CONTENTS:

SHEET DESCRIPTION

1 TITLE SHEET

2 LEGEND

3 REPORT

4 SITE PLAN

5 PROFILE

6-8 BORE LOG & CORE REPORTS

9 SOIL TEST RESULTS

10 SCOUR REPORT

SITE PHOTOGRAPH

STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION

DIVISION OF HIGHWAYS
GEOTECHNICAL UNIT

STRUCTURE SUBSURFACE INVESTIGATION

STATE PROJECT 33420.1.1 I.D. NO. B-4055

F.A. PROJECT BRSTP-1124(4)

COUNTY CARTERET

PROJECT DESCRIPTION BRIDGE NO. 22

ON SR 1124 OVER BRANCH OF

NEWPORT RIVER

STATE STATE PROJECT REFERENCE NO. SECT STATES

N.C. 33420.1.1 (B-4055) 1 11

STATE PROJ.NO. F.A.PROJ.NO. DESCRIPTION
P.E.
P.E.
CONST.

CAUTION NOTICE

THE SUBSURFACE INFORMATION AND THE SUBSURFACE INVESTIGATION ON WHICH IT IS BASED WAS MADE FOR THE PURPOSE OF STUDY, PLANNING AND DESIGN, AND NOT FOR CONSTRUCTION OR PAY PURPOSES. THE VAROUS FIELD BORING LOGS, ROCK CORES, AND SOIL TEST DATA AVAILABLE MAY BE REVIEWED OR INSPECTED IN RALEIGH BY CONTACTING THE N. C. DEPARTMENT OF TRANSPORTATION, GEOTECHNICAL UNIT @ (99) 250-4088. NEITHER THE SUBSURFACE PLANS AND REPORTS, NOR THE FIELD BORING LOGS, ROCK CORES, OR SOIL TEST DATA IS PART OF THE CONTRACT.

GENERAL SOIL AND ROCK STRATA DESCRIPTIONS AND INDICATED BOUNDARIES ARE BASED ON A GEOTECHNICAL INTERPRETATION OF ALL AVAILABLE SUBSURFACE DATA AND MAY NOT NECESSARILY REFLECT THE ACTUAL SUBSURFACE CONDITIONS BETWEEN BORNESS OR BETWEEN SAMPLED STRATA WITHIN THE BOREHOLE. THE LABORATORY SAMPLED DATA AND THE IN SITU (IN-PLACE) TEST DATA CAN BE RELIED ON ONLY TO THE DEGREE OF RELIABILITY INHERENT IN THE STANDARD TEST METHOD. THE OBSERVED WATER LEVELS OR SOIL MOISTURE CONDITIONS INDICATED IN THE SUBSURFACE INVESTIGATIONS ARE AS RECORDED AT THE TIME OF THE INVESTIGATION. THESE WATER LEVELS OR SOIL MOISTURE CONDITIONS WITH THE ACCORDING TO CLIMATIC CONDITIONS INCLUDING TEMPERATURES, PRECIPITATION AND WIND, AS WELL AS OTHER NON-CLIMATIC FACTORS.

THE BIDDER OR CONTRACTOR IS CAUTIONED THAT DETAILS SHOWN ON THE SUBSURFACE PLANS ARE PRELMMARY ONLY AND IN MANY CASES THE FINAL DESIGN DETAILS ARE DIFFERENT, FOR BIDDING AND CONSTRUCTION PLANS AND DOOUMENTS FOR FINAL DESIGN INFORMATION ON THIS PROJECT, THE DEPARTMENT DOES NOT WARRANT OR CHARANTEE THE SUFFICIENCY OR ACCURACY OF THE INVESTIGATION MADE, NOR THE INTERPRETATIONS NADE OR OPINION OF THE DEPARTMENT AS TO THE TYPE OF MATERIALS AND CONDITIONS TO BE ENCOUNTERED. THE BIDDER OR CONTRACTOR IS CAUTIONED TO MAKE SUCH INDEPENDENT SUBSURFACE INVESTIGATIONS AS HE DEEMS INCESSARY TO SATISFY HIMSELF AS TO CONDITIONS TO BE ENCOUNTERED ON THIS PROJECT. THE CONTRACTOR SHALL HAVE NO CLAIM FOR ADDITIONAL COMPENSATION OR FOR AN EXTENSION OF TIME FOR ANY REASON RESULTING FROM THE ACTUAL CONDITIONS ENCOUNTERED AT THE SITE DIFFERING FROM THOSE INDICATED IN THE SUBSURFACE INFORMATION.

INVESTIGATED BY F.M. WESCOTT

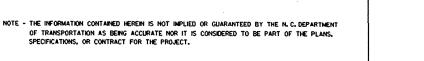
CHECKED BY D.N. ARGENBRIGHT

SUBMITTED BY D.N. ARGENBRIGHT

DATE FEBRUARY 2006

RES

HRC





NOTE - BY HAVING REQUESTED THIS INFORMATION THE CONTRACTOR SPECIFICALLY WAIVES ANY CLAIMS FOR INCREASED COMPENSATION OR EXTENSION OF TIME BASED ON DIFFERENCES BETWEEN THE CONDITIONS INDICATED HEREIN AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

DIVISION OF HIGHWAYS

GEOTECHNICAL UNIT

SUBSURFACE INVESTIGATION

	SOIL AND ROCK L	EGEND, TERMS, SYMBO	DLS, AND ABBREV	IATIONS	^				
SOIL DESCRIPTION	GRADATION		ROCK	DESCRIPTION	TERMS AND DEFINITIONS				
SOIL IS CONSIDERED TO BE THE UNCONSOLIDATED, SEMI-CONSOLIDATED OR WEATHERED EARTH MATERIALS WHICH CAN BE PENETRATED WITH A CONTINUOUS FLIGHT POWER AUGER, AND WHICH YIELDS LESS THAN 180 BLOWS PER FOOT ACCORDING TO STANDARD PENETRATION TEST (ASAPTO 1208, ASTIN D-1586, SOIL CLASSIFICATION IS BASED ON THE AGENTO SYSTEM AND BASIC DESCRIPTIONS GENERALLY SHALL INCLUDE: CONSISTENCY, COLOR, TEXTURE, HOISTURE, AGSHTO CLASSIFICATION, AND OTHER PERTINENT FACTORS SUCH AS MINERALGOICAL COMPOSITION, ANDLARITY, STRUCTURE, PLASTICITY, ETC. EXAMPLE:	WELL GRADED- INDICATES A GOOD REPRESENTATION OF PARTICLE SIZES FROM FINE T UNIFORM- INDICATES THAT SOIL PARTICLES ARE ALL APPROXIMATELY THE SAME SIZE. POORLY GRADED SOIL PARTICLES ARE ALL APPROXIMATELY THE SAME SIZE. GAP-GRADED- INDICATES A MIXTURE OF UNIFORM PARTICLES OF TWO OR MORE SIZES. ANGULARITY OF GRAINS THE ANGULARITY OR ROUNDIESS OF SOIL GRAINS ARE DESIGNATED BY THE TERMS; AN SUBANDIL AR, SUBROUNDED, OR ROUNDED.	ACALSO ROCK LINE SPT REFUSA IN NON-COA OF WEATHER	IS NON-COASTAL PLAIN MATERIAL TH INDICATES THE LEVEL AT WHICH NON IL IS PENETRATION BY A SPLIT SPOO STAL PLAIN MATERIAL, THE TRANSIT IED ROCK. KIALS ARE TYPICALLY DIVIDED AS FO	NAT WHEN TESTED, WOULD YIELD SPT REFUSAL, AN INFERRED I-COASTAL PLAIN MATERIAL WOULD YIELD SPT REFUSAL. IN SAMPLER EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS, ION BETWEEN SOIL AND ROCK IS OFTEN REPRESENTED BY A ZONE LOWS:	ALLUVIUM (ALLUV) - SOILS WHICH HAVE BEEN TRANSPORTED BY WATER.				
VERY STAFF, GRAY SLTY CLAY, MOST WITH INTERBEDDED FINE SAND LAVERS, HIGHLY PLASTIC, A-7-6		ROCK (WR)	PER FOOT.	PLAIN MATERIAL THAT YIELDS SPT N VALUES > 100 BLOWS	ARTESIAN - GROUND WATER THAT IS UNDER SUFFICIENT PRESSURE TO RISE ABOVE THE LEVEL				
SOIL LEGEND AND AASHTO CLASSIFICATION GENERAL GRANULAR MATERIALS SILT-CLAY MATERIALS (25% PASSING *200) CRGANIC MATERIALS (35% PASSING *200) CRGANIC MATERIALS	MINERAL OGICAL COMPOSITION MINERAL NAMES SUCH AS QUARTZ, FELDSPAR, MICA, TALC, KAQLIN, ETC. ARE USED IN DESEMBLE OF SIGNIFICANCE.	SCRIPTIONS CRYSTALLINE ROCK (CR)	WOULD YIELD S		AT WHICH IS IS ENCOUNTERED, BUT WHICH DOES NOT NECESSARILY RISE TO OR ABOVE THE GROUND SURFACE. CALCAREOUS (CALC.) - SOILS WHICH CONTAIN APPRECIABLE AMOUNTS OF CALCIUM CARBONATE.				
GROUP A-1 A-3 A-2 A-4 A-5 A-6 A-7 A-1, A-2 A-4, A-5 CLASS. A-1-a A-1-b A-2-4 A-2-5 A-2-6 A-2-7 A-5 A-5 A-3 A-6, A-7	COMPRESSIBILITY	NON-CRYSTALL ROCK (NCR)	SEDIMENTARY F	SE GRAIN METAMORPHIC AND NON-COASTAL PLAIN ROCK THAT WOULD YEILD SPT REFUSAL IF TESTED. ROCK TYPE	COLLUVIUM - ROCK FRAGMENTS MIXED WITH SOIL DEPOSITED BY GRAVITY ON SLOPE OR AT BOTTOM				
SYMBOL 8000 8000 8000 8000 8000 8000 8000 80	SLIGHTLY COMPRESSIBLE LIQUID LIMIT LESS THAN MODERATELY COMPRESSIBLE LIQUID LIMIT 31-50 HIGHLY COMPRESSIBLE PERCENTAGE OF MATERIAL	COASTAL PLAIN	COASTAL PLAIN	LLITE, SLATE, SANDSTONE, ETC. NEORIMENTS CEMENTED INTO ROCK, BUT MAY NOT YIELD ROCK TYPE INCLUDES LIMESTONE, SANDSTONE, CEMENTED TC.	OF SLOPE. CORE RECOVERY (REC.) - TOTAL LENGTH OF ALL MATERIAL RECOVERED IN THE CORE BARREL DIVIDED BY TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE.				
# 10 50 MX GRANULAR SILT- MUCK,	ORGANIC MATERIAL GRANULAR SILT- CLAY	TEDIAL	WE	EATHERING	DIKE - A TABULAR BODY OF IGNEOUS ROCK THAT CUTS ACROSS THE STRUCTURE OF ADJACENT ROCKS OR CUTS MASSIVE ROCK.				
* 40 30 MX50 MX51 MN PEAT SOILS SOILS SOILS PEAT * 200 15 MX 25 MX 105 MX 1	SOILS SOILS	1 - 10%		JOINTS MAY SHOW SLIGHT STAINING ROCK RINGS UNDER	DIP - THE ANGLE AT WHICH A STRATUM OR ANY PLANAR FEATURE IS INCLINED FROM THE				
LIQUID LIMIT	LITTLE ORGANIC MATTER 3 - 5% 5 - 12% LITTLE 1	10 - 20% 20 - 35% 20 - 35% 400 ABOVE (V. SLI.)	HAMMER IF CRYSTALLINE. ROCK GENERALLY FRESH, JOINTS STA: CRYSTALS ON A BROKEN SPECIMEN FO OF A CRYSTALLINE NATURE.	INED, SOME JOINTS MAY SHOW THIN CLAY COATINGS IF OPEN, ACE SHINE BRIGHTLY. ROCK RINGS UNDER HAMMER BLOWS IF	HORIZONTAL. DIP DIRECTION (DIP AZIMUTH) - THE DIRECTION OR BEARING OF THE HORIZONTAL TRACE OF THE LINE OF DIP, MEASURED CLOCKWISE FROM NORTH.				
GROUP NOCK 0 0 0 4 MX 8 MX 12 MX 16 MX No MX MUDERAIL ORGANIC USUAL TYPES STONE FRACS. OF MAJOR GRAVEL AND SAND SOILS SOILS SOILS SOILS SOILS SAND GRAVEL AND SAND SOILS SOILS MATTER	GROUND WATER WATER LEVEL IN BORE HOLE IMMEDIATELY AFTER DRILLING. STATIC WATER LEVEL AFTER 24 HOURS.	SLIGHT (SLI,)	1 INCH. OPEN JOINTS MAY CONTAIN C	INED AND DISCOLORATION EXTENDS INTO ROCK UP TO LAY. IN GRANITOID ROCKS SOME OCCASIONAL FELDSPAR D. CRYSTALLINE ROCKS RING UNDER HAMMER BLOWS.	FAULT - A FRACTURE OR FRACTURE ZONE ALONG WHICH THERE HAS BEEN DISPLACEMENT OF THE SIDES RELATIVE TO ONE ANOTHER PARALLEL TO THE FRACTURE. FISSILE - A PROPERTY OF SPLITTING ALONG CLOSELY SPACED PARALLEL PLANES.				
GEN. RATING AS A SUBGRADE EXCELLENT TO GOOD FAIR TO POOR POOR POOR POOR UNSUITABLE	E PERCHED WATER, SATURATED ZONE OR WATER BEARING STRAT	(MOD.)	GRANITOID ROCKS, MOST FELDSPARS A	W DISCOLORATION AND WEATHERING EFFECTS. IN ARE DULL AND DISCOLORED, SOME SHOW CLAY. ROCK HAS AND SHOWS SIGNIFICANT LOSS OF STRENGTH AS COMPARED	FLOAT - ROCK FRAGMENTS ON SURFACE NEAR THEIR ORIGINAL POSITION AND DISLODGED FROM PARENT MATERIAL.				
P.I. OF A-7-5 ≤ L.L 30 : P.I. OF A-7-6 > L.L 30	SPRING OR SEEPAGE	i i	WITH FRESH ROCK.		FLOOD PLAIN (F.P.) - LAND BORDERING A STREAM, BUILT OF SEDIMENTS DEPOSITED BY THE STREAM.				
CONSISTENCY OR DENSENESS PRIMARY SDIL TYPE COMPACTNESS OR RANGE OF STANDARD RANGE OF UNCONFINED PENETRATION RESISTENCE COMPRESSIVE STRENGTH	MISCELLANEOUS SYMBOLS RDADWAY EMBANKMENT WITH SDIL DESCRIPTION WITH SDIL DESCRIPTION WITH SDILL DE	SEVERE (MOD, SEV.)	AND DISCOLORED AND A MAJORITY SH AND CAN BE EXCAVATED WITH A GEOL	ED OR STAINED. IN GRANITOID ROCKS, ALL FELDSPARS DULL HOW KAOLINIZATION. ROCK SHOWS SEVERE LOSS OF STRENGTH LOGIST'S PICK. ROCK GIVES 'CLUNK' SOUND WHEN STRUCK.	FORMATION (FM.) - A MAPPABLE GEOLOGIC UNIT THAT CAN BE RECOGNIZED AND TRACED IN THE FIELD.				
CONSISTENCY CONSISTENCY (N-VALUE) (TONS/FT2)	WITH SOIL DESCRIPTION DESCRIPTION DESCRIPTION		IF TESTED, WOULD YIELD SPT REFUSA	ሚ RED OR STAINED ROCK FABRIC CLEAR AND EVIDENT BUT REDUCED	JOINT - FRACTURE IN ROCK ALONG WHICH NO APPRECIABLE MOVEMENT HAS OCCURRED.				
GENERALLY	l m ²	BULK SAMPLE (SEV.)	IN STRENGTH TO STRONG SOIL. IN GE EXTENT. SOME FRAGMENTS OF STRON IF TESTED. YIELDS SPT N VALUES >	RANITOID ROCKS ALL FELDSPARS ARE KAOLINIZED TO SOME IG ROCK USUALLY REMAIN.	ITS LATERAL EXTENT.				
MATERIAL	ROADWAY EMBANKMENTS - CORE BORING ST- INFERRED SOIL BOUNDARIES MONITORING WELL	SAMPLE VERY SEVERE (V. SEV.)	ALL ROCK EXCEPT OUARTZ DISCOLORE THE MASS IS EFFECTIVELY REDUCED REMAINING. SAPROLITE IS AN EXAMPL	100 EPT. ED OR STAINED. ROCK FABRIC ELEMENTS ARE DISCERNIBLE BUT TO SOIL STATUS, WITH DNLY FRAGMENTS OF STRONG ROCK E. OF ROCK WEATHERED TO A DEGREE SUCH THAT ONLY MINOR BRIC FRAMIA. IE TESTED, VIELDS SPT. N VALUES 4, 100 BPF	LENS - A BODY OF SOIL OR ROCK THAT THINS OUT IN ONE OR MORE DIRECTIONS. MOTTLED (MOT.) - IRREGULARLY MARKED WITH SPOTS OF DIFFERENT COLORS, MOTTLING IN SOILS USUALLY INDICATES POOR AERATION AND LACK OF GOOD DRAINAGE. PERCHED WATER - WATER MAINTAINED ABOVE THE NORMAL GROUND WATER LEVEL BY THE PRESENCE OF AN INTERVENING IMPERVIOUS STRATUM.				
SILT-CLAY MEDIUM STIFF 4 TO 8 0.25 TO 1	ALLUVIAL SOIL BOUNDARY ALLUVIAL SOIL BOUNDARY 25/825 DIP/DIP DIRECTION OF INSTALLATION INSTALLATION CBB	TRIAVIAL CAMPLE	ROCK REDUCED TO SOIL. ROCK FABRIO SCATTERED CONCENTRATIONS. QUARTZ ALSO AN EXAMPLE.	C NOT DISCERNIBLE, OR DISCERNIBLE ONLY IN SMALL AND MAY BE PRESENT AS DIKES OR STRINGERS, SAPROLITE IS	RESIDUAL SOIL - SOIL FORMED IN PLACE BY THE WEATHERING OF ROCK. ROCK QUALITY DESIGNATION (R.O.D.) - A MEASURE OF ROCK QUALITY DESCRIBED BY: TOTAL LENGTH OF ROCK SEGMENTS EQUAL TO DR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF CORE RUN AND				
TEXTURE OR GRAIN SIZE	ROCK STRUCTURES SPT N-VALUE	VERY HARD		K HARDNESS R SHARP PICK, BREAKING OF HAND SPECIMENS REDUIRES	EXPRESSED AS A PERCENTAGE. SAPROLITE (SAP.) - RESIDUAL SOIL WHICH RETAINS THE RELIC STRUCTURE OR FABRIC OF THE				
U.S. STD. SIEVE SIZE 4 10 40 60 200 270 DPENING (MM) 4.76 2.0 0.42 0.25 0.075 0.053	● - SOUNDING ROD REFUSAL ABBREVIATIONS	HARD	SEVERAL HARD BLOWS OF THE GEOL		PARENT ROCK. SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND				
BOULDER	AR - AUGER REFUSAL PMT - PRESSUREMETER 1 BT - BORTNO TERMINATED SD SAND, SANDY	TEST MODERATELY	TO DETACH HAND SPECIMEN. CAN BE SCRATCHED BY KNIFE OR PI	ick. Gouges or grooves to 0.25 inches deep can be cologists pick. Hand specimens can be detached	RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, WHICH HAS BEEN EMPLACED PARALLEL TO THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS SLICKENSIDE - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT OR				
GRAIN MM 305 75 2.0 0.25 0.05 0.005 SIZE IN 12' 3'	CL CLAY SL SILT, SILTY CPT - CONE PENETRATION TEST SLI SLIGHTLY	, naro	BY MODERATE BLOWS.	COLOGISTS PICK. HAND SPECIMENS CAN BE DETACHED	SLIP PLANE. STANDARD PENETRATION TEST (PENETRATION RESISTANCE) (SPT) - NUMBER OF BLOWS (N OR B.P.F.) OF				
SOIL MOISTURE - CORRELATION OF TERMS SOIL MOISTURE SCALE FIELD MOISTURE GILDE FOR FIELD MOISTURE DESCRIPTION	CSE COARSE DMT - DILATOMETER TEST DPT - DYNAMIC PENETRATION TEST • - VOID RATIO TCR - TRICONE REFUSAL 7 - UNIT WEIGHT 7d - DRY UNIT WEIGHT	HARD	CAN BE EXCAVATED IN SMALL CHIPS POINT OF A GEOLOGISTS PICK.	INCHES DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT. S TO PEICES 1 INCH MAXIMUM SIZE BY HARD BLOWS OF THE Y BY KNIFE OR PICK, CAN BE EXCAYATED IN FRAGMENTS	STROUGHO FENE TRATION TEST GENETIATION RESISTANCE/TSF17 NUMBER OF BLOWS ON OR BEFF. JOIN A 140 LB, HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF 1 FOOT INTO SOIL WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER, SPT REFUSAL IS LESS THAN 0.1 FOOT PENETRATION WITH 60 BLOWS.				
(ATTERBERG LIMITS) DESCRIPTION ONDE TO ATTERBE HOUSING ESSENTIAL OF THE CONTROL O	- SATURATED - USUALLY LIQUID; VERY WET, USUALLY FOSS FOSSILIFEROUS V VERY				STRATA CORE RECOVERY (SREC.) - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY TOTAL LENGTH OF STRATUM AND EXPRESSED AS A PERCENTAGE.				
LL LIOUID LIMIT (SAT.) FROM BELOW THE GROUND WATER TABLE PLASTIC RANGE - WET - (W) SEMISOLID, REQUIRES DRYING TO	FRAC FRACTURED VST - VANE SHEAR TEST FRAGS FRAGMENTS MED MEDIUM	SOFT		E EXCAVATED READILY WITH POINT OF PICK, PIECES I INCH DIKEN BY FINGER PRESSURE, CAN BE SCRATCHED READILY BY	STRATA ROCK QUALITY DESIGNATION (S.R.D.D.) - A MEASURE OF ROCK QUALITY DESCRIBED BY: TOTAL LENGTH OF ROCK SEGMENTS WITHIN A STRATUM EQUAL TO OR GREATER THAN 10 CENTIMETERS DIVIDED BY THE TOTAL LENGTH OF STRATA AND EXPRESSED AS A PERCENTAGE.				
RANGE - WET - (W) ATTAIN OPTIMUM MOISTURE (P1) PL PLASTIC LIMIT	EQUIPMENT USED ON SUBJECT PROJECT		ACTURE SPACING	BEDDING	IOPSOIL (T.S.) - SURFACE SOILS USUALLY CONTAINING ORGANIC MATTER.				
	DRILL UNITS: ADVANCING TOOLS: HAMMER	· · ·	SPACING	IERM THICKNESS VERY THICKLY BEDDED > 4 FEET	BENCH MARK: RAILROAD SPIKE IN 30° PINE -BL- STA. 12+34, 139 LEFT				
OM OPTIMUM MOISTURE - MOIST - (M) SOLID: AT OR NEAR OPTIMUM MOISTURE SL SHRINKAGE LIMIT	MOBILE B- CLAY BITS X AUT	MANOAL WIDE	3 TO 10 FEET	THICKLY BEDDED 1.5 - 4 FEET THINLY BEDDED 0.16 - 1.5 FEET	ELEVATION: 9.37				
REDUIRES ADDITIONAL WATER TO	6° CONTINUOUS FLIGHT AUGER CORE SI	MODERATEL CLOSE	Y CLOSE 1 TO 3 FEET 0.16 TO 1 FEET	VERY THINLY BEDDED 0.03 - 0.16 FEET					
- DRY - (D) ATTAIN OPTIMUM MOISTURE	CME 550-X B* HOLLOW AUGERS -B_	VERY CLOS	E LESS THAN 0.16 FEET	THICKLY LAMINATED 0.008 - 0.03 FEET THINLY LAMINATED < 0.008 FEET	NOTES:				
PLASTICITY	CME-45C HARD FACED FINGER BITSN_	EOD CEDIMENTA		DURATION INING OF THE MATERIAL BY CEMENTING, HEAT, PRESSURE, ETC.					
PLASTICITY INDEX (PI) DRY STRENGTH NONPLASTIC 0-5 VERY LOW LOW PLASTICITY 6-15 SLIGHT	TUNG,-CARBIDE INSERTS X CASING W/ ADVANCER HAND T	FRI	RUBBIN	INING OF THE MATERIAL BY CEMENTING, HEAT, PRESSURE, ETC. BYTH FINGER FREES NUMEROUS GRAINS; BLOW BY HAMMER DISINTEGRATES SAMPLE.					
MED. PLASTICITY 16-25 MEDIUM HIGH PLASTICITY 26 OR MORE HIGH	PORTABLE HOIST X TRICONE 219/16 STEEL TEETH P	!		CAN BE SEPARATED FROM SAMPLE WITH STEEL PROBE; S EASILY WHEN HIT WITH HAMMER.					
COLOR	I IXI OTHER CMF 45B	1		ARE DIFFICULT TO SEPARATE WITH STEEL PROBE;					
DESCRIPTIONS MAY INCLUDE COLOR OR COLOR COMBINATIONS (TAM, RED, YEL-BRN, BLUE-GRAY) MODIFIERS SUCH AS LIGHT, DARK, STREAKED, ETC. ARE USED TO DESCRIBE APPEARANCE.	X OTHER CME 550X OTHER	ANE SHEAR TEST	REMELY INDURATED SHARP	ULT TO BREAK WITH HAMMER. HAMMER BLOWS REDUIRED TO BREAK SAMPLE; E BREAKS ACROSS GRAINS.					

STATE PROJECT NO. SHEET NO. TOTAL SHEETS
33420.1.1 2 11

ID B-4055



STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

MICHAEL F. EASLEY
GOVERNOR

LYNDO TIPPETT
SECRETARY

February 27, 2006

STATE PROJECT:

33420.1.1 B-4055

F. A. PROJECT:

BRSTP-1124(4)

COUNTY:

Carteret

DESCRIPTION:

Bridge No. 22 on SR 1124 over Branch of Newport River

SUBJECT:

Geotechnical Report - Bridge Foundation Investigation for

SR 1124 over Branch of Newport River at -L- Station 16+72

Site Description

The proposed bridge site is located at the existing SR 1124 over a Branch of the Newport River in Newport. The replacement structure will be constructed along the existing SR 1124 alignment. Based on the proposed design, the new structure will have two spans with a total length of 140 feet. The bents will have a skew of 110 degrees.

One Standard Penetration Test (SPT) boring was made at or near each proposed bent location to provide subsurface information relative to foundation design. The borings were made with ATV mounted CME-550 drill machine and were advanced by rotary drill methods using bentonite drilling fluid.

The bridge site is located in the Coastal Plain Physiographic Province and is underlain by Recent alluvial deposits, Pliocene age sediments of the Duplin Formation and Oligocene age soils of the River Bend Formation. Topography at the site is nearly flat to gentle sloping. Elevations at the site range from -1± feet along the channel bed to 15± feet along the existing SR 1124 roadway. Artesian flow was noted in all borings drilled at the site. The hydrostatic head was measured at an elevation of 12± feet at EB1-B and 10± feet at EB2-A. The true water levels will generally match the stream flow line. The surface of Branch of Newport River was at an elevation of 3± feet.

MAILING ADDRESS:
NC DEPARTMENT OF TRANSPORTATION
GEOTECHNICAL ENGINEERING UNIT
1589 MAIL SERVICE CENTER
RALEIGH NC 27699-1589

TELEPHONE: 919-250-4088 FAX: 919-250-4237

WEBSITE: WWW.DOH.DOT.STATE.NC.US

LOCATION:
CENTURY CENTER COMPLEX
ENTRANCE B-2
1020 BIRCH RIDGE DRIVE
RALFIGH NC

Soil Description

Subsurface conditions at the site are relatively uniform. Surficial alluvial soils generally consist of 4± to 10± feet of very loose to dense sand (A-2-4, A-3). Soils belonging to the Pliocene age Duplin Formation underlie the Recent deposits at elevations ranging from 2± to -6± feet. Soils of the Duplin Formation consist of 15± to 22± feet of very loose to medium dense sand (A-1-b, A-2-4) underlain by 8± to 15± feet of soft to stiff sandy silt (A-4). Shell fragments were noted throughout the Duplin Formation. The Duplin deposits are underlain at elevations ranging from -27± to -35± feet by the Oligocene age River Bend Formation. Soils within the River Bend formation consist of limestone with sand layers underlain by medium stiff to hard sandy silt (A-4). Approximately 2 feet of limestone was encountered beneath the silt in boring B1-B and EB2-A at an elevation of -77± feet. Dense clayey sand (A-2-4) was below the limestone layer in boring B1-B.

Based on the proposed design, the existing grade will be raised $1\pm$ feet at the proposed bridge site. The existing roadway embankment at the end bents consists of $6\pm$ feet of very loose sand (A-2-4). The proposed end bent slopes will be mainly constructed within the existing embankment. Some additional fill will be required for construction of the end bent and side slopes. Borrow meeting Coastal Plain criteria is available in nearby areas.

The Geotechnical foundation report is based on the Bridge Survey and Hydraulic Design Report dated August 18, 2005. If significant changes are made in the design or location of the proposed structure, the subsurface information should be reviewed and modified as necessary.

Prepared By:

Fred M. Wescott III

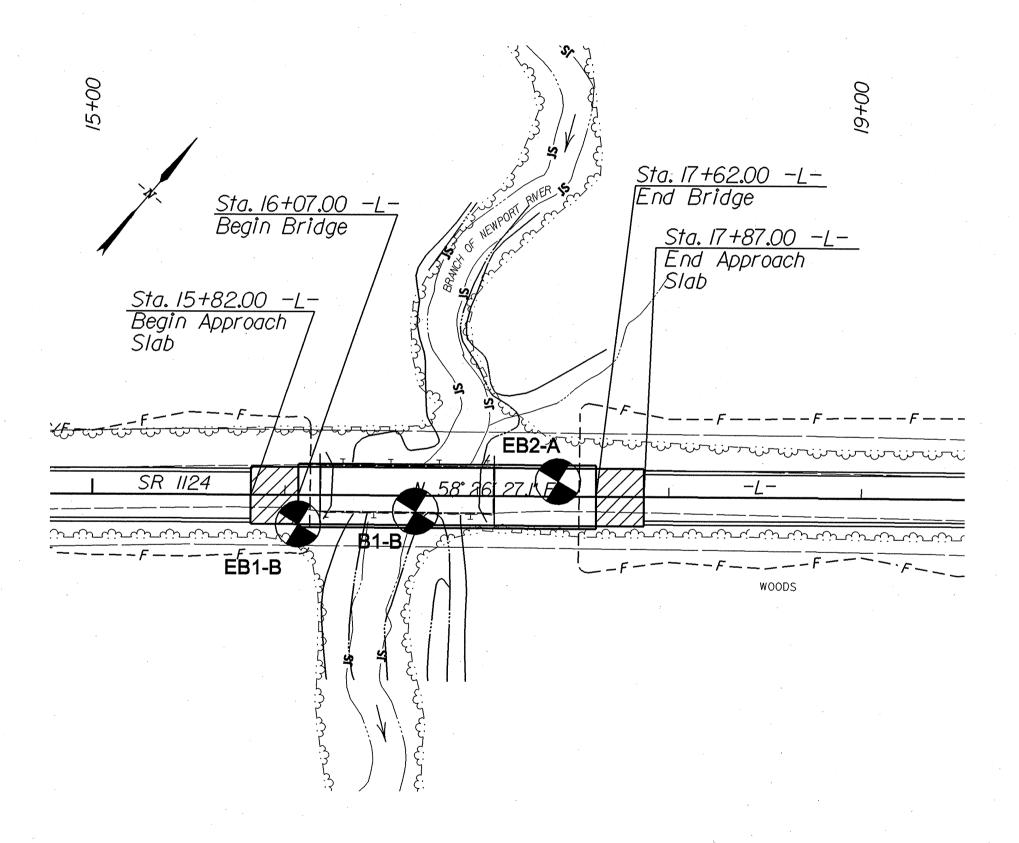
Project Engineering Geologist

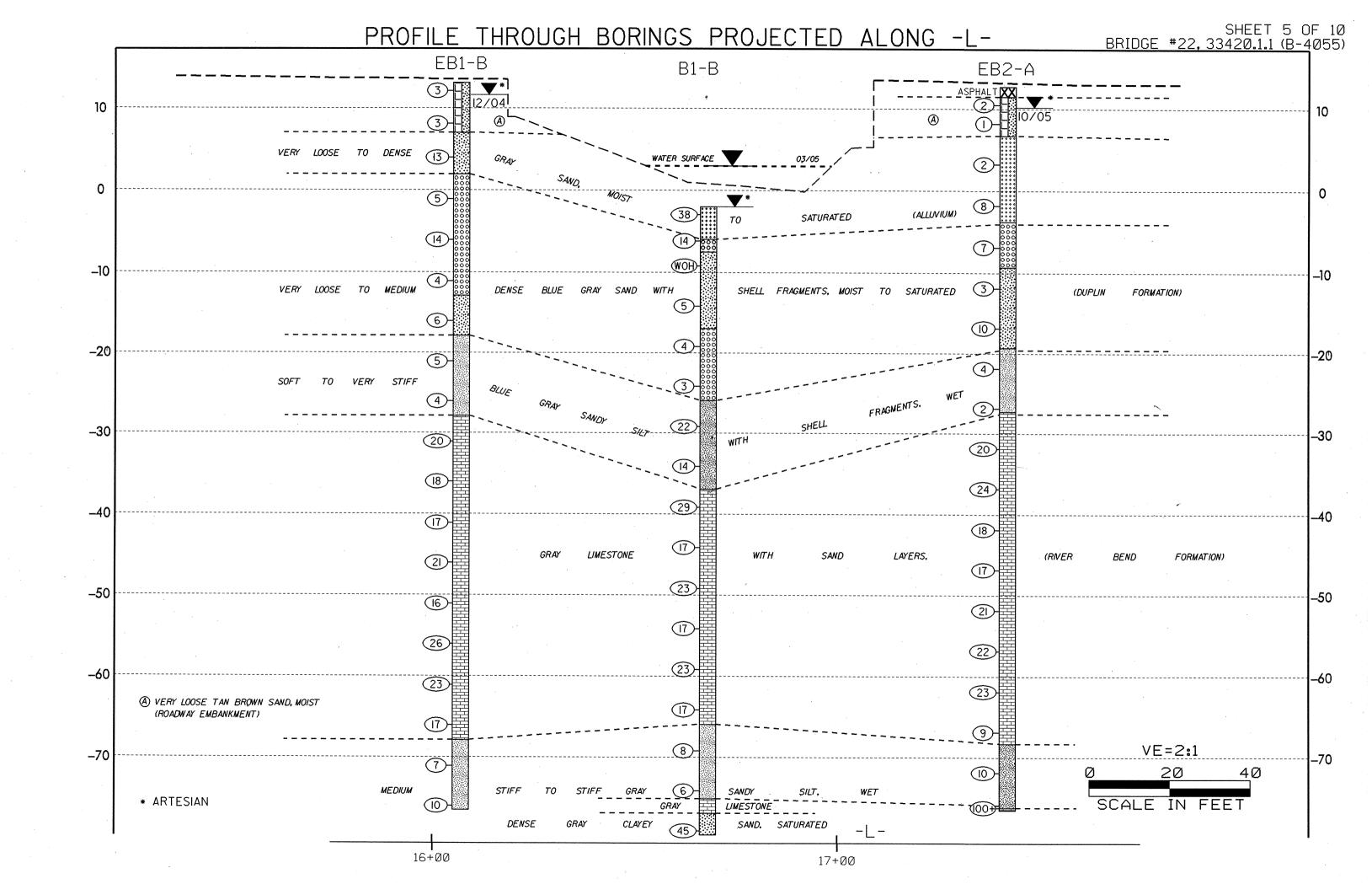
Ful m wew Ty

50 100 PROJECT REFERENCE NO. SHEET

33420.1.1 (B-4055) 4

TEST SITE PLAN





North Carolina department of transportation North Carolina department of transportation GEOTECHNICAL UNIT BORING LOG

GEOTECHNICAL UNIT BORING LOG

PROJECT	NO 334	20 11		III	D 4055	COLUMN			ODOL 4														SHEET	6 OF II
SITE DESC					B-4055	COUNT R 1124 OVE			GEOLG		M. M. HAGE		PROJECT						COUNTY CARTE		GEOLO	GIST !	M. M. HAGER	
BORING N					ATION	16+07		15'RT		MENT		GROUND WATER O HR. NM	BODING A	CKIPTION	RF	RODE	NO. 2		OVER BRANCH					OUND WATER
COLLAR E					NORTHING		Ortob	EASTIN		VPIEN I	-L-	24 HR. ARTESIAN	BORING 1					CATION 16+0	(OFFSE	r 15′ RT	<u>l</u>	MENT -		R. NM
TOTAL DE					IINE CME		DRILL MET	THOD ROT		ALID.	HAMMED TY	YPE AUTOMATIC	COLLAR					NORTHING	I DAY 1 100	EASTIN				IR. ARTESIAN
START DA						ATE 02/18		RFACE WAT		VIOD	DEPTH TO		TOTAL D					HINE CME-45B			ARY W/N	IUD	HAMMER TYPE A	
	DEPTH BI				·	OWS PER	FOOT	10111515	1 /1	T				DEPTH				PLETION DATE C	72718704 SI PER FOOT	RFACE WAT	7 7 7		DEPTH TO ROC	
ELEV.	(FT.) O			1		25 50	7,5 10	SAMPLE NUMBER	MOL 6		SOIL AND DESCRIF	TION	ELEV.	3	1	0.510	- 1			SAMPLE NUMBER	1 / 1111		SOIL AND ROODESCRIPTION	CK
13.1	0.0					+		-	7					+	0.5	0.5 0	.5 " 1.	 		HOMBER	MOI. Ğ			· · · · · · · · · · · · · · · · · · ·
13.1	_ 0.0		12		M				Y H		TAN BRO	WN SAND.	-70 . 0 -	± 83.2	4	4	3 1	7		SS-8		/ "	GRAY LIMESTONE SAND LAYERS IVER BEND FORMA	
10.0	4.0	ı 2	, 1	1.	V_3]					DRY TO S	ATURATED		‡ · · · ·								/ IK	IVER BEND FORMA	TION
	_ '	' -		'	 	I = = = I = =			🖽		(ROADWAY E	EMBANKMENT)	75.0	‡										
5.0	- - 8.2 3	3 4	. 9	1	- -	 		SS-I			CRAY SAND	, SATURATED	-75 . 0 -	± 88.2	3	4 (5 I	<u>*</u> 10-				G	RAY SANDY SILT,	WET
	- 0.2	' '	'	'	 - 	<u> </u>		33-1			(ALLU			Ŧ				BOBING -	TERMINATED					
	_					+			000				-80.0	I-				11 1	ATION -76.67					
-0.0	<u>-</u> 13.2 2	2 1	4		 \ 5 -	 			000					\pm				1 1	VDY_SILT		.			
1 7	-					 			000	BL.	UE GRAY SAN	ND WITH SHELL	-85 . 0 -	<u></u>										
-5.0	18.2	5 7	7	1	- X -4	#		SS-2	0000		FRAGMENTS, S	CATUDATEN		‡										
1	-					I = = I = I			0000	'	I MADIVILIATO,	SATUNATED		‡										
-10.0	- -23.2 2	2 2	2		7				000		(DUPLIN FO	ORMATION)	-90 . 0 -	†					-				•	
	- 25.2 2	- -	-	'		 -	-		000					ŧ										
	-				 							• .	-95 . 0 -	<u> </u>										
-15.0	-28 . 2	3 3	3	1	<u> </u>	-		SS-3						<u> </u>		Ì		[====================================					•	•
1 7	-					 -							-100.0 -	<u> </u>						-				·
-20.0	33.2	2 2	3	1	 	 -		SS-4					100.0	‡.										
	-				EE==]				RITIE	- CDAY CLAY	EY SANDY SILT	105.0	‡					-				•	
-25.0	38.2 2	, 2	2		-								-105.0 -	F										
2010		٦	-	'		 				WII	TH SHELL FR	AGMENTS, WET		Ŧ.										
700	-		١			 			8888				-110.0 -	_										
-30.0	- 43.2 E	1 9	11		7	2 0		SS-5					-	<u>+</u> .										
1 =					F#:								-115.0 -	‡				 +	-					
-35.0	-48.2 E	10	8 1		[*+	8							-	‡				 						
1 ‡	.												120.0	‡					- +					
-40.0	53.2 E	. 7	10	,		 		SS-6					-120 . 0 -	F					-					
					====	 		33 6						Ī										
1 45 0					11	1					GRAY LIMEST	ONE WITH	-I25 . 0 -	 -										
-45.0	-58.2	12	9		 	<u> 2t </u>							_	±				[.			
1 7					# -	‡ = = - = = = =					SAND LA	YERS	-130.0 -	<u>L</u>				<u> </u>						
-50.0	-63 . 2 8	8	8		FX-16			,			DU/ED DE//D		100.0	‡										
1 ±					[==]					(1	RIVER BEND F	- ORMATION)	-	‡			ľ							
-55.0	-68.2	10	16			X-26		CC-7					-I35 . 0 -	F										
55.5	00.2	ا	10		<u> </u>	[]		SS-7					-	Ŧ		1								
‡													-140.0 -	<u> </u>							-			
-60.0 +	73.2 12	2 11	12		*	23							_	E										
‡					= = -/=	-							-145 . 0 -	<u> </u>				 						
-65 . 0 	78.2 8	7	10	.	F = -*	7							-	‡					+					
<u> </u>					<u> </u>	<u> </u>			註:					<u> </u>				<u> </u>	+					l

NORTH CAROLINA DEPAR	
	UNIT BORING LOG SHEET 7 OF 11
PROJECT NO. 33420.I.I ID. B-4055 COUNTY SITE DESCRIPTION BRIDGE NO. 22 ON SR II24 OVER	
BORING NO. BI-B BORING LOCATION 16+68	BRANCH OF NEWPORT RIVER OFFSET 8' RT ALIGNMENT -L- O HR. NM
COLLAR ELEVATION -2.0' NORTHING	EASTING 24 HR. NM
TOTAL DEPTH 77.7' DRILL MACHINE CME 550-X	DRILL METHOD ROTARY W/MUD HAMMER TYPE AUTOMATIC
START DATE 10/10/05 COMPLETION DATE 10/11/0	
ELEV. (FT.) 0.510.510.5 (FT.) 0 25 50	TOOT SAMPLE O SOIL AND ROCK DESCRIPTION
-2.0 0.0 2 5 33 1 × 38 	
-5.0 + 3.2 6 6 8 1	GRAY SAND, SATURATED (ALLUVIUM)
6.2 WOHWOHWOH I X WOH	SS-18 ***********************************
-10.0 + 0.2	
# II.2 1 1 4 I ********************************	
-15.0 +	BLUE GRAY SAND WITH SHELL
+ 16.2 1 1 3 1 1 4 4	BLUE GRAY SAND WITH SHELL SS-20 FRAGMENTS, SATURATED OUPLIN FORMATION)
-20.0 +	
21.2 1 1 2 1 7 3	
-25.0 +	
26.2 8 11 11 1 122	
-30.0 +	 BLUE GRAY SANDY SILT WITH
31.2 4 7 7 1 - 14	
-35.0 +	SHELL FRAGMENTS, WET
36.2 11 12 17 1 X 29	SS-22 SS-22
-40.0 +	SS-22 SS-23 GRAY LIMESTONE WITH SAND LAYERS (RIVER BEND FORMATION)
41.2 8 9 8 1	
+ 46.2 7 11 12 1 23	
51.2 8 8 9 1	GRAY LIMESTONE WITH
-55.0 + 51.2 0 0 1	SAND LAYERS
56.2 9 9 14 1 23	(RIVER BEND FORMATION)
-60.0 	
+ 61.2 5 10 7 1 1 1	
-65.0 + BORING TERI	
+ 66.2 5 2 6 1 -/-AT ELEVATIO	N79.7-
-70.0 +	
# 71.2 2 3 3 1 # *	GRAY SANDY SILT, WET
-75.0 + 76.2 2 2 2 2 4 2 4 5 4 5 4 5 4 5 4 5 4 5 4	GRAY LIMESTONE
+ 76.2 2 33 12 1 =45X	GRAY LIMESTONE GRAY CLAYEY SAND, SATURATED
-80.0	F······

NORTH CAROLINA DEPARTMENT OF TRANSPORTATION NORTH CAROLINA DEPARTMENT OF TRANSPORTATION GEOTECHNICAL UNIT BORING LOG

GEOTECHNICAL UNIT BORING LOG

PROJECT NO. 33420.I.I ID. B-4055 COUNTY CARTERET GEOLOGIST J. L. STONE	SHEET 8 OF II
	PROJECT NO. 33420.I.I ID. B-4055 COUNTY CARTERET GEOLOGIST J. L. STONE
SITE DESCRIPTION BRIDGE NO. 22 ON SR 1124 OVER BRANCH OF NEWPORT RIVER GROUND WATER	SITE DESCRIPTION BRIDGE NO. 22 ON SR 1124 OVER BRANCH OF NEWPORT RIVER GROUND WATER
BORING NO. EB2-A BORING LOCATION 17+42 OFFSET 8'LT ALIGNMENT -L- 0 HR. NM	BORING NO. EB2-A BORING LOCATION 17+42 OFFSET 8'LT ALIGNMENT -L- OHR. NM
	COLLAR ELEVATION 12.7' NORTHING EASTING 24 HR. ARTESIAN
TOTAL DEPTH 89.3' DRILL MACHINE CME 550-X DRILL METHOD ROTARY W/MUD HAMMER TYPE AUTOMATIC	TOTAL DEPTH 89.3' DRILL MACHINE CME 550-X DRILL METHOD ROTARY W/MUD HAMMER TYPE AUTOMATIC
START DATE 10/11/05 COMPLETION DATE 10/12/05 SURFACE WATER DEPTH DEPTH TO ROCK	START DATE 10/11/05 COMPLETION DATE 10/12/05 SURFACE WATER DEPTH TO ROCK
ELEV. DEPTH BLOW COUNT PEN. BLOWS PER FOOT SAMPLE ON NUMBER MOI. G SOIL AND ROCK DESCRIPTION	DEPTHBLOW COUNTIPEN. BLOWS PER FOOT SAMPLE VILL SOULAND BOOK
(FT.) 0.510.510.5 (FT.) 0 25 50 75 100 NUMBER MOI. G DESCRIPTION	ELEV. (FT.) 0.5(0.5(0.5(0.5 (FT.) 0 25 50 75 100 NUMBER MOI. G DESCRIPTION
12.7 + 0.0	GRAY LIMESTONE WITH
I 1.2 1 1 1 1 1 1 1 1 1	-70.0 + 83.6 2 4 6 1 - 10 SS-17 SAND LAYERS
	GRAY SANDY SILT, WET
5.0 + 0.5 1 1 1 1 1 1 1 1 1	1 + 88.6 4 96
	BORING TERMINAJED AT
	-80.0
$\begin{vmatrix} 0.0 & + 13.5 \end{vmatrix} \begin{vmatrix} 4 & 6 & 2 \end{vmatrix} \begin{vmatrix} 1 & + 1 \end{vmatrix} \begin{vmatrix} 1 & 1 & 1 \end{vmatrix} \begin{vmatrix} 1 & 1 & 1 & 1 \\ 1 & 1 & 1 & 1 \end{vmatrix} \begin{vmatrix} 1 & 1 & 1 & 1 & 1 \\ 1 & 1 & 1 & 1 & 1 \end{vmatrix} \begin{vmatrix} 1 & 1 & 1 & 1 & 1 \\ 1 & 1 & 1 & 1 & 1 &$	
	1 -85.0 +
0.0	
18.6 3 3 4 1	
<u> </u>	-90.0 +
-10.0 + 23.6 1 1 2 1 X-3	
± 25.5 1 1 2 1 1 2 2 3 2 3 2 3 3 3 3	
$\begin{vmatrix} -15.0 & + \\ -2.0 & - \end{vmatrix}$ (DUPLIN FORMATION)	-95.0 +
28.6 3 4 6 1 F-X-10	
	-100.0 +
$\begin{vmatrix} -20.0 & + \\ & + & 33.6 \end{vmatrix}$	<u>╣</u>
33.6 1 2 2 1 米4	
	-105.0 +
-25.0 + 38.6 WOH 1 1 1 1 2+	
T	╢-IIO.O +
-30.0 + 43.6 6 11 9 1 +	
-35.0 + 43.6 6 11 9 1	
-35.0 + GRAY LIMESTONE WITH	-115.0 +
-35.0 + 48.6 5 11 13 1 * 24 GRAY LIMESTONE WITH	
	-120.0 +
$ -40.0 \pm 53.6 = -40$	
T 55.6 6 7 7 7 7 1 1 1 1 1 1 1 1 1 1 1 1 1 1	
	-125.0 +
-45.0 + 58.6 6 7 10 I SS-I5 (RIVER BEND FORMATION)	
T	-130.0 +
30.0	
	-135.0 +
-55.0 + 68.6 7 9 13 1 = = = = = = = = =	
	$\ \cdot \ _{-14000} + \ \cdot \ _{-140000} + \ \cdot \ _{-140000} + \ \cdot \ _{-1400000} + \ \cdot \ _{-1400000000} + \ \cdot \ _{-14000000000000000000000000000000000000$
-60.0	-140.0 +
73.6 10 11 12 1 1 = - × 23 SS-16	
	-145.0 +
$\begin{vmatrix} -65.0 + 78.6 \\ 3 & 5 & 4 \\ 1 & 5 & 6 \end{vmatrix}$	

B-4055 Bridge No. 22 on SR 1124 over Branch of Newport River

HOLE#	SAMPLE#	PASS 10	PASS 40	PASS 200	CSESAND	FINESAND	SI	CL	LL	PI	CLASS	DEPTH	MOIST.	ORG.
EB1-B	SS-1	99	97	12	16.3	72.3	6.2	5.2	15	NP	A24(0)	8.2-9.7		
	SS-2	37	27	14	37.6	28.5	18.6	15.3	28	5	A1a(0)	18.2-19.7		
	SS-3	90	80	24	22.9	52.9	10.9	13.3	20	2	A24(0)	28.2-29.7		
	SS-4	92	88	55	6.4	46.9	25.4	21.3	32	3	A4(4)	33.2-34.7		
	SS-5	57	19	11	72.8	11.8	10.2	5.2	26	5	A1b(0)	43.2-44.7		
	SS-6	71	19	8	81.0	8.6	5.2	5.2	18	NP	A1b(0)	53.2-54.7		
	SS-7	54	15	6	80.8	9.7	2.3				A1b(0)	68.2-69.7		
	SS-8	87	80	40	17.1	40.0	31.6				A4(1)	83.2-84.7		
EB2-A	SS-9	100	95	16	15.3	70.5	6.2	8.0	20	NP	A24(0)	1.2-2.7		
	SS-10	100	74	0	69.9	29.3	1.2	2.0	14	NP	A3(0)	13.5-15.0		
	SS-11	35	19	9	55.4	19.7	6.8	18.1	20	NP	A1a(0)	18.6-20.1		
	SS-12	62	52	18	23.7	51.0	7.2	18.1	20	NP	A24(0)	23.6-25.1		
	SS-13	79	72	39	11.2	51.2	21.5	16.1	29	1	A4(0)	33.6-35.1		
	SS-14	59	20	10	74.9	9.2	5.8	10.0	21	3	A1b(0)	43.6-45.1		
	SS-15	81	19	8	83.8	7.0	0.8	8.4	20	NP	A1b(0)	58.6-60.1		
	SS-16	70	23	7	76.4	15.0	1.8	10.4	19	2	A1b(0)	73.6-75.1		
	SS-17	100	93	39	17.8	45.6					A4(0)	83.6-85.1		
B1-B	SS-18	73	36	21	55.4	18.6	7.6	18.4	LM	LM	A1b(0)	3.2-4.7		
	SS-19	67	55	17	31.2	45.4	7.0	16.4	17	4	A24(0)	11.2-12.7		
	SS-20	91	18	7	87.0	5.8	1.2	8.4	29	6	A1b(0)	16.2-17.7		
	SS-21	74	69	38	8.8	51.2	19.6	20.4	18	NP	A4(0)	26.2-27.7		
	SS-22	48	14	6	80.0	9.6	0.0	10.4	30	NP	A1a(0)	36.2-37.7		
	SS-23	81	29	12	74.6	10.6	2.4				A1b(0)	46.2-47.7		
	SS-24	70	16	7	83.4	7.6					A1b(0)	56.2-57.7		
	SS-25	99	93	45	15.4	42.2	12.0				A4(0)	66.2-67.7		
	SS-26	59	55	29	13.0	39.4	11.2				A24(0)	76.2-77.7		

^{*}LM= Lack of material

GEOTECHNICAL UNIT FIELD SCOUR REPORT

PROJECT: 33420.1.1 ID: B-4055 COUNTY: Carteret
DESCRIPTION(1): Bridge No. 22 on SR 1124 over Branch of Newport River
INFORMATION ON EXISTING BRIDGE Information obtained from:
BR. NO.: 22 BR. LENGTH: 90' NO. BENTS: 4 NO. BENTS IN: CHANNEL: 2 FLOODPLAIN: 2
FOUNDATION TYPE:Timber piles
EVIDENCE OF SCOUR(2):
ABUTMENTS OR END BENT SLOPES: Undermining of end walls and end slopes
INTERIOR BENTS: None noted
CHANNEL BED: None noted
CHANNEL BANKS: None noted
EXISTING SCOUR PROTECTION:
TYPE(3): Wooden and concrete end wall
EXTENT(4): 8 feet from outside edge of bridge
EFFECTIVENESS(5): Appears satisfactory
OBSTRUCTIONS(6) (DAMS,DEBRIS,ETC.): Debris in Channel
<u>DESIGN INFORMATION</u>
CHANNEL BED MATERIAL(7): Fine to coarse sand
CHANNEL BANK MATERIAL(8): Fine to coarse sand (SS-1)
CHANNEL BANK COVER(9): Wooded
FLOOD PLAIN WIDTH(10): 1500 +/- feet
FLOOD PLAIN COVER(11): Wooded

SHEET 10

DESIGN INFORMATION CONT.
STREAM IS:DEGRADING EQUILIBRIUM (12)
OTHER OBSERVATIONS AND COMMENTS:
P.
CHANNEL MIGRATION TENDENCY (13): Northeast toward end bent 2
GEOTECHNICALLY ADJUSTED SCOUR ELEVATIONS(14):
It is unlikely that overtopping scour will extend below an elevation of -6+/- feet at B1-B.
An elevation of -6+/- feet generally corresponds with the base of recent sediments and is 9+/- feet
higher than the theoretical scour elevation provided by the Hydraulics Unit.
REPORTED BY: 1 M West I DATE: 2/17/06

INSTRUCTIONS

- (1) GIVE THE DESCRIPTION OF THE SPECIFIC SITE, INCLUDING ROUTE NUMBER AND BODY OF WATER CROSSED.
- NOTE ANY EVIDENCE OF SCOUR AT THE EXISTING END BENTS OR ABUTMENTS (UNDERMINING, SLOUGHING, SCOUR LOCATIONS, DEGRADATIONS, ETC.)
- 3) NOTE ANY EXISTING SCOUR PROTECTION (RIR RAP, ETC.)
- (4) DESCRIBE THE EXTENT OF ANY EXISTING SCOUR PROTECTION.
- (5) DESCRIBE WHETHER OR NOT THE SCOUR PROTECTION APPEARS TO BE WORKING.
- (6) NOTE ANY DAMS, FALLEN TREES, DEBRIS AT BENTS, ETC.
- 7) DESCRIBE THE CHANNEL BED MATERIAL BASED ON OBSERVATION AND/OR SAMPLES.
- (8) DESCRIBE THE CHANNEL BANK MATERIAL BASED ON OBSERVATION AND/OR SAMPLES.
- (9) DESCRIBE THE BANK COVERING (GRASS, TREES, RIP RAP, NONE, ETC.)
- (10) GIVE THE APPROXIMATE FLOOD PLAIN WIDTH (ESTIMATE).
- (11) DESCRIBE THE FLOOD PLAIN COVERING (GRASS, TREES, CROPS, ETC.)
- (12) CHECK THE APPROPRIATE SPACE AS TO WHETHER THE STREAM IS DEGRADING OR AGGRADING.
- (13) DESCRIBE THE POTENTIAL OF THE BODY OF WATER TO MIGRATE LATERALLY DURING THE LIFE OF THE BRIDGE (APPROXIMATELY 100 YEARS).
- (14) GIVE THE GEOTECHNICALLY ADJUSTED SCOUR ELEVATION EXPECTED OVER THE LIFE OF THE BRIDGE (APPROXIMATELY 100 YEARS). THIS CAN BE GIVEN AS AN ELEVATION RANGE ACROSS THE SITE, OR ON A BENT BY BENT BASIS WHERE VARIATIONS EXIST. DISCUSS THE RELATIONSHIP BETWEEN THE HYDRAULICS THEORETICAL SCOUR AND THE GEOTECHNICALLY ADJUSTED SCOUR ELEVATION. IF THE GEOTECHNICALLY ADJUSTED SCOUR ELEVATION IS DEPENDENT ON SCOUR COUNTER MEASURES, EXPLAIN. (RIPRAP ARMORING ON SLOPES, ETC.) THE GEOTECHNICALLY ADJUSTED SCOUR ELEVATION IS BASED ON THE ERODABILITY OF MATERIALS WITH CONSIDERATION FOR JOINTING, FOLIATION, BEDDING ORIENTATION AND FREQUENCY, CORE RECOVERY PERCENTAGE, PERCENTAGE RQD, DIFFERENTIAL WEATHERING, SHEAR STRENGTH, OBSERVATIONS AT EXISTING STRUCTURES, OTHER TESTS DEEMED APPROPRIATE, AND OVERALL GEOLOGIC CONDITIONS AT THE SITE.

33420.1.1 B-4055 Carteret Co. Bridge on SR 1124 over Branch of Newport River



View Looking Northeast Toward End Bent 2