FRANKLIN COUNTY VICINITY MAP NEAREST SHIPPING POINT: LOUISBURG ON CSX R.R. APPROX. 17 MILES FROM PROJECT

DIVISION OF HIGHWAYS

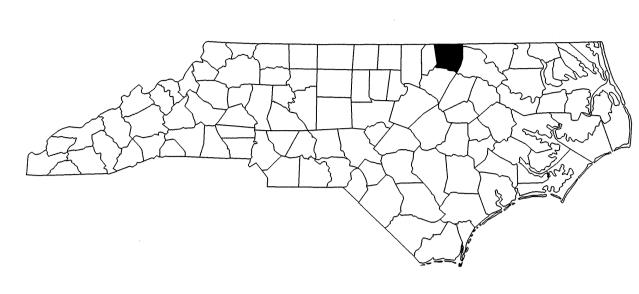
STATE OF NORTH CAROLINA

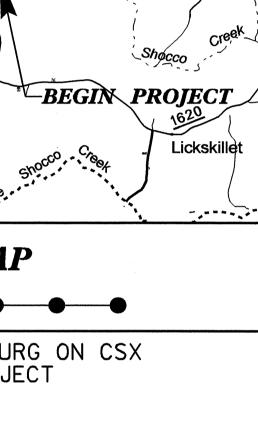
WARREN COUNTY

B-4312 BRZ-1613(2) 33649.1.1 P.E. BRZ-1613(2) 33649.2.1 R/W, UTIL. CONST. 33649.3.1 BRZ-1613(2)

LOCATION: BRIDGE NO. 42 OVER SHOCCO CREEK AND APPROACHES ON SR 1613 (SHOCCO SPRINGS ROAD)

TYPE OF WORK: GRADING, PAVING, DRAINAGE, AND STRUCTURE





-END PROJECT

-L- STA. 15 + 50.00 BEGIN TIP PROJECT B-4312 BEGIN BRIDGE

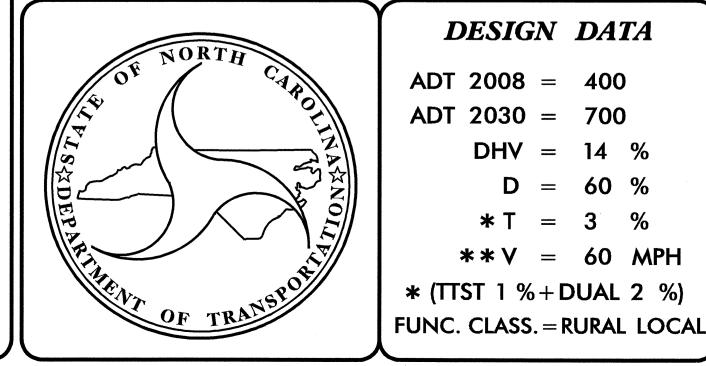
-L-STA. 19 + 98.00

-L- STA. 26+80.00 END TIP PROJECT B-4312

SR 1613 (SHOCCO SPRINGS ROAD)

END BRIDGE -L- STA. 20 + 98.00

** DESIGN EXCEPTIONS FOR HORIZONTAL ALIGNMENT, HORIZONTAL STOPPING SIGHT DISTANCE, VERTICAL ALIGNMENT, AND VERTICAL STOPPING SIGHT DISTANCE ARE REQUIRED.



DESIGN DATA

ADT 2008 = 400

ADT 2030 = 700DHV = 14 %

> D = 60 %= 3 %

**V = 60 MPH * (TTST 1 % + DUAL 2 %)

PROJECT LENGTH

LENGTH ROADWAY TIP PROJECT B-4312 = 0.195 MILE LENGTH STRUCTURE TIP PROJECT B-4312 = 0.019 MILE

TOTAL LENGTH TIP PROJECT B-4312 = 0.214 MILE

Prepared in the Office of: **DIVISION OF HIGHWAYS**

1000 Birch Ridge Dr., Raleigh NC, 27610

2006 STANDARD SPECIFICATIONS

LETTING DATE:

JULY 15, 2008

N.N. BULLOCK, P.E. PROJECT ENGINEER

A.K. PASCHAL, P.E. PROJECT DESIGN ENGINEER

STRUCTURE DESIGN UNIT

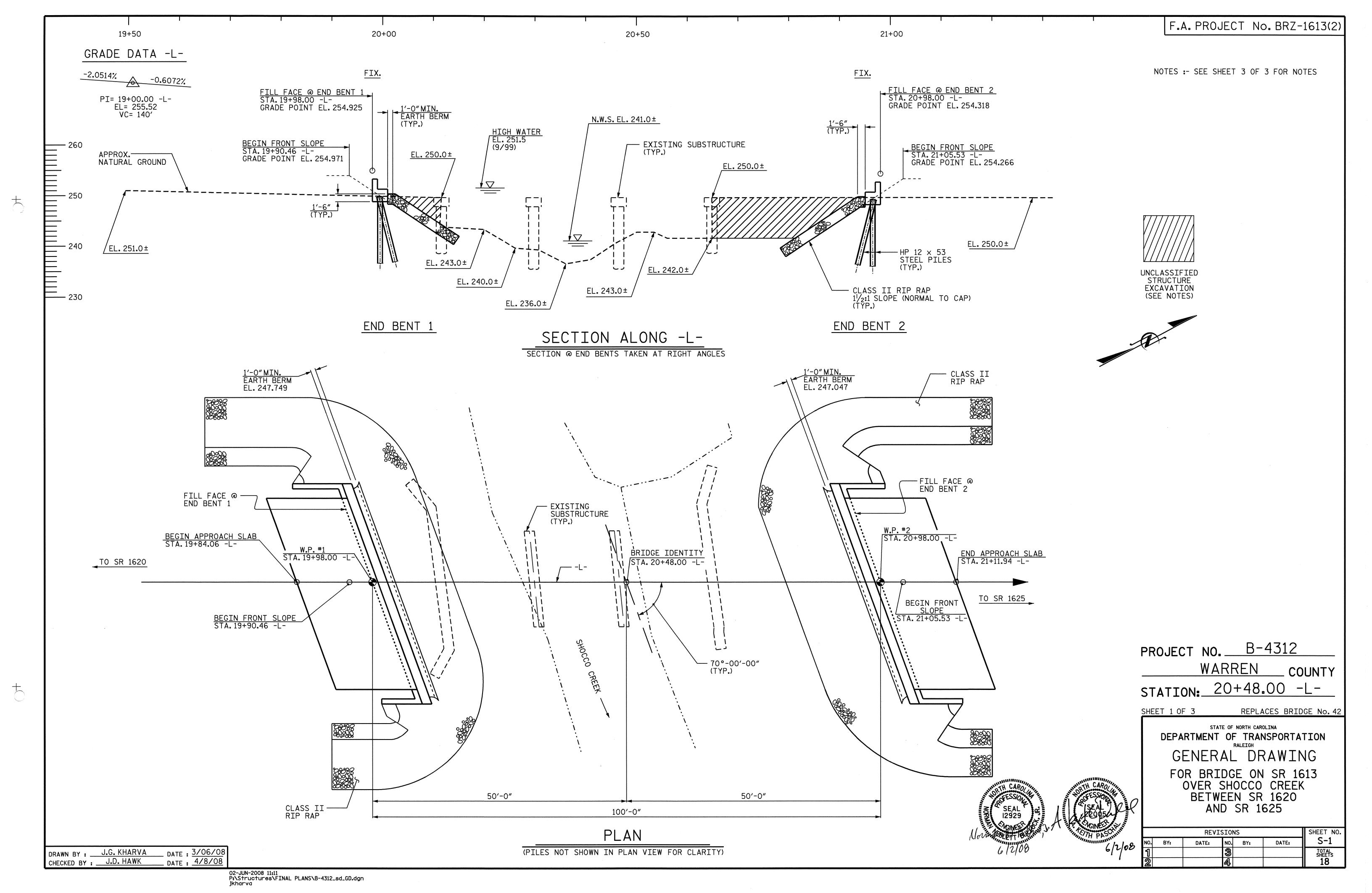
DIVISION OF HIGHWAYS STATE OF NORTH CAROLINA

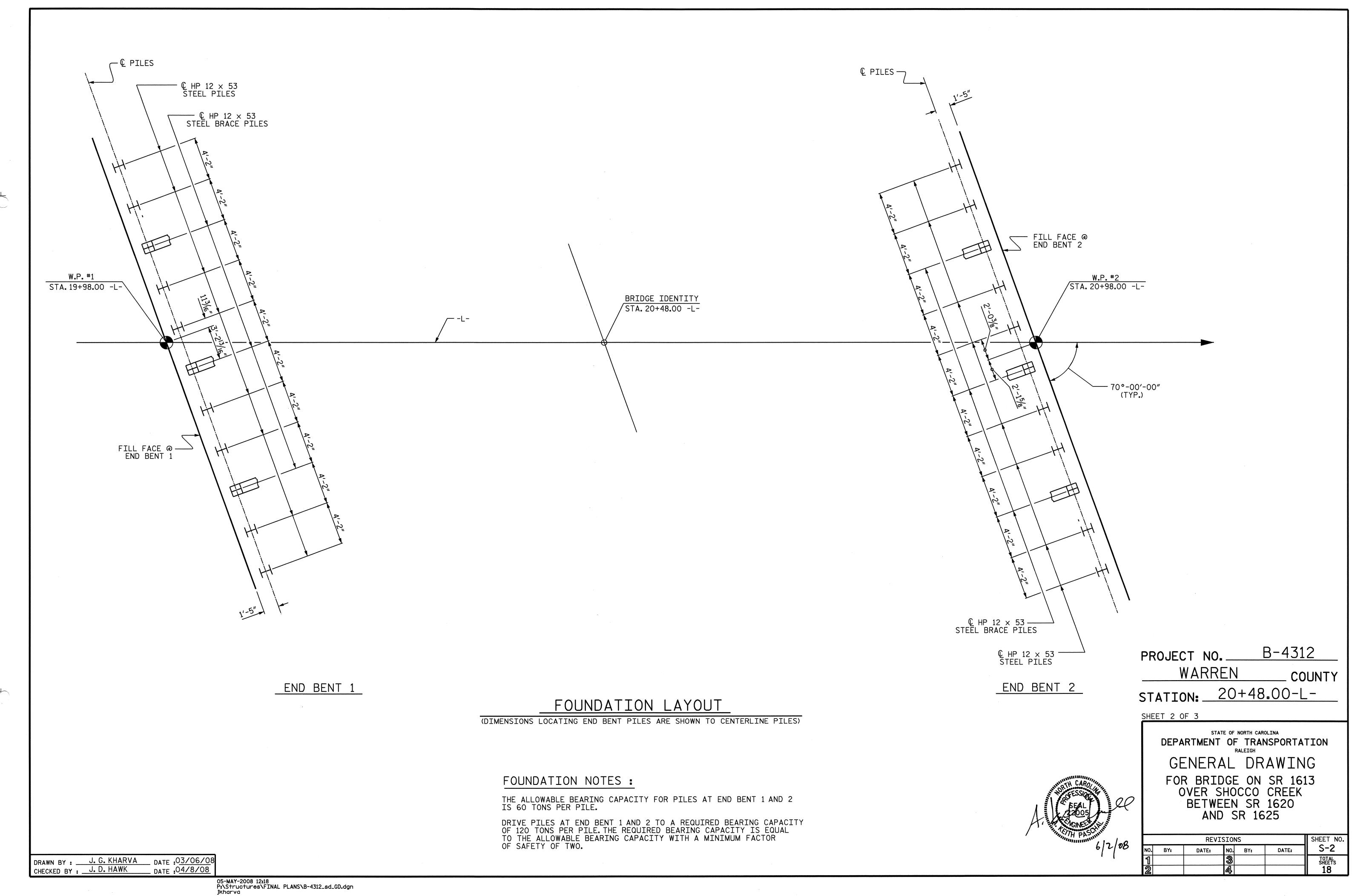
STATE DESIGN ENGINEER

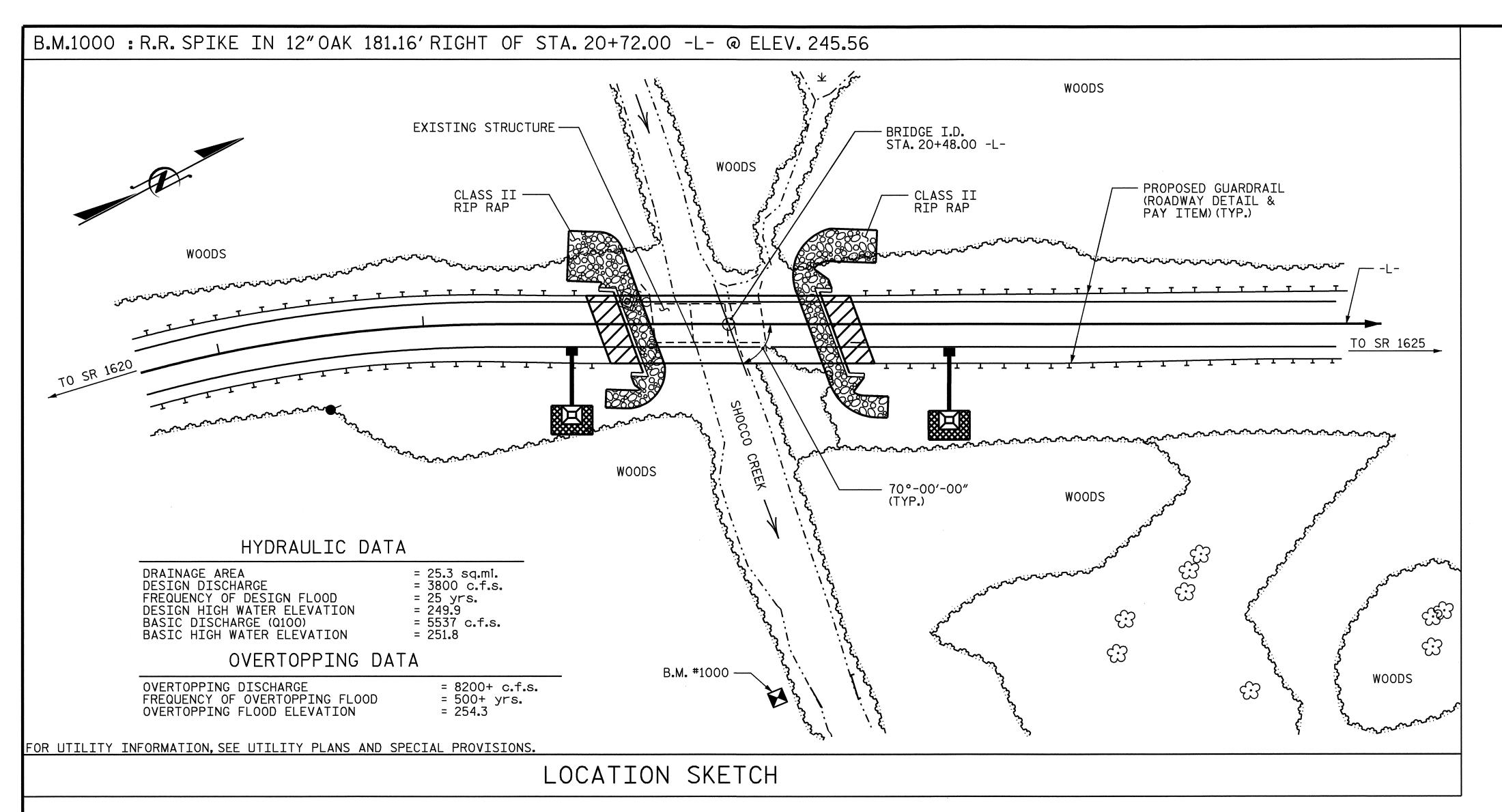
DEPARTMENT OF TRANSPORTATION FEDERAL HIGHWAY ADMINISTRATION

<u>APPROVED</u> DIVISION ADMINISTRATOR

05-MAY-2008 10:46 p:\struct@|\finalp@|\B-4312_ jkharva







	TOTAL BILL OF MATERIAL												
	REMOVAL OF EXISTING STRUCTURE	UNCLASSIFIED STRUCTURE EXCAVATION	CLASS A CONCRETE	BRIDGE APPROACH SLABS	REINFORCING STEEL		P 12 X 53 EEL PILES	CONCRETE BARRIER RAIL	RIP RAP CLASS II (2'-0" THICK)	FILTER FABRIC FOR DRAINAGE	ELASTOMERIC BEARINGS	PRE C	O" × 3'-3" STRESSED ONCRETE X BEAMS
	LUMP SUM	LUMP SUM	CU. YDS.	LUMP SUM	LBS.	NO.	LIN.FT.	LIN.FT.	TONS	SQ. YDS.	LUMP SUM	No.	LIN.FT.
SUPERSTRUCTURE				LUMP SUM				195.21			LUMP SUM	12	1171.25
END BENT 1		LUMP SUM	22.2		3431	11	220		170	189			
END BENT 2		LUMP SUM	22.7		3470	11	220		174	193			
TOTAL	LUMP SUM	LUMP SUM	44.9	LUMP SUM	6901	22	440	195.21	344	382	LUMP SUM	12	1171.25

NOTES

ASSUMED LIVE LOAD = HS 20 OR ALTERNATE LOADING, EXCEPT THAT THE BOX BEAMS HAVE BEEN DESIGNED FOR HS 25.

FOR OTHER DESIGN DATA AND GENERAL NOTES, SEE SHEET SN.

FOR EROSION CONTROL MEASURES SEE EROSION CONTROL PLANS.

THIS BRIDGE HAS BEEN DESIGNED BY THE STRENGTH DESIGN METHOD AS SPECIFIED IN AASHTO STANDARD SPECIFICATIONS.

THE EXISTING STRUCTURE CONSISTING OF THREE 1 @ 18'-6",1 @ 16'-9" AND 1 @ 18-4" TIMBER DECK ON TIMBER JOISTS SPANS WITH A CLEAR ROADWAY WIDTH OF 19'-1" SUPPORTED BY TIMBER ABUTMENTS AND TIMBER CAPS ON TIMBER PILES AND LOCATED AT THE PROPOSED STRUCTURE SHALL BE REMOVED. THE EXISTING BRIDGE IS PRESENTLY POSTED BELOW THE LEGAL LOAD LIMIT.

REMOVAL OF THE EXISTING BRIDGE SHALL BE PERFORMED SO AS NOT TO ALLOW DEBRIS TO FALL INTO THE WATER. THE CONTRACTOR SHALL REMOVE THE BRIDGE AND SUBMIT PLANS FOR DEMOLITION IN ACCORDANCE WITH ARTICLE 402-2 OF THE STANDARD SPECIFICATIONS.

THE SUBSTRUCTURE OF THE EXISTING BRIDGE INDICATED ON THE PLANS IS FROM THE BEST INFORMATION AVAILABLE. SINCE THIS INFORMATION IS SHOWN FOR THE CONVENIENCE OF THE CONTRACTOR, THE CONTRACTOR SHALL HAVE NO CLAIM WHATSOEVER AGAINST THE DEPARTMENT OF TRANSPORTATION FOR ANY DELAYS OR ADDITIONAL COST INCURRED BASED ON DIFFERENCES BETWEEN THE EXISTING BRIDGE SUBSTRUCTURE SHOWN ON THE PLANS AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

THE MATERIAL SHOWN IN THE CROSS-HATCHED AREA ON SHEET S-1 SHALL BE EXCAVATED FOR A DISTANCE OF 30 FT. EACH SIDE OF CENTERLINE ROADWAY AS DIRECTED BY THE ENGINEER. THE THIS WORK WILL BE PAID FOR AT THE CONTRACT LUMP SUM PRICE FOR UNCLASSIFIED STRUCTURE EXCAVATION.

ASPHALT WEARING SURFACE IS INCLUDED IN ROADWAY QUANTITY ON ROADWAY PLANS.

THIS BRIDGE HAS BEEN DESIGNED IN ACCORDANCE WITH THE REQUIREMENTS OF THE AASHTO STANDARD SPECIFICATIONS FOR SEISMIC DESIGN OF HIGHWAY BRIDGES FOR SEISMIC PERFORMANCE CATEGORY A.

THE CONTRACTOR SHALL PROVIDE INDEPENDENT ASSURANCE SAMPLES OF REINFORCING STEEL AS FOLLOWS: FOR PROJECTS REQUIRING UP TO 400 TONS OF REINFORCING STEEL, ONE 30 INCH SAMPLE OF EACH SIZE BAR USED, AND FOR PROJECTS REQUIRING OVER 400 TONS OF REINFORCING STEEL, TWO 30 INCH SAMPLES OF EACH SIZE BAR USED. THE BARS FROM WHICH THE SAMPLES ARE TAKEN MUST THEN BE SPLICED WITH REPLACEMENT BARS OF THE SIZE AND LENGTH OF THE SAMPLE, PLUS A MINIMUM LAP SPLICE OF THIRTY BAR DIAMETERS.

THIS STRUCTURE HAS BEEN DESIGNED IN ACCORDANCE WITH HEC 18, "EVALUATING SCOUR AT BRIDGES", MAY, 2001.

FOR SUBMITTAL OF WORKING DRAWINGS, SEE SPECIAL PROVISIONS.

FOR FALSEWORK AND FORMWORK, SEE SPECIAL PROVISIONS.

FOR CRANE SAFETY, SEE SPECIAL PROVISIONS.

FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.

FOR PRESTRESSED CONCRETE MEMBERS, SEE SPECIAL PROVISIONS.

PROJECT NO. B-4312

WARREN COUNTY

STATION: 20+48.00 -L-

SHEET 3 OF 3

DEPARTMENT OF TRANSPORTATION

GENERAL DRAWING

FOR BRIDGE ON SR 1613

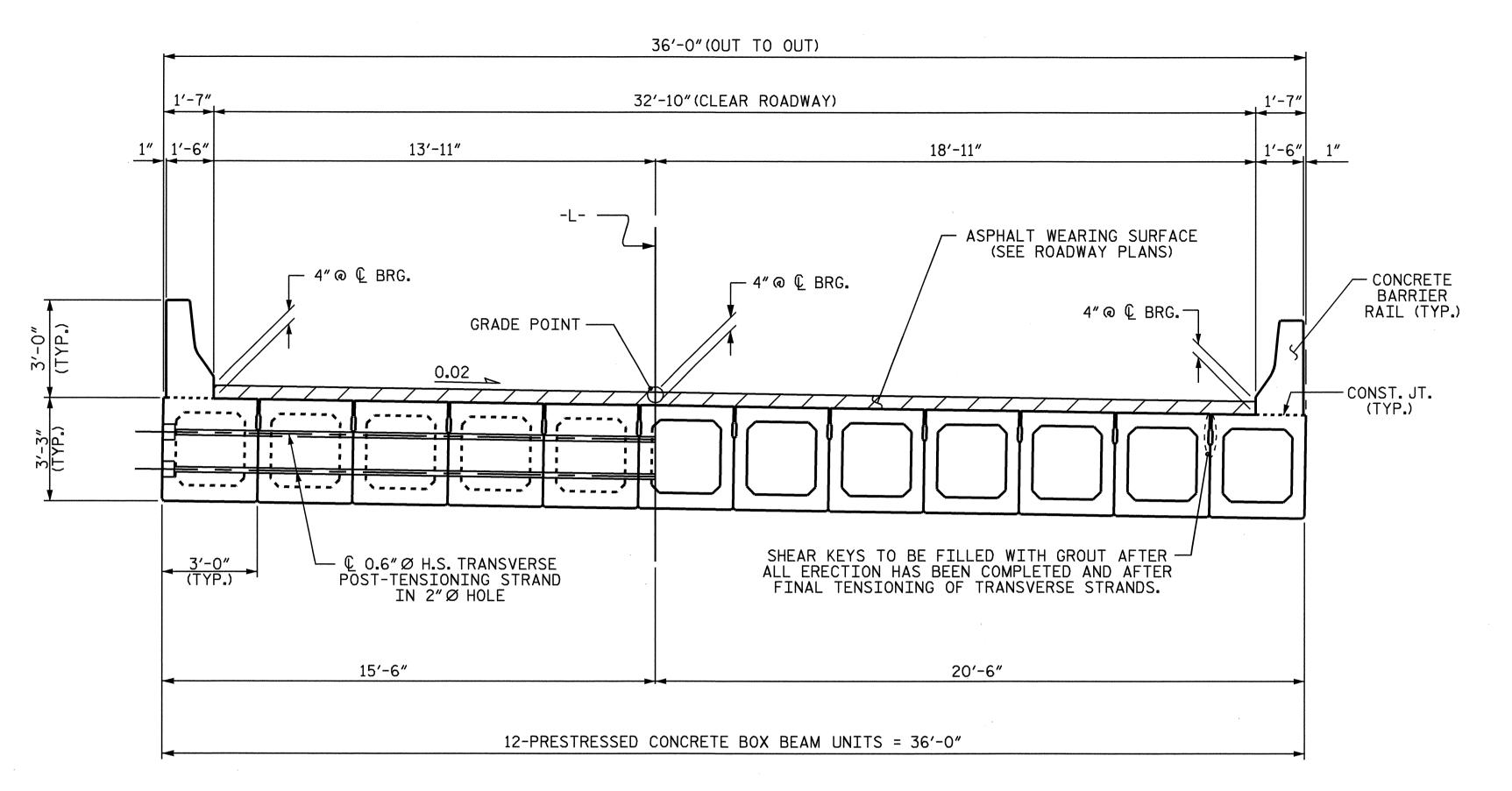
OVER SHOCCO CREEK

BETWEEN SR 1620

AND SR 1625

		SHEET NO.				
NO.	BY:	DATE:	NO.	BY:	DATE:	S-3
1			3			TOTAL SHEETS
2			A			ll 18

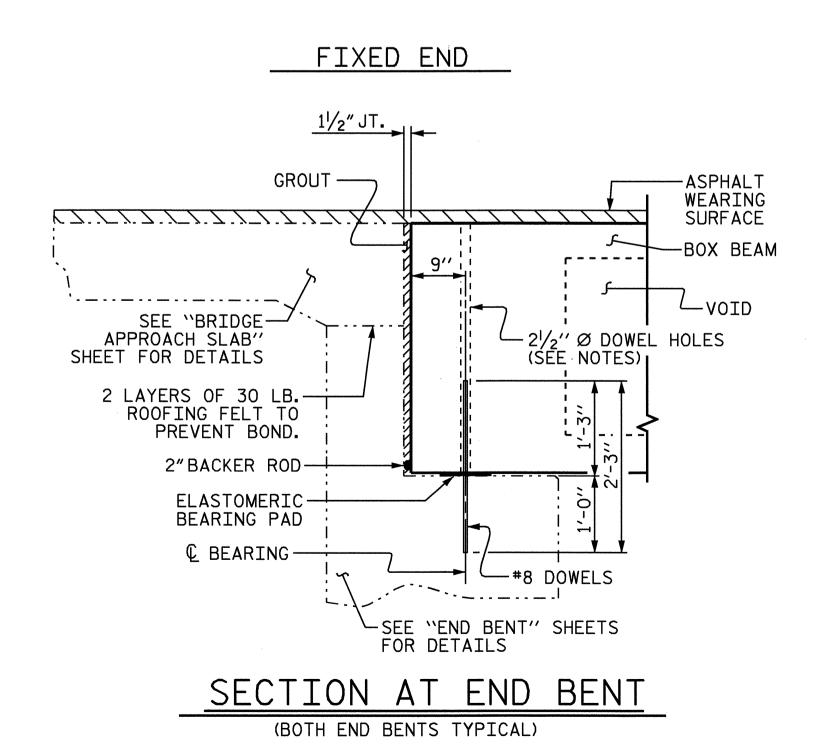
DRAWN BY: J.G. KHARVA DATE: 3/06/08
CHECKED BY: J.D. HAWK DATE: 4/06/08



HALF SECTION @ DIAPHRAGMS

HALF SECTION @ VOIDS

TYPICAL SECTION



ASSEMBLED BY : J. G. KHARVA DATE: 03-06-08 CHECKED BY : J. D. HAWK DATE : 04-15-08 ADDED 7/II/05R REV. 5/I/06R KMM/GM DRAWN BY: TLA 5/05 CHECKED BY: GM 6/05

NOTES

ALL PRESTRESSING STRANDS SHALL BE 7-WIRE LOW RELAXATION GRADE 270 STRANDS AND SHALL CONFORM TO AASHTO M203 EXCEPT FOR SAMPLING REQUIREMENTS WHICH SHALL BE IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

ALL REINFORCING STEEL CAST WITH THE BOX BEAM SECTIONS SHALL BE GRADE 60 AND SHALL BE INCLUDED IN THE UNIT PRICE BID FOR PRESTRESSED CONCRETE BOX BEAMS.

RECESSES FOR TRANSVERSE STRANDS SHALL BE GROUTED AFTER THE TENSIONING OF THE STRANDS.

THE $2^{1}\!/_{2}$ $^{\prime\prime}$ $^{\prime\prime}$ DOWEL HOLES AT FIXED ENDS OF BOX BEAM SECTIONS SHALL BE FILLED WITH NON-SHRINK GROUT.

THE 2" Ø BACKER ROD SHALL CONFORM TO THE REQUIREMENTS OF TYPE M BOND BREAKER. SEE SECTION 1028 OF THE STANDARD SPECIFICATIONS.

THE TRANSFER OF LOAD FROM THE ANCHORAGES TO THE BOX BEAM UNIT SHALL BE DONE WHEN THE CONCRETE HAS REACHED A COMPRESSIVE STRENGTH OF NOT LESS THAN 5600 PSI.

ALL REINFORCING STEEL IN BARRIER RAILS SHALL BE EPOXY COATED.

PRESTRESSING STRANDS SHALL BE CUT FLUSH WITH THE BOX BEAM UNIT ENDS.

APPLY EPOXY PROTECTIVE COATING TO BOX BEAM UNIT ENDS.

VERTICAL GROOVED CONTRACTION JOINTS, 1/2" IN DEPTH, SHALL BE TOOLED IN ALL EXPOSED FACES OF THE BARRIER RAIL AND IN ACCORDANCE WITH ARTICLE 825-10(B) OF THE STANDARD SPECIFICATIONS. A VERTICAL CONTRACTION JOINT SHALL BE LOCATED AT EACH THIRD POINT BETWEEN BARRIER RAIL EXPANSION JOINTS. ONLY ONE CONTRACTION JOINT IS REQUIRED AT MIDPOINT OF BARRIER RAIL SEGMENTS LESS THAN 20 FEET IN LENGTH AND NO CONTRACTION JOINTS ARE REQUIRED FOR THOSE SEGMENTS LESS THAN 10 FEET IN LENGTH.

THE LOCATION OF THE VOID DRAINS MAY BE SHIFTED SLIGHTLY WHERE NECESSARY TO CLEAR PRESTRESSING STRANDS OR TRANSVERSE REINFORCING

ELASTOMER IN ALL BEARINGS SHALL BE 60 DUROMETER HARDNESS.

FOR PRESTRESSED CONCRETE MEMBERS, SEE SPECIAL PROVISIONS.

FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.

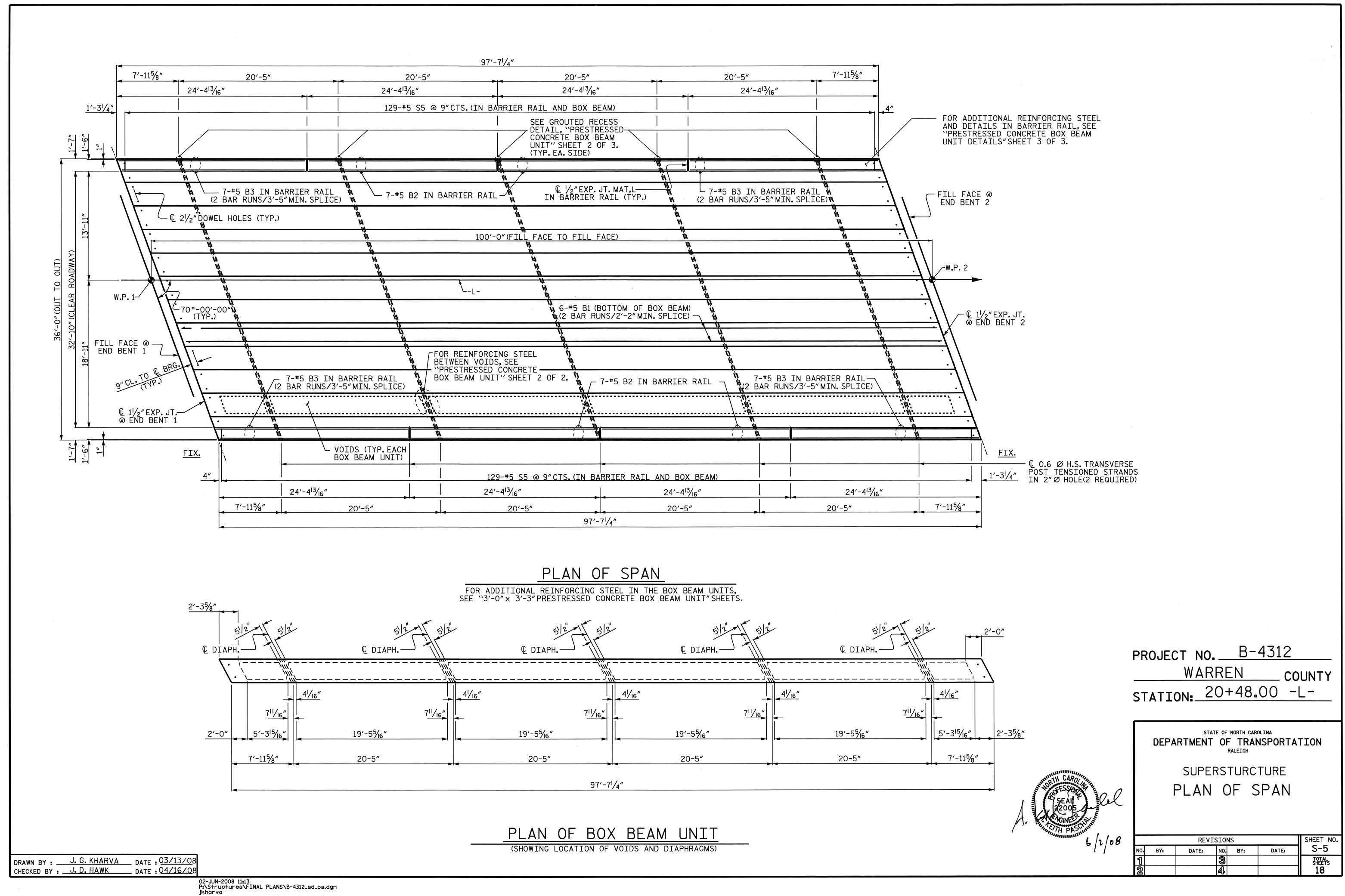
B-4312 PROJECT NO.____ WARREN COUNTY 20+48.00 -L-STATION:_

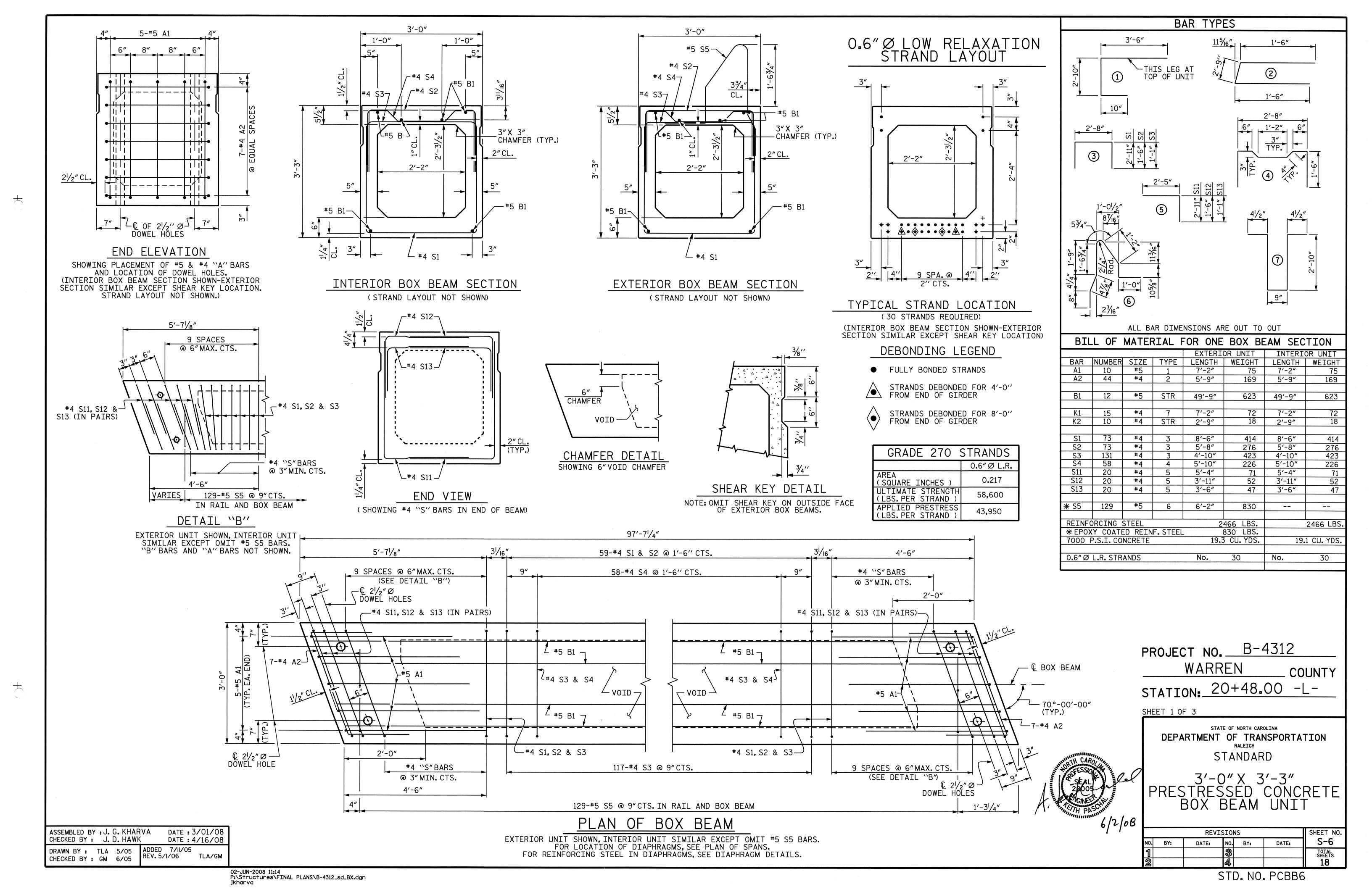
STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION STANDARD

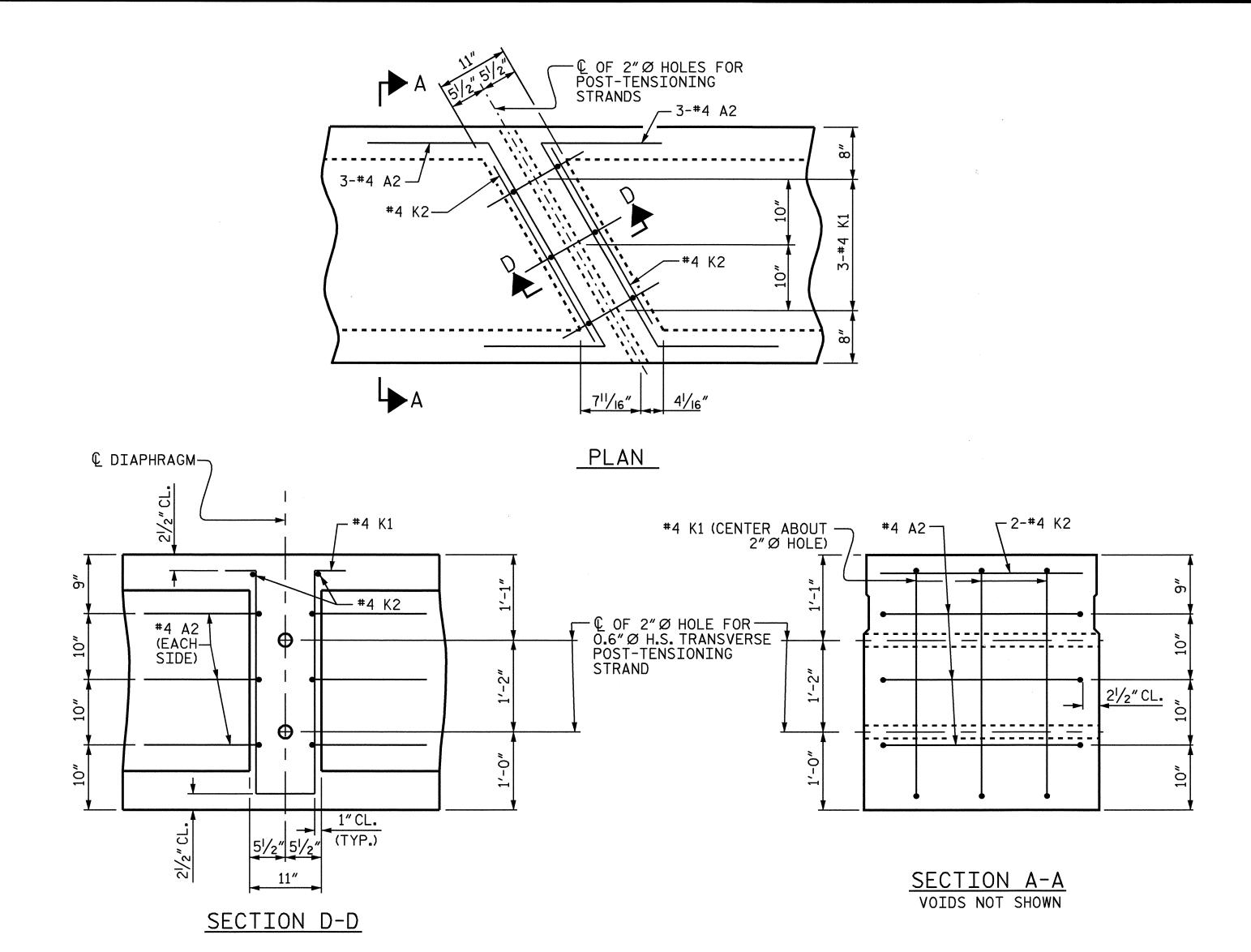
3'-0" X 3'-3" PRESTRESSED CONCRETE BOX BEAM UNIT

REVISIONS	SHEET NO.
NO. BY: DATE: NO. BY: DATE:	S-4
1 3	TOTAL SHEETS
2 4	18

STD. NO. PCBB1

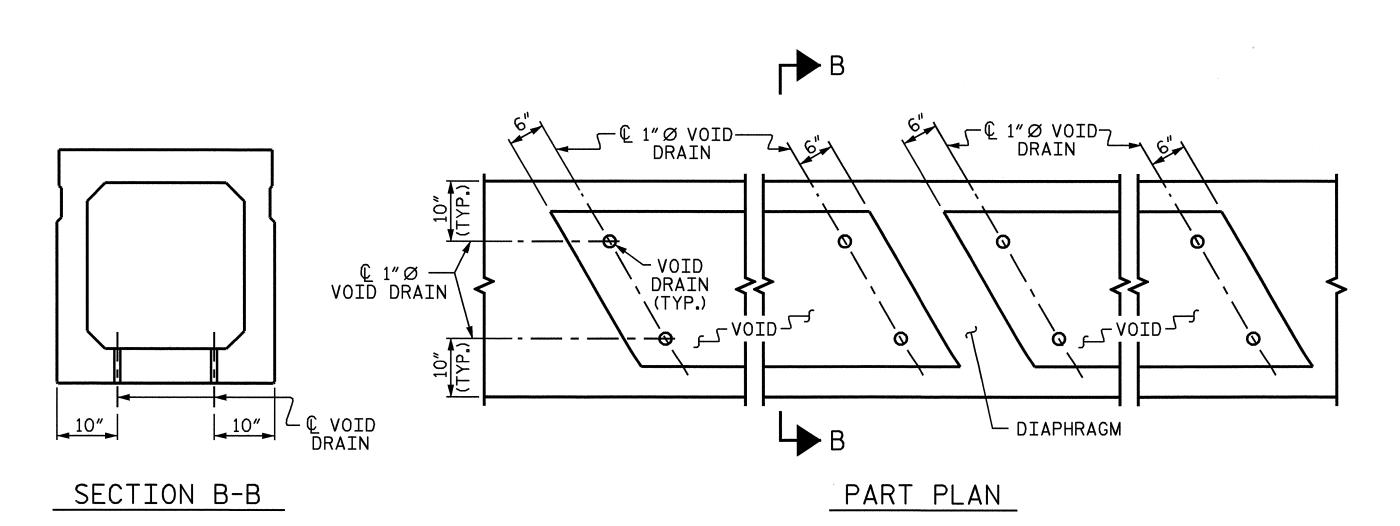






DOUBLE DIAPHRAGM DETAILS

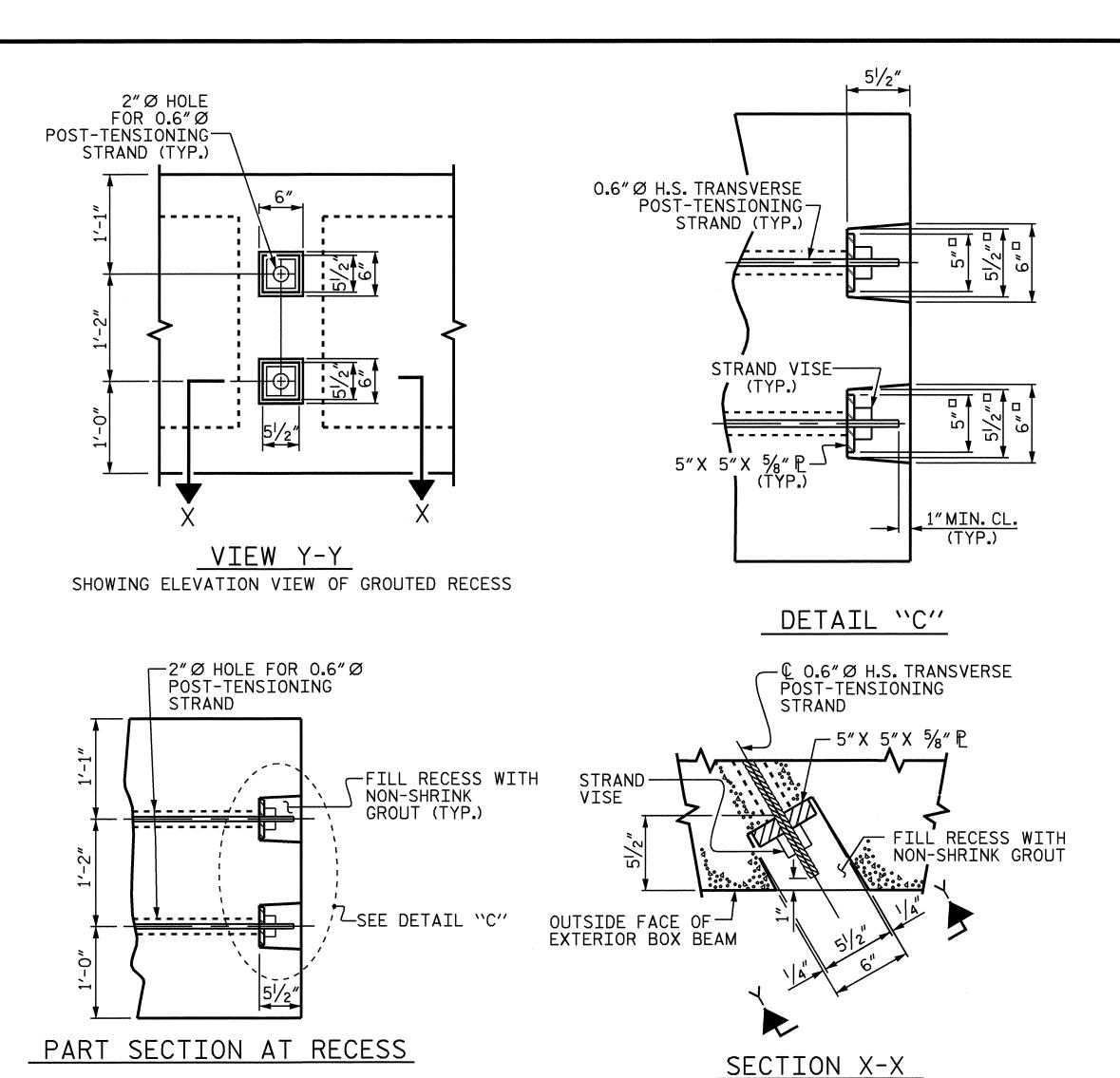
#4 "S" BARS NOT SHOWN. #4 "S" BARS MAY BE SHIFTED SLIGHTLY TO CLEAR 2" Ø HOLE.



VOID DRAIN DETAILS

(DIMENSIONS SHOWN ARE TYPICAL FOR EACH VOID)

ASSEMBLED BY : J. G. KHARVA DATE : 2/29/08 CHECKED BY : J. D. HAWK DATE : 4/16/08 DRAWN BY: TLA 5/05 ADDED 7/II/05 REV. 5/I/06 TLA/GM



GROUTED RECESS DETAIL AT END OF POST-TENSIONED STRANDS OF EXTERIOR BOX BEAM

DEAD LOAD DEFLECTION	AND CAMBER
	3'-0"× 3'-3"
	0.6"Ø L.R. STRAND
	SPAN
CAMBER (BEAM ALONE IN PLACE)	311/16"
DEFLECTION DUE TO SUPERIMPOSED DEAD LOAD **	↓ ¹³ / ₁₆ "
FINAL CAMBER	27/8"

** INCLUDES FUTURE ASPHALT WEARING SURFACE

PROJECT NO. B-4312 WARREN COUNTY STATION: 20+48.00 -L-

SHOWING PLAN VIEW OF GROUTED RECESS

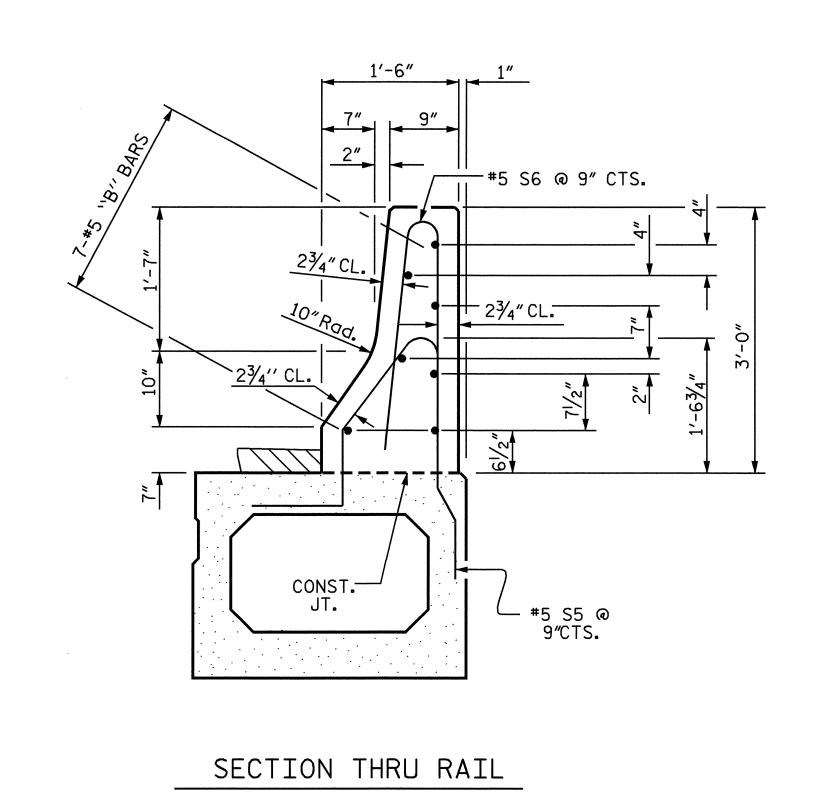
SHEET 2 OF 3

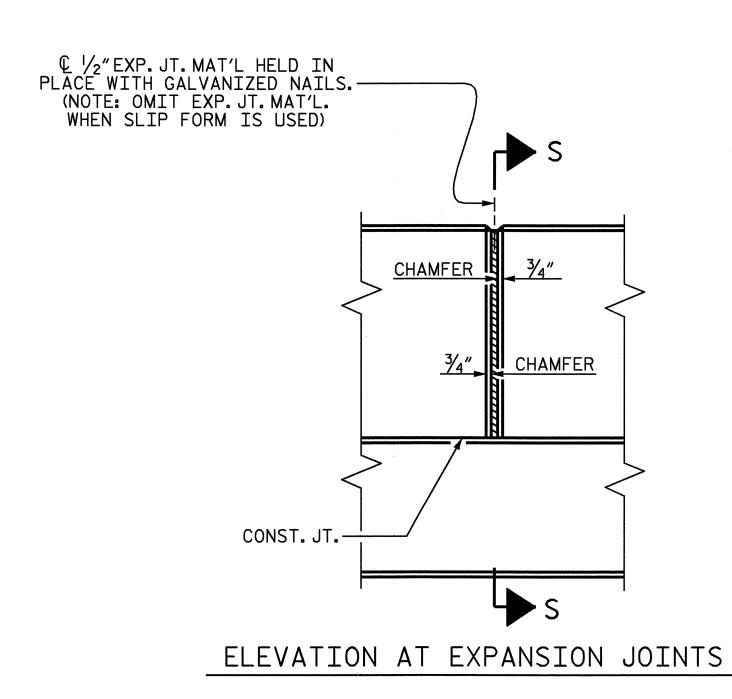
STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION STANDARD

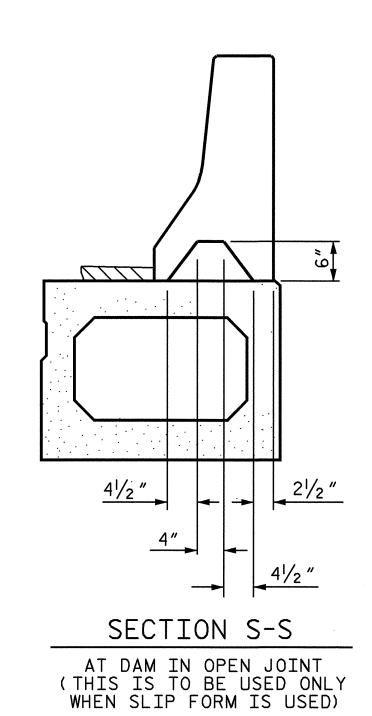
3'-0" X 3'-3" PRESTRESSED CONCRETE BOX BEAM UNIT

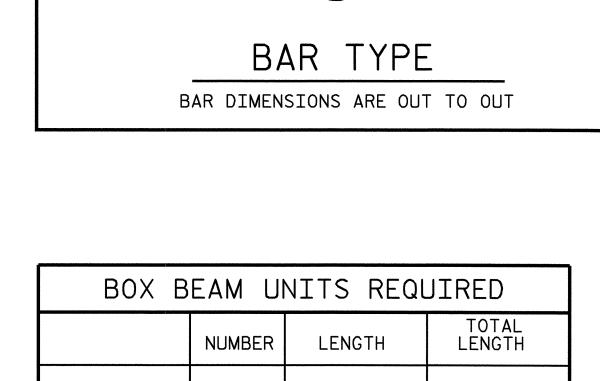
	SHEET N					
NO.	BY:	DATE:	NO.	BY:	DATE:	S-7
1			3			TOTAL SHEETS
2			4			ll 18

STD. NO. PCBB7









12

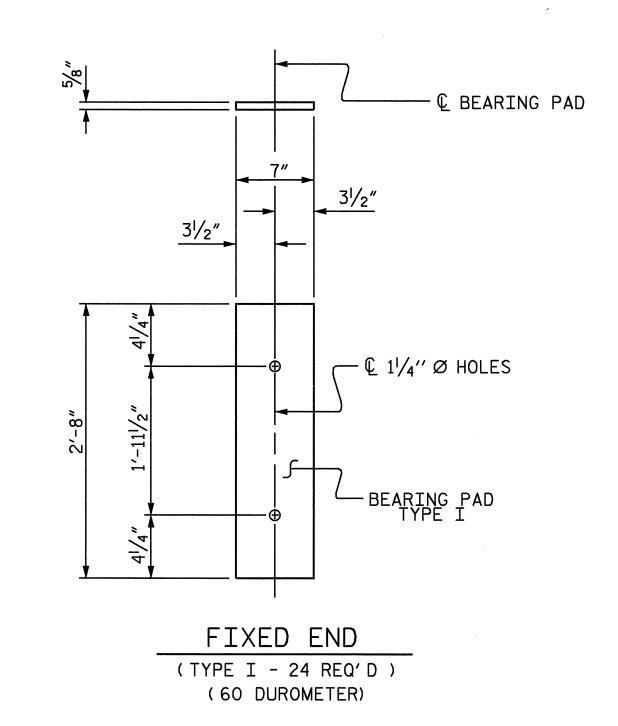
TOTAL

97'-71/4"

1171′-3″

71/4"

BARRIER RAIL DETAILS



ELASTOMERIC BEARING DETAILS

BILL OF MATERIAL FOR CONCRETE BARRIER RAIL							
BAR	TOTAL NO.	SIZE	TYPE	LENGTH	WEIGHT		
N = 5	00	#_	CTD	04/ 0#	704		
★ B2	28	#5	STR	24'-0"	701		
★ B3	56	#5	STR	14'-0"	818		
∗ S6	258	#5	2	5′-10″	1570		
* EPOXY COATED REINFORCING STEEL LBS. 3089							
CLASS AA CONCRETE CU.YDS. 23.2							
TOT	AL LIN.FT.OF CO	NCRETE BA	ARRIER RA	IL 195.21			

PROJECT NO. B-4312 WARREN _ COUNTY 20+48.00 -L-STATION:_

SHEET 3 OF 3

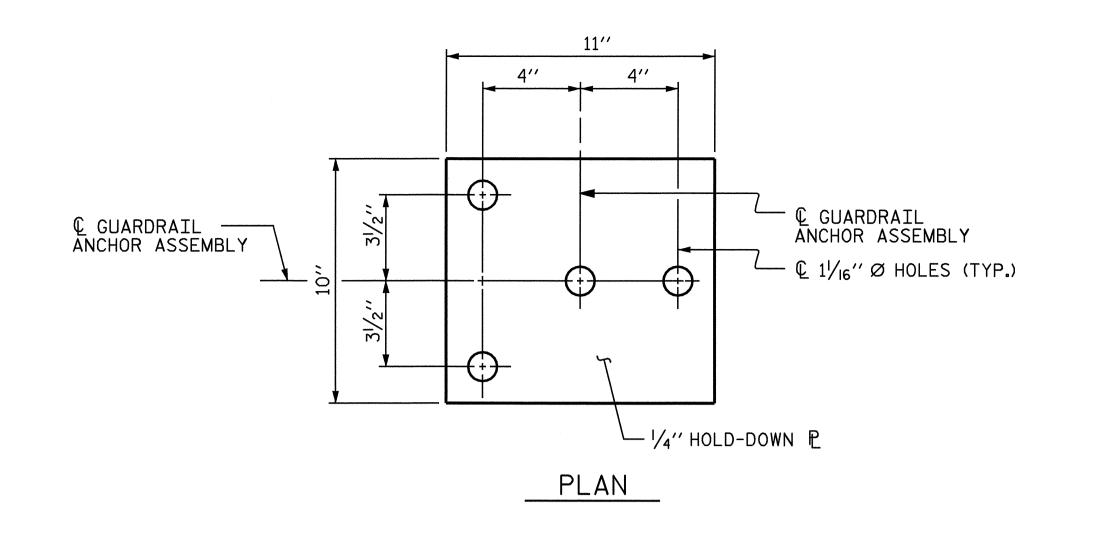
STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION
RALEIGH STANDARD 3'-0" X 3'-3" PRESTRESSED CONCRETE BOX BEAM UNIT DETAILS

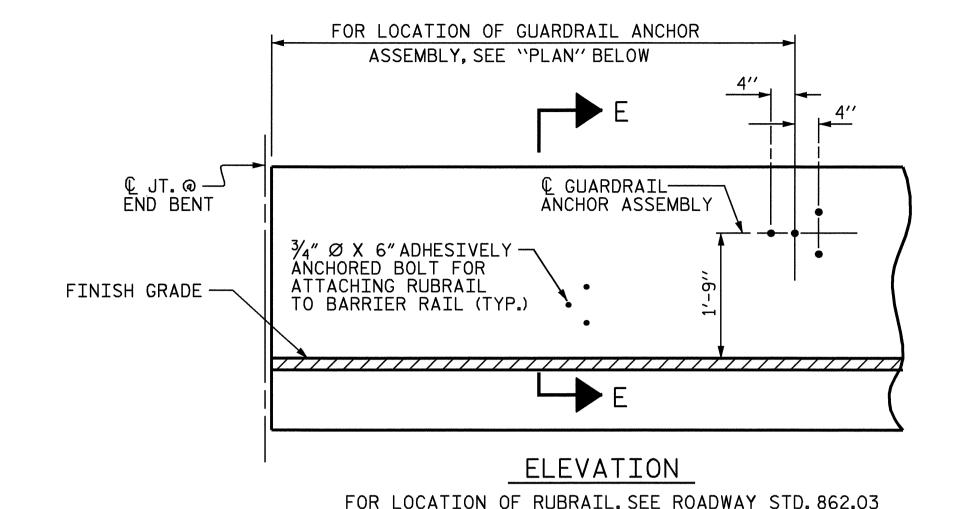
REVISIONS SHEET NO. NO. BY: S-8 DATE: TOTAL SHEETS 18

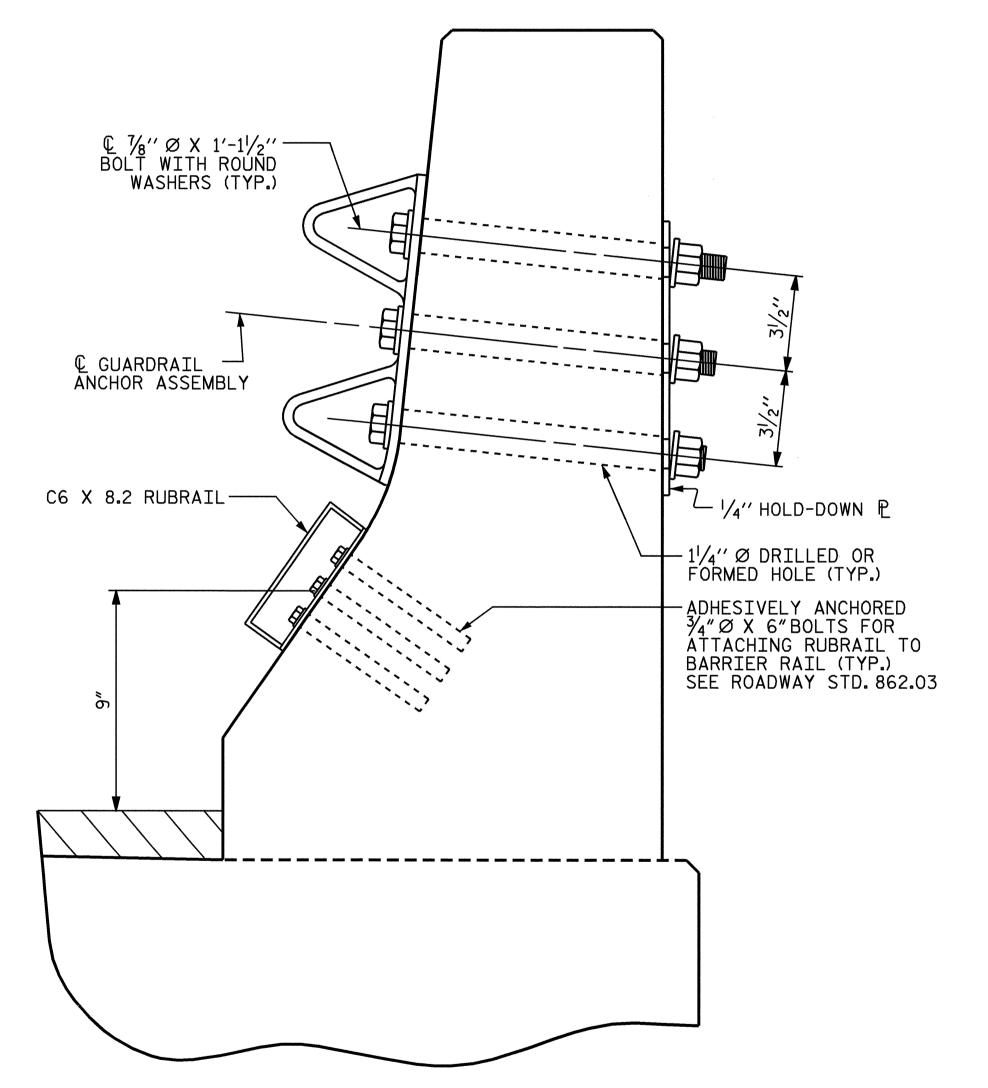
ASSEMBLED BY: J.G.KHARVA CHECKED BY: J.D.HAWK DATE : 2/26/08 DATE : 4/16/08 DRAWN BY: TLA 5/05 ADDED 7/II/05R REV. 5/I/06 TLA/GM

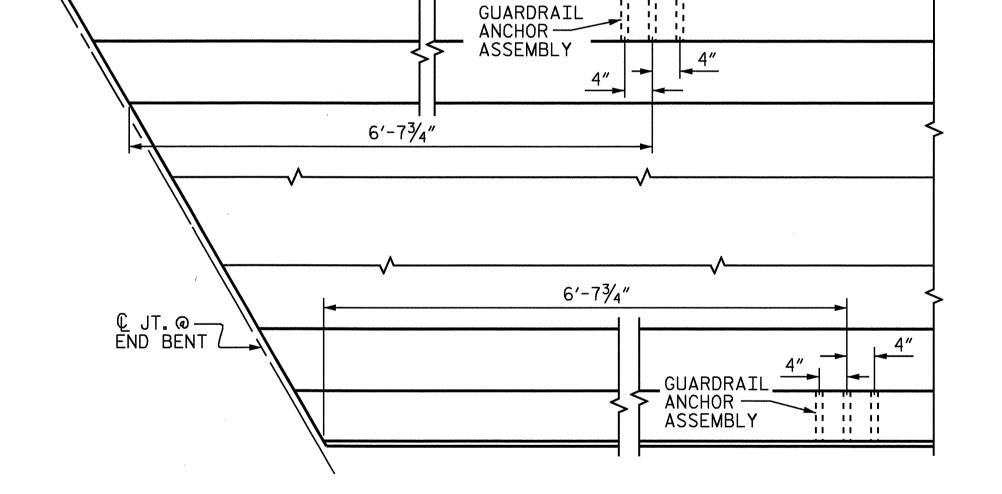
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STD. NO. PCBB8









LOCATION OF ANCHORS FOR GUARDRAIL

END BENT 1 SHOWN, END BENT 2 SIMILAR.

NOTES

THE GUARDRAIL ANCHOR ASSEMBLY SHALL CONSIST OF A $\frac{1}{4}$ " HOLD DOWN PLATE AND 4 - $\frac{7}{8}$ " Ø BOLTS WITH NUTS AND WASHERS, RUBRAIL, AND ADHESIVELY ANCHORED BOLTS.

THE HOLD-DOWN PLATE SHALL CONFORM TO AASHTO M270 GRADE 36. AFTER FABRICATION, THE HOLD-DOWN PLATE SHALL BE HOT-DIP GALVANIZED IN ACCORDANCE WITH AASHTO M111.

BOLTS SHALL CONFORM TO THE REQUIREMENTS OF ASTM A307 AND NUTS SHALL CONFORM TO THE REQUIREMENTS OF AASHTO M291. BOLTS, NUTS AND WASHERS SHALL BE GALVANIZED. (AT THE CONTRACTOR'S OPTION, STAINLESS STEEL BOLTS, NUTS AND WASHERS MAY BE USED AS AN ALTERNATE FOR THE 1/8" Ø GALVANIZED BOLTS, NUTS AND WASHERS. THEY SHALL CONFORM TO OR EXCEED THE MECHANICAL REQUIREMENTS OF ASTM A307. THE USE OF THIS ALTERNATE SHALL BE APPROVED BY THE ENGINEER.)

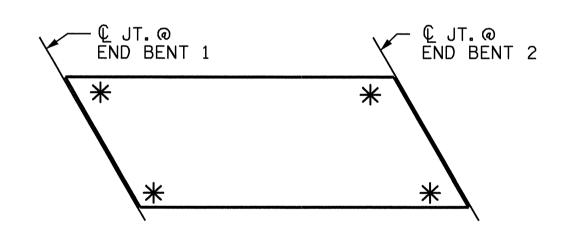
THE GUARDRAIL ANCHOR ASSEMBLY IS REQUIRED AT ALL POINTS WHERE APPROACH GUARDRAIL IS TO BE ATTACHED TO THE END OF BARRIER RAIL.FOR POINTS OF ATTACHMENT, SEE SKETCH.

AFTER INSTALLATION, THE EXPOSED THREAD OF THE BOLT SHALL BE BURRED WITH A SHARP POINTED TOOL.

THE COST OF THE GUARDRAIL ANCHOR ASSEMBLY SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR CONCRETE BARRIER RAIL.

THE 1 1/4" Ø HOLES SHALL BE FORMED OR DRILLED WITH A CORE BIT. IMPACT TOOLS WILL NOT BE PERMITTED. ANY CONCRETE DAMAGED BY THIS WORK SHALL BE REPAIRED TO THE SATISFACTION OF THE ENGINEER.

THE C6 X 8.2 RUBRAIL IS TO BE ADHESIVELY ANCHORED TO THE RAIL USING THREE 3/4" Ø X 6"BOLTS WITH WASHERS.LEVEL 1 FIELD TESTING IS REQUIRED, AND THE YIELD LOAD OF THE ANCHOR BOLTS IS 12 KIPS.FOR ADHESIVELY ANCHORED ANCHOR BOLTS OR DOWELS, SEE SPECIAL PROVISIONS. SEE ROADWAY STANDARD 862.03 FOR DETAILS AND LOCATION OF THE RUBRAIL.



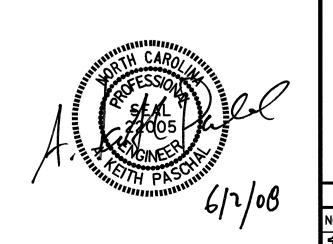
SKETCH SHOWING POINTS OF ATTACHMENTS

* DENOTES GUARDRAIL ANCHOR ASSEMBLY

PROJECT NO. B-4312

WARREN COUNTY

STATION: 20+48.00 -L-



STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH

GUARDRAIL ANCHORAGE FOR BARRIER RAIL

	SHEET NO.				
BY:	DATE:	ISION	BY:	DATE:	S-9
		3			TOTAL SHEETS
		4			18

02-JUN-2008 11:15
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Jkharva

ASSEMBLED BY : J. G. KHARVA

DRAWN BY: TLA 5/06 CHECKED BY: GM 5/06

J. D. HAWK

CHECKED BY :

DATE: 3/6/08
DATE: 4/16/08

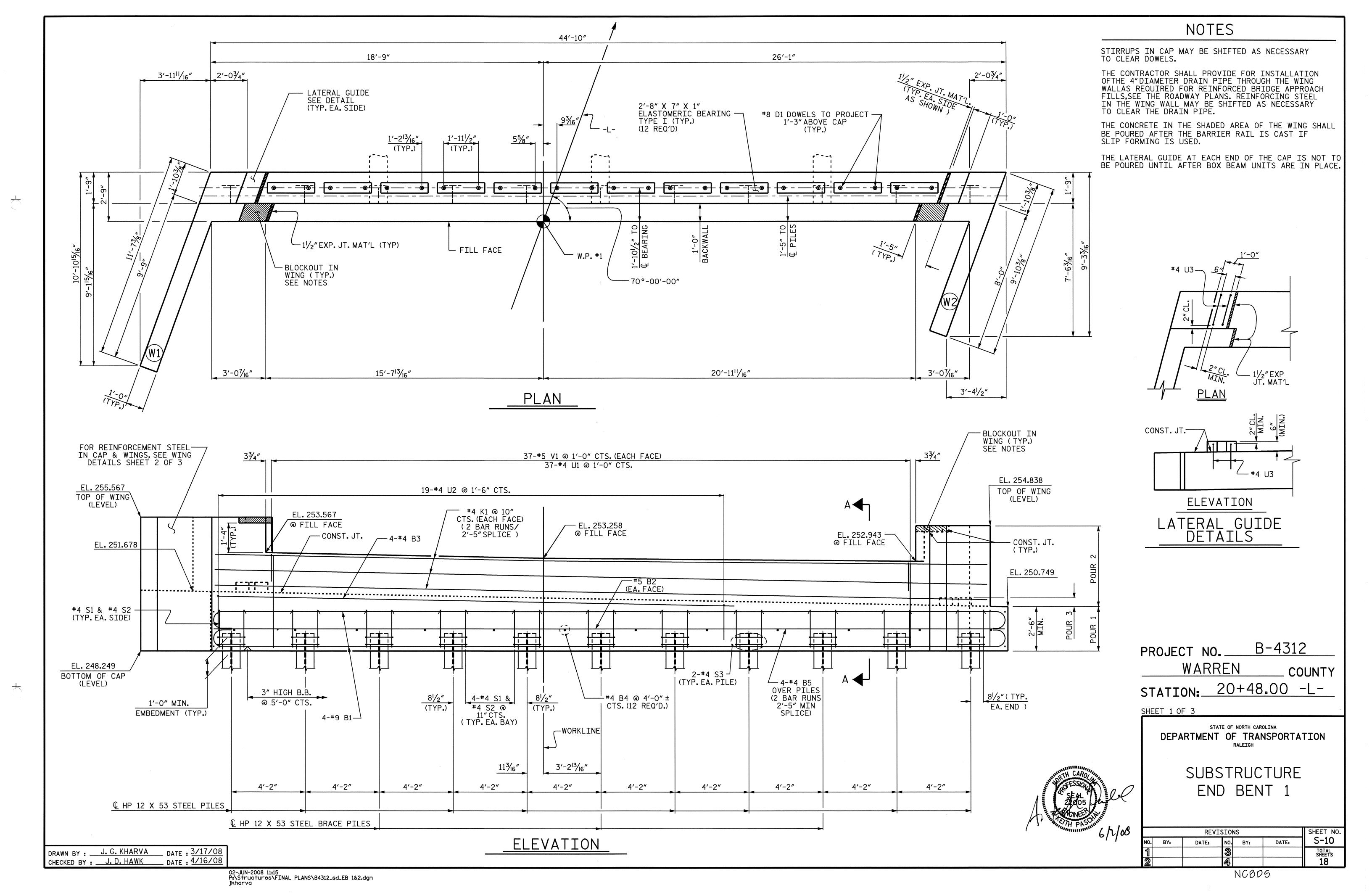
SECTION E-E

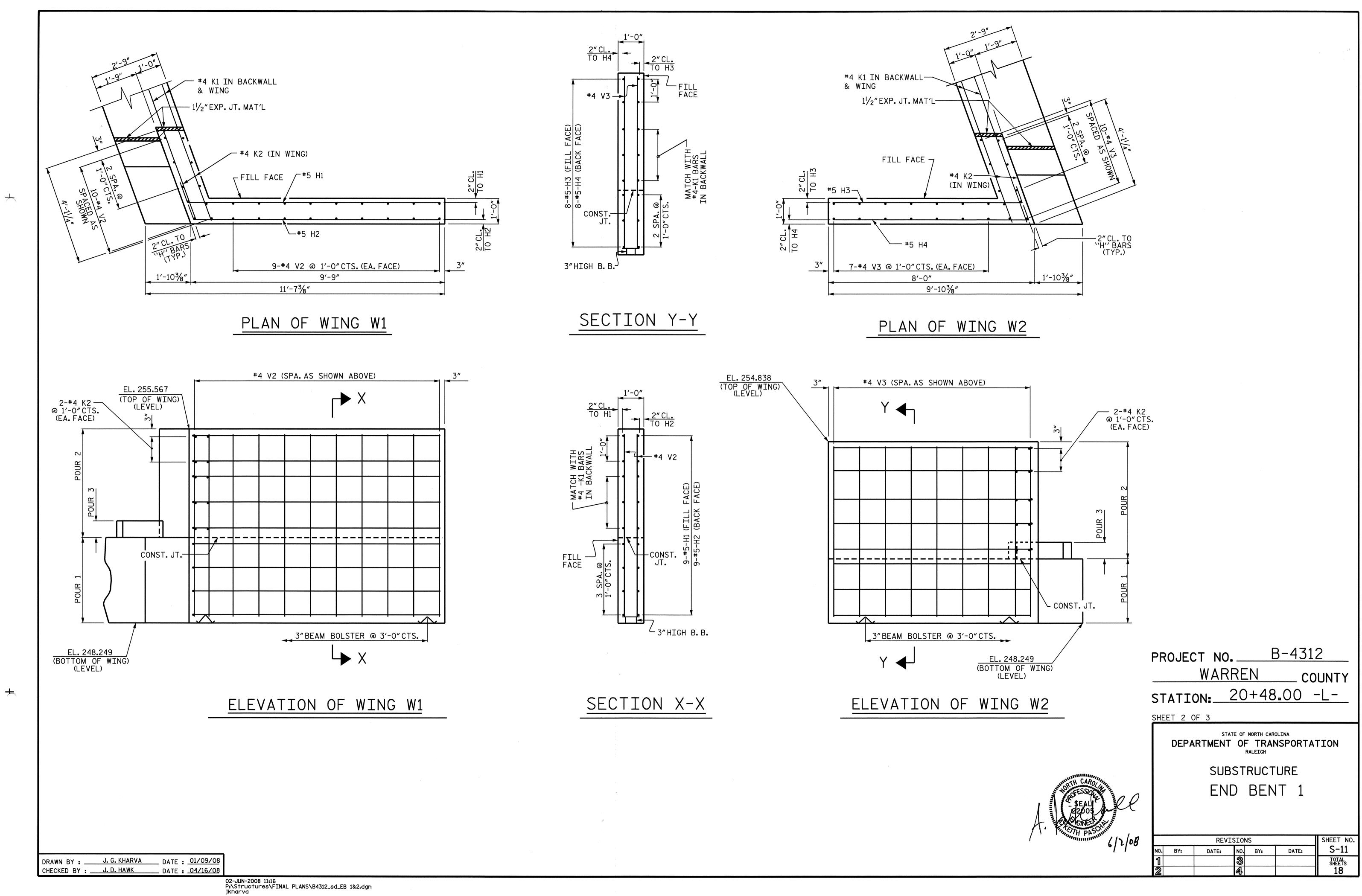
GUARDRAIL ANCHOR ASSEMBLY DETAILS

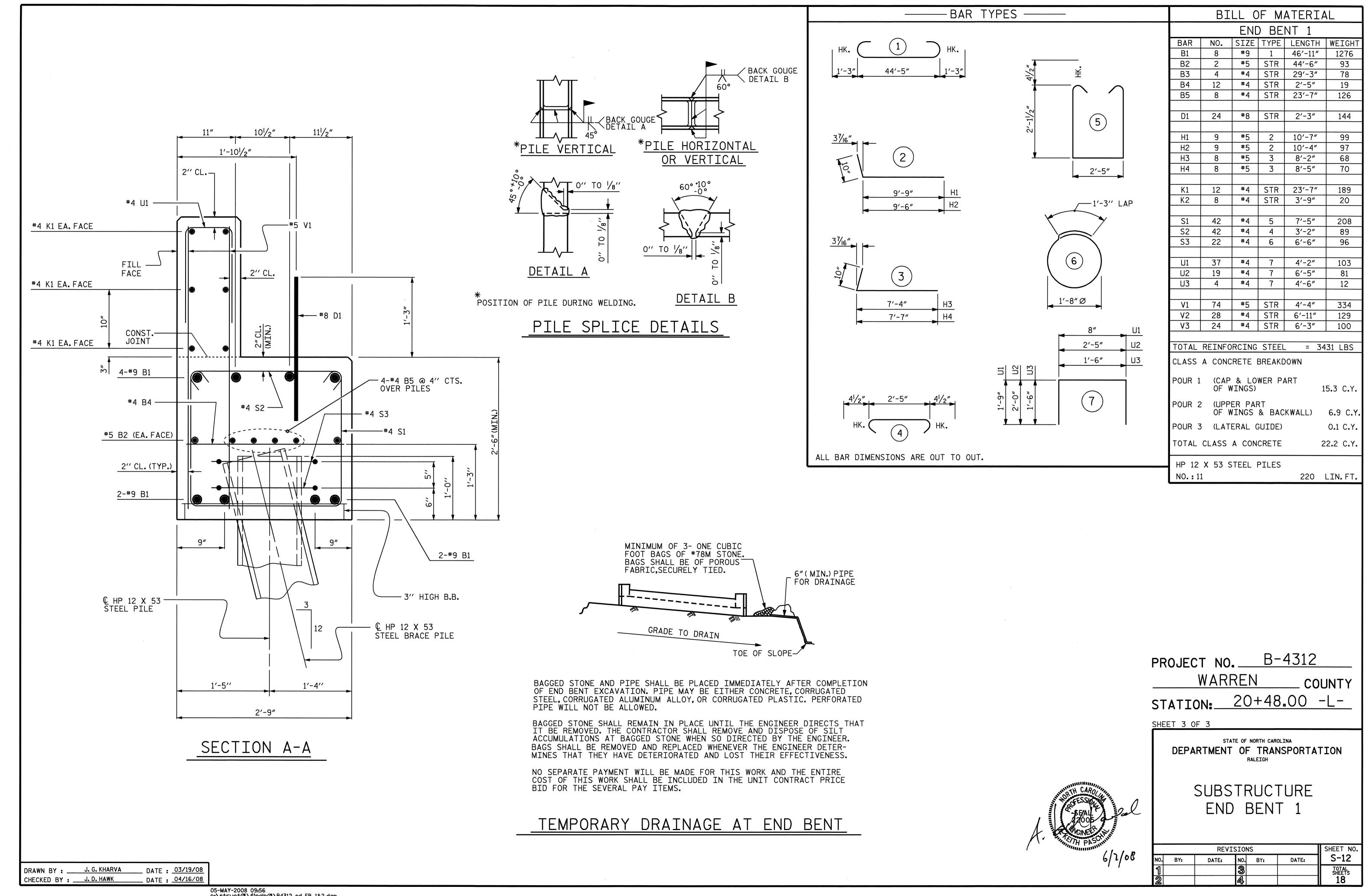
STANDARD

REVISIONS SHEET I

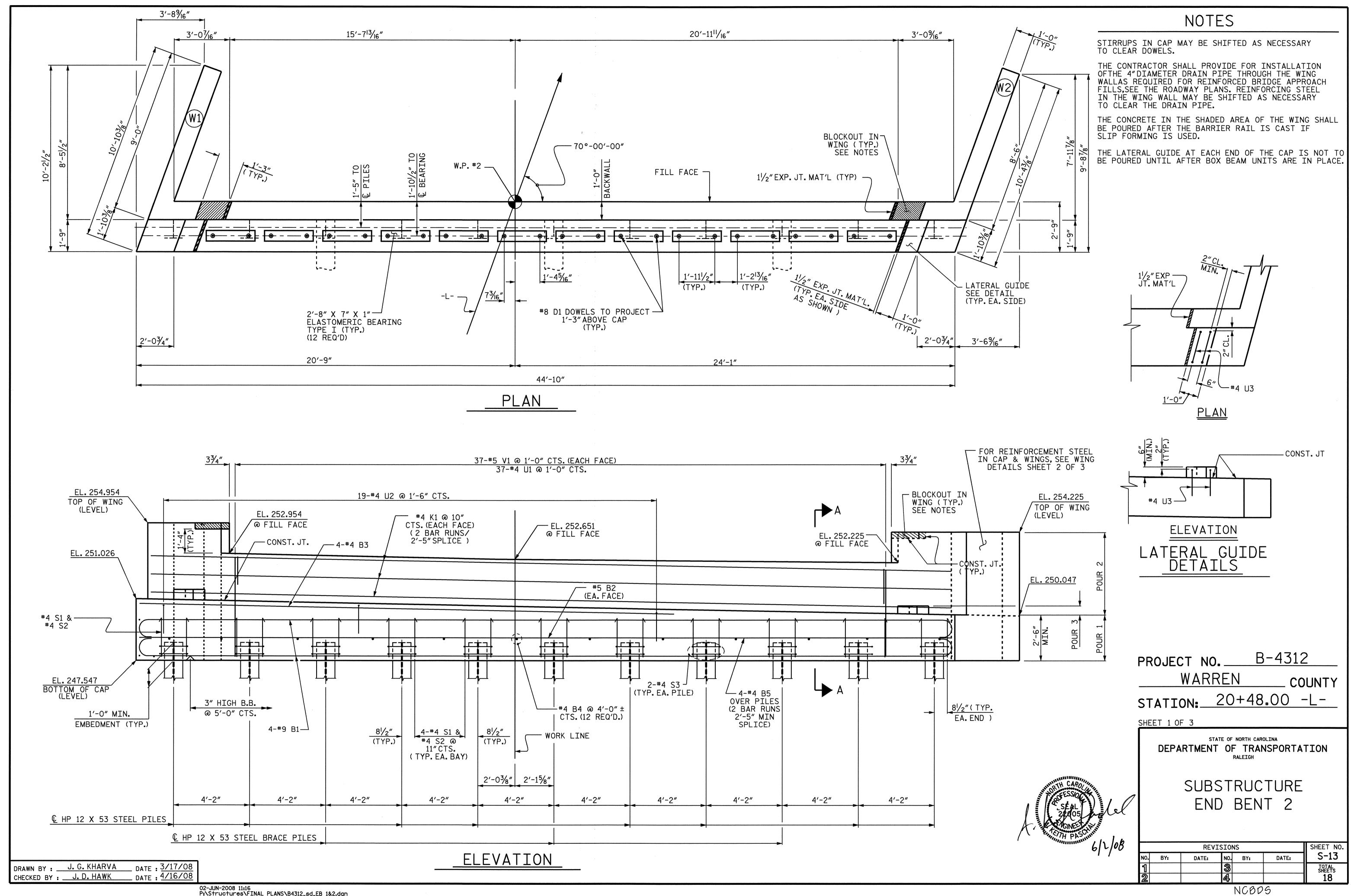
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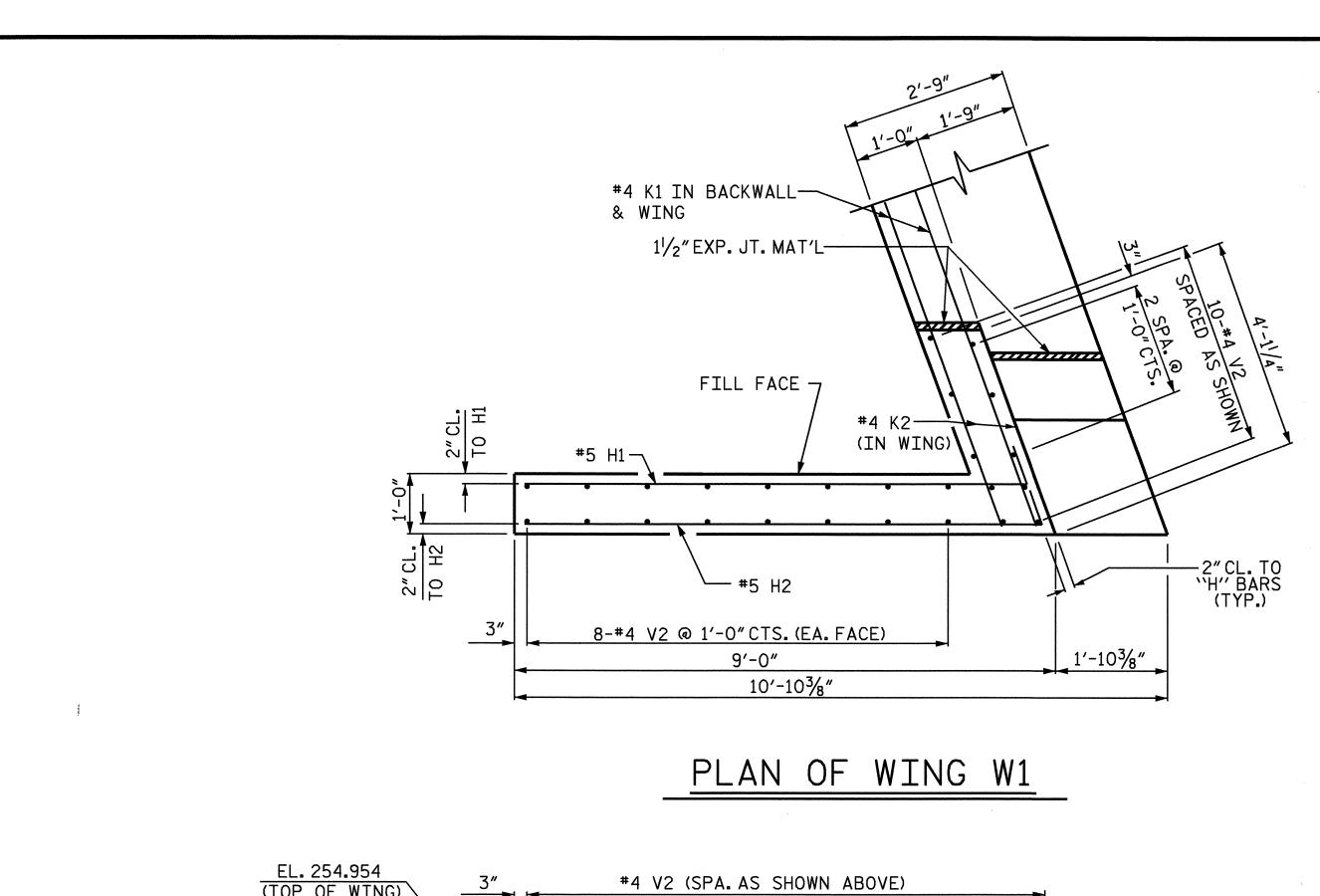


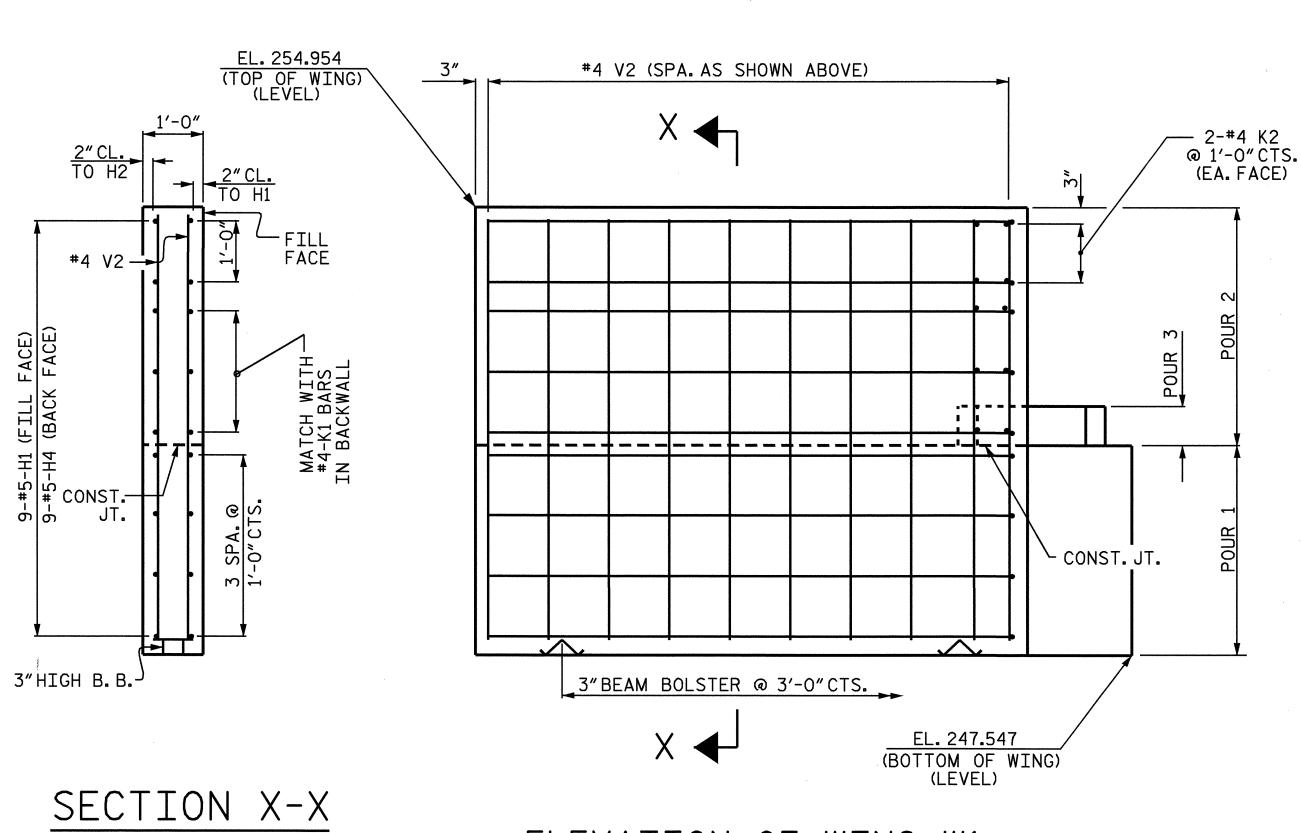




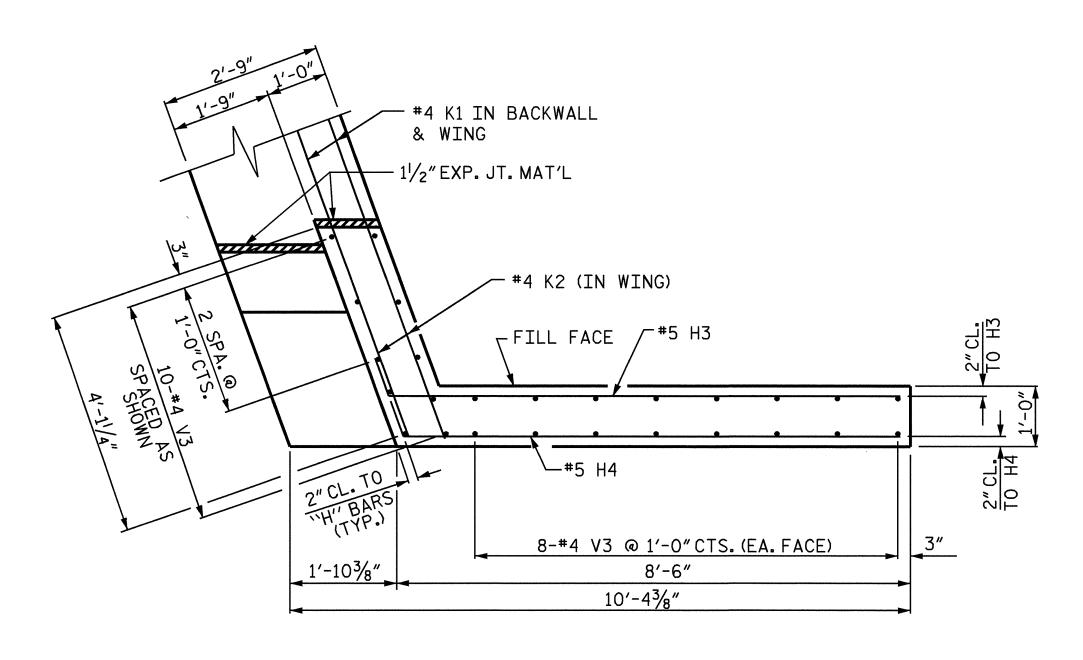
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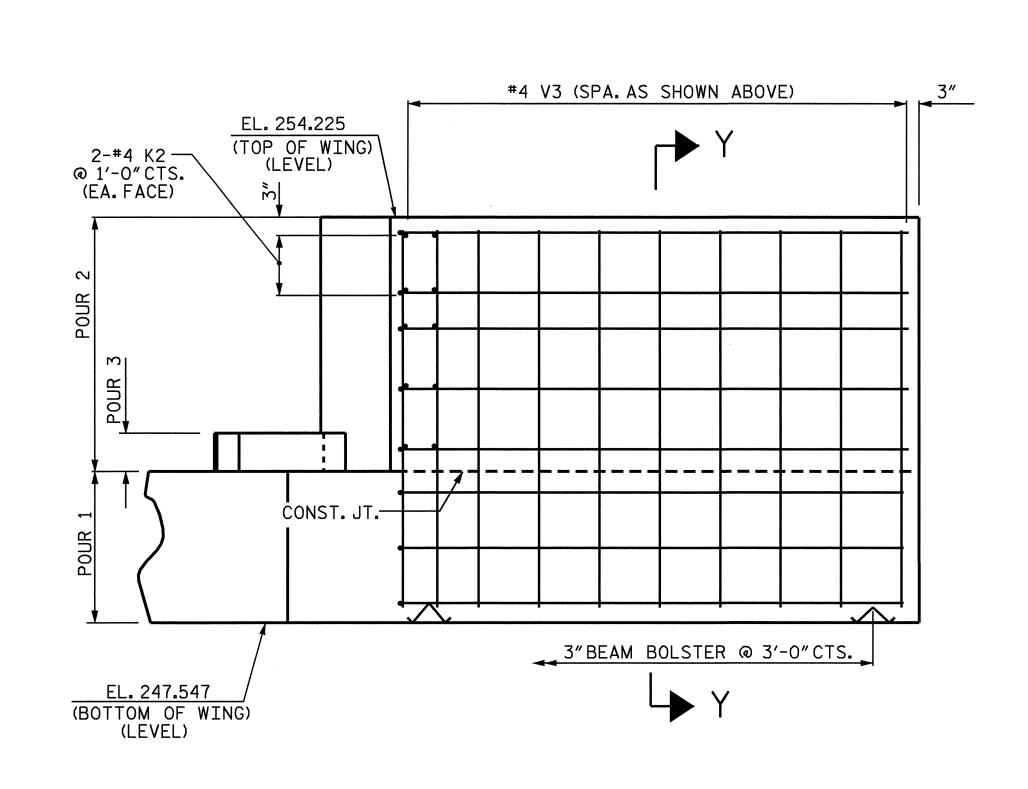




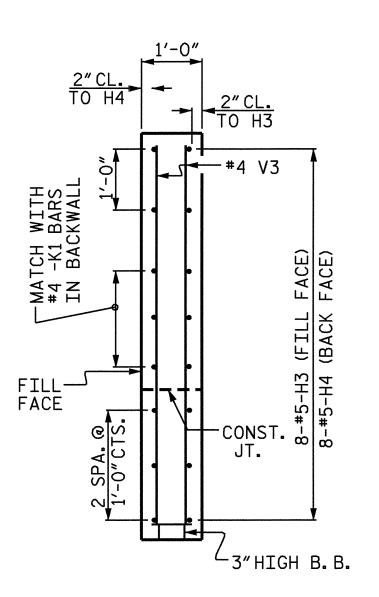
ELEVATION OF WING W1



PLAN OF WING W2



ELEVATION OF WING W2



SECTION Y-Y

PROJECT NO. B-4312

WARREN COUNTY

STATION: 20+48.00 -L-

SHEET 2 OF 3

DEPARTMENT OF TRANSPORTATION
RALEIGH
SUBSTRUCTURE

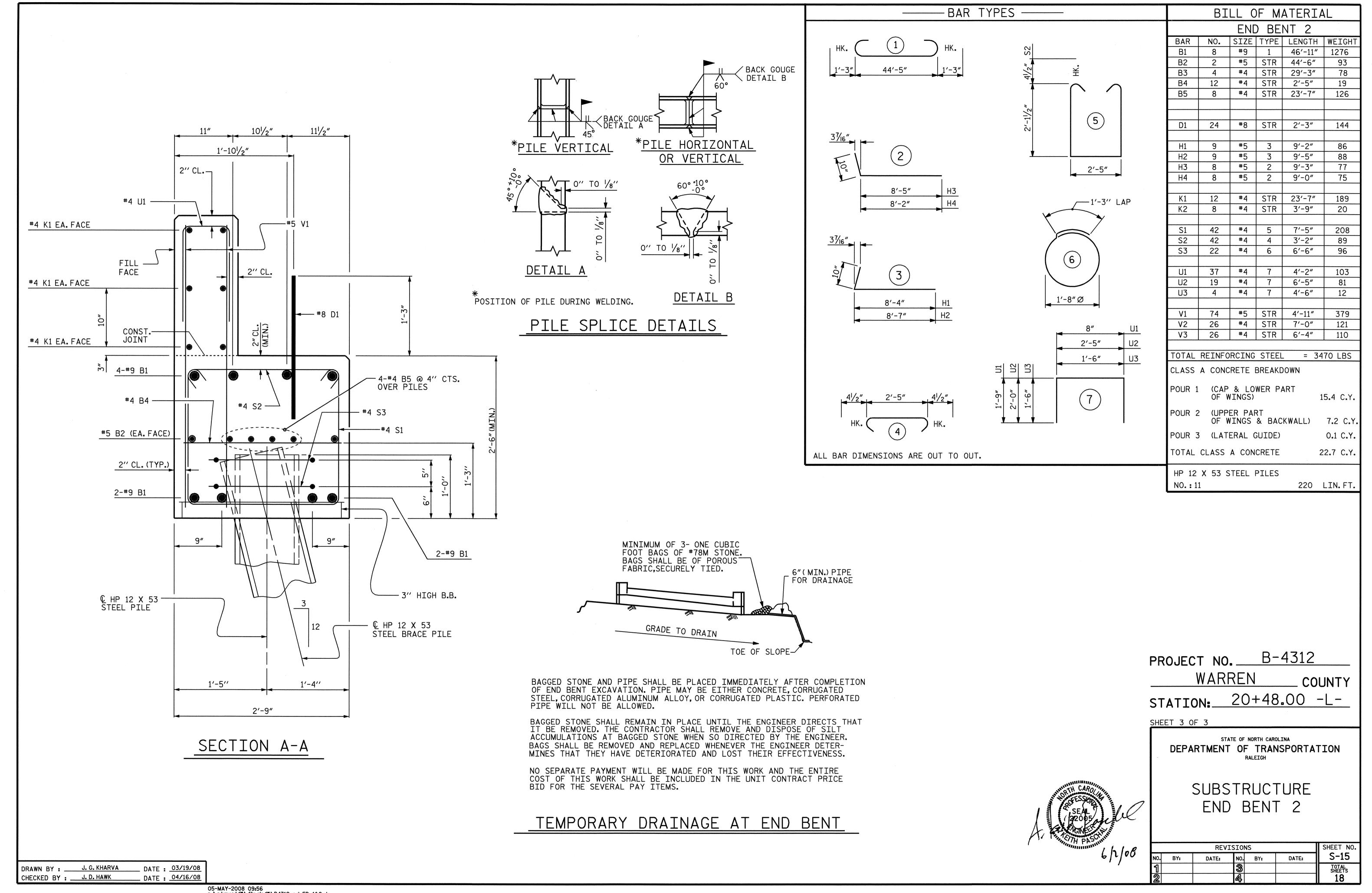
END BENT 2

REVISIONS

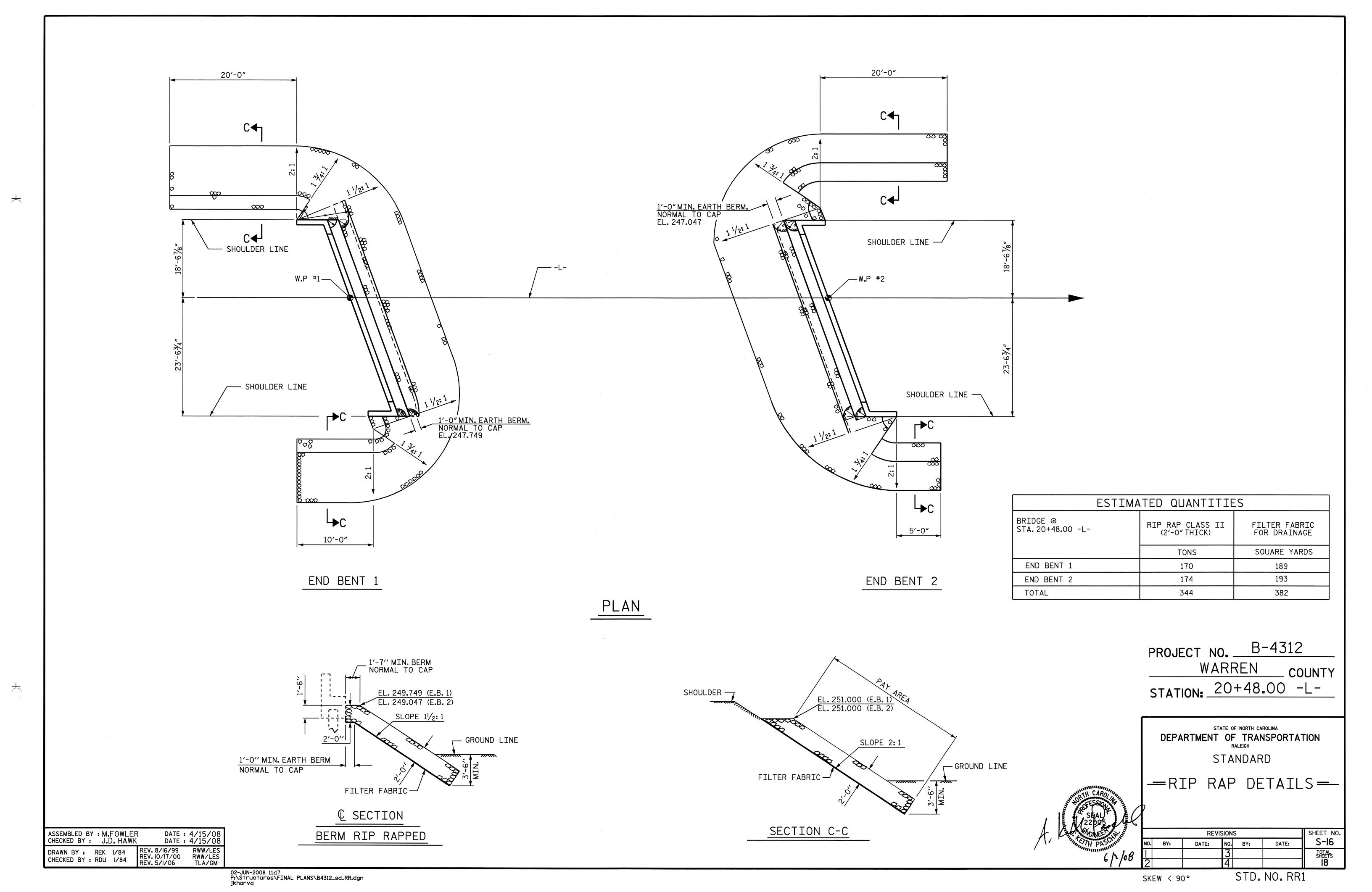
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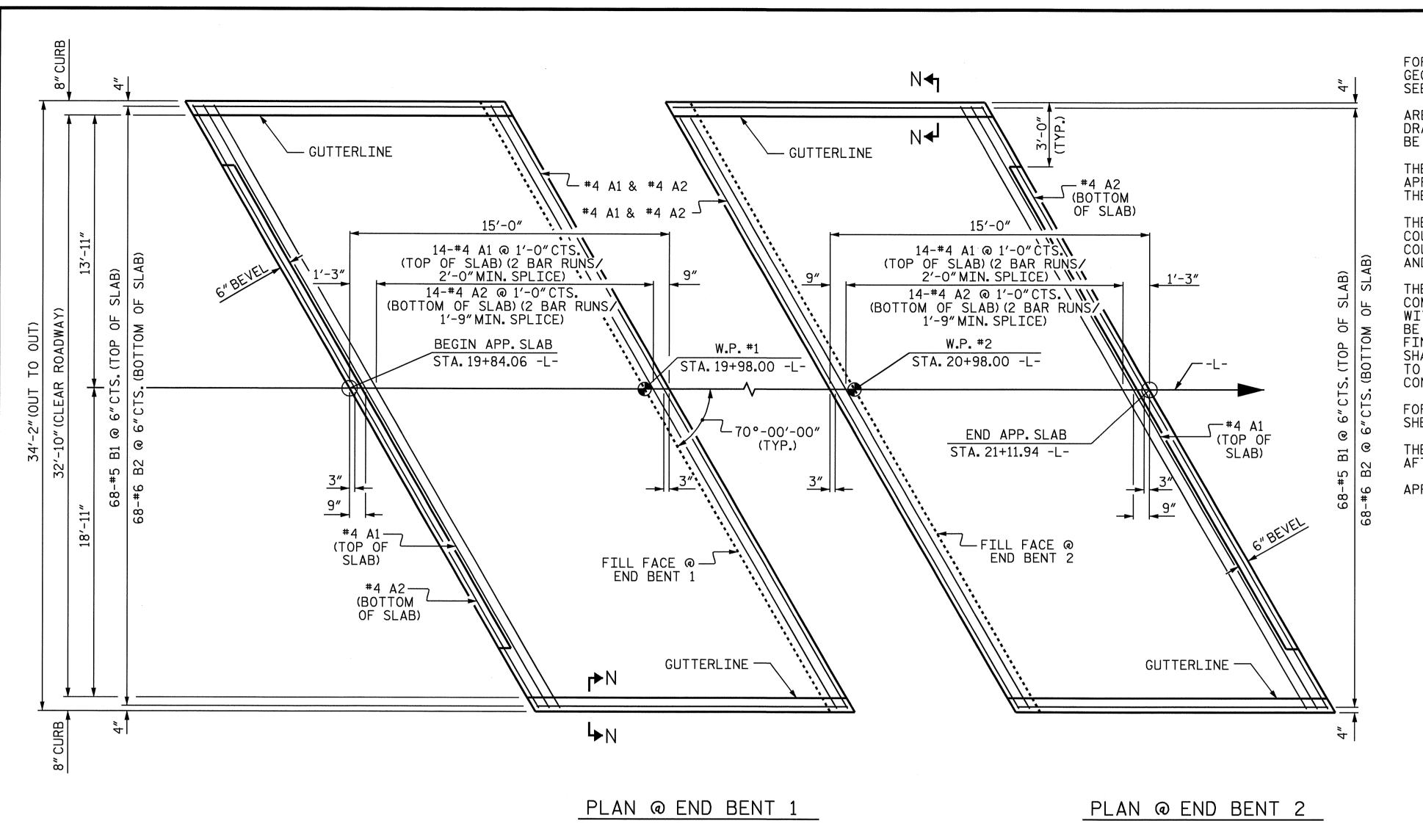
3 TOTAL SHEETS
18

-L-



05-MAY-2008 09:56 p:\structØl\finalpØl\B4312_sd_EB 1&2.dgn jkharva





DIMENSIONS SHOWN ARE TYPICAL FOR BOTH APPROACH SLABS

FOR REINFORCED BRIDGE APPROACH FILL INCLUDING FABRIC, IMPERMEABLE GEOMEMBRANE, 4" Ø DRAINAGE PIPE, #78M STONE, AND SELECT MATERIAL, SEE ROADWAY PLANS.

AREA BETWEEN THE WINGWALL AND APPROACH SLAB SHALL BE GRADED TO DRAIN THE WATER AWAY FROM THE FILL FACE OF THE BRIDGE AND SHALL BE PAVED. SEE ROADWAY PLANS.

THE 6"COMP. A.B.C. SHALL BE FLUSH WITH THE ROADWAY END OF THE APPROACH SLAB AND SHALL EXTEND 1'-0"OUTSIDE OF EACH EDGE OF THE APPROACH SLAB.

THE CONTRACTOR MAY USE 4"TYPE B-25.0B ASPHALT CONCRETE BASE COURSE IN LIEU OF 6"COMP. A.B.C. IF THIS OPTION IS USED, THE BASE COURSE SHALL BE FLUSH WITH THE ROADWAY END OF THE APPROACH SLAB, AND THE WIDTH SHALL BE THE SAME AS THAT OF THE APPROACH SLAB.

THE CONTRACTOR MAY USE 5"CLASS "A" CONCRETE BASE IN LIEU OF 6" COMP. A.B.C. IF THIS OPTION IS USED, THE CONCRETE BASE SHALL BE FLUSH WITH THE ROADWAY END OF THE APPROACH SLAB, AND THE WIDTH SHALL BE THE SAME AS THAT OF THE APPROACH SLAB. THE CONCRETE SHALL BE FINISHED TO A SMOOTH SURFACE AND A LAYER OF 30 LB ROOFING FELT SHALL BE PLACED BETWEEN THE CONCRETE BASE AND THE APPROACH SLAB TO PREVENT BOND. THE APPROACH SLAB SHALL NOT BE CAST UNTIL THE CONCRETE BASE HAS REACHED AN AGE OF THREE CURING DAYS.

FOR JOINT DETAILS, SEE "PRESTRESSED CONCRETE CORED SLAB UNIT" SHEETS.

THE JOINT AT THE END BENT SHALL BE GROUTED AS SOON AS PRACTICAL AFTER THE CONSTRUCTION OF THE APPROACH SLABS.

APPROACH SLAB GROOVING IS NOT REQUIRED.

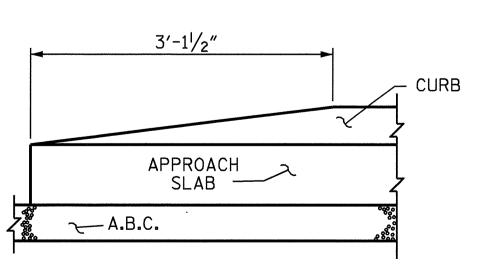
BILL OF MATERIAL								
FOR ONE APPROACH SLAB (2 REQ'D)								
BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT			
* A1	32	#4	STR	19'-0"	406			
A2	32	#4	STR	18'-11"	404			
₩ B1	68	#5	STR	14'-3"	1011			
B2	68	#6	STR	14'-8"	1498			
REINFORCING STEEL LBS. 1902								
* EPOXY COATED REINFORCING STEEL LBS. 1417								
CLASS	CLASS AA CONCRETE C. Y. 19.5							

PROPOSED ASPHALT - $_{\prime}$ 5 $^{\prime}/_{4}$ " CONTINUOUS HIGH CHAIR UPPER (CHCU)@ 3'-0"CTS. ACROSS SLAB **PAVEMENT** JOINT †2:1 SLOPE ——7 THE THE TAX TH - GROUT -APPROVED WIRE BAR SUPPORTS @ 3'-O"CTS. ___ ВЕАМ ___ 6"COMP. A.B.C. UNIT ROADWAY --LIMITS OF REINFORCED BRIDGE APPROACH FILL (ROADWAY PAY - ITEM, SEE NOTES) BACKER ROD FABRIC -† NORMAL TO END BENT SELECT MATERIAL #78M STONE--2 LAYERS OF 30 LB. ROOFING FELT TO 4"Ø CORRUGATED PREVENT BOND PERFORATED DRAINAGE PIPE -IMPERMEABLE GEOMEMBRANE

SECTION THRU SLAB

WIN WILL

SECTION N-N



END OF CURB WITHOUT SHOULDER BERM GUTTER

(OMIT TAPER WHEN SHOULDER BERM GUTTER IS REQUIRED

CURB DETAILS

PROJECT NO. B-4312

WARREN COUNTY

STATION: 20+48.00 -L-

SHEET 1 OF 2

DEPARTMENT OF TRANSPORTATION
RALEIGH

STANDARD
BRIDGE APPROACH SLAB
FOR PRESTRESSED
CONCRETE BOX BEAM

	SHEET NO.				
BY:	DATE:	NO.	BY:	DATE:	S-17
		3			TOTAL SHEETS
		4			18

J. G. KHARVA DATE: 03/10/08

DATE: 04/16/08

LES/RDR RWW/JTE KMM/GM

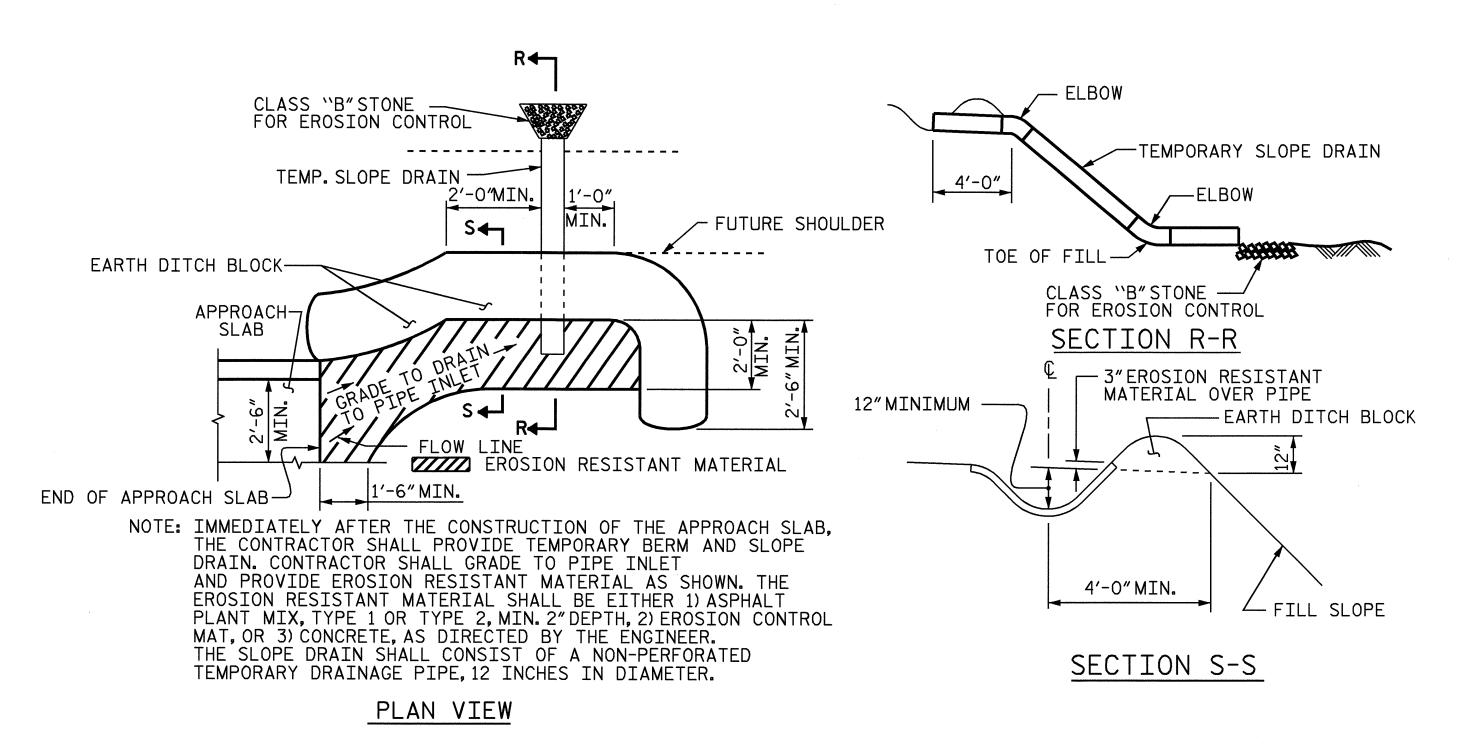
J. D. HAWK

DRAWN BY: FCJ 6/87 REV. 7/10/01 REV. 5/7/03R REV. 5/1/06R

ASSEMBLED BY :

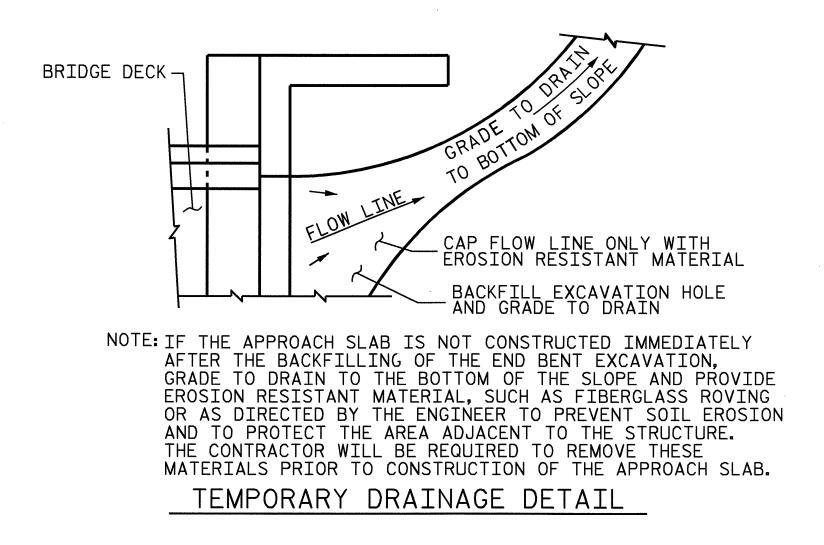
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TEMPORARY BERM AND SLOPE DRAIN DETAILS

(TO BE USED WHEN SHOULDER BERM GUTTER IS REQUIRED)



PROJECT NO. B-4312 WARREN COUNTY STATION: 20+48.00 -L-

SHEET 2 OF 2

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH

STANDARD BRIDGE APPROACH SLAB DETAILS

	SHEET NO.					
NO.	BY:	DATE:	NO.	BY:	DATE:	S-18
			3			TOTAL SHEETS
2			4			18
5						

STD. NO. BAS10

ASSEMBLED BY : J.G.KHARVA DATE: 03/10/08 CHECKED BY: J. D. HAWK DATE : 04/16/08 DRAWN BY: FCJ | 11/88 | REV. 10/17/00 | REV. 5/7/03 | REV. 5/1/06 RWW/LES RWW/JTE TLA/GM

STANDARD NOTES

DESIGN DATA:

A.A.S.H.T.O. (CURRENT) **SPECIFICATIONS** ---- SEE PLANS LIVE LOAD ---- SEE A.A.S.H.T.O. IMPACT ALLOWANCE STRESS IN EXTREME FIBER OF STRUCTURAL STEEL - AASHTO M270 GRADE 36 - 20,000 LBS. PER SQ. IN. - AASHTO M270 GRADE 50W - 27,000 LBS. PER SQ. IN. - AASHTO M270 GRADE 50 - 27,000 LBS. PER SQ. IN. REINFORCING STEEL IN TENSION GRADE 60 -- 24,000 LBS. PER SQ. IN. ---- 1.200 LBS. PER SQ. IN. CONCRETE IN COMPRESSION ---- SEE A.A.S.H.T.O. CONCRETE IN SHEAR STRUCTURAL TIMBER - TREATED OR ---- 1.800 LBS. PER SQ. IN. UNTREATED - EXTREME FIBER STRESS COMPRESSION PERPENDICULAR TO GRAIN OF TIMBER 375 LBS. PER SQ. IN. ----30 LBS. PER CU. FT. EQUIVALENT FLUID PRESSURE OF EARTH

MATERIAL AND WORKMANSHIP:

EXCEPT AS MAY OTHERWISE BE SPECIFIED ON PLANS OR IN THE SPECIAL PROVISIONS, ALL MATERIAL AND WORKMANSHIP SHALL BE IN ACCORDANCE WITH THE 2002 STANDARD SPECIFICATIONS "FOR ROADS AND STRUCTURES" OF THE N. C. DEPARTMENT OF TRANSPORTATION.

(MINIMUM)

STEEL SHEET PILING FOR PERMANENT OR TEMPORARY APPLICATIONS SHALL BE HOT ROLLED.

CONCRETE:

UNLESS OTHERWISE REQUIRED ON PLANS, CLASS A CONCRETE SHALL BE USED FOR ALL PORTIONS OF ALL STRUCTURES WITH THE EXCEPTION THAT: CLASS AA CONCRETE SHALL BE USED IN BRIDGE SUPERSTRUCTURES, ABUTMENT BACKWALLS, AND APPROACH SLABS; CLASS B CONCRETE SHALL BE USED FOR SLOPE PROTECTION AND RIP RAP; AND CLASS S SHALL BE USED FOR UNDERWATER FOOTING SEALS.

CONCRETE CHAMFERS:

UNLESS OTHERWISE NOTED ON THE PLANS, ALL EXPOSED CORNERS ON STRUCTURES SHALL BE CHAMFERED 3/4"WITH THE FOLLOWING EXCEPTIONS: TOP CORNERS OF CURBS MAY BE ROUNDED TO 1-1/2"RADIUS WHICH IS BUILT INTO CURB FORMS; CORNERS OF TRANSVERSE FLOOR EXPANSION JOINTS SHALL BE ROUNDED WITH A 1/4"FINISHING TOOL UNLESS OTHERWISE REQUIRED ON PLANS; AND CORNERS OF EXPANSION JOINTS IN THE ROADWAY FACES AND TOPS OF CURBS AND SIDEWALKS SHALL BE ROUNDED TO A 1/4"RADIUS WITH A FINISHING STONE OR TOOL UNLESS OTHERWISE REQUIRED ON PLANS.

DOWELS:

DOWELS WHEN INDICATED ON PLANS AS FOR CULVERT EXTENSIONS, SHALL BE EMBEDDED AT LEAST 12" INTO THE OLD CONCRETE AND GROUTED INTO PLACE WITH 1:2 CEMENT MORTAR.

ALLOWANCE FOR DEAD LOAD DEFLECTION, SETTLEMENT, ETC. IN CASTING SUPERSTRUCTURES:

BRIDGES SHALL BE BUILT ON THE GRADE OR VERTICAL CURVE SHOWN ON PLANS. SLABS, CURBS AND PARAPETS SHALL CONFORM TO THE GRADE OR CURVE.

ALL DIMENSIONS WHICH ARE GIVEN IN SECTION AND ARE AFFECTED BY DEAD LOAD DEFLECTIONS ARE DIMENSIONS AT CENTER LINE OF BEARING UNLESS OTHERWISE NOTED ON PLANS. IN SETTING FORMS FOR STEEL BEAM BRIDGES AND PRESTRESSED CONCRETE GIRDER BRIDGES, ADJUSTMENTS SHALL BE MADE DUE TO THE DEAD LOAD DEFLECTIONS FOR THE ELEVATIONS SHOWN. WHERE BLOCKS ARE SHOWN OVER BEAMS FOR BUILDING UP TO THE SLAB, THE VERTICAL DIMENSIONS OF THE BLOCKS SHALL BE ADJUSTED BETWEEN BEARINGS TO COMPENSATE FOR DEAD LOAD DEFLECTIONS, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER. WHERE BOTTOM OF SLAB IS IN LINE WITH BOTTOM OF TOP FLANGES, DEPTH OF SLAB BETWEEN BEARINGS SHALL BE ADJUSTED TO COMPENSATE FOR DEAD LOAD DEFLECTION, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER.

IN SETTING FALSEWORK AND FORMS FOR REINFORCED CONCRETE SPANS, AN ALLOWANCE SHALL BE MADE FOR DEAD LOAD DEFLECTIONS, SETTLEMENT OF FALSEWORK, AND PERMANENT CAMBER WHICH SHALL BE PROVIDED FOR IN ADDITION TO THE ELEVATIONS SHOWN. AFTER REMOVAL OF THE FALSEWORK, THE FINISHED STRUCTURES SHALL CONFORM TO THE PROFILE AND ELEVATIONS SHOWN ON THE PLANS AND CONSTRUCTION ELEVATIONS FURNISHED BY THE ENGINEER.

DETAILED DRAWINGS FOR FALSEWORK OR FORMS FOR BRIDGE SUPERSTRUCTURE AND ANY STRUCTURE OR PARTS OF A STRUCTURE AS NOTED ON THE PLANS SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL BEFORE CONSTRUCTION OF THE FALSEWORK OR FORMS IS STARTED.

REINFORCING STEEL:

ALL REINFORCING STEEL SHALL BE DEFORMED WITH THE EXCEPTION OF #2
BARS WHICH MAY BE FABRICATED FROM COLD DRAWN STEEL WIRE. DIMENSIONS
RELATIVE TO PLACEMENT OF REINFORCING ARE TO CENTERS OF BARS UNLESS OTHERWISE
INDICATED IN THE PLANS. DIMENSIONS ON BAR DETAILS ARE TO CENTERS OF BARS
OR ARE OUT TO OUT AS INDICATED ON PLANS.

WIRE BAR SUPPORTS SHALL BE PROVIDED FOR REINFORCING STEEL WHERE
INDICATED ON THE PLANS. WHEN BAR SUPPORT PIECES ARE PLACED IN CONTINUOUS
LINES, THEY SHALL BE SO PLACED THAT THE ENDS OF THE SUPPORTING WIRES SHALL
BE LAPPED TO LOCK LEGS ON ADJOINING PIECES.

STRUCTURAL STEEL:

AT THE CONTRACTOR'S OPTION, HE MAY SUBSTITUTE 7/8" SHEAR STUDS FOR THE 3/4" STUDS SPECIFIED ON THE PLANS. THIS SUBSTITUTION SHALL BE MADE AT THE RATE OF 3 - 7/8" STUDS FOR 4 - 3/4" STUDS, AND STUD SPACING CHANGES SHALL BE MADE AS NECESSARY TO PROVIDE THE SAME EQUIVALENT NUMBER OF 7/8" STUDS ALONG THE BEAM AS SHOWN FOR 3/4" STUDS BASED ON THE RATIO OF 3 - 7/8" STUDS FOR 4 - 3/4" STUDS. STUDS OF THE LENGTH SPECIFIED ON THE PLANS MUST BE PROVIDED. THE MAXIMUM SPACING SHALL BE 2'-0".

EXCEPT AT THE INTERIOR SUPPORTS OF CONTINUOUS BEAMS WHERE THE COVER PLATE IS IN CONTACT WITH BEARING PLATE, THE CONTRACTOR MAY, AT HIS OPTION, SUBSTITUTE FOR THE COVER PLATES DESIGNATED ON THE PLANS COVER PLATES OF THE EQUIVALENT AREA PROVIDED THESE PLATES ARE AT LEAST 5/16" IN THICKNESS AND DO NOT EXCEED A WIDTH EQUAL TO THE FLANGE WIDTH LESS 2" OR A THICKNESS EQUAL TO 2 TIMES THE FLANGE THICKNESS. THE SIZE OF FILLET WELDS SHALL CONFORM TO THE REQUIREMENTS OF THE CURRENT ANSI/AASHTO/AWS "BRIDGE WELDING CODE". ELECTROSLAG WELDING WILL NOT BE PERMITTED.

PLACEMENT OF BEAM OR GIRDER MEMBERS ON TRUCKS FOR HAULING SHALL
BE DONE IN COMPLIANCE WITH LIMITS SHOWN ON SKETCHES PROVIDED TO THE MATERIALS
AND TEST UNIT APPROVED BY THE STRUCTURE DESIGN UNIT DATED MAY 8, 1991.
THESE SKETCHES PRIMARILY LIMIT THE UNSUPPORTED CANTILEVER LENGTH OF MEMBERS.
WHEN THE CONTRACTOR WISHES TO PLACE MEMBERS ON TRUCKS NOT IN ACCORDANCE
WITH THESE LIMITS, TO SHIP BY RAIL, TO ATTACH SHIPPING RESTRAINTS TO THE
MEMBERS OR TO INVERT MEMBERS, HE SHALL SUBMIT A SKETCH FOR APPROVAL
PRIOR TO SHIPPING. SEE ALSO ARTICLE 1072-11.

WITH THE SOLE EXCEPTION OF EDGES AT SURFACES WHICH BEAR ON OTHER SURFACES, ALL SHARP EDGES AND ENDS OF SHAPES AND PLATES SHALL BE SLIGHTLY ROUNDED BY SUITABLE MEANS TO A RADIUS OF APPROXIMATELY 1/16 INCH OR EQUIVALENT FLAT SURFACE AT A SUITABLE ANGLE PRIOR TO PAINTING, GALVANIZING, OR METALLIZING.

HANDRAILS AND POSTS:

METAL STANDARDS AND FACES OF THE CONCRETE END POSTS FOR THE METAL RAIL SHALL BE SET NORMAL TO THE GRADE OF THE CURB, UNLESS OTHERWISE SHOWN ON PLANS. THE METAL RAIL AND TOPS OF CONCRETE POSTS USED WITH THE ALUMINUM RAIL SHALL BE BUILT PARALLEL TO THE GRADE OF THE CURB.

METAL HANDRAILS SHALL BE IN ACCORDANCE WITH THE PLANS. RAILS SHALL BE AS MANUFACTURED FOR BRIDGE RAILING. CASTINGS SHALL BE OF A UNIFORM APPEARANCE. FINS AND OTHER DEFORMATIONS RESULTING FROM CASTING OR OTHERWISE SHALL BE REMOVED IN A MANNER SO THAT A UNIFORM COLORING OF THE COMPLETED CASTING SHALL BE OBTAINED. CASTINGS WITH DISCOLORATIONS OR OF NON-UNIFORM COLORING WILL NOT BE ACCEPTED. CERTIFIED MILL REPORTS ARE REQUIRED FOR METAL RAILS AND POSTS.

SPECIAL NOTES:

GENERALLY, IN CASE OF DISCREPANCY, THIS STANDARD SHEET OF NOTES SHALL GOVERN OVER THE SPECIFICATIONS, BUT THE REMAINDER OF THE PLANS SHALL GOVERN OVER NOTES HEREON, AND SPECIAL PROVISIONS SHALL GOVERN OVER ALL. SEE SPECIFICATIONS ARTICLE 105-4.

ENGLISH