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STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION
DIVISION OF HIGHWAYS
GEOTECHNICAL ENGINEERING UNIT

ROADWAY SUBSURFACE INVESTIGATION

PROJ. REFERENCE NO. 33183.1.1

F.A. PROJ. **BRSTP-1505(1)**

COUNTY **CHEROKEE**

PROJECT DESCRIPTION APPROACHES TO BRIDGE NO. 26 OVER

JUNALUSKA CREEK ON SR 1505 (JUNALUSKA ROAD)

INVENTORY

STA. 19+50.00 -L- END TIP PROJECT B-3635

CAUTION NOTICE

THE SUBSURFACE INFORMATION AND THE SUBSURFACE INVESTIGATION ON WHICH IT IS BASED WERE MADE FOR THE PURPOSE OF STUDY, PLANNING, AND DESIGN, AND NOT FOR CONSTRUCTION OR PAY PURPOSES. THE VARIOUS FIELD BORING, LOGS, ROCK CORES, AND SOUL TEST DATA AVAILABLE MAY BE REVIEWED OR INSPECTED IN RALEIGH BY CONTACTING THE N.C. DEPARTMENT OF TRANSPORTATION, GEOTECHNICAL ENCINEERING UNIT 1991 250-4088, NETHER THE SUBSURFACE PLANS AND REPORTS, NOR THE FIELD BORING LOGS, ROCK CORES, OR SOIL TEST DATA ARE PART OF THE CONTRACT.

GENERAL SOIL AND ROCK STRATA DESCRIPTIONS AND INDICATED BOUNDARIES ARE BASED ON A GEOTECHNICAL INTERPRETATION OF ALL AVAILABLE SUBSURFACE DATA AND MAY NOT NECESSARLY REFLECT THE ACTUAL SUBSURFACE CONDITIONS BETWEEN BORNOS OR BETWEEN SAMPLED STRATA ANTIHN THE BORENOLE, THE LABORATORY SAMPLE DATA AND THE IN SITU (8N-PLACE) TEST DATA CAN BE RELIED ON ONLY TO THE DEGREE OF RELABULTY INHERENT IN THE STANDARD TEST METHOD. THE OBSERVED WATER LEVELS OR SOIL MOSTURE CONDITIONS INDICATED IN THE SUBSURFACE INVESTICATIONS ARE AS RECORDED AT THE TIME OF THE INVESTICATION. THESE WATER LEVELS OR SOIL MOOSTURE CONDITIONS AND VARY CONSIDERABLY WITH TIME ACCORDING TO CLIMATIC CONDITIONS INCLUDING TEMPERATURES, PRECIPITATION, AND WIND, AS WELL AS OTHER NON-CLIMATIC FACTORS.

THE BIDDER OR CONTRACTOR IS CAUTIONED THAT DETAILS SHOWN ON THE SUBSURFACE PLANS ARE PRELIMINARY ONLY AND IN MANY CASES THE FINAL DESIGN DETAILS ARE DIFFERENT, FOR BIDDING AND CONSTRUCTION PLANS AND DOSCUMENTS FOR FINAL DESIGN INFORMATION ON THIS PROJECT. THE DEPARTMENT DESIGN INFORMATION ON THIS PROJECT, THE DEPARTMENT DESIGN INFORMATION ON THIS PROJECT, THE DEPARTMENT AS TO THE INVESTIGATION MADE, FOR PRINCIPLE OF THE DEPARTMENT AS TO THE INTERPRETATIONS AS DECEMBER OF THE DEPARTMENT AS TO THE TYPE OF MATERIALS AND CONDITIONS TO BE ENCOUNTERED. THE BIDDER OR CONTRACTOR IS CAUTIONED TO MAKE SUCH INDEPENDENT SUBSURFACE INVESTIGATIONS AS HE DEEMS NECESSART TO SATISFY HINNELF AS TO CONDITIONS TO BE ENCOUNTERED ON THIS PROJECT. THE CONTRACTOR SHALL HAVE NO CLAIM FOR ADDITIONAL COMPENSATION OF FOR AN EXTENSION OF TIME FOR ANY PREASON RESULTING FROM THE ACTUAL CONDITIONS ENCOUNTERED AT THE SITE DIFFERING FROM THOSE INDICATED IN THE SUBSURFACE INFORMATION.

PERSONNEL **D.P. MURPHY**

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INVESTIGATED BY L.L. ACKER

CHECKED BY W.D. FRYE

SUBMITTED BY W.D. FRYE

DATE 3/8/

SE AL John Lohn

DRAWN BY: J.T. WILLIAMS

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NOTE - BY HAVING REDUESTED THIS INFORMATION THE CONTRACTOR SPECIFICALLY WAIVES ANY CLAIMS FOR INCREASED COMPENSATION OR EXTENSION OF TIME BASED ON DIFFERENCES BETWEEN THE CONDITIONS INDICATED HEREIN AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

DIVISION OF HIGHWAYS

GEOTECHNICAL ENGINEERING UNIT

SUBSURFACE INVESTIGATION

SOIL AND ROCK LEGEND, TERMS, SYMBOLS, AND ABBREVIATIONS SOIL DESCRIPTION ROCK DESCRIPTION
ROCK IS NON-COASTAL PLAIN MATERIAL THAT IF TESTED, WOULD YIELD SPT REFUSAL, AN INFERRED TERMS AND DEFINITIONS <u>WELL GRADED</u> - INDICATES A GOOD REPRESENTATION OF PARTICLE SIZES FROM FINE TO COARSE. UNIFORM - INDICATES THAT SOIL PARTICLES ARE ALL APPROXIMATELY THE SAME SIZE, (ALSO SOIL IS CONSIDERED TO BE THE UNCONSOLIDATED, SEMI-CONSOLIDATED, OR WEATHERED EARTH MATERIALS THAT CAN BE PENETRATED WITH A CONTINUOUS FLIGHT POWER AUGER, AND YIELD LESS THAN 108 BLOWS PER FOOT ACCORDING TO STANDARD PENETRATION TEST (AASHTO 1206, ASTM D-1586). SOIL CLASSIFICATION IS BASED ON THE AASHTO SYSTEM, BASIC DESCRIPTIONS GENERALLY SHALL INCLUDE: ALLUVIUM (ALLUV.) - SOILS THAT HAVE BEEN TRANSPORTED BY WATER. ROCK LINE INDICATES THE LEVEL AT WHICH NON-COASTAL PLAIN MATERIAL WOULD VIELD SYT REFUSAL, AN INFERRED
SPT REFUSAL IS PENETRATION BY A SPLIT SPOON SAMPLER EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS.
IN NON-COASTAL PLAIN MATERIAL, THE TRANSITION BETWEEN SOIL AND ROCK IS OFTEN REPRESENTED BY A ZONE AQUIFER - A WATER BEARING FORMATION OR STRATA GAP-GRADED - INDICATES A MIXTURE OF UNIFORM PARTICLES OF TWO OR MORE SIZES. ANGULARITY OF GRAINS ARENACEOUS - APPLIED TO ROCKS THAT HAVE BEEN DERIVED FROM SAND OR THAT CONTAIN SAND. CONSISTENCY, COLOR, TEXTURE, MOISTURE, AASHTO CLASSIFICATION, AND OTHER PERTINENT FACTORS SUCH ROCK MATERIALS ARE TYPICALLY DIVIDED AS FOLLOWS: ARGILLACEOUS - APPLIED TO ALL ROCKS OR SUBSTANCES COMPOSED OF CLAY MINERALS, AS MINERALOGICAL COMPOSITION, ANGULARITY, STRUCTURE, PLASTICITY, ETC. EXAMPLE: THE ANGULARITY OR ROUNDNESS OF SOIL GRAINS IS DESIGNATED BY THE TERMS: ANGULAR, NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT N VALUES > 100 OR HAVING A NOTABLE PROPORTION OF CLAY IN THEIR COMPOSITION, AS SHALE, SLATE, ETC. SUBANGULAR, SUBROUNDED, OR ROUNDED. VERY STIFF, GRAY, SULTY CLAY, MOIST WITH INTERBEDDED FINE SAND LAYERS, HIGHLY PLASTIC, A-7-6 ARTESIAN - GROUND WATER THAT IS UNDER SUFFICIENT PRESSURE TO RISE ABOVE THE LEVEL BLOWS PER FOOT IF TESTED. MINERALOGICAL COMPOSITION SOIL LEGEND AND AASHTO CLASSIFICATION T WHICH IT IS ENCOUNTERED, BUT WHICH DOES NOT NECESSARILY RISE TO OR ABOVE THE FINE TO COARSE GRAIN IGNEOUS AND METAMORPHIC ROCK THA CRYSTALLINE ROCK (CR) MINERAL NAMES SUCH AS QUARTZ, FELDSPAR, MICA, TALC, KAOLIN, ETC. ARE USED IN DESCRIPTIONS WOULD YIELD SPT REFUSAL IF TESTED, ROCK TYPE INCLUDES GRANITE, CNEISS, GABBRO, SCHIST, ETC. GROUND SURFACE. GENERAL ORGANIC MATERIALS WHENEVER THEY ARE CONSIDERED OF SIGNIFICANCE. CLASS. CALCAREOUS (CALC.) - SOILS THAT CONTAIN APPRECIABLE AMOUNTS OF CALCIUM CARBONATE. FINE TO COARSE GRAIN METAMORPHIC AND NON-COASTAL PLAIN A-4 A-5 A-6 A-7 A-1, A-2 A-4, A-5 A-6, A-7 NON-CRYSTALLINE ROCK (NCR) GROUP A-1 A-3 SEDIMENTARY ROCK THAT WOULD YELD SPT REFUSAL IT TESTED. ROCK TYPE INCLUDES PHYLLITE, SLATE, SANDSTONE, ETC.

COASTAL PLAIN SEDIMENTS CEMENTED INTO ROCK, BUT MAY NOT YIELD COLLUVIUM - ROCK FRAGMENTS MIXED WITH SOIL DEPOSITED BY GRAVITY ON SLOPE OR AT BOTTOM CLASS. 1-a A-1-t SLIGHTLY COMPRESSIBLE LIQUID LIMIT LESS THAN 31 MODERATELY COMPRESSIBLE HIGHLY COMPRESSIBLE COASTAL PLAIN SEDIMENTARY ROCK <u>CORE RECOVERY (REC.)</u> - TOTAL LENGTH OF ALL MATERIAL RECOVERED IN THE CORE BARREL DIVIDED BY TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE. SYMBOL LIQUID LIMIT GREATER THAN 50 SPT REFUSAL. ROCK TYPE INCLUDES LIMESTONE, SANDSTONE, CEMENTED SHELL BEDS, ETC. PASSING PERCENTAGE OF MATERIAL WEATHERING DIKE - A TABULAR BODY OF IGNEOUS ROCK THAT CUTS ACROSS THE STRUCTURE OF ADJACENT # 10 # 40 ORGANIC MATERIAL PEAT OTHER MATERIAL SOILS SOILS SOILS SOILS ROCK FRESH, CRYSTALS BRIGHT, FEW JOINTS MAY SHOW SLIGHT STAINING, ROCK RINGS UNDER DIP - THE ANGLE AT WHICH A STRATUM OR ANY PLANAR FEATURE IS INCLINED FROM THE * 200 TRACE OF ORGANIC MATTER 1 - 10% 5 - 12% LITTLE 10 - 20% TOUR LINE ROCK GENERALLY FRESH, JOINTS STAINED, SOME JOINTS MAY SHOW THIN CLAY COATINGS IF OPEN, SOUS WITH MODERATELY ORGANIC 5 - 10% 12 - 20% SOME DIP DIRECTION (DIP AZIMUTH) - THE DIRECTION OR BEARING OF THE HORIZONTAL TRACE OF LASTIC INDEX √P |10 MX |10 MX |11 MN |11 MN |10 MX |10 MX |11 MN |11 MN LITTLE OR HIGHLY ORGANIC HIGHLY V SLI.) CRYSTALS ON A BROKEN SPECIMEN FACE SHINE BRIGHTLY, ROCK RINGS UNDER HAMMER BLOWS IF 35% AND ABOVE THE LINE OF DIP MEASURED CLOCKWISE FROM NORTH. OF A CRYSTALLINE NATURE. GROUP INDEX 8 MX 12 MX 16 MX No MODERATE 4 MX GROUND WATER FAULT - A FRACTURE OR FRACTURE ZONE ALONG WHICH THERE HAS BEEN DISPLACEMENT OF THE USUAL TYPES STONE FRAGS. FINE SILTY OR CLATE:
OF MAJOR GRAVEL, AND SAND GRAVEL AND SAND ROCK GENERALLY FRESH, JOINTS STAINED AND DISCOLORATION EXTENDS INTO ROCK UP TO 1 INCH. DPEN JOINTS MAY CONTAIN CLAY. IN GRANITOID ROCKS SOME OCCASIONAL FELDSPAR AMOUNTS OF SOILS SIDES RELATIVE TO DNE ANOTHER PARALLEL TO THE FRACTURE. WATER LEVEL IN BORE HOLE IMMEDIATELY AFTER DRILLING SILTY CLAYEY (SLI.) FISSILE - A PROPERTY OF SPLITTING ALONG CLOSELY SPACED PARALLEL PLANES. MATTER **Y**_ CRYSTALS ARE DULL AND DISCOLORED, CRYSTALLINE ROCKS RING UNDER HAMMER BLOWS. STATIC WATER LEVEL AFTER 24 HOURS MODERATE SIGNIFICANT PORTIONS OF ROCK SHOW DISCOLORATION AND WEATHERING EFFECTS. IN $\underline{\mathsf{FLOAT}}$ - ROCK FRAGMENTS ON SURFACE NEAR THEIR ORIGINAL POSITION AND DISLODGED FROM PARENT MATERIAL. GEN. RATIN ∇_{PW} RANITOID ROCKS, MOST FELDSPARS ARE DULL AND DISCOLORED, SOME SHOW CLAY. ROCK HAS FAIR TO PERCHED WATER, SATURATED ZONE, OR WATER BEARING STRATA (MOD.) AS A EXCELLENT TO GOOD FAIR TO POOR POOR NSUITABL DULL SOUND UNDER HAMMER BLOWS AND SHOWS SIGNIFICANT LOSS OF STRENGTH AS COMPARED POOR FLOOD PLAIN (FP) - LAND BORDERING A STREAM, BUILT OF SEDIMENTS DEPOSITED BY THE STREAM. OM-WITH FRESH BOCK. SPRING OR SEEP PI OF A-7-5 SUBGROUP IS ≤ LL - 30 : PI OF A-7-6 SUBGROUP IS > LL - 30 MODERATEL ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. IN GRANITOID ROCKS, ALL FELDSPARS DULL MISCELLANEOUS SYMBOLS CONSISTENCY OR DENSENESS FORMATION (FM.) - A MAPPABLE GEOLOGIC UNIT THAT CAN BE RECOGNIZED AND TRACED IN AND DISCOLORED AND A MAJORITY SHOW KAOLINIZATION, ROCK SHOWS SEVERE LOSS OF STRENGTH BANGE OF UNCONFINED (MDD, SEV.) AND CAN BE EXCAVATED WITH A GEOLOGIST'S PICK. ROCK GIVES 'CLUNK' SOUND WHEN STRUCK, COMPACTNESS OR CONSISTENCY ROADWAY EMBANKMENT SPT CPT
DPT DNT TEST BORING PRIMARY SOIL TYPE COMPRESSIVE STRENGTH (TONS/FT²) IF TESTED, WOULD YIELD SPT REFUSAL JOINT - FRACTURE IN ROCK ALONG WHICH NO APPRECIABLE MOVEMENT HAS OCCURRED. WITH SOIL DESCRIPTION (N-VALUE) DESIGNATIONS ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED ROCK FABRIC CLEAR AND EVIDENT BUT REDUCED SEVERE $\underline{\texttt{LEDGE}}$ - A SHELF-LIKE RIDGE OR PROJECTION OF ROCK WHOSE THICKNESS IS SMALL COMPARED TO ITS LATERAL EXTENT. \oplus IN STRENGTH TO STRONG SOIL. IN GRANITOID ROCKS ALL FELDSPARS ARE KAOLINIZED TO SOME S - BULK SAMPLE EXTENT. SOME FRAGMENTS OF STRONG ROCK USUALLY REMAIN. 4 TO 10 LOOSE GRANUL AR MEDIUM DENSE N/A LENS - A BODY OF SOIL OR ROCK THAT THINS OUT IN ONE OR MORE DIRECTIONS. 10 TO 30 ARTIFICIAL FILL OTHER THAN SS - SPLIT SPOON IF TESTED, YIELDS SPT N VALUES > 100 BPF DENSE 30 TO 50 CORE BORING ROADWAY EMBANKMENT MOTTLED (MOT.) - IRREGULARLY MARKED WITH SPOTS OF DIFFERENT COLORS. MOTTLING IN (NON-COHESIVE) SAMPLE VERY SEVERE ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED, ROCK FARRIC FLEMENTS ARE DISCERNIBLE BUT VERY DENSE ST - SHELBY TUBE THE MASS IS EFFECTIVELY REDUCED TO SOIL STATUS, WITH ONLY FRAGMENTS OF STRONG ROCK SOILS USUALLY INDICATES POOR AERATION AND LACK OF GOOD DRAINAGE. SAMPLE REMAINING, SAPROLITE IS AN EXAMPLE OF ROCK WEATHERED TO A DEGREE SUCH THAT DNLY MINDE PERCHED WATER - WATER MAINTAINED ABOVE THE NORMAL GROUND WATER LEVEL BY THE PRESENCE OF AN <0.25 MONITORING WELL 2 TO 4 4 TO 8 SOFT VESTIGES OF THE ORIGINAL ROCK FABRIC REMAIN. IF TESTED, YIELDS SPT N VALUES < 180 BPF TERVENING IMPERVIOUS STRATUM. GENERALL' 0.25 TO 0.50 TITE INFERRED ROCK LINE RS - ROCK SAMPLE MEDIUM STIFF PIEZOMETER SILT-CLAY 0.5 TO 1.0 ROCK REDUCED TO SOIL. ROCK FABRIC NOT DISCERNIBLE, OR DISCERNIBLE ONLY IN SMALL AND RESIDUAL SOIL - SOIL FORMED IN PLACE BY THE WEATHERING OF ROCK. INSTALLATION 1 TO 2 2 TO 4 MATERIAL STIFF 8 TO 15 RT - RECOMPACTED SCATTERED CONCENTRATIONS, QUARTZ MAY BE PRESENT AS DIKES OR STRINGERS, SAPROLITE IS VERY STIFF ROCK QUALITY DESIGNATION (RQD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF CORE RUN AND 15 TO 30 TRIAXIAL SAMPLE SLOPE INDICATOR \bigcirc DIP & DIP DIRECTION OF ROCK STRUCTURES CBR - CALIFORNIA BEARING RATIO SAMPLE ROCK HARDNESS EXPRESSED AS A PERCENTAGE. TEXTURE OR GRAIN SIZ 0-SPT N-VALUE SAPROLITE (SAP.) - RESIDUAL SOIL THAT RETAINS THE RELIC STRUCTURE OR FABRIC OF THE CANNOT BE SCRATCHED BY KNIFE OR SHARP PICK. BREAKING OF HAND SPECIMENS REQUIRES SOUNDING ROD U.S. STD. SIEVE SIZE REF-SPT_REFUSAL SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK. 0.42 0.075 SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY, HARD HAMMER BLOWS REQUIRED ABBREVIATIONS RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, THAT HAS BEEN EMPLACED PARALLEL COARSE TO DETACH HAND SPECIMEN. TO THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS. COBBLE (COB.) - AUGER REFUSAL SLICKENSIDE - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT OR SLIP PLANE. CLAY HI. - HIGHLY V - VERY CAN BE SCRATCHED BY KNIFE OR PICK. GOUGES OR GROOVES TO 0.25 INCHES DEEP CAN BE (BLDR.) (GRJ) (SL.) BT - BORING TERMINATED MED. - MEDIUM VST - VANE SHEAR TEST CL. - CLAY 7 - UNIT WEIGHT EXCAVATED BY HARD BLOW OF A GEOLOGIST'S PICK, HAND SPECIMENS CAN BE DETACHED 2.0 0.25 0.05 0.005 7 - DRY UNIT WEIGHT CPT - CONE PENETRATION TEST MOD. - MODERATELY STANDARD PENETRATION TEST (PENETRATION RESISTANCE)(SPT) - NUMBER OF BLOWS (N OR BPF) OF A 140 LB. HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF 1 FOOT INTO SOIL WITH CAN BE GROOVED OR GOUGED 0.05 INCHES DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT - DILATOMETER TEST CAN BE EXCAVATED IN SMALL CHIPS TO PEICES 1 INCH MAXIMUM SIZE BY HARD BLOWS OF THE PMT - PRESSUREMETER TEST SOIL MOIS JRE - CORRELA A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER, SPT REFUSAL IS PENETRATION EQUAL TO OR LESS SAP. - SAPROLITIC - DYNAMIC PENETRATION TEST POINT OF A GEOLOGIST'S PICK. SOIL MOISTURE SCALE GUIDE FOR FIELD MOISTURE DESCRIPTION CAN BE GROVED OR GOLIGED READLY BY KNIFE OR PICK. CAN BE EXCAVATED IN FRAGMENTS e - VOID RATIO SD. - SAND, SANDY SOFT (ATTERBERG LIMITS) DESCRIPTION STRATA CORE RECOVERY (SREC.) - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY TOTAL LENGTH OF STRATUM AND EXPRESSED AS A PERCENTAGE. FROM CHIPS TO SEVERAL INCHES IN SIZE BY MODERATE BLOWS OF A PICK POINT, SMALL, THIN SL. - SILT, SILTY FOSS. - FOSSILIFEROUS SLI, - SLIGHTLY PIECES CAN BE BROKEN BY FINGER PRESSURE. - SATURATED USUALLY LIQUID; VERY WET, USUALLY FRAC. - FRACTURED TCR - TRICONE REFUSAL STRATA ROCK DUALITY DESIGNATION (SROD) - A MEASURE OF ROCK DUALITY DESCRIBED BY CAN BE CARVED WITH KNIFE, CAN BE EXCAVATED READILY WITH POINT OF PICK, PIECES 1 INCH (SAT.) FROM BELOW THE GROUND WATER TABLE LIQUID LIMIT FRAGS. - FRAGMENTS W - MOISTURE CONTENT OTAL LENGTH OF ROCK SEGMENTS WITHIN A STRATUM EQUAL TO OR GREATER THAN 4 INCHES DIVIDED OR MORE IN THICKNESS CAN BE BROKEN BY FINGER PRESSURE. CAN BE SCRATCHED READILY BY BY THE TOTAL LENGTH OF STRATA AND EXPRESSED AS A PERCENTAGE. PLASTIC FINGERNATI. SEMISOLID: REQUIRES DRYING TO RANGE TOPSOIL (TS.) - SURFACE SOILS USUALLY CONTAINING ORGANIC MATTER. - WET - (W) EQUIPMENT USED ON SUBJECT PROJECT FRACTURE SPACING ATTAIN OPTIMUM MOISTURE (PI) PLASTIC LIMIT TERM THICKNESS TERM SPACING BENCH MARK: XXX ADVANCING TOOLS DRILL UNITS: VERY THICKLY BEDDED > 4 FEET VERY WIDE MORE THAN 10 FEET X AUTOMATIC MANUAL 1.5 - 4 FEET 0.16 - 1.5 FEET XXX OPTIMUM MOISTURE - MOIST - (M) SOLID: AT OR NEAR OPTIMUM MOISTURE CLAY BITS XXX ELEVATION: XXX SHRINKAGE LIMIT MOBILE B-THINLY BEDDED MODERATELY CLOSE 1 TO 3 FEET 0.03 - 0.16 FEET VERY THINLY BEDDED 6 CONTINUOUS FLIGHT AUGER REQUIRES ADDITIONAL WATER TO NOTES: - DRY - (D) THICKLY LAMINATED 0.008 - 0.03 FEET BK-51 ATTAIN OPTIMUM MOISTURE VERY CLOSE LESS THAN 0.16 FEET 8" HOLLOW AUGERS П-в___ THINLY LAMINATED < 0.008 FFFT PLASTICI HARD FACED FINGER BITS CME-45C X -N____ FOR SEDIMENTARY ROCKS, INDURATION IS THE HARDENING OF THE MATERIAL BY CEMENTING, HEAT, PRESSURE, ETC. PLASTICITY INDEX (PI) DRY STRENGTH TUNG.-CARBIDE INSERTS ___н__ X CME-550 RUBBING WITH FINGER FREES NUMEROUS GRAINS X CASING X W/ ADVANCER LOW PLASTICITY 6-15 SLIGHT HAND TOOLS: GENTLE BLOW BY HAMMER DISINTEGRATES SAMPLE. MED, PLASTICITY MEDIUM PORTABLE HOIST X TRICONE GRAINS CAN BE SEPARATED FROM SAMPLE WITH STEEL PROBE; *STEEL TEETH HIGH PLASTICITY POST HOLE DIGGER MODERATELY INDURATED 26 OR MORE BREAKS EASILY WHEN HIT WITH HAMMER. TRICONE HAND AUGER COLOR OTHER GRAINS ARE DIFFICULT TO SEPARATE WITH STEEL PROBE; SOUNDING ROD X CORE BIT DESCRIPTIONS MAY INCLUDE COLOR OR COLOR COMBINATIONS (TAN. RED. YELLOW-RROWN, BLUE-GRAY). DIFFICULT TO BREAK WITH HAMMER. VANE SHEAR TEST OTHER MODIFIERS SUCH AS LIGHT, DARK, STREAKED, ETC. ARE USED TO DESCRIBE APPEARANCE. SHARP HAMMER BLOWS REQUIRED TO BREAK SAMPLE; SAMPLE BREAKS ACROSS GRAINS. EXTREMELY INDURATED OTHER

PROJECT REFERENCE NO.

B-3635

SHEET NO.

2 of 8



STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

MICHAEL F. EASLEY GOVERNOR

LYNDO TIPPETT SECRETARY

March 6, 2006

STATE PROJECT:

33183.1.1

TIP NO:

B-3635

COUNTY:

Cherokee

DESCRIPTION:

Approaches to Bridge No. 26 on SR 1505 over Junaluska Creek

SUBJECT:

Geotechnical Report – Inventory

Site Description

This project is located in eastern Cherokee County, approximately 2 miles east of the city of Andrews. Junaluska Creek is a major tributary in the headwaters of Valley River. It is a bold, mountain stream in a channel 20 to 30 feet wide, with a shallow bed of gravel and boulders and intermittent exposures of bedrock. SR 1505 closely follows the creek on a sinuous track below steep, forested slopes. Small house sites lie close beside the road or on excavated benches low on the slopes.

Plans call for construction of a replacement bridge and its approaches on new alignment -L- upstream of the existing bridge. Construction is to begin at Station 14+20 and end at Station 20+00. Construction activities and their approximate locations on -L- are as follows:

Station 14+20 to 17+00, Left Side – fill of 3 feet or less

Station 15+50 to 17+00, Right Side – cut 10 feet or less, cut slope 20 feet high

Station 17+10 to 17+55 – bridge

Station 1755 to 20+00 – cuts of 5 feet or less, fills 2 feet or less

The Geotechnical Engineering Unit conducted a subsurface investigation in February, 2004, on an alignment now abandoned in favor of the present -L-. Four borings were

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LOCATION: CENTURY CENTER COMPLEX

BUILDING B 1020 BIRCH RIDGE DRIVE RALEIGH NC 27610

made using a CME 45 power drilling machine. The borings were advanced through surficial materials with an NX casing advancer, and Standard Penetration Tests were performed at regular intervals. Soil samples were submitted to a DOT laboratory for quality testing. Rock was cored at two borings with NXWL coring equipment. Subsurface information was supplemented with observations of rock outcrops and soil cuts at the site.

Items of Special Geotechnical Interest

Hard Rock in Cut

Hard rock will be encountered in a proposed cut on the Right Side between Stations 15+50 and 17+00.

Acid Rock

The bedrock at this site is part of the Nantahala Formation, a rock unit known to be acidic. The lithology at the site is dark gray, mica phyllite containing graphite and abundant visible iron pyrite. Tests of Net Neutralization Potential (NNP) have confirmed the acidic nature of the lithology at this site.

Soil and Rock Materials

Alluvial floodplain gravel, alluvial terrace sediments, colluvium, and hard rock are the principal materials encountered on this project. Small amounts of saprolite and weathered rock are present also.

Alluvial sediment on the floodplain consists of loose, coarse, sandy gravel, cobbles, and boulders in a bed approximately 4 to 8 feet thick overlying weathered or fresh rock.

Alluvial terrace sediments are found on a natural bench 10 feet to more than 20 feet above the floodplain, on the Right Side, back station from the stream crossing. The terrace sediments comprise deeply weathered soils in two layers – an upper, soft, red, sandy clay (A-7-6) with a liquid limit of 44 and a plastic index of 19; and a basal, loose, yellow, silty, sandy gravel (A-1-b).

A very thin layer of hard, silty saprolite (A-4) was found below the terrace gravel.

Colluvial soil lies along the base of the slope on the Left Side of the existing road up station from the stream crossing, where it will be encountered in cuts from there to near the end of the project. The colluvium is a very loose, very micaceous mixture of platy rock fragments in all sizes from silt and sand to cobbles and small boulders.

The bedrock lithology is dark gray, mica phyllite containing graphite and pyrite. The fresh rock is only moderately hard due to its very high mica content. A very well developed platy cleavage, which dips moderately to the east, allows the rock to break

easily into platy or tabular pieces. Visual inspection of the rock indicates a high concentration of iron sulfide, an indicator of acid rock. A composite sample from one of the core borings has been submitted for tests of Net Neutralization Potential, and the acidic nature of the rock has been confirmed.

Geotechnical Descriptive Analysis

Stations 14+20 to 17+10

A cut is to be constructed on the Right Side of this segment. The maximum depth of cut at the ditch line will be approximately 10 feet, and the exposed cut face will be about 20 feet high, depending on the angle of the cut. Fresh rock crops out on the natural slope in the cut area. A boring on the Right Side of Station 16+77 found fresh rock at a depth of 11.9 feet, with a thin cap of hard saprolite and weathered rock, overlain by 10 feet of weathered alluvial terrace sediments. The terrace sediments comprise about 5 feet of soft, sandy clay (A-7-6) and 5 feet of very silty, coarse sand and gravel (A-1-b). A cut at this site will encounter the alluvial terrace soils in the upper 5 to 10 feet overlying hard rock and weathered rock. The rock will be acidic.

Stations 17+10 to 17+55

The replacement bridge is to be constructed in this interval.

Station 17+55 to 18+00

Plans call for a few feet of embankment to be placed over the floodplain in this segment. Borings on the floodplain indicate 5 to 8 feet of coarse, bouldery alluvium overlies weathered to fresh rock.

Station 18+00 to 20+00

A cut of 5 feet or less on the Left Side is called for in this segment of the project. Soil material at the site was inspected in a low cut beside the existing ditchline. It consisted of colluvium composed of loose, platy rock fragments in a very micaceous, sandy matrix.

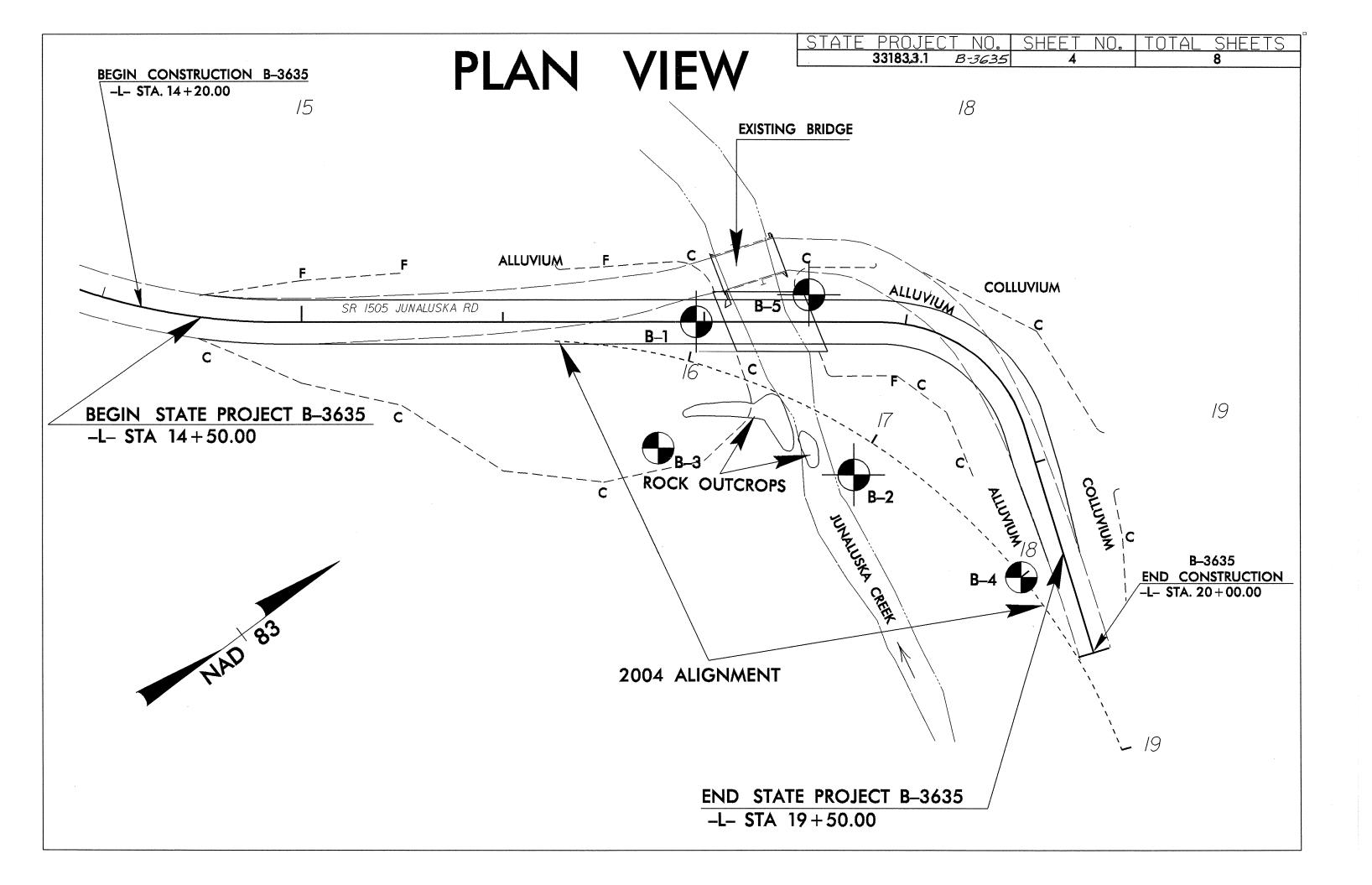
Respectfully submitted,

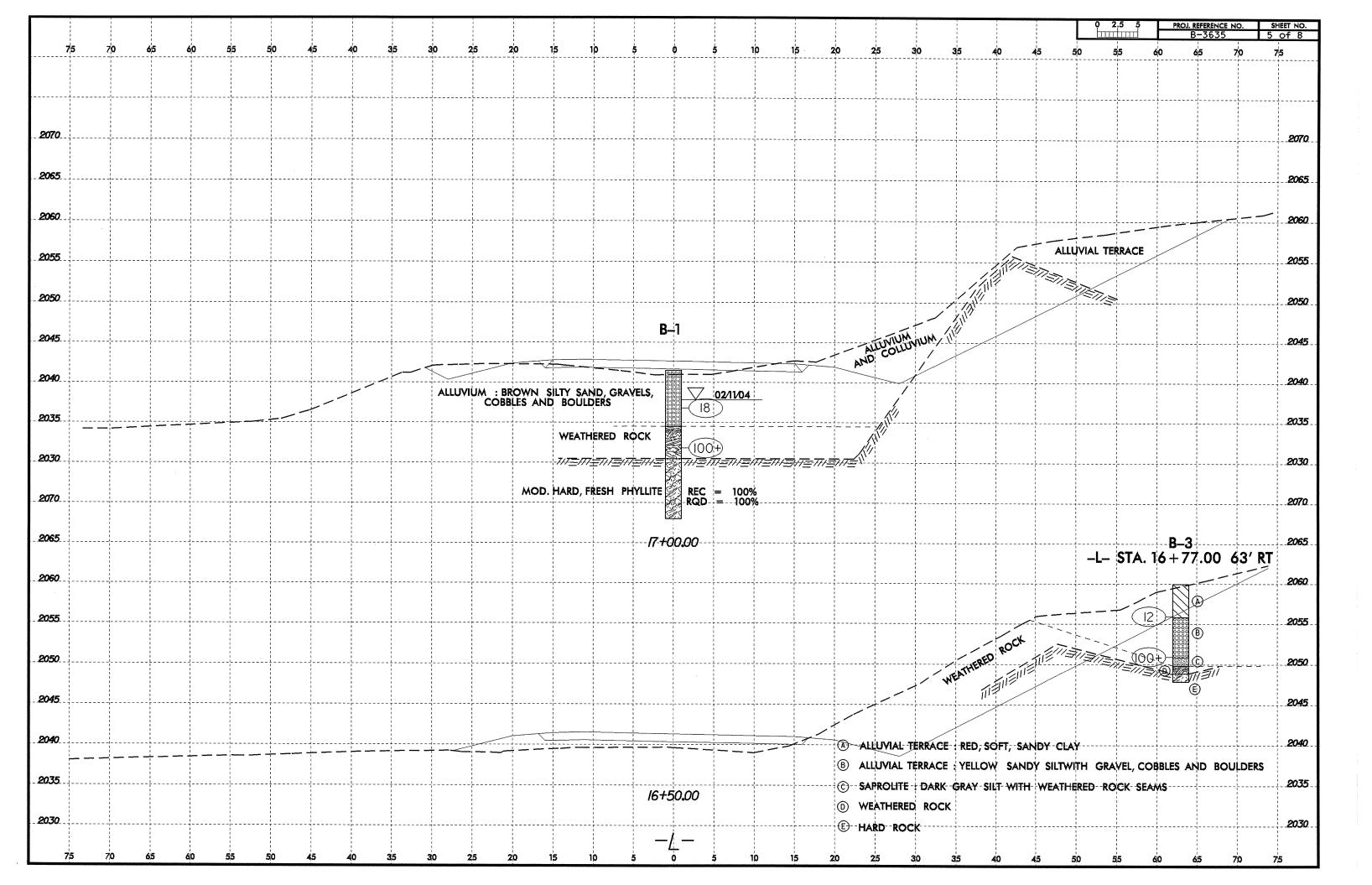
Louis L. Acker, LG

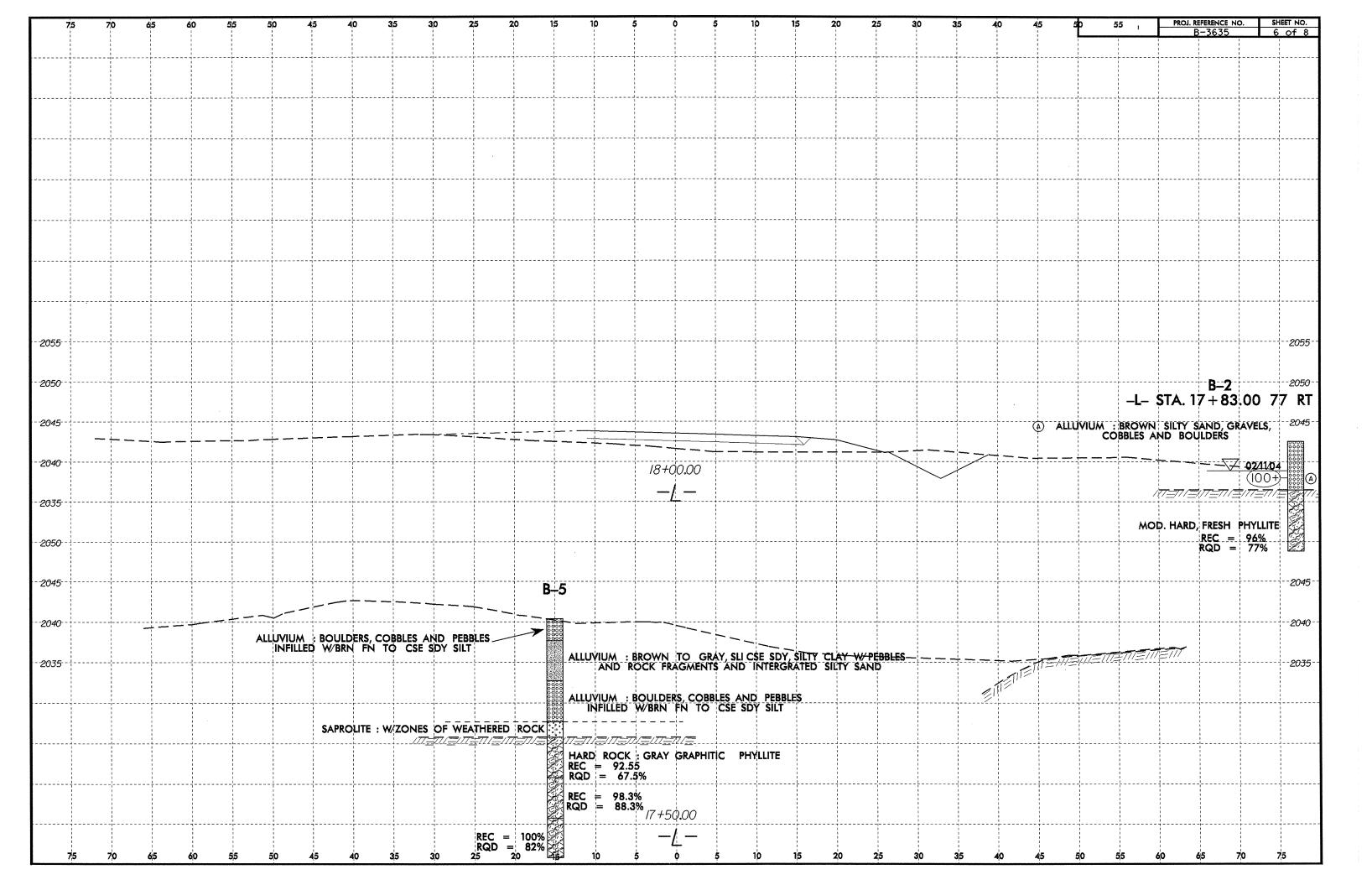
EARTHWORK BALANCE SHEET

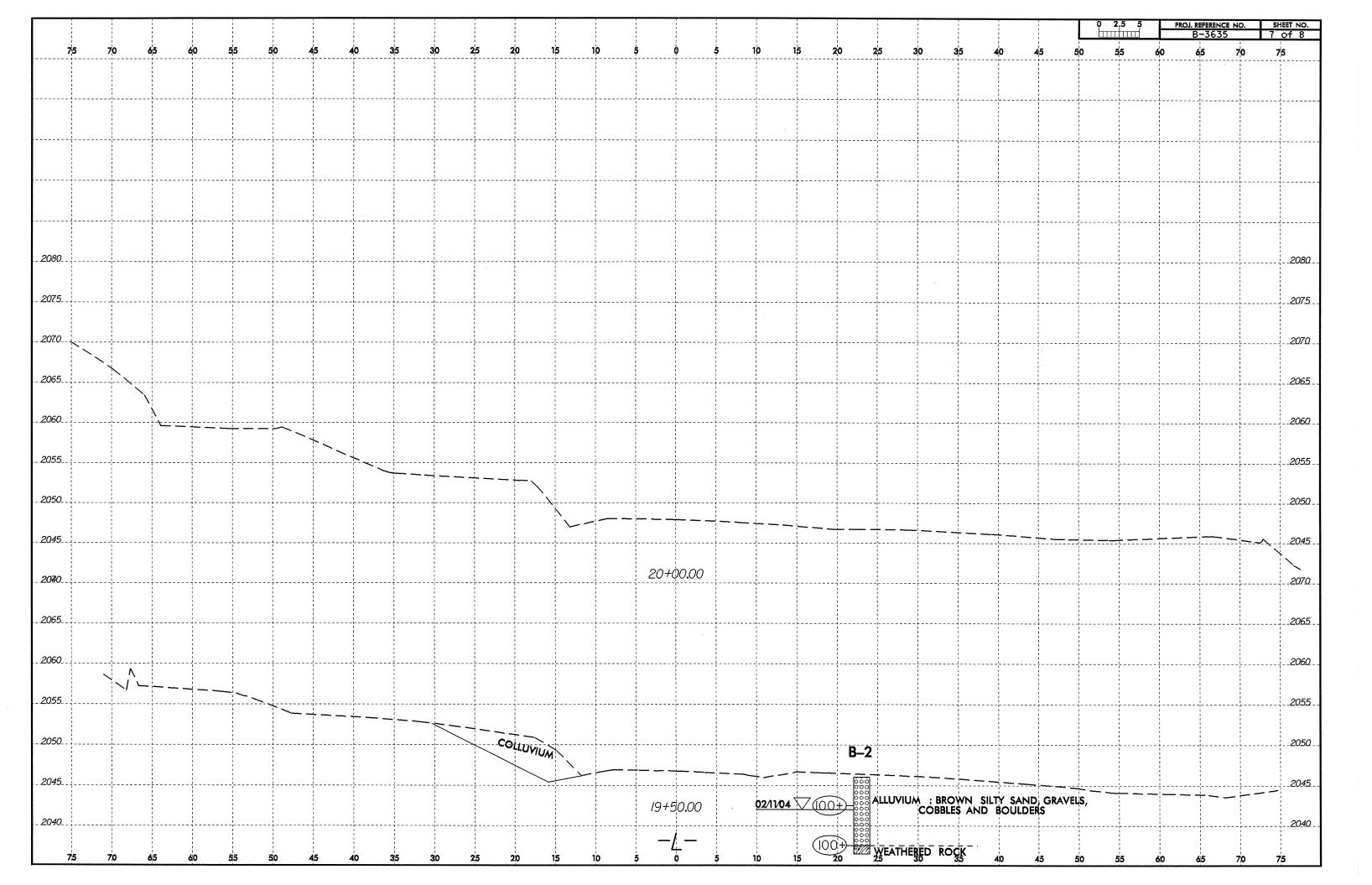
IN CUBIC YARDS

TIP NO.	B-3635						DATE:		19-Jul-05				······································					
COUNTY: CALCULATED BY: CHECKED BY:	CHEROKEE DWG PJS 2/21/08				•		SHRINKAGI	E FACTOR:	15%			IN CUBIC YA	ARDS					
LINE	STATION	то	STATION	TOTAL EXCAV. (UNCL)	ROCK EXCAV.	UNDERCUT EXCAV.	UNSUIT EXCAV.	SUITABLE EXCAV.	TOTAL EMBANK.		UNDERCUT EXCAV.	EARTH EMBANK.	EMBANK. +15%	BORROW	ROCK WASTE	SUIT. WASTE	UNSUIT. WASTE	TOTAL WASTE
B-3635				(ONOL)	LXOAV.													
SUMMARY NO. 1 -L-	14+50.00	+	17+10.00 (BB)	534	37			497	297		1	297	342		37	155	37	192
TOTAL SUMMARY NO.	1			534	37			497	297			297	342		37	155	37	192
				334	37			437	231			291	342		31	100	31	192
SUMMARY NO. 2 -L-	17+55.00 (EB	+	20+00.00	224				224	133		1	133	153			71		71
TOTAL SUMMARY NO.	. <u>2</u>	+		224				224	133			133	153			71		71
		-									<u> </u>				•			
													=======================================					
SUMMARY TOTALS		\vdash		758	37			721	430		-	430	495		37	226	37	263
LOSS DUE TO C & G				-100				-100								-100		-100
SUB-TOTALS				658	37			621	430			430	495	-	37	126	37	163
Rock Excavation x1.25		\vdash													9		9	9
Rock Excavation x1.15															7		7	7
PROJECT B-3635 GRA	ND TOTAL			658	37			621	430			430	495	-	53	126	53	179
SAY				700														
DDE = 40 CY																		
UNDERCUT EXCAVAT	TON = 500 CY LASS IV = 600 T	ON	•															
SELECT GRANULAR M	ATERIAL = 500	CY																
FABRIC FOR SOIL STA	ABILIZATION = 5	00 51	<u> </u>															
		-									-							
																		•
		\vdash				**************************************											<u> </u>	
		\vdash				······································	 				 							
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M&T 503E

JJL NORTH CAROLINA DEPARTMENT OF TRANSPORTATION DIVISION OF HIGHWAYS-MATERIALS AND TESTS UNIT SOILS TEST REPORT-SOILS LABORATORY

PROJECT:		COUN	ΓY: Ch	erokee		Owner:			***************************************
DATE SAMPLED:	2-19-04	DATE REC		2-20-04		DATE REI	PORTED:	3-5-04	<u> </u>
SAMPLED FROM:	Rdwy -L-	 		ED BY:	L L Ac			1	
UBMITTED BY:	W D Frye				2002		DARD SPE	CIFICA	TION
ABORATORY:	Asheville				L				
							· · · · · · · · · · · · · · · · · · ·		
		TH	EST RES	SULTS				•	
roject Sample No.	S-2				<u> </u>	1			
ab Sample No.	A-144490		<u>-</u>		_				
liCAMS Sample #									
Retained #4 Sieve %									
assing #10 Sieve %	99		•						
assing #40 Sieve %	96								
assing #200 Sieve %	79			,					
		MINU	S #10 F	RACTIO	N				
oil Mortar - 100%	Т		T		T				
oarse Sand -Ret. #60	6								
ine Sand - Ret. #270	20				-	l			
ilt 0.05-0.005 mm %	22								
lay < 0.005 mm %	52				_	 			
assing # 40 Sieve %									
assing # 200 Sieve %									
		L				<u>-</u>			
iquid Limit	44			····			· 1	· · · · · · · · · · · · · · · · · · ·	
lastic Index	19	·				· · · · · · · · · · · · · · · · · · ·		· ·	
ASHTO Classification	A-7-6 (12)								
Quantity	11,0(12)				_				
exture									
tation	16+00 Rt								
lole No.	10.001				_				
epth (ft) From:	1.0				- 				
To:	2.0				-				·
Remarks:	<u> </u>						L		
Comming.							···		
7.0									
CC:				····					
V D Frye						·			
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J Lail ïle					· ************************************	**************************************		·	

8-19-2000

G:/Everyone. . . /M&T Forms/Regional Lab Statesville/Soils Test Report M&T 503E

8 OF 8

NORTH CAROLINA DEPARTMENT OF TRANSPORTATION MATERIALS & TESTS UNIT P.O. BOX 25201 - RALEIGH, NORTH CAROLINA April 08, 2004

TEST REPORT FOR:

ROCK

TYPE OF MATERIAL: HARD ROCK

DATE SAMPLED: 03/30/04

RECEIVED: 04/01/04

COUNTY: CHEROKEE

SAMPLE #: 1-6 PROJECT #:

33183.3.1

SAMPLED BY: L. ACKER

TEST RESULTS

SAMPLE #	SULFUR %	NEUTRAL.	ACID	NET NEUTRAL
		POTENTIAL	POTENTIAL	POTENTIAL
1	2.822	42.519	88.188	-45.669
2	2.922	42.269	91.300	-49.031
3	3.403	42.020	106.344	-64.324
4	3.021	42.269	94.413	-52.143
5	3.536	42.519	110.494	-67.975
6	3.303	42.269	103.231	-60.962

NET NEUTRALIZATION POTENTIAL (NNP) REPORTED IN TONS CaCO3 PER 1000 TONS OF MATERIAL. *NNP VALUES LESS THAN NEGATIVE FIVE ARE CONSIDERED HARMFUL (HOT).

CHRISTOPHER A. PEOPLES CHEMICAL TESTING ENGINEER

W.D. FRYE L. ACKER

FILE