THE USE OF THIS ALTERNATE SHALL BE APPROVED BY THE ENGINEER.)
- C. WIRE STRUTS SHOWN IN THE GUARDRAIL ANCHOR ASSEMBLY FOR CULVERTS DETAIL ARE MINIMUM ALLOWABLE SIZE AND SHALL HAVE A MINIMUM TENSILE STRENGTH
 OF 100,000 P.S.I. AS AN OPTION, A 7/6" Ø WIRE STRUT WITH A MINIMUM TENSILE
 STRENGTH OF 90,000 PSI IS ACCEPTABLE.

GUARDRAIL ANCHOR ASSEMBLY WITH BOLTS SHALL BE ASSEMBLED IN THE SHOP. BOLT THREADS MAY BE RECUT AS NECESSARY TO INSURE FIT.

THE COST OF THE GUARDRAIL ANCHOR ASSEMBLY FOR CULVERTS COMPLETE IN PLACE, SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR CLASS "A" CONCRETE.

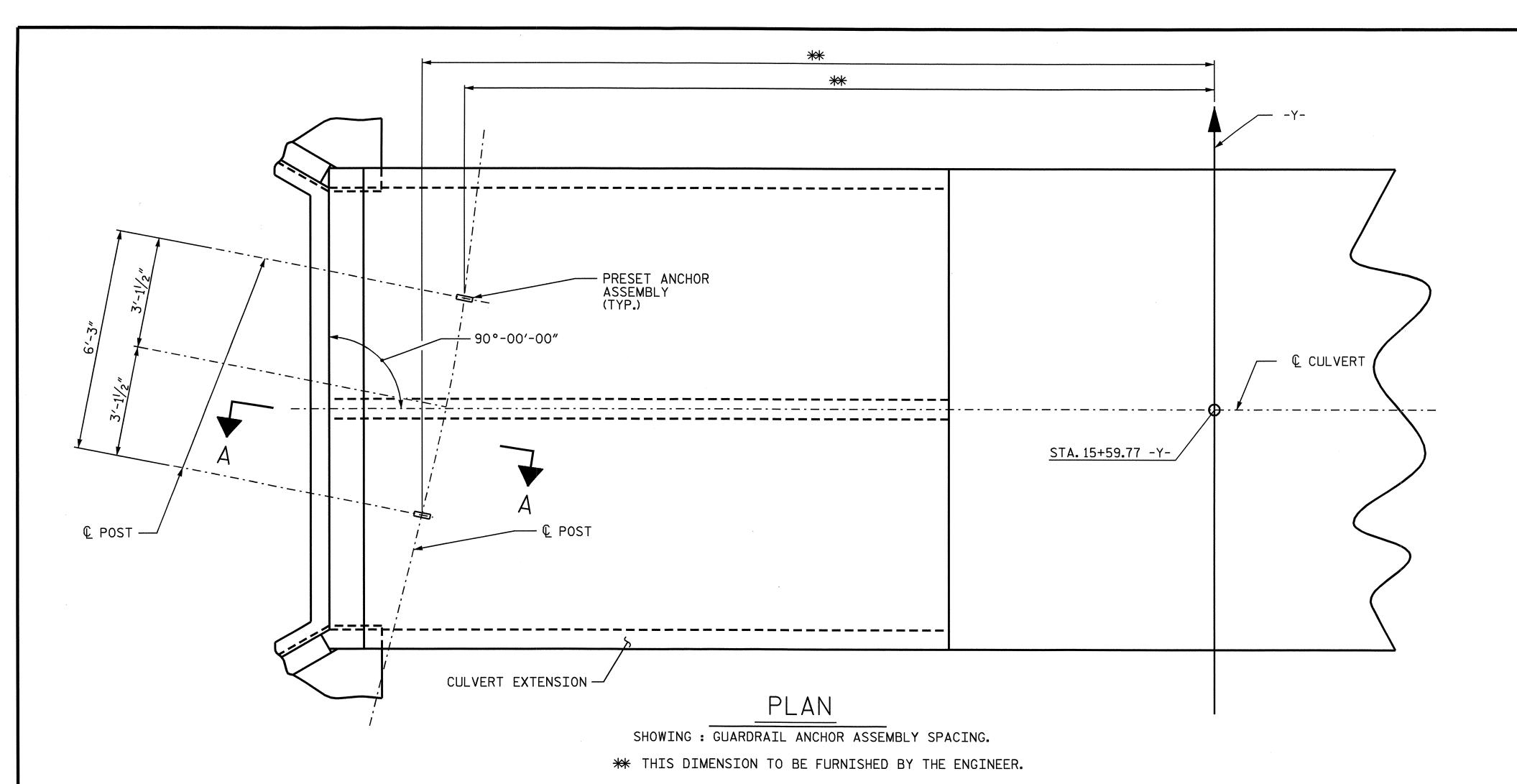
FERRULES TO BE PLUGGED DURING POURING OF SLAB AS RECOMMENDED BY THE MANUFACTURER.

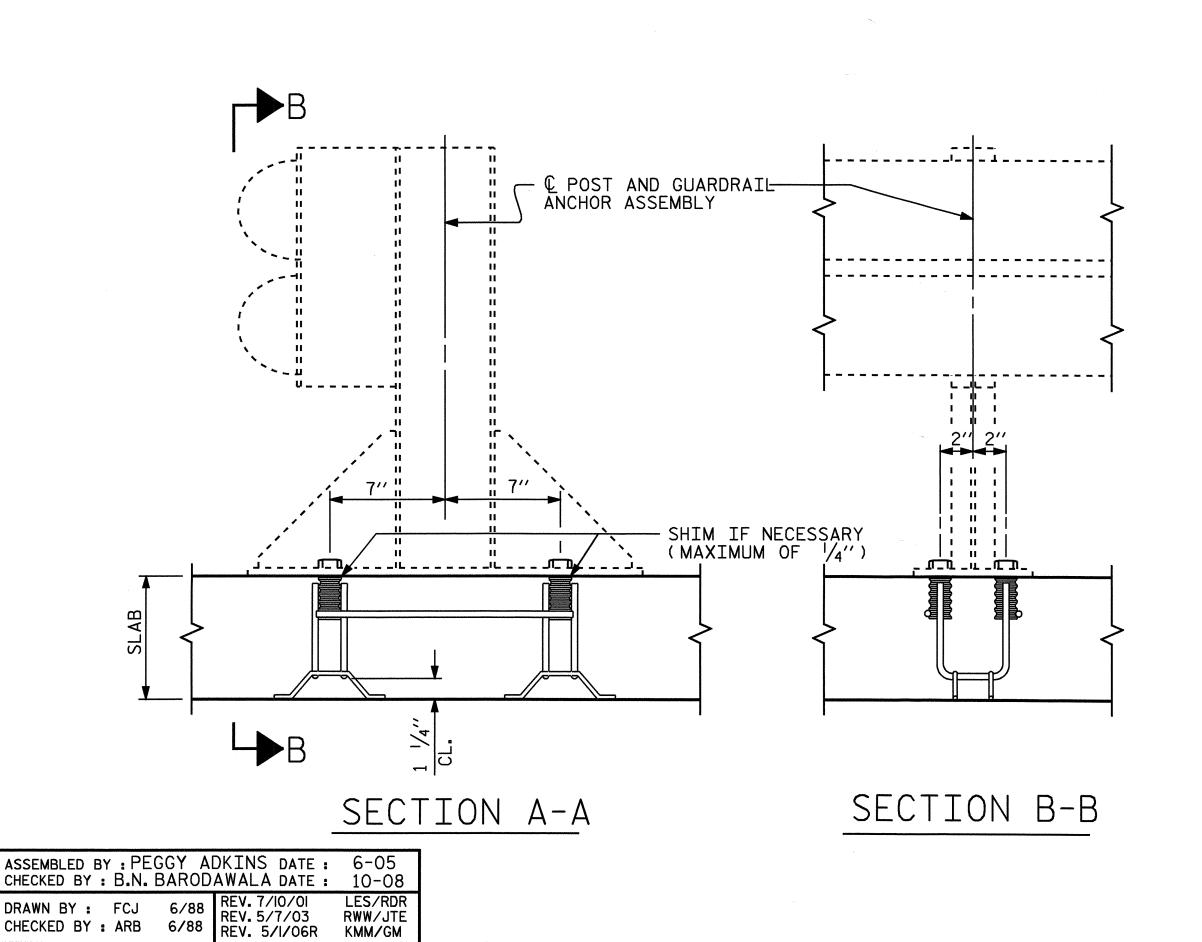
AT THE CONTRACTOR'S OPTION, FERRULES WITH OPEN OR CLOSED ENDS MAY BE USED.

PAYMENT FOR GUARDRAIL, POSTS, AND POST BASE PLATES IS INCLUDED IN ROADWAY PAY ITEMS.

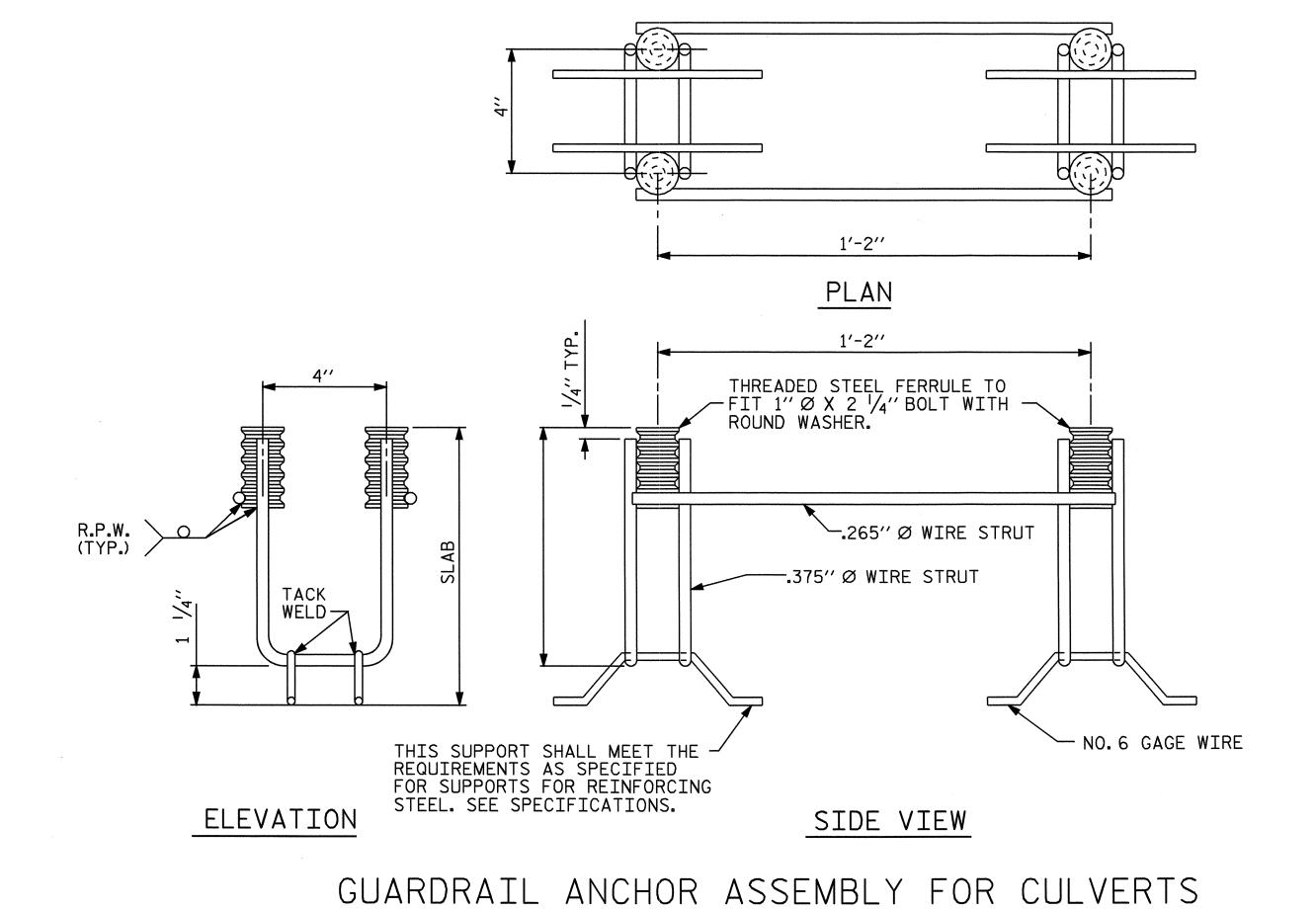
SLAB REINFORCING STEEL MAY BE SHIFTED AS NECESSARY TO CLEAR GUARDRAIL ANCHOR ASSEMBLY. CARE SHOULD BE TAKEN TO KEEP THE SHIFTING OF REINFORCING STEEL TO A MINIMUM.

THE CONTRACTOR MAY USE ADHESIVELY ANCHORED ANCHOR BOLTS IN PLACE OF GUARDRAIL ANCHOR ASSEMBLY. LEVEL TWO FIELD TESTING IS REQUIRED, AND THE YIELD LOAD OF THE 1" Ø BOLT IS 21.8 KIPS. FOR ADHESIVELY ANCHORED ANCHOR BOLTS OR DOWELS, SEE SPECIAL PROVISIONS.





DRAWN BY: FCJ 6/88 CHECKED BY: ARB 6/88



PROJECT NO. U-3462 BRUNSWICK COUNTY 15+59.77 -Y-STATION:_

SHEET 4 OF 5

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

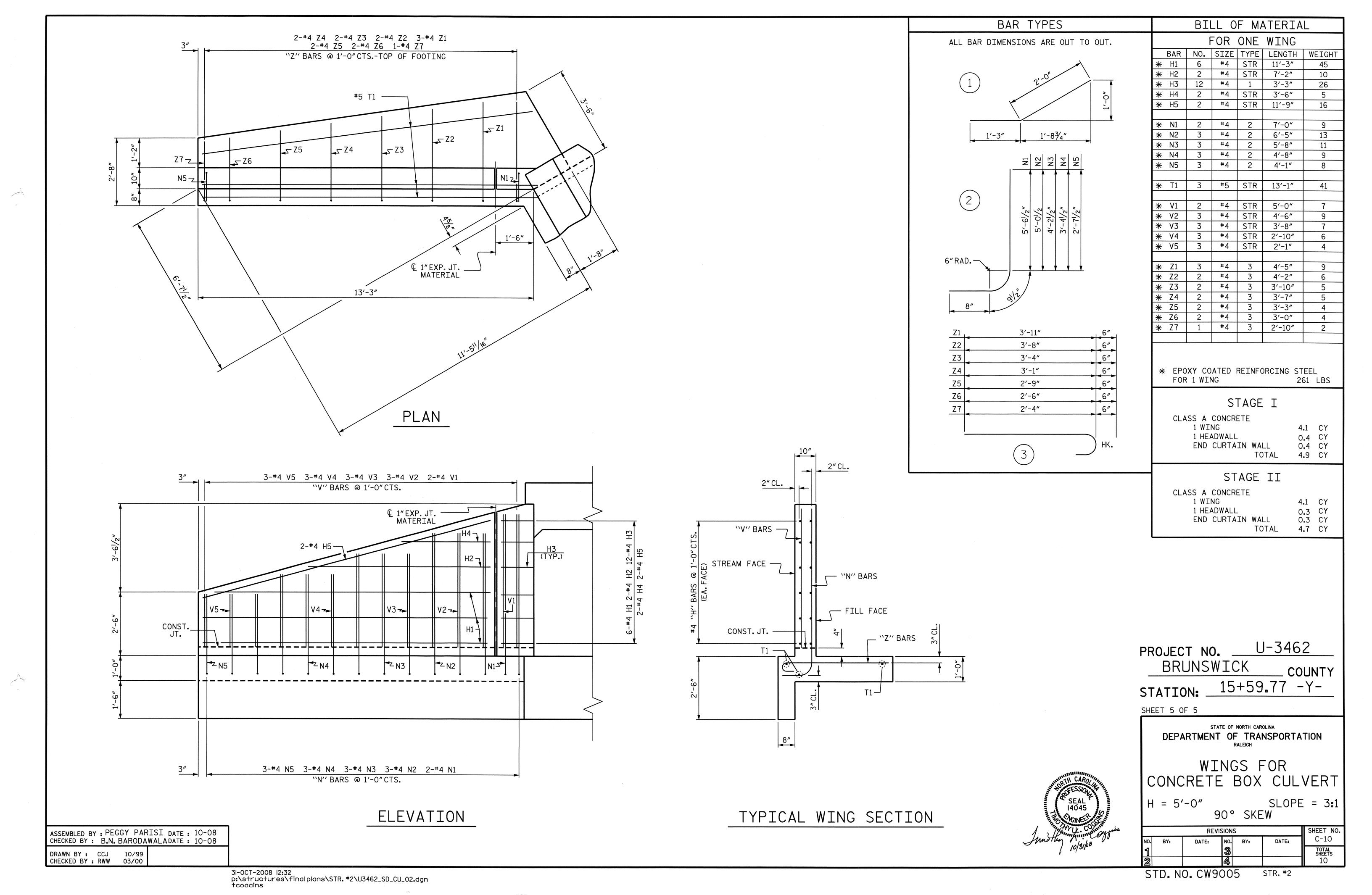
STANDARD

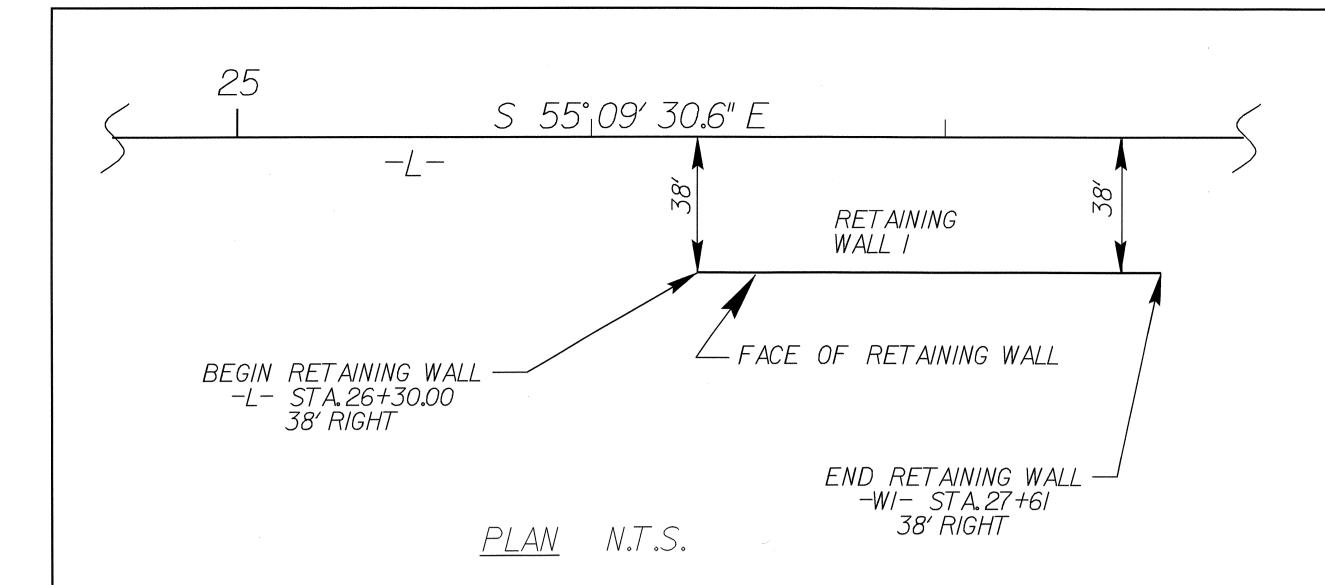
ANCHORAGE DETAILS FOR GUARDRAIL ANCHOR ASSEMBLY FOR CULVERTS

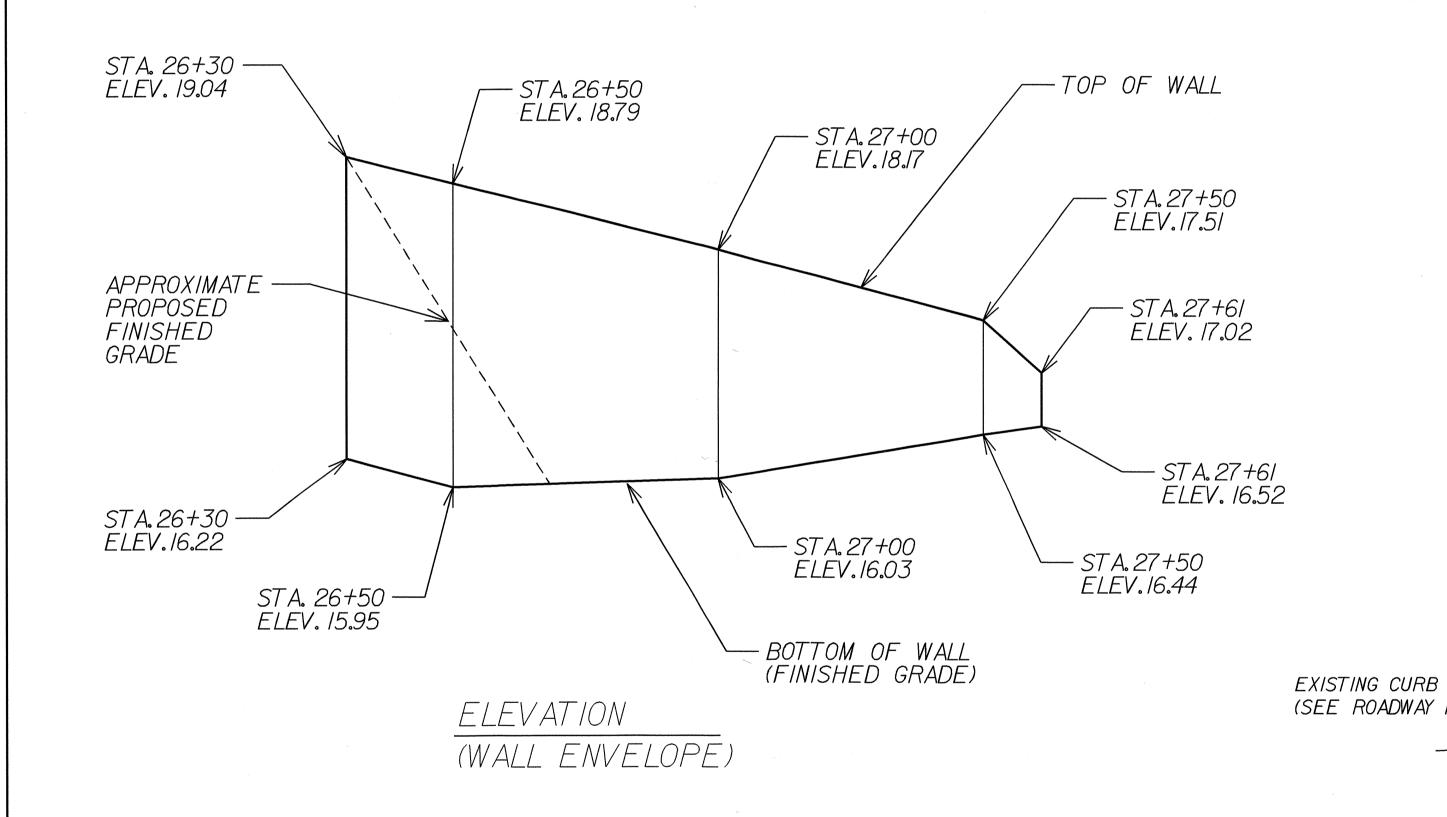
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NO.	BY:	DATE:	NO.	BY:	DATE:	C-9
1			3			TOTAL SHEETS
2			4			10
		1				

31-0CT-2008 12:32 p:\structures\finalplans\STR. #2\U3462_SD_CU_02.dgn tcoggins

STR. #2 STD. NO. GRA1







STATION -L-	OFFSET	GRADE ELEVATION	TOP OF WALL ELEV.	BOTTOM OF WALL ELEV.
26+30.000	38.00 ft RT	18.54 ft	19.04 ft	16.22 ft
26+50.000	38.00 ft RT	18.29 ft	18.79 ft	15.99 ft
27+00.000	38.00 ft RT	17.67 ft	18.17 ft	16.03 ft
27+50.000	38.00 ft RT	17.01 ft	17.51 ft	16.44 ft
27+61.000	38.00 ft RT	16.52 ft	17.02 ft	16.52 ft

PREPARED BY: T.T. WALKER DATE: 10/08

REVIEWED BY: J.E. GODWIN DATE: 10/08

NOTES ON PLANS

FOR SEGMENTAL GRAVITY RETAINING WALLS, SEE SEGMENTAL GRAVITY RETAINING WALLS SPECIAL PROVISION.

BEFORE BEGINNING WALL DESIGN FOR RETAINING WALL NO. 1, SURVEY ALL EXISTING GROUND ELEVATIONS SHOWN ON THE PLANS AND SUBMIT A REVISED WALL ENVELOPE FOR REVIEW. DO NOT START WALL DESIGN OR CONSTRUCTION UNTIL THIS ENVELOPE IS ACCEPTED.

DESIGN RETAINING WALL NO. 1 FOR A WALL HEIGHT EQUAL TO THE DESIGN HEIGHT (DIFFERENCE BETWEEN GRADE ELEVATION AND BOTTOM OF WALL ELEVATION) PLUS EMBEDMENT (DIFFERENCE BETWEEN BOTTOM OF WALL ELEVATION AND TOP OF LEVEL PAD ELEVATION).

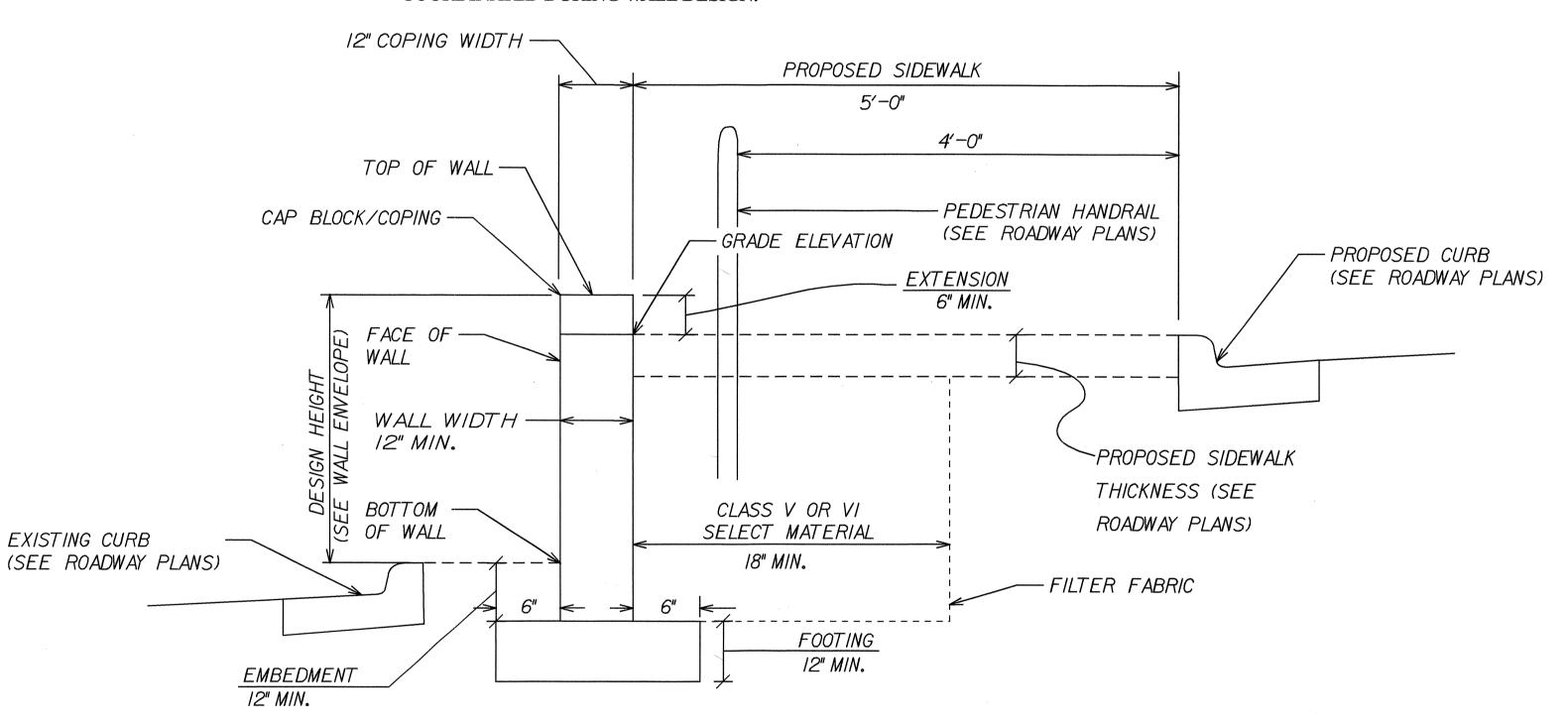
DESIGN RETAINING WALL NO. 1 FOR THE FOLLOWING:

- 1) MINIMUM SERVICE LIFE = 75 YEARS
- 2) ALLOWABLE BEARING CAPACITY = 3,000 PSF
- 3) IN-SITU ASSUMED MATERIAL PARAMETERS:

MATERIAL TYPE	UNIT WEIGHT (γ) PCF	FRICTION ANGLE (\$) DEGREES	COHESION (c) PSF
BACKFILL	100	38	0
FOUNDATION	120	32	0

DESIGN WALL NO. 1 FOR A LIVE LOAD (TRAFFIC) SURCHARGE.

HANDRAIL POST FOUNDATION AND RETAINING WALL BLOCKS SHOULD BE COORDINATED DURING WALL DESIGN.



TYPICAL SECTION NTS

TOTAL WALL AREA: 300 SQUARE FEET

PROJECT NO.: 34953.1.1

GEOTECHNICAL ENGINEER

SEAL 28919 4/3/8

ENGINEER

BRUNSWICK COUNTY
STATION: 27+61 TO 26+30 -L-

SHEET 1 OF 1

GEOTECHNICAL ENGINEERING UNIT

- EASTERN REGIONAL OFFICE

 WESTERN REGIONAL OFFICE

STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH

SEGMENTAL GRAVITY RETAINING WALL

REVISIONS

BY DATE NO. BY DATE

3 TOTAL SHEETS
4

STANDARD NOTES

DESIGN DATA:

CONCRETE IN COMPRESSION ----- 1,200 LBS. PER SQ. IN CONCRETE IN SHEAR ----- SEE A.A.S.H.T.O.

STRUCTURAL TIMBER - TREATED OR

UNTREATED - EXTREME FIBER STRESS - - - - - 1,800 LBS. PER SQ. IN.

COMPRESSION PERPENDICULAR TO GRAIN
OF TIMBER ----

375 LBS. PER SQ. IN.

EQUIVALENT FLUID PRESSURE OF EARTH ----

375 LBS. PER SQ. I

30 LBS. PER CU. FT. (MINIMUM)

MATERIAL AND WORKMANSHIP:

EXCEPT AS MAY OTHERWISE BE SPECIFIED ON PLANS OR IN THE SPECIAL PROVISIONS, ALL MATERIAL AND WORKMANSHIP SHALL BE IN ACCORDANCE WITH THE 2006 "STANDARD SPECIFICATIONS FOR ROADS AND STRUCTURES" OF THE N. C. DEPARTMENT OF TRANSPORTATION.

STEEL SHEET PILING FOR PERMANENT OR TEMPORARY APPLICATIONS SHALL BE HOT ROLLED.

CONCRETE:

UNLESS OTHERWISE REQUIRED ON PLANS, CLASS A CONCRETE SHALL BE USED FOR ALL PORTIONS OF ALL STRUCTURES WITH THE EXCEPTION THAT: CLASS AA CONCRETE SHALL BE USED IN BRIDGE SUPERSTRUCTURES, ABUTMENT BACKWALLS, AND APPROACH SLABS; AND CLASS B CONCRETE SHALL BE USED FOR SLOPE PROTECTION AND RIP RAP.

CONCRETE CHAMFERS:

UNLESS OTHERWISE NOTED ON THE PLANS, ALL EXPOSED CORNERS ON STRUCTURES SHALL BE CHAMFERED 3/4"WITH THE FOLLOWING EXCEPTIONS: TOP CORNERS OF CURBS MAY BE ROUNDED TO 1-1/2"RADIUS WHICH IS BUILT INTO CURB FORMS; CORNERS OF TRANSVERSE FLOOR EXPANSION JOINTS SHALL BE ROUNDED WITH A 1/4"FINISHING TOOL UNLESS OTHERWISE REQUIRED ON PLANS; AND CORNERS OF EXPANSION JOINTS IN THE ROADWAY FACES AND TOPS OF CURBS AND SIDEWALKS SHALL BE ROUNDED TO A 1/4"RADIUS WITH A FINISHING STONE OR TOOL UNLESS OTHERWISE REQUIRED ON PLANS.

DOWELS:

DOWELS WHEN INDICATED ON PLANS AS FOR CULVERT EXTENSIONS, SHALL BE EMBEDDED AT LEAST 12"INTO THE OLD CONCRETE AND GROUTED INTO PLACE WITH 1:2 CEMENT MORTAR.

ALLOWANCE FOR DEAD LOAD DEFLECTION, SETTLEMENT, ETC. IN CASTING SUPERSTRUCTURES:

BRIDGES SHALL BE BUILT ON THE GRADE OR VERTICAL CURVE SHOWN ON PLANS, SLABS, CURBS AND PARAPETS SHALL CONFORM TO THE GRADE OR CURVE.

ALL DIMENSIONS WHICH ARE GIVEN IN SECTION AND ARE AFFECTED BY DEAD LOAD DEFLECTIONS ARE DIMENSIONS AT CENTER LINE OF BEARING UNLESS OTHERWISE NOTED ON PLANS. IN SETTING FORMS FOR STEEL BEAM BRIDGES AND PRESTRESSED CONCRETE GIRDER BRIDGES, ADJUSTMENTS SHALL BE MADE DUE TO THE DEAD LOAD DEFLECTIONS FOR THE ELEVATIONS SHOWN. WHERE BLOCKS ARE SHOWN OVER BEAMS FOR BUILDING UP TO THE SLAB, THE VERTICAL DIMENSIONS OF THE BLOCKS SHALL BE ADJUSTED BETWEEN BEARINGS TO COMPENSATE FOR DEAD LOAD DEFLECTIONS, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER. WHERE BOTTOM OF SLAB IS IN LINE WITH BOTTOM OF TOP FLANGES, DEPTH OF SLAB BETWEEN BEARINGS SHALL BE ADJUSTED TO COMPENSATE FOR DEAD LOAD DEFLECTION, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER.

IN SETTING FALSEWORK AND FORMS FOR REINFORCED CONCRETE SPANS, AN ALLOWANCE SHALL BE MADE FOR DEAD LOAD DEFLECTIONS, SETTLEMENT OF FALSEWORK, AND PERMANENT CAMBER WHICH SHALL BE PROVIDED FOR IN ADDITION TO THE ELEVATIONS SHOWN. AFTER REMOVAL OF THE FALSEWORK, THE FINISHED STRUCTURES SHALL CONFORM TO THE PROFILE AND ELEVATIONS SHOWN ON THE PLANS AND CONSTRUCTION ELEVATIONS FURNISHED BY THE ENGINEER.

DETAILED DRAWINGS FOR FALSEWORK OR FORMS FOR BRIDGE SUPERSTRUCTURE AND ANY STRUCTURE OR PARTS OF A STRUCTURE AS NOTED ON THE PLANS SHALL

DETAILED DRAWINGS FOR FALSEWORK OR FORMS FOR BRIDGE SUPERSTRUCTURE AND ANY STRUCTURE OR PARTS OF A STRUCTURE AS NOTED ON THE PLANS SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL BEFORE CONSTRUCTION OF THE FALSEWORK OR FORMS IS STARTED.

REINFORCING STEEL:

ALL REINFORCING STEEL SHALL BE DEFORMED. DIMENSIONS RELATIVE TO PLACEMENT OF REINFORCING ARE TO CENTERS OF BARS UNLESS OTHERWISE INDICATED IN THE PLANS. DIMENSIONS ON BAR DETAILS ARE TO CENTERS OF BARS OR ARE OUT TO OUT AS INDICATED ON PLANS.

WIRE BAR SUPPORTS SHALL BE PROVIDED FOR REINFORCING STEEL WHERE

INDICATED ON THE PLANS. WHEN BAR SUPPORT PIECES ARE PLACED IN CONTINUOUS LINES, THEY SHALL BE SO PLACED THAT THE ENDS OF THE SUPPORTING WIRES SHALL BE LAPPED TO LOCK LEGS ON ADJOINING PIECES.

STRUCTURAL STEEL:

AT THE CONTRACTOR'S OPTION, HE MAY SUBSTITUTE 7/8" Ø SHEAR STUDS FOR THE 3/4" Ø STUDS SPECIFIED ON THE PLANS. THIS SUBSTITUTION SHALL BE MADE AT THE RATE OF 3 - 7/8" Ø STUDS FOR 4 - 3/4" Ø STUDS, AND STUD SPACING CHANGES SHALL BE MADE AS NECESSARY TO PROVIDE THE SAME EQUIVALENT NUMBER OF 7/8" Ø STUDS ALONG THE BEAM AS SHOWN FOR 3/4" Ø STUDS BASED ON THE RATIO OF 3 - 7/8" Ø STUDS FOR 4 - 3/4" Ø STUDS. STUDS OF THE LENGTH SPECIFIED ON THE PLANS MUST BE PROVIDED. THE MAXIMUM SPACING SHALL BE 2'-0".

EXCEPT AT THE INTERIOR SUPPORTS OF CONTINUOUS BEAMS WHERE THE COVER PLATE TS THE CONTRACTOR MAY AT HIS OPTION.

BE PROVIDED. THE MAXIMUM SPACING SHALL BE 2'-O".

EXCEPT AT THE INTERIOR SUPPORTS OF CONTINUOUS BEAMS WHERE THE COVER PLATE IS IN CONTACT WITH BEARING PLATE, THE CONTRACTOR MAY, AT HIS OPTION, SUBSTITUTE FOR THE COVER PLATES DESIGNATED ON THE PLANS COVER PLATES OF THE EQUIVALENT AREA PROVIDED THESE PLATES ARE AT LEAST 5/16" IN THICKNESS AND DO NOT EXCEED A WIDTH EQUAL TO THE FLANGE WIDTH LESS 2" OR A THICKNESS EQUAL TO 2 TIMES THE FLANGE THICKNESS. THE SIZE OF FILLET WELDS SHALL CONFORM TO THE REQUIREMENTS OF THE CURRENT ANSI/AASHTO/AWS "BRIDGE WELDING CODE". ELECTROSLAG WELDING WILL NOT BE PERMITTED.

WITH THE SOLE EXCEPTION OF EDGES AT SURFACES WHICH BEAR ON OTHER SURFACES, ALL SHARP EDGES AND ENDS OF SHAPES AND PLATES SHALL BE SLIGHTLY ROUNDED BY SUITABLE MEANS TO A RADIUS OF APPROXIMATELY 1/16 INCH OR EQUIVALENT FLAT SURFACE AT A SUITABLE ANGLE PRIOR TO PAINTING, GALVANIZING, OR METALLIZING.

HANDRAILS AND POSTS:

METAL STANDARDS AND FACES OF THE CONCRETE END POSTS FOR THE METAL RAIL SHALL BE SET NORMAL TO THE GRADE OF THE CURB, UNLESS OTHERWISE SHOWN ON PLANS. THE METAL RAIL AND TOPS OF CONCRETE POSTS USED WITH THE ALUMINUM RAIL SHALL BE BUILT PARALLEL TO THE GRADE OF THE CURB.

METAL HANDRAILS SHALL BE IN ACCORDANCE WITH THE PLANS. RAILS SHALL BE AS MANUFACTURED FOR BRIDGE RAILING. CASTINGS SHALL BE OF A UNIFORM APPEARANCE. FINS AND OTHER DEFORMATIONS RESULTING FROM CASTING OR OTHERWISE SHALL BE REMOVED IN A MANNER SO THAT A UNIFORM COLORING OF THE COMPLETED CASTING SHALL BE OBTAINED. CASTINGS WITH DISCOLORATIONS OR OF NON-UNIFORM COLORING WILL NOT BE ACCEPTED. CERTIFIED MILL REPORTS ARE REQUIRED FOR METAL RAILS AND POSTS.

SPECIAL NOTES:

GENERALLY, IN CASE OF DISCREPANCY, THIS STANDARD SHEET OF NOTES SHALL GOVERN OVER THE SPECIFICATIONS, BUT THE REMAINDER OF THE PLANS SHALL GOVERN OVER NOTES HEREON, AND SPECIAL PROVISIONS SHALL GOVERN OVER ALL. SEE SPECIFICATIONS ARTICLE 105-4.

ENGLISH

JANUARY, 1990