STATE	STATE PROJECT REFERENCE NO.	SHEET NO.	TOTAL SHEETS
N.C.	B-4177	1	13

STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
DIVISION OF HIGHWAYS
GEOTECHNICAL ENGINEERING UNIT

STRUCTURE SUBSURFACE INVESTIGATION

PROJ. REFERENCE NO. 33524.1.1 F.A. PROJ. BRZ-1193 (5)

COUNTY LINCOLN

PROJECT DESCRIPTION BR 142 OVER HOWARDS CREEK ON SR 1193 BETWEEN NC 27 & SR 1113

SITE DESCRIPTION

CONTENTS

CAUTION NOTICE

THE SUBSURFACE INFORMATION AND THE SUBSURFACE INVESTIGATION ON WHICH IT IS BASED WERE MADE FOR THE PURPOSE OF STUDY, PLANNING, AND DESIGN, AND NOT FOR CONSTRUCTION OR PAY PURPOSES. THE VARIOUS FIELD BORING LOGS, ROCK CORES, AND SOIL TEST DATA AVAILABLE MAY BE REVIEWED OR INSPECTED IN RALEIGH BY CONTACTING THE N.C. DEPARTMENT OF TRANSPORTATION, GEOTECHNICAL ENGINEERING UNIT AT (919) 250-4088. NEITHER THE SUBSURFACE PLANS AND REPORTS, NOR THE FIELD BORING LOGS, ROCK CORES, OR SOIL TEST DATA ARE PART OF THE CONTRACT.

GENERAL SOIL AND ROCK STRATA DESCRIPTIONS AND INDICATED BOUNDARIES ARE BASED ON A GEOTECHNICAL INTERPRETATION OF ALL AVAILABLE SUBSURFACE DATA AND MAY NOT NECESSARILY REFLECT THE ACTUAL SUBSURFACE CONDITIONS BETWEEN BORINGS OR BETWEEN SAMPLED STRATA WITHIN THE BOREHOLE. THE LABORATORY SAMPLE DATA AND THE IN SITU (IN-PLACE) TEST DATA CAN BE RELIED ON ONLY TO THE DEGREE OF RELIABILITY INHERENT IN THE STANDARD TEST METHOD. THE OBSERVED WATER LEVELS OR SOIL MOISTURE CONDITIONS INDICATED IN THE SUBSURFACE INVESTIGATIONS ARE AS RECORDED AT THE TIME OF THE INVESTIGATION. THESE WATER LEVELS OR SOIL MOISTURE CONDITIONS MAY VARY CONSIDERABLY WITH TIME ACCORDING TO CLIMATIC CONDITIONS INCLUDING TEMPERATURES, PRECIPITATION, AND WIND, AS WELL AS OTHER NON-CLIMATIC FACTORS.

THE BIDDER OR CONTRACTOR IS CAUTIONED THAT DETAILS SHOWN ON THE SUBSURFACE PLANS ARE PRELIMINARY ONLY AND IN MANY CASES THE FINAL DESIGN DETAILS ARE DIFFERENT. FOR BIDDING AND CONSTRUCTION PURPOSES, REFER TO THE CONSTRUCTION PLANS AND DOCUMENTS FOR FINAL DESIGN INFORMATION ON THIS PROJECT. THE DEPARTMENT DOES NOT WARRANT OR GUARANTEE THE SUFFICIENCY OR ACCURACY OF THE INVESTIGATION MADE, NOR THE INTERPRETATIONS MADE, OR OPINION OF THE DEPARTMENT AS TO THE TYPE OF MATERIALS AND CONDITIONS TO BE ENCOUNTERED. THE BIDDER OR CONTRACTOR IS CAUTIONED TO MAKE SUCH INDEPENDENT SUBSURFACE INVESTIGATIONS AS HE DEEMS NECESSARY TO SATISFY HIMSELF AS TO CONDITIONS TO BE ENCOUNTERED ON THIS PROJECT. THE CONTRACTOR SHALL HAVE NO CLAIM FOR ADDITIONAL COMPENSATION OR FOR AN EXTENSION OF TIME FOR ANY REASON RESULTING FROM THE ACTUAL CONDITIONS ENCOUNTERED AT THE SITE DIFFERING FROM THOSE INDICATED IN THE SUBSURFACE INFORMATION.

PERSONNEL

J. K. STICKNEY

C. L. SMITH

H. K. WISE

INVESTIGATED BY J. E. BEVERLY

CHECKED BY C. B. LITTLE

SUBMITTED BY C. B. LITTLE

DATE AUGUST 2007

NORTH CAROLINA LICENSED SEAL 11041 GEOLOGIST CLINTON B. LITTLE 8-23-07

PROJECT: 33524.1.1

ID: B-4177

DRAWN BY: C. E. BURRIS

NOTE - THE INFORMATION CONTAINED HEREIN IS NOT IMPLIED OR GUARANTEED BY THE N. C. DEPARTMENT OF TRANSPORTATION AS BEING ACCURATE NOR IT IS CONSIDERED TO BE PART OF THE PLANS, SPECIFICATIONS, OR CONTRACT FOR THE PROJECT.

NOTE - BY HAVING REQUESTED THIS INFORMATION THE CONTRACTOR SPECIFICALLY WAIVES ANY CLAIMS FOR INCREASED COMPENSATION OR EXTENSION OF TIME BASED ON DIFFERENCES BETWEEN THE CONDITIONS INDICATED HEREIN AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

PROJECT REFERENCE NO.	SHEET NO.
B-4177	2

NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

DIVISION OF HIGHWAYS

GEOTECHNICAL ENGINEERING UNIT

SUBSURFACE INVESTIGATION

SOIL AND ROCK LEGEND, TERMS, SYMBOLS, AND ABBREVIATIONS

SOIL DESCRIPTION

SOIL IS CONSIDERED TO BE THE UNCONSOLIDATED, SEMI-CONSOLIDATED, OR WEATHERED EARTH MATERIALS THAT CAN BE PENETRATED WITH A CONTINUOUS FLIGHT POWER AUGER, AND YIELD LESS THAN 100 BLOWS PER FOOT ACCORDING TO STANDARD PENETRATION TEST (AASHTO T206, ASTM D-1586). SOIL CLASSIFICATION IS BASED ON THE AASHTO SYSTEM. BASIC DESCRIPTIONS GENERALLY SHALL INCLUDE: CONSISTENCY, COLOR, TEXTURE, MOISTURE, AASHTO CLASSIFICATION, AND OTHER PERTINENT FACTORS SUCH AS MINERALOGICAL COMPOSITION, ANGULARITY, STRUCTURE, PLASTICITY, ETC. EXAMPLE:

VERY STIFF, GRAY, SILTY CLAY, MOIST WITH INTERBEDDED FINE SAND LAYERS, HIGHLY PLASTIC, A-7-6

SOIL LEGEND AND AASHTO CLASSIFICATION

GENERAL CLASS.	GRANULAR MATERIALS (≤ 35% PASSING #200)							SILT-CLAY MATERIALS (> 35% PASSING #200)				ORGANIC MATERIALS		
GROUP CLASS.	A-1		A-3	A-2				A-4	A-5	A-6	A-7	A-1, A-2 A-3	A-4, A-5 A-6, A-7	
	A-1-a	A-1-b		A-2-4	A-2-5	A-2-6	A-2-7				A-7-5 A-7-6			
SYMBOL														
% PASSING # 10 # 40 # 200	50 MX 30 MX 15 MX	50 MX 25 MX	51 MN 10 MX	35 MX	35 MX	35 MX	35 MX	36 MN	36 MN	36 MN	36 MN	GRANULAR SOILS	SILT-CLAY SOILS	MUCK, PEAT
LIQUID LIMIT	6 MX		NP	40 MX	41 MN	40 MX	41 MN	40 MX	41 MN	40 MX	41 MN	SOILS WITH LITTLE OR MODERATE AMOUNTS OF ORGANIC MATTER		HIGHLY ORGANIC SOILS
PLASTIC INDEX				10 MX	10 MX	11 MN	11 MN	10 MX	10 MX	11 MN	11 MN			
GROUP INDEX	0		0	0		4 MX		8 MX	12 MX	16 MX	No MX			
USUAL TYPES OF MAJOR MATERIALS	STONE FRAGS. GRAVEL, AND SAND		FINE SAND	SILTY OR CLAYEY GRAVEL AND SAND				SILTY SOILS		CLAYEY SOILS				
GEN. RATING AS A SUBGRADE	EXCELLENT TO GOOD							FAIR TO POOR				FAIR TO POOR	POOR	UNSUITABLE

PI OF A-7-5 SUBGROUP IS ≤ LL - 30 ; PI OF A-7-6 SUBGROUP IS > LL - 30

CONSISTENCY OR DENSENESS

PRIMARY SOIL TYPE	COMPACTNESS OR CONSISTENCY	RANGE OF STANDARD PENETRATION RESISTENCE (N-VALUE)	RANGE OF UNCONFINED COMPRESSIVE STRENGTH ($TONS/FT^2$)
GENERALLY GRANULAR MATERIAL (NON-COHESIVE)	VERY LOOSE LOOSE MEDIUM DENSE DENSE VERY DENSE	<4 4 TO 10 10 TO 30 30 TO 50 >50	N/A
GENERALLY SILT-CLAY MATERIAL (COHESIVE)	VERY SOFT SOFT MEDIUM STIFF STIFF VERY STIFF HARD	<2 2 TO 4 4 TO 8 8 TO 15 15 TO 30 >30	<0.25 0.25 TO 0.50 0.5 TO 1.0 1 TO 2 2 TO 4 >4

TEXTURE OR GRAIN SIZE

U.S. STD. SIEVE SIZE	4	10	40	60	200	270
OPENING (MM)	4.76	2.00	0.42	0.25	0.075	0.053

BOULDER (BLDR.)	COBBLE (COB.)	GRAVEL (GR.)	COARSE SAND (CSE. SD.)	FINE SAND (F SD.)	SILT (SL.)	CLAY (CL.)

GRAIN SIZE	MM	305	75	2.0	0.25	0.05	0.005
	IN.	12	3				

SOIL MOISTURE - CORRELATION OF TERMS

SOIL MOISTURE SCALE (ATTERBERG LIMITS)	FIELD MOISTURE DESCRIPTION	GUIDE FOR FIELD MOISTURE DESCRIPTION
	- SATURATED - (SAT.)	USUALLY LIQUID; VERY WET, USUALLY FROM BELOW THE GROUND WATER TABLE
LL — LIQUID LIMIT		
PLASTIC RANGE (PI)	- WET - (W)	SEMISOLID; REQUIRES DRYING TO ATTAIN OPTIMUM MOISTURE
PL — PLASTIC LIMIT		
OM — OPTIMUM MOISTURE	- MOIST - (M)	SOLID; AT OR NEAR OPTIMUM MOISTURE
SL — SHRINKAGE LIMIT		
	- DRY - (D)	REQUIRES ADDITIONAL WATER TO ATTAIN OPTIMUM MOISTURE

PLASTICITY

	PLASTICITY INDEX (PI)	DRY STRENGTH
NONPLASTIC	0-5	VERY LOW
LOW PLASTICITY	6-15	SLIGHT
MED. PLASTICITY	16-25	MEDIUM
HIGH PLASTICITY	26 OR MORE	HIGH

COLOR

DESCRIPTIONS MAY INCLUDE COLOR OR COLOR COMBINATIONS (TAN, RED, YELLOW-BROWN, BLUE-GRAY), MODIFIERS SUCH AS LIGHT, DARK, STREAKED, ETC. ARE USED TO DESCRIBE APPEARANCE.

GRADATION

WELL GRADED - INDICATES A GOOD REPRESENTATION OF PARTICLE SIZES FROM FINE TO COARSE.
UNIFORM - INDICATES THAT SOIL PARTICLES ARE ALL APPROXIMATELY THE SAME SIZE. (ALSO POORLY GRADED)
GAP-GRADED - INDICATES A MIXTURE OF UNIFORM PARTICLES OF TWO OR MORE SIZES.

ANGULARITY OF GRAINS

THE ANGULARITY OR ROUNDNESS OF SOIL GRAINS IS DESIGNATED BY THE TERMS: ANGULAR, SUBANGULAR, SUBROUNDED, OR ROUNDED.

MINERALOGICAL COMPOSITION

MINERAL NAMES SUCH AS QUARTZ, FELDSPAR, MICA, TALC, KAOLIN, ETC. ARE USED IN DESCRIPTIONS WHENEVER THEY ARE CONSIDERED OF SIGNIFICANCE.

COMPRESSIBILITY

SLIGHTLY COMPRESSIBLE	LIQUID LIMIT LESS THAN 31
MODERATELY COMPRESSIBLE	LIQUID LIMIT EQUAL TO 31-50
HIGHLY COMPRESSIBLE	LIQUID LIMIT GREATER THAN 50

PERCENTAGE OF MATERIAL

ORGANIC MATERIAL	GRANULAR SOILS	SILT - CLAY SOILS	OTHER MATERIAL	
TRACE OF ORGANIC MATTER	2 - 3%	3 - 5%	TRACE	1 - 10%
LITTLE ORGANIC MATTER	3 - 5%	5 - 12%	LITTLE	10 - 20%
MODERATELY ORGANIC	5 - 10%	12 - 20%	SOME	20 - 35%
HIGHLY ORGANIC	>10%	>20%	HIGHLY	35% AND ABOVE

GROUND WATER

- WATER LEVEL IN BORE HOLE IMMEDIATELY AFTER DRILLING
- STATIC WATER LEVEL AFTER 24 HOURS
- PW — PERCHED WATER, SATURATED ZONE, OR WATER BEARING STRATA
- SPRING OR SEEP

MISCELLANEOUS SYMBOLS

- ROADWAY EMBANKMENT (RE) WITH SOIL DESCRIPTION
- SOIL SYMBOL
- ARTIFICIAL FILL (AF) OTHER THAN ROADWAY EMBANKMENT
- INFERRED SOIL BOUNDARY
- INFERRED ROCK LINE
- ALLUVIAL SOIL BOUNDARY
- 25/025 — DIP & DIP DIRECTION OF ROCK STRUCTURES
- SOUNDING ROD
- SPT CPT DPT DMT VST PMT — TEST BORING
- AUGER BORING
- CORE BORING
- MW — MONITORING WELL
- PIEZOMETER INSTALLATION
- SLOPE INDICATOR INSTALLATION
- SPT N-VALUE
- REF — SPT REFUSAL

SAMPLE DESIGNATIONS

S - BULK SAMPLE
SS - SPLIT SPOON SAMPLE
ST - SHELBY TUBE SAMPLE
RS - ROCK SAMPLE
RT - RECOMPACTED TRIAXIAL SAMPLE
CBR - CALIFORNIA BEARING RATIO SAMPLE

ABBREVIATIONS

AR - AUGER REFUSAL
BT - BORING TERMINATED
CL. - CLAY
CPT - CONE PENETRATION TEST
CSE. - COARSE
DMT - DILATOMETER TEST
DPT - DYNAMIC PENETRATION TEST
e - VOID RATIO
F - FINE
FOSS. - FOSSILIFEROUS
FRAC. - FRACTURED, FRACTURES
FRAGS. - FRAGMENTS
HI. - HIGHLY
MED. - MEDIUM
MICA. - MICACEOUS
MOD. - MODERATELY
NP - NON PLASTIC
ORG. - ORGANIC
PMT - PRESSUREMETER TEST
SAP. - SAPROLITIC
SD. - SAND, SANDY
SL. - SILT, SILTY
SLI. - SLIGHTLY
TCR - TRICONE REFUSAL
w - MOISTURE CONTENT
V - VERY
VST - VANE SHEAR TEST
WEA. - WEATHERED
γ - UNIT WEIGHT
γ_d - DRY UNIT WEIGHT

EQUIPMENT USED ON SUBJECT PROJECT

DRILL UNITS:
- [ ] MOBILE B-___
- [ ] BK-51
- [ ] CME-45C
- [X] CME-550
- [ ] PORTABLE HOIST
- [ ] _____
- [ ] _____

ADVANCING TOOLS:
- [ ] CLAY BITS
- [ ] 6" CONTINUOUS FLIGHT AUGER
- [X] 8" HOLLOW AUGERS
- [ ] HARD FACED FINGER BITS
- [X] TUNG.-CARBIDE INSERTS
- [ ] CASING [ ] W/ ADVANCER
- [ ] TRICONE _____" STEEL TEETH
- [ ] TRICONE _____" TUNG.-CARB.
- [X] CORE BIT
- [ ] _____

HAMMER TYPE:
- [X] AUTOMATIC [ ] MANUAL

CORE SIZE:
- [ ] -B_____
- [X] -N NQ
- [ ] -H_____

HAND TOOLS:
- [ ] POST HOLE DIGGER
- [ ] HAND AUGER
- [ ] SOUNDING ROD
- [ ] VANE SHEAR TEST
- [ ] _____

ROCK DESCRIPTION

HARD ROCK IS NON-COASTAL PLAIN MATERIAL THAT IF TESTED, WOULD YIELD SPT REFUSAL. AN INFERRED ROCK LINE INDICATES THE LEVEL AT WHICH NON-COASTAL PLAIN MATERIAL WOULD YIELD SPT REFUSAL. SPT REFUSAL IS PENETRATION BY A SPLIT SPOON SAMPLER EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS. IN NON-COASTAL PLAIN MATERIAL, THE TRANSITION BETWEEN SOIL AND ROCK IS OFTEN REPRESENTED BY A ZONE OF WEATHERED ROCK.
ROCK MATERIALS ARE TYPICALLY DIVIDED AS FOLLOWS:

WEATHERED ROCK (WR)	NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT N VALUES > 100 BLOWS PER FOOT IF TESTED.
CRYSTALLINE ROCK (CR)	FINE TO COARSE GRAIN IGNEOUS AND METAMORPHIC ROCK THAT WOULD YIELD SPT REFUSAL IF TESTED. ROCK TYPE INCLUDES GRANITE, GNEISS, GABBRO, SCHIST, ETC.
NON-CRYSTALLINE ROCK (NCR)	FINE TO COARSE GRAIN METAMORPHIC AND NON-COASTAL PLAIN SEDIMENTARY ROCK THAT WOULD YEILD SPT REFUSAL IF TESTED. ROCK TYPE INCLUDES PHYLLITE, SLATE, SANDSTONE, ETC.
COASTAL PLAIN SEDIMENTARY ROCK (CP)	COASTAL PLAIN SEDIMENTS CEMENTED INTO ROCK, BUT MAY NOT YIELD SPT REFUSAL. ROCK TYPE INCLUDES LIMESTONE, SANDSTONE, CEMENTED SHELL BEDS, ETC.

WEATHERING

FRESH	ROCK FRESH, CRYSTALS BRIGHT, FEW JOINTS MAY SHOW SLIGHT STAINING. ROCK RINGS UNDER HAMMER IF CRYSTALLINE.
VERY SLIGHT (V SLI.)	ROCK GENERALLY FRESH, JOINTS STAINED, SOME JOINTS MAY SHOW THIN CLAY COATINGS IF OPEN, CRYSTALS ON A BROKEN SPECIMEN FACE SHINE BRIGHTLY. ROCK RINGS UNDER HAMMER BLOWS IF OF A CRYSTALLINE NATURE.
SLIGHT (SLI.)	ROCK GENERALLY FRESH, JOINTS STAINED AND DISCOLORATION EXTENDS INTO ROCK UP TO 1 INCH. OPEN JOINTS MAY CONTAIN CLAY. IN GRANITOID ROCKS SOME OCCASIONAL FELDSPAR CRYSTALS ARE DULL AND DISCOLORED. CRYSTALLINE ROCKS RING UNDER HAMMER BLOWS.
MODERATE (MOD.)	SIGNIFICANT PORTIONS OF ROCK SHOW DISCOLORATION AND WEATHERING EFFECTS. IN GRANITOID ROCKS, MOST FELDSPARS ARE DULL AND DISCOLORED, SOME SHOW CLAY. ROCK HAS DULL SOUND UNDER HAMMER BLOWS AND SHOWS SIGNIFICANT LOSS OF STRENGTH AS COMPARED WITH FRESH ROCK.
MODERATELY SEVERE (MOD. SEV.)	ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. IN GRANITOID ROCKS, ALL FELDSPARS DULL AND DISCOLORED AND A MAJORITY SHOW KAOLINIZATION. ROCK SHOWS SEVERE LOSS OF STRENGTH AND CAN BE EXCAVATED WITH A GEOLOGIST'S PICK. ROCK GIVES "CLUNK" SOUND WHEN STRUCK. *IF TESTED, WOULD YIELD SPT REFUSAL*
SEVERE (SEV.)	ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. ROCK FABRIC CLEAR AND EVIDENT BUT REDUCED IN STRENGTH TO STRONG SOIL. IN GRANITOID ROCKS ALL FELDSPARS ARE KAOLINIZED TO SOME EXTENT. SOME FRAGMENTS OF STRONG ROCK USUALLY REMAIN. *IF TESTED, YIELDS SPT N VALUES > 100 BPF*
VERY SEVERE (V SEV.)	ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. ROCK FABRIC ELEMENTS ARE DISCERNIBLE BUT THE MASS IS EFFECTIVELY REDUCED TO SOIL STATUS, WITH ONLY FRAGMENTS OF STRONG ROCK REMAINING. SAPROLITE IS AN EXAMPLE OF ROCK WEATHERED TO A DEGREE SUCH THAT ONLY MINOR VESTIGES OF THE ORIGINAL ROCK FABRIC REMAIN. *IF TESTED, YIELDS SPT N VALUES < 100 BPF*
COMPLETE	ROCK REDUCED TO SOIL. ROCK FABRIC NOT DISCERNIBLE, OR DISCERNIBLE ONLY IN SMALL AND SCATTERED CONCENTRATIONS. QUARTZ MAY BE PRESENT AS DIKES OR STRINGERS. SAPROLITE IS ALSO AN EXAMPLE.

ROCK HARDNESS

VERY HARD	CANNOT BE SCRATCHED BY KNIFE OR SHARP PICK. BREAKING OF HAND SPECIMENS REQUIRES SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK.
HARD	CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY. HARD HAMMER BLOWS REQUIRED TO DETACH HAND SPECIMEN.
MODERATELY HARD	CAN BE SCRATCHED BY KNIFE OR PICK. GOUGES OR GROOVES TO 0.25 INCHES DEEP CAN BE EXCAVATED BY HARD BLOW OF A GEOLOGIST'S PICK. HAND SPECIMENS CAN BE DETACHED BY MODERATE BLOWS.
MEDIUM HARD	CAN BE GROOVED OR GOUGED 0.05 INCHES DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT. CAN BE EXCAVATED IN SMALL CHIPS TO PEICES 1 INCH MAXIMUM SIZE BY HARD BLOWS OF THE POINT OF A GEOLOGIST'S PICK.
SOFT	CAN BE GROVED OR GOUGED READILY BY KNIFE OR PICK. CAN BE EXCAVATED IN FRAGMENTS FROM CHIPS TO SEVERAL INCHES IN SIZE BY MODERATE BLOWS OF A PICK POINT. SMALL, THIN PIECES CAN BE BROKEN BY FINGER PRESSURE.
VERY SOFT	CAN BE CARVED WITH KNIFE. CAN BE EXCAVATED READILY WITH POINT OF PICK. PIECES 1 INCH OR MORE IN THICKNESS CAN BE BROKEN BY FINGER PRESSURE. CAN BE SCRATCHED READILY BY FINGERNAIL.

FRACTURE SPACING

TERM	SPACING
VERY WIDE	MORE THAN 10 FEET
WIDE	3 TO 10 FEET
MODERATELY CLOSE	1 TO 3 FEET
CLOSE	0.16 TO 1 FEET
VERY CLOSE	LESS THAN 0.16 FEET

BEDDING

TERM	THICKNESS
VERY THICKLY BEDDED	> 4 FEET
THICKLY BEDDED	1.5 - 4 FEET
THINLY BEDDED	0.16 - 1.5 FEET
VERY THINLY BEDDED	0.03 - 0.16 FEET
THICKLY LAMINATED	0.008 - 0.03 FEET
THINLY LAMINATED	< 0.008 FEET

INDURATION

FOR SEDIMENTARY ROCKS, INDURATION IS THE HARDENING OF THE MATERIAL BY CEMENTING, HEAT, PRESSURE, ETC.

FRIABLE	RUBBING WITH FINGER FREES NUMEROUS GRAINS; GENTLE BLOW BY HAMMER DISINTEGRATES SAMPLE.
MODERATELY INDURATED	GRAINS CAN BE SEPARATED FROM SAMPLE WITH STEEL PROBE; BREAKS EASILY WHEN HIT WITH HAMMER.
INDURATED	GRAINS ARE DIFFICULT TO SEPARATE WITH STEEL PROBE; DIFFICULT TO BREAK WITH HAMMER.
EXTREMELY INDURATED	SHARP HAMMER BLOWS REQUIRED TO BREAK SAMPLE; SAMPLE BREAKS ACROSS GRAINS.

TERMS AND DEFINITIONS

ALLUVIUM (ALLUV.) - SOILS THAT HAVE BEEN TRANSPORTED BY WATER.

AQUIFER - A WATER BEARING FORMATION OR STRATA.

ARENACEOUS - APPLIED TO ROCKS THAT HAVE BEEN DERIVED FROM SAND OR THAT CONTAIN SAND.

ARGILLACEOUS - APPLIED TO ALL ROCKS OR SUBSTANCES COMPOSED OF CLAY MINERALS, OR HAVING A NOTABLE PROPORTION OF CLAY IN THEIR COMPOSITION, AS SHALE, SLATE, ETC.

ARTESIAN - GROUND WATER THAT IS UNDER SUFFICIENT PRESSURE TO RISE ABOVE THE LEVEL AT WHICH IT IS ENCOUNTERED, BUT WHICH DOES NOT NECESSARILY RISE TO OR ABOVE THE GROUND SURFACE.

CALCAREOUS (CALC.) - SOILS THAT CONTAIN APPRECIABLE AMOUNTS OF CALCIUM CARBONATE.

COLLUVIUM - ROCK FRAGMENTS MIXED WITH SOIL DEPOSITED BY GRAVITY ON SLOPE OR AT BOTTOM OF SLOPE.

CORE RECOVERY (REC.) - TOTAL LENGTH OF ALL MATERIAL RECOVERED IN THE CORE BARREL DIVIDED BY TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE.

DIKE - A TABULAR BODY OF IGNEOUS ROCK THAT CUTS ACROSS THE STRUCTURE OF ADJACENT ROCKS OR CUTS MASSIVE ROCK.

DIP - THE ANGLE AT WHICH A STRATUM OR ANY PLANAR FEATURE IS INCLINED FROM THE HORIZONTAL.

DIP DIRECTION (DIP AZIMUTH) - THE DIRECTION OR BEARING OF THE HORIZONTAL TRACE OF THE LINE OF DIP, MEASURED CLOCKWISE FROM NORTH.

FAULT - A FRACTURE OR FRACTURE ZONE ALONG WHICH THERE HAS BEEN DISPLACEMENT OF THE SIDES RELATIVE TO ONE ANOTHER PARALLEL TO THE FRACTURE.

FISSILE - A PROPERTY OF SPLITTING ALONG CLOSELY SPACED PARALLEL PLANES.

FLOAT - ROCK FRAGMENTS ON SURFACE NEAR THEIR ORIGINAL POSITION AND DISLODGED FROM PARENT MATERIAL.

FLOOD PLAIN (FP) - LAND BORDERING A STREAM, BUILT OF SEDIMENTS DEPOSITED BY THE STREAM.

FORMATION (FM.) - A MAPPABLE GEOLOGIC UNIT THAT CAN BE RECOGNIZED AND TRACED IN THE FIELD.

JOINT - FRACTURE IN ROCK ALONG WHICH NO APPRECIABLE MOVEMENT HAS OCCURRED.

LEDGE - A SHELF-LIKE RIDGE OR PROJECTION OF ROCK WHOSE THICKNESS IS SMALL COMPARED TO ITS LATERAL EXTENT.

LENS - A BODY OF SOIL OR ROCK THAT THINS OUT IN ONE OR MORE DIRECTIONS.

MOTTLED (MOT.) - IRREGULARLY MARKED WITH SPOTS OF DIFFERENT COLORS. MOTTLING IN SOILS USUALLY INDICATES POOR AERATION AND LACK OF GOOD DRAINAGE.

PERCHED WATER - WATER MAINTAINED ABOVE THE NORMAL GROUND WATER LEVEL BY THE PRESENCE OF AN INTERVENING IMPERVIOUS STRATUM.

RESIDUAL (RES.) SOIL - SOIL FORMED IN PLACE BY THE WEATHERING OF ROCK.

ROCK QUALITY DESIGNATION (RQD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE.

SAPROLITE (SAP.) - RESIDUAL SOIL THAT RETAINS THE RELIC STRUCTURE OR FABRIC OF THE PARENT ROCK.

SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, THAT HAS BEEN EMPLACED PARALLEL TO THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS.

SLICKENSIDE - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT OR SLIP PLANE.

STANDARD PENETRATION TEST (PENETRATION RESISTANCE) (SPT) - NUMBER OF BLOWS (N OR BPF) OF A 140 LB. HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF 1 FOOT INTO SOIL WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT REFUSAL IS PENETRATION EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS.

STRATA CORE RECOVERY (SREC.) - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY TOTAL LENGTH OF STRATUM AND EXPRESSED AS A PERCENTAGE.

STRATA ROCK QUALITY DESIGNATION (SRQD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS WITHIN A STRATUM EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF STRATA AND EXPRESSED AS A PERCENTAGE.

TOPSOIL (TS.) - SURFACE SOILS USUALLY CONTAINING ORGANIC MATTER.

BENCH MARK: BM 2: R.R. SPIKE IN BASE OF 10" OAK TREE -BL- STATION 21+27.68 (57.03' LT.)

N 646552 E 1297347 ELEVATION: 831.74 FT.

NOTES:

REVISED 02/23/06

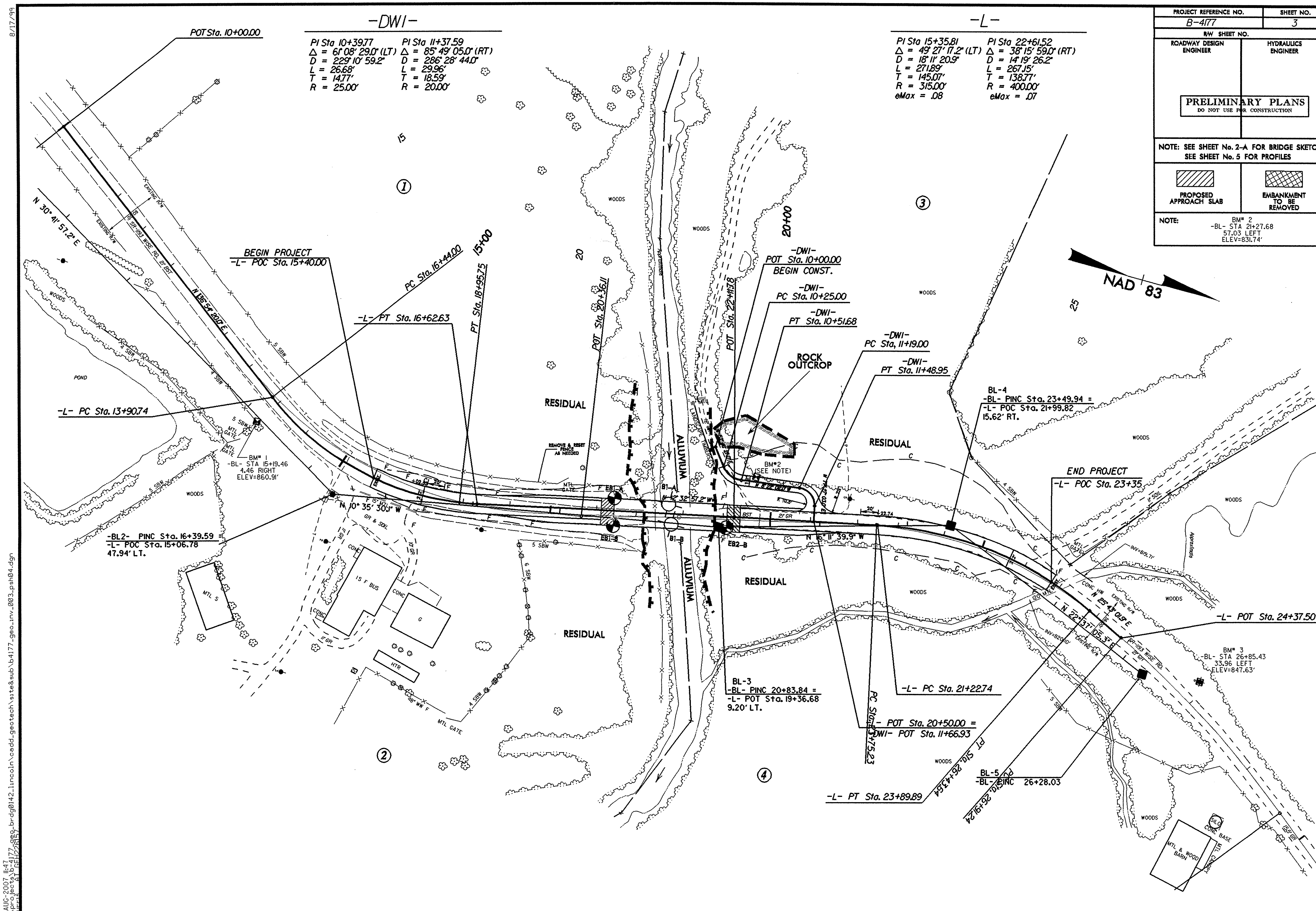
PROJECT REFERENCE NO.	SHEET NO.
B-4177	3
R/W SHEET NO.	
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER

PRELIMINARY PLANS
DO NOT USE FOR CONSTRUCTION

NOTE: SEE SHEET No. 2-A FOR BRIDGE SKETCH
SEE SHEET No. 5 FOR PROFILES

PROPOSED APPROACH SLAB	EMBANKMENT TO BE REMOVED

NOTE: BM# 2
-BL- STA 21+27.68
57.03 LEFT
ELEV=831.74'



5/14/99

PROJECT REFERENCE NO.	SHEET NO.
B-4177	4
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER

PRELIMINARY PLANS
DO NOT USE FOR CONSTRUCTION

-L-

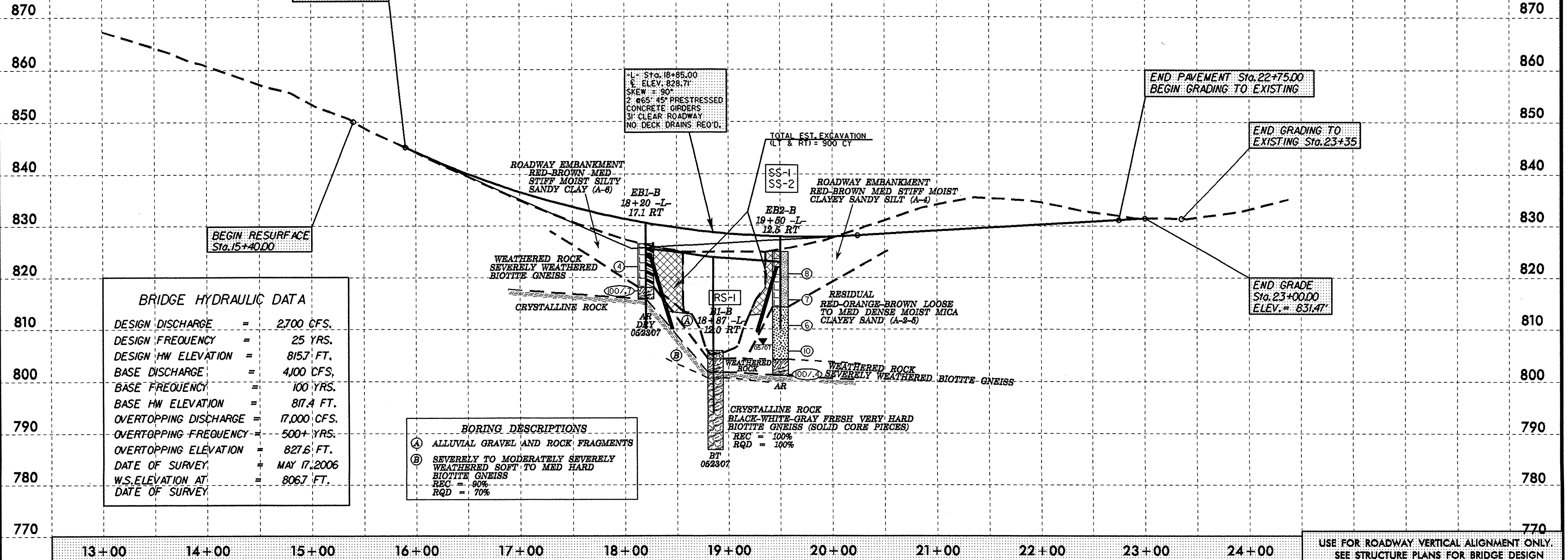
BM 1: R.R. SPIKE IN BASE OF WOOD FENCE CORNER
-BL- Sta. 15+19.46 (4.46' RT.)
ELEV. = 860.91'
N 645984 E 1297438

BM 2: R.R. SPIKE IN BASE OF 10" OAK TREE
-BL- Sta. 21+27.68 (57.03' LT.)
ELEV. = 831.74'
N 646552 E 1297347

PI = 18+07.00
EL = 825.49'
VC = 434'
K = 42

SOIL TEST RESULTS

SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C. SAND	F. SAND	SILT	CLAY	10	40	200		
SS-1	12.5 RT	19+50	3.70 - 4.70	A-4(1)	27	9	24.6	33.5	15.4	26.4	84	71	41		
SS-2	12.5 RT	19+50	13.70 - 14.70	A-2-5(0)	46	NP	28.0	48.8	8.90	14.2	98	88	30		



BRIDGE HYDRAULIC DATA

DESIGN DISCHARGE	=	2,700 CFS.
DESIGN FREQUENCY	=	25 YRS.
DESIGN HW ELEVATION	=	815.7 FT.
BASE DISCHARGE	=	4,100 CFS.
BASE FREQUENCY	=	100 YRS.
BASE HW ELEVATION	=	817.4 FT.
OVERTOPPING DISCHARGE	=	17,000 CFS.
OVERTOPPING FREQUENCY	=	500+ YRS.
OVERTOPPING ELEVATION	=	827.6 FT.
DATE OF SURVEY	=	MAY 17, 2006
W.S. ELEVATION AT DATE OF SURVEY	=	806.7 FT.

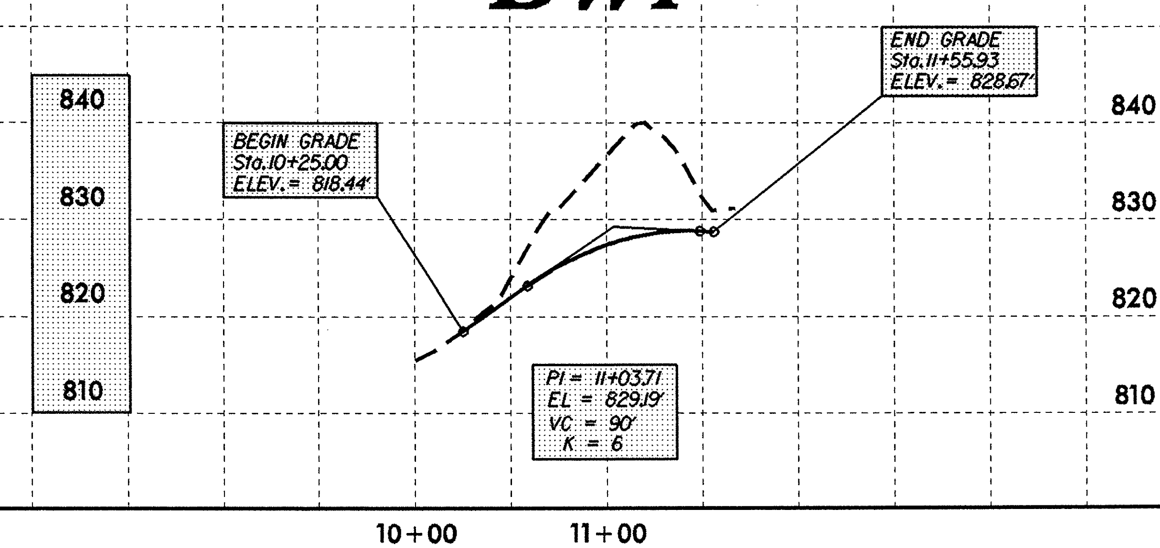
BORING DESCRIPTIONS

(A) ALLUVIAL GRAVEL AND ROCK FRAGMENTS

(B) SEVERELY TO MODERATELY SEVERELY WEATHERED SOFT TO MED HARD BIOTITE GNEISS
REC = 90%
RQD = 70%

USE FOR ROADWAY VERTICAL ALIGNMENT ONLY.
SEE STRUCTURE PLANS FOR BRIDGE DESIGN

-DW1-



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5/28/99

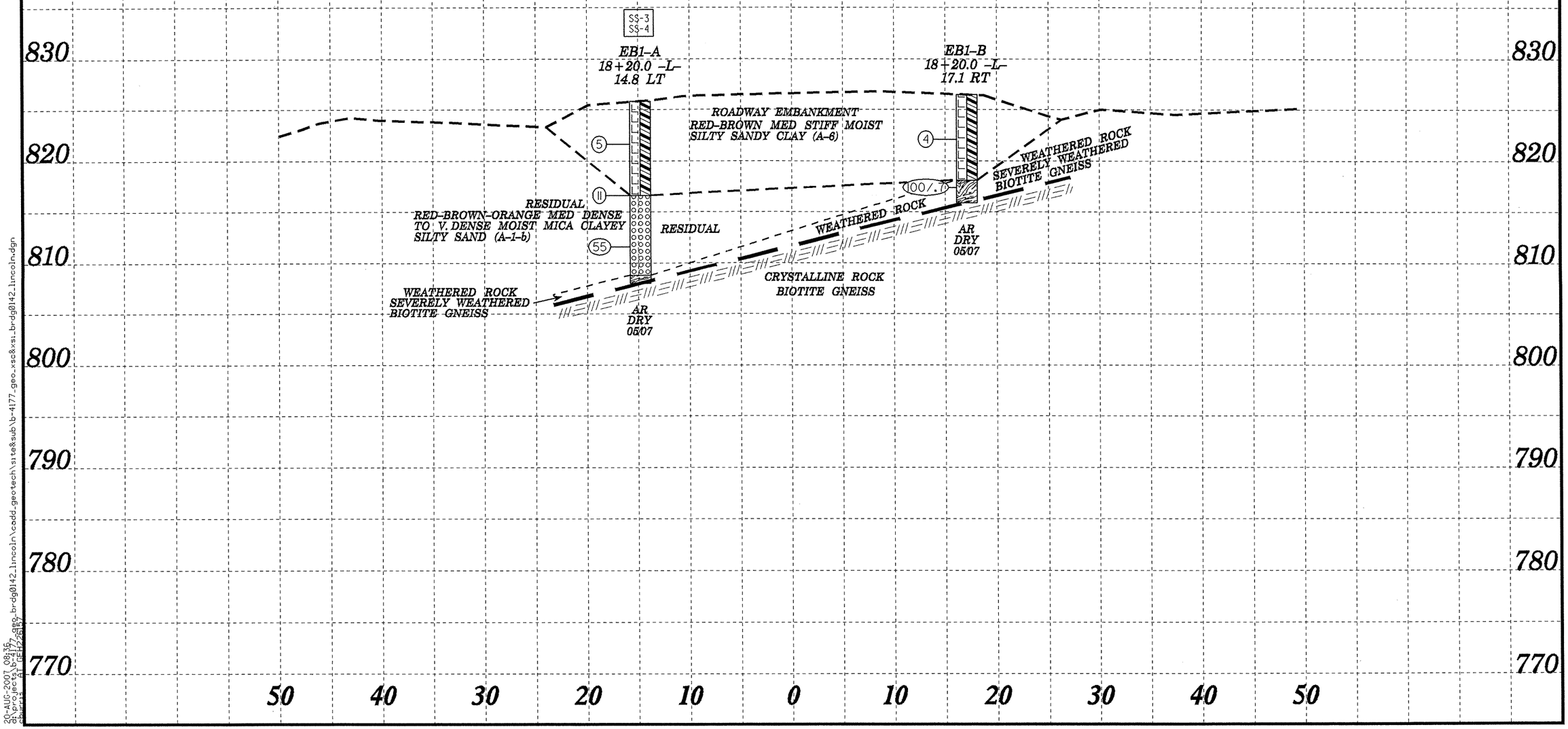
PROJECT REFERENCE NO.	SHEET
B-4177	5

SECTION THRU END BENT 1
STA. 18+20 -L- SKEW=90°

0 5 10
FEET

SOIL TEST RESULTS

SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C. SAND	F. SAND	SILT	CLAY	10	40	200		
SS-3	14.80 LT	18+20	3.70 - 4.70	A-6(7)	37	18	18.7	27.8	10.8	42.7	97	87	67		
SS-4	14.80 LT	18+20	13.70 - 14.70	A-1-b(0)	34	NP	35.4	41.3	13.2	10.2	41	32	13		



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5/28/99

PROJECT REFERENCE NO.	SHEET
B-4177	6

SECTION THRU BENT 1
STA. 18+85 -L- SKEW=90°

0 5 10
FEET

850 840 830 820 810 800 790 780 770 760

BRIDGE DECK

RS-2
B1-A
18+83 -L-
11.1 LT

RS-1
B1-B
18+87 -L-
12.0 RT

WATER SURFACE ON 05-23-07

WEATHERED ROCK
BROWN-GRAY-WHITE SEVERELY WEATHERED BIOTITE GNEISS
REC = 47%
RQD = 0%

CRYSTALLINE ROCK
BROWN-GRAY-WHITE SEVERE TO MOD. WEATHERED MOD. HARD TO HARD BIOTITE GNEISS W/V. CLOSE TO CLOSE FRACTURE SPACING
REC = 88%
RQD = 34%

GRAY-WHITE-BLACK FRESH VERY HARD BIOTITE GNEISS (EACH RUN CONSISTS OF A SOLID CORE PIECE)
REC = 100%
RQD = 100%

BT
05/07

ALLUVIAL GRAVEL AND ROCK FRAGMENTS

WEATHERED ROCK
SEVERELY WEATHERED BIOTITE GNEISS
REC = 90%
RQD = 70%

CRYSTALLINE ROCK
SEVERELY TO MODERATELY SEVERELY WEATHERED SOFT TO MED HARD BIOTITE GNEISS
REC = 100%
RQD = 100%

CRYSTALLINE ROCK
BLACK-WHITE-GRAY FRESH VERY HARD BIOTITE GNEISS (SOLID CORE PIECES)

BT
05/07

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PROJECT REFERENCE NO.	SHEET
B-4177	7

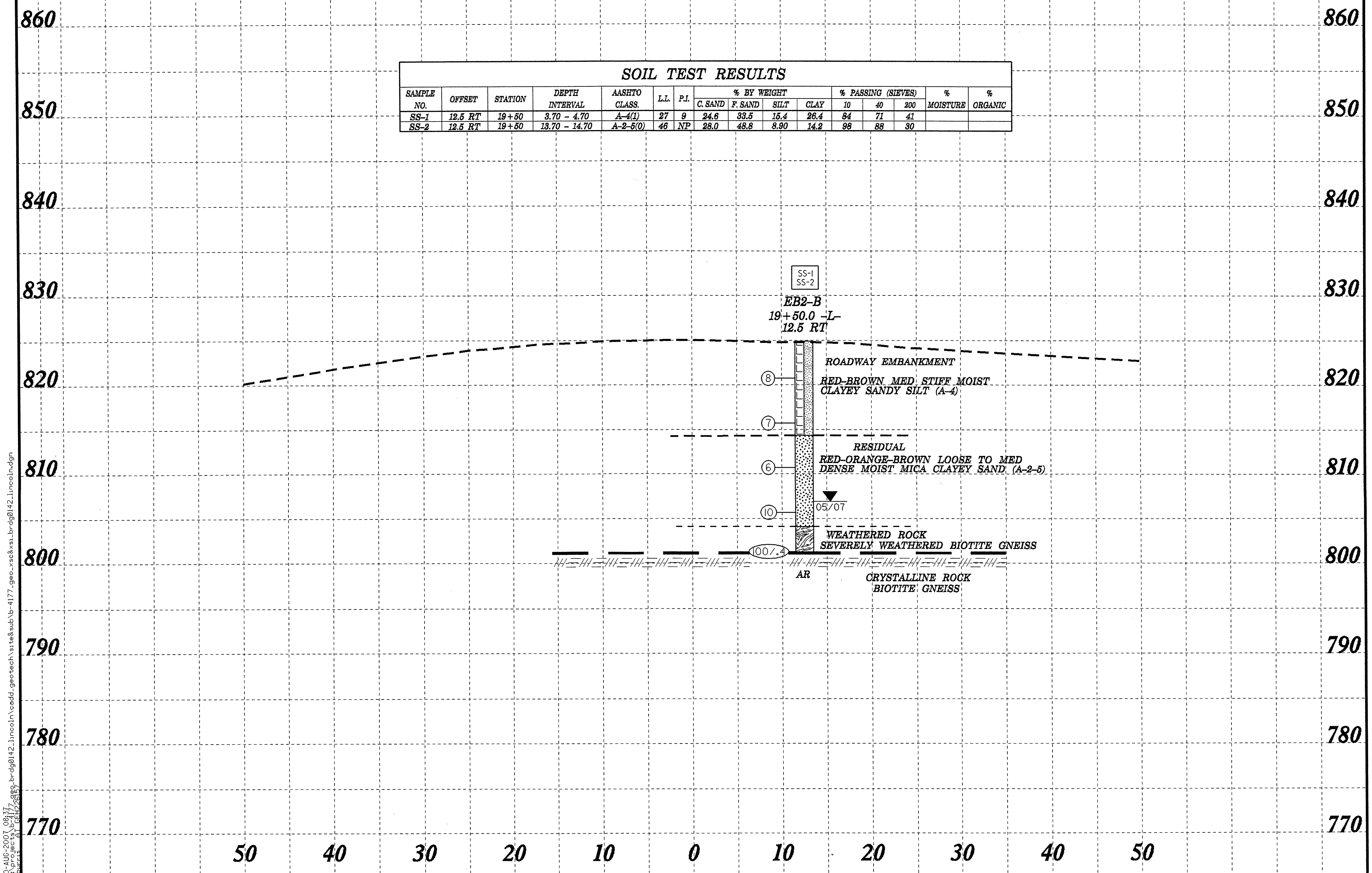
SECTION THRU END BENT 2
STA. 19+50 -L- SKEW=90°

Scale: 0 5 10 FEET

5/28/99

SOIL TEST RESULTS

SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C. SAND	F. SAND	SILT	CLAY	10	40	200		
SS-1	12.5 RT	19+50	3.70 - 4.70	A-4(1)	27	9	24.6	33.5	15.4	26.4	84	71	41		
SS-2	12.5 RT	19+50	13.70 - 14.70	A-2-5(0)	46	NP	28.0	48.8	8.90	14.2	98	88	30		



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NCDOT GEOTECHNICAL ENGINEERING UNIT
BORELOG REPORT

PROJECT NO. 33524.1	**ID.** B-4177	**COUNTY** LINCOLN	**GEOLOGIST** Stickney, J. K.
SITE DESCRIPTION BRIDGE #142 OVER HOWARDS CREEK ON SR 1193 BETWEEN NC 27 AND SR 1113			**GROUND WTR (ft)**
BORING NO. EB1-A	**STATION** 18+20	**OFFSET** 15ft LT	**ALIGNMENT** -L- **0 HR.** Dry
COLLAR ELEV. 826.5 ft	**TOTAL DEPTH** 17.8 ft	**NORTHING** 646,402	**EASTING** 1,297,414 **24 HR.** FIAD
DRILL MACHINE CME-550X	**DRILL METHOD** H.S. Augers		**HAMMER TYPE** Automatic
START DATE 05/24/07	**COMP. DATE** 05/24/07	**SURFACE WATER DEPTH** N/A	**DEPTH TO ROCK** 17.8 ft

ELEV (ft)	DEPTH (ft)	BLOW COUNT 0.5ft	0.5ft	0.5ft	BLOWS PER FOOT 0 25 50 75 100	SAMP. NO.	MOI	LOG	SOIL AND ROCK DESCRIPTION ELEV. (ft)	DEPTH (ft)
830									826.5 GROUND SURFACE	0.0
823.3	3.2	1	2	3	5	SS-3	M		**ROADWAY EMBANKMENT** RED-BROWN MED STIFF MOIST SILTY SANDY CLAY (A-6)	
818.3	8.2	2	3	8	11		M		817.3 **RESIDUAL** RED-BROWN-ORANGE MED DENSE TO V. DENSE MOIST MICA CLAYEY SILTY SAND (A-1-b)	9.2
813.3	13.2	10	30	25	55	SS-4	D		809.5	17.0
									808.7 **WEATHERED ROCK** SEVERELY WEATHERED BIOTITE GNEISS Boring Terminated by Auger Refusal at Elevation 808.7 ft ON BIOTITE GNEISS	17.8

PROJECT NO. 33524.1	**ID.** B-4177	**COUNTY** LINCOLN	**GEOLOGIST** Stickney, J. K.
SITE DESCRIPTION BRIDGE #142 OVER HOWARDS CREEK ON SR 1193 BETWEEN NC 27 AND SR 1113			**GROUND WTR (ft)**
BORING NO. EB1-B	**STATION** 18+20	**OFFSET** 17ft RT	**ALIGNMENT** -L- **0 HR.** Dry
COLLAR ELEV. 826.5 ft	**TOTAL DEPTH** 10.6 ft	**NORTHING** 646,409	**EASTING** 1,297,445 **24 HR.** FIAD
DRILL MACHINE CME-550X	**DRILL METHOD** H.S. Augers		**HAMMER TYPE** Automatic
START DATE 05/24/07	**COMP. DATE** 05/24/07	**SURFACE WATER DEPTH** N/A	**DEPTH TO ROCK** 10.6 ft

ELEV (ft)	DEPTH (ft)	BLOW COUNT 0.5ft	0.5ft	0.5ft	BLOWS PER FOOT 0 25 50 75 100	SAMP. NO.	MOI	LOG	SOIL AND ROCK DESCRIPTION
830									826.5 GROUND SURFACE 0.0
823.1	3.4	1	2	2	4		M		**ROADWAY EMBANKMENT** RED-BROWN MED STIFF MOIST SILTY SANDY CLAY (A-6)
818.1	8.4	49	51/.2		100/.7		D		818.1 8.4 **WEATHERED ROCK** SEVERELY WEATHERED BIOTITE GNEISS 10.6 Boring Terminated by Auger Refusal at Elevation 815.9 ft ON BIOTITE GNEISS

NCDOT GEOTECHNICAL ENGINEERING UNIT
CORE BORING REPORT

PROJECT NO. 33524.1	**ID.** B-4177	**COUNTY** LINCOLN	**GEOLOGIST** Stickney, J. K.
SITE DESCRIPTION BRIDGE #142 OVER HOWARDS CREEK ON SR 1193 BETWEEN NC 27 AND SR 1113			**GROUND WTR (ft)**
BORING NO. B1-A	**STATION** 18+83	**OFFSET** 11ft LT	**ALIGNMENT** -L- **0 HR.** N/A
COLLAR ELEV. 805.0 ft	**TOTAL DEPTH** 21.2 ft	**NORTHING** 646,464	**EASTING** 1,297,404 **24 HR.** N/A
DRILL MACHINE CME-550X	**DRILL METHOD** Core Boring		**HAMMER TYPE** Automatic
START DATE 05/23/07	**COMP. DATE** 05/23/07	**SURFACE WATER DEPTH** 1.7ft	**DEPTH TO ROCK** 3.2 ft
CORE SIZE CNWL	**TOTAL RUN** 21.2 ft	**DRILLER** Smith, C. L.	

ELEV (ft)	DEPTH (ft)	RUN (ft)	DRILL RATE (Min/ft)	RUN REC. (ft) %	RUN RQD (ft) %	SAMP. NO.	STRATA REC. (ft) %	STRATA RQD (ft) %	LOG	DESCRIPTION AND REMARKS
805										Ground Surface
805.0	0.0	3.2	2:07/3.2	(1.5) 47%	(0.0) 0%		(1.5) 47%	(0.0) 0%		**WEATHERED ROCK** BROWN-GRAY-WHITE SEVERELY WEATHERED BIOTITE GNEISS
801.8	3.2									801.8 3.2
		5.0	2:18/5.0	(3.4) 68%	(1.7) 34%		(3.4) 68%	(1.7) 34%		**CRYSTALLINE ROCK** BROWN-GRAY-WHITE SEVERE TO MODERATLY WEATHERED MOD.-HARD TO HARD BIOTITE GNEISS W/ V. CLOSE TO CLOSE FRACTURE SPACING
796.8	8.2									796.8 8.2
		5.0	6:24/5.0	(5.0) 100%	(5.0) 100%		(13.0) 100%	(13.0) 100%		**CRYSTALLINE ROCK** GRAY-WHITE-BLACK FRESH VERY HARD BIOTITE GNEISS (EACH RUN CONSISTS OF A SOLID CORE PIECE)
791.8	13.2									
		5.0	7:30/5.0	(5.0) 100%	(5.0) 100%	RS-2				
786.8	18.2									
		3.0	4:12/3.0	(3.0) 100%	(3.0) 100%					
783.8	21.2									783.8 21.2
										Boring Terminated at Elevation 783.8 ft IN BIOTITE GNEISS

PROJECT NO. 33524.1	**ID.** B-4177	**COUNTY** LINCOLN	**GEOLOGIST** Stickney, J. K.
SITE DESCRIPTION BRIDGE #142 OVER HOWARDS CREEK ON SR 1193 BETWEEN NC 27 AND SR 1113			**GROUND WTR (ft)**
BORING NO. B1-B	**STATION** 18+87	**OFFSET** 12ft RT	**ALIGNMENT** -L- **0 HR.** N/A
COLLAR ELEV. 805.8 ft	**TOTAL DEPTH** 19.1 ft	**NORTHING** 646,473	**EASTING** 1,297,426 **24 HR.** N/A
DRILL MACHINE CME-550X	**DRILL METHOD** Core Boring		**HAMMER TYPE** Automatic
START DATE 05/23/07	**COMP. DATE** 05/23/07	**SURFACE WATER DEPTH** 0.9ft	**DEPTH TO ROCK** 4.1 ft
CORE SIZE CNWL	**TOTAL RUN** 19.1 ft	**DRILLER** Smith, C. L.	

ELEV (ft)	DEPTH (ft)	RUN (ft)	DRILL RATE (Min/ft)	RUN REC. (ft) %	RUN RQD (ft) %	SAMP. NO.	STRATA REC. (ft) %	STRATA RQD (ft) %	LOG	DESCRIPTION AND REMARKS
805.8										Ground Surface
805.8	0.0	4.1								**ALLUVIAL** GRAVEL AND ROCK FRAGMENTS 804.2 1.6
801.7	4.1									**WEATHERED ROCK** BROWN-GRAY-WHITE SEVERELY WEATHERED BIOTITE GNEISS 801.7 4.1
		5.0	7:29/5.0	(4.5) 90%	(3.5) 70%		(0.7) 64%	(0.0) 0%		**CRYSTALLINE ROCK** SEVERELY TO MODERATELY SEVERELY WEATHERED SOFT TO MED HARD BIOTITE GNEISS 800.6 5.2
796.7	9.1						(13.9) 100%	(13.9) 100%		**CRYSTALLINE ROCK** BLACK-WHITE-GRAY FRESH VERY HARD BIOTITE GNEISS (SOLID CORE PIECES)
		5.0	8:03/5.0	(5.0) 100%	(5.0) 100%					
791.7	14.1					RS-1				
		5.0	7:39/5.0	(5.0) 100%	(5.0) 100%					
786.7	19.1									786.7 19.1
										Boring Terminated at Elevation 786.7 ft IN BIOTITE GNEISS

NCDOT GEOTECHNICAL ENGINEERING UNIT
BORELOG REPORT

PROJECT NO. 33524.1	**ID.** B-4177	**COUNTY** LINCOLN	**GEOLOGIST** Stickney, J. K.	
SITE DESCRIPTION BRIDGE #142 OVER HOWARDS CREEK ON SR 1193 BETWEEN NC 27 AND SR 1113				**GROUND WTR (ft)**
BORING NO. EB2-B	**STATION** 19+50	**OFFSET** 13ft RT	**ALIGNMENT** -L-	**0 HR.** 23.7
COLLAR ELEV. 824.9 ft	**TOTAL DEPTH** 23.8 ft	**NORTHING** 646,534	**EASTING** 1,297,413	**24 HR.** 18.0
DRILL MACHINE CME-550X	**DRILL METHOD** H.S. Augers		**HAMMER TYPE** Automatic	
START DATE 05/23/07	**COMP. DATE** 05/23/07	**SURFACE WATER DEPTH** N/A	**DEPTH TO ROCK** 23.8 ft	

ELEV (ft)	DEPTH (ft)	BLOW COUNT 0.5ft	0.5ft	0.5ft	BLOWS PER FOOT 0 25 50 75 100	SAMP. NO.	MOI	LOG	SOIL AND ROCK DESCRIPTION
									ELEV. (ft) DEPTH (ft)
825									824.9 GROUND SURFACE 0.0
821.7	3.2								**ROADWAY EMBANKMENT** RED-BROWN MED STIFF MOIST CLAYEY SANDY SILT (A-4)
816.7	8.2	2	3	5	8	SS-1	M		
814.3	10.6	2	3	4	7		M		
811.7	13.2								**RESIDUAL** RED-ORANGE-BROWN LOOSE TO MED DENSE MOIST MICA CLAYEY SAND (A-2-5)
806.7	18.2	3	3	3	6	SS-2	M		
804.1	20.8	4	6	4	10		M		
801.7	23.2	100/.4			100/.4				**WEATHERED ROCK** SEVERELY WEATHERED BIOTITE GNEISS
801.1	23.8								Boring Terminated by Auger Refusal at Elevation 801.1 ft ON BIOTITE GNEISS

NORTH CAROLINA DEPARTMENT OF TRANSPORTATION
GEOTECHNICAL ENGINEERING UNIT

FIELD SCOUR REPORT

WBS: 33524 TIP: B-4177 COUNTY: LINCOLN

DESCRIPTION(1): BRIDGE 142 OVER HOWARDS CREEK ON SR 1113

EXISTING BRIDGE

Information from: Field Inspection X Microfilm ____ (reel ____ pos: ____)
Other (explain) ____

Bridge No.: 142 Length: 80 Total Bents: 3 Bents in Channel: 1 Bents in Floodplain: 2
Foundation Type: TIMBER ABUTMENTS, INTERIOR FOOTINGS

EVIDENCE OF SCOUR(2)

Abutments or End Bent Slopes: PREVIOUSLY REPAIRED SLOPE WASHOUT AT END BENT ONE
ERODING AGAIN

Interior Bents: NONE OBSERVED

Channel Bed: NONE OBSERVED

Channel Bank: NONE OBSERVED

EXISTING SCOUR PROTECTION

Type(3): NONE

Extent(4): ____

Effectiveness(5): ____

Obstructions(6): NONE

DESIGN INFORMATION

Channel Bed Material(7): SAND & GRAVEL OVER WEATHERED ROCK

Channel Bank Material(8): SAND

Channel Bank Cover(9): SHRUBS, TREES

Floodplain Width(10): 100'

Floodplain Cover(11): TREES, GRASS

Stream is(12): Aggrading ____ Degrading X Static ____

Channel Migration Tendency(13): MINIMAL, TOWARDS END BENT ONE

Observations and Other Comments: ____

DESIGN SCOUR ELEVATIONS(14) Feet X Meters ____

BENTS

B1	B2	B3	B4							
802										

Comparison of DSE to Hydraulics Unit theoretical scour:
B1 (100 YR) THEORETICAL SCOUR =795, RAISED DUE TO PRESENCE OF WEATHERED ROCK / ROCK

SOIL ANALYSIS RESULTS FROM CHANNEL BED AND BANK MATERIAL

Bed or Bank							
Sample No.							
Retained #4							
Passed #10							
Passed #40							
Passed #200							
Coarse Sand							
Fine Sand							
Silt							
Clay							
LL							
PI							
AASHTO							
Station							
Offset							
Depth							

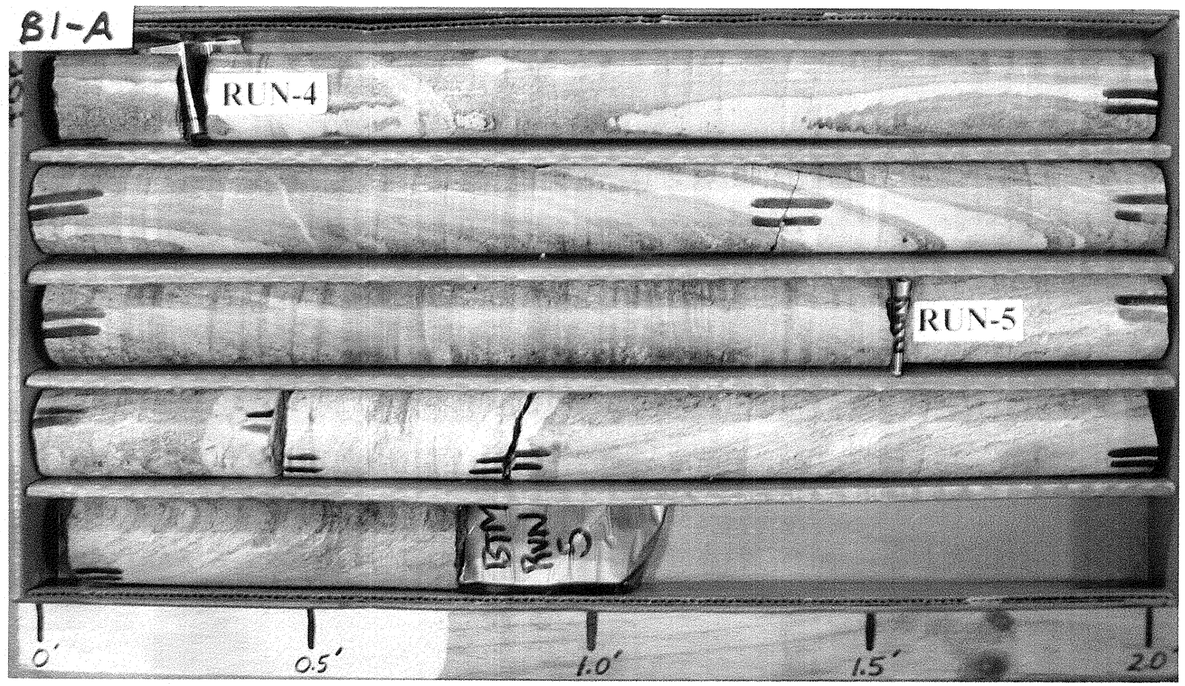
Template Revised 02/07/06

Reported by: C. LITTLE **Date:** 8/22/2007

INSTRUCTIONS

1 Describe the specific site's location, including route number and body of water crossed.
2 Note scour evidence at existing end bents or abutments (e.g. undermining, sloughing, degradations).
3 Note existing scour protection (e.g. rip rap).
4 Describe extent of existing scour protection.
5 Describe whether or not the scour protection appears to be working.
6 Note obstructions such as dams, fallen trees, debris at bents, etc.
7 Describe the channel bed material based on observation and/or samples. Include any lab results with report.
8 Describe the channel bank material based on observation and/or samples. Include any lab results with report.
9 Describe the material covering the banks (e.g. grass, trees, rip rap, none).
10 Determine the approximate floodplain width from field observation or a topographic map.
11 Describe the material covering the floodplain (e.g. grass, trees, crops).
12 Use professional judgement to specify if the stream is degrading, aggrading, or static.
13 Describe potential and direction of the stream to migrate laterally during the bridge's life (approx. 100 years).
14 Give the design scour elevation (DSE) expected over the life of the bridge (approx. 100 years). This elevation can be given as a range across the site, or for each bent. Discuss the relationship between the Hydraulics Unit theoritical scour and the DSE. If the DSE is dependent on scour counter measures, explain (e.g. rip rap armoring on slopes). The DSE is based on the erodability of materials, giving consideration to the influence of joints, foliation, bedding characteristics, % core recovery, % RQD, differential weathering, shear strength, observations at existing structures, other tests deemed appropriate, and overall geologic conditions at the site.

33524.1.1 B-4177
LINCOLN COUNTY
BR 142 OVER HOWARDS
CREEK ON SR 1193 BETWEEN
NC 27 & SR 113
PHOTOS



33524.1.1 B-4177
LINCOLN COUNTY
BR 142 OVER HOWARDS
CREEK ON SR 1193 BETWEEN
NC 27 & SR 113
PHOTOS