

STATE OF NORTH CAROLINA DIVISION OF HIGHWAYS

RANDOLPH COUNTY

B-4244 F. A. PROJ. NO. BRZ-2215(1) 33587.1.1 P.E. BRZ-2215(1) 33587.2.1 UTIL. & R/W BRZ-2215(1) 33587.3.1 CONST.

LOCATION: BRIDGE NO. 140 OVER GABRIEL'S CREEK ON SR 2215 TYPE OF WORK: GRADING, DRAINAGE, PAVING, & STRUCTURE

END TIP PROJECT B-4244 -L-STA. 23 + 20.00BEGIN TIP PROJECT B-4244 -L-STA.15+00.00**END BRIDGE** STA 18+50.00 -L-TO NORTH ASHEBORO SR 2215 HENLEY COUNTRY ROAD TO US 64 BEGIN BRIDGE -L- STA 17+70.00

VICINITY MAP

DENOTES OFF-SITE DETOUR

DHV = 12 %

D = 60 %

* TTST=1% DUALS=2%

PROJECT LENGTH

LENGTH ROADWAY OF F.A. PROJECT = 0.140 MI LENGTH STRUCTURE OF F.A. PROJECT = 0.015 MI

TOTAL LENGTH OF STATE PROJECT = 0.155 MI

DIVISION OF HIGHWAYS 2006 STANDARD SPECIFICATIONS J. C. FRYE, P.E. PROJECT ENGINEER LETTING DATE: W.A. DAVIS, P.E. JULY 15, 2008 PROJECT DESIGN ENGINEER

Prepared in the Office of:

STRUCTURE DESIGN UNIT 1000 BIRCH RIDGE DR.

DIVISION OF HIGHWAYS STATE OF NORTH CAROLINA RALEIGH, N.C. 27610

STATE DESIGN ENGINEER

DEPARTMENT OF TRANSPORTATION FEDERAL HIGHWAY ADMINISTRATION

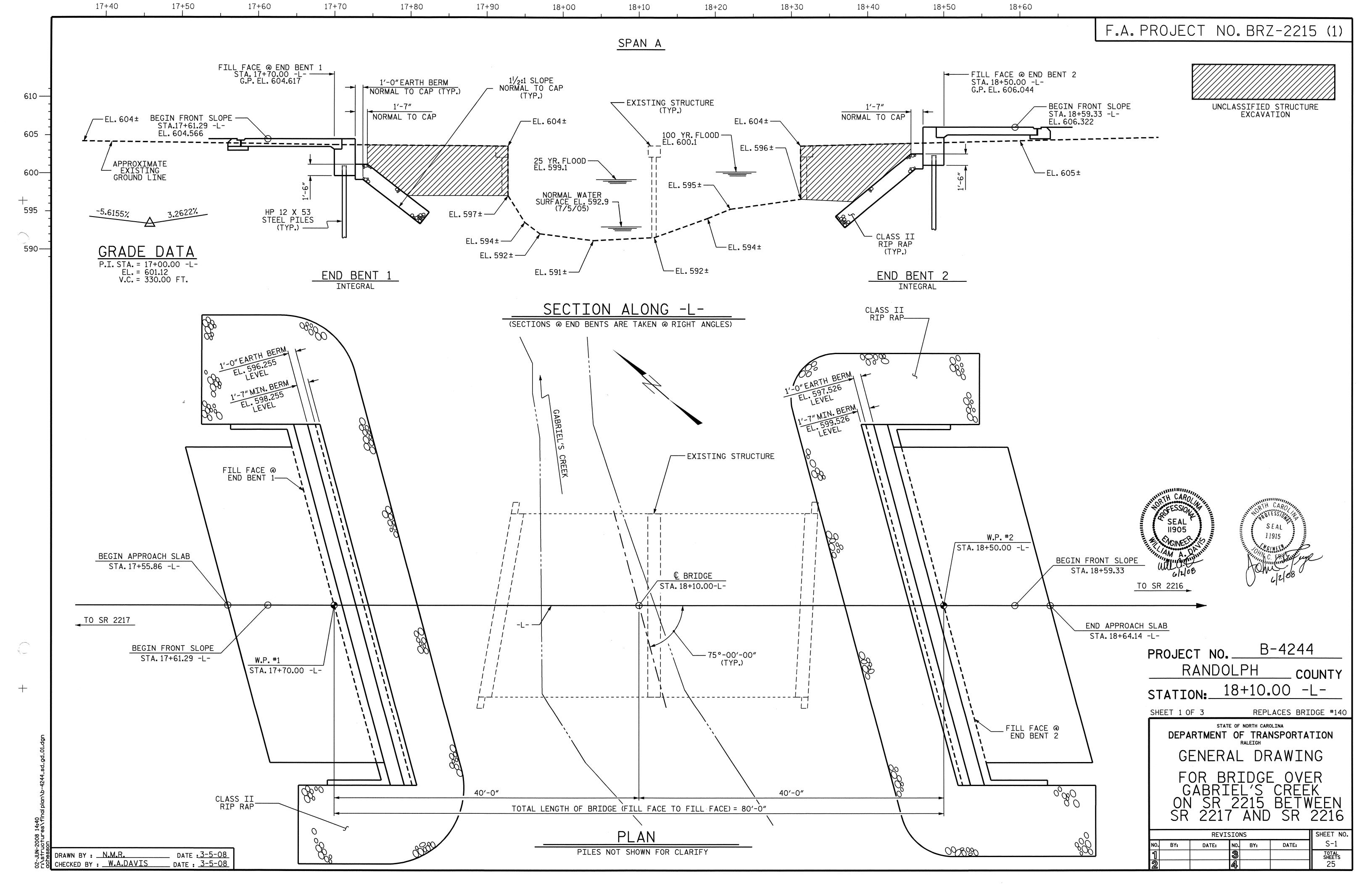
APPROVED
DIVISION ADMINISTRATOR DATE

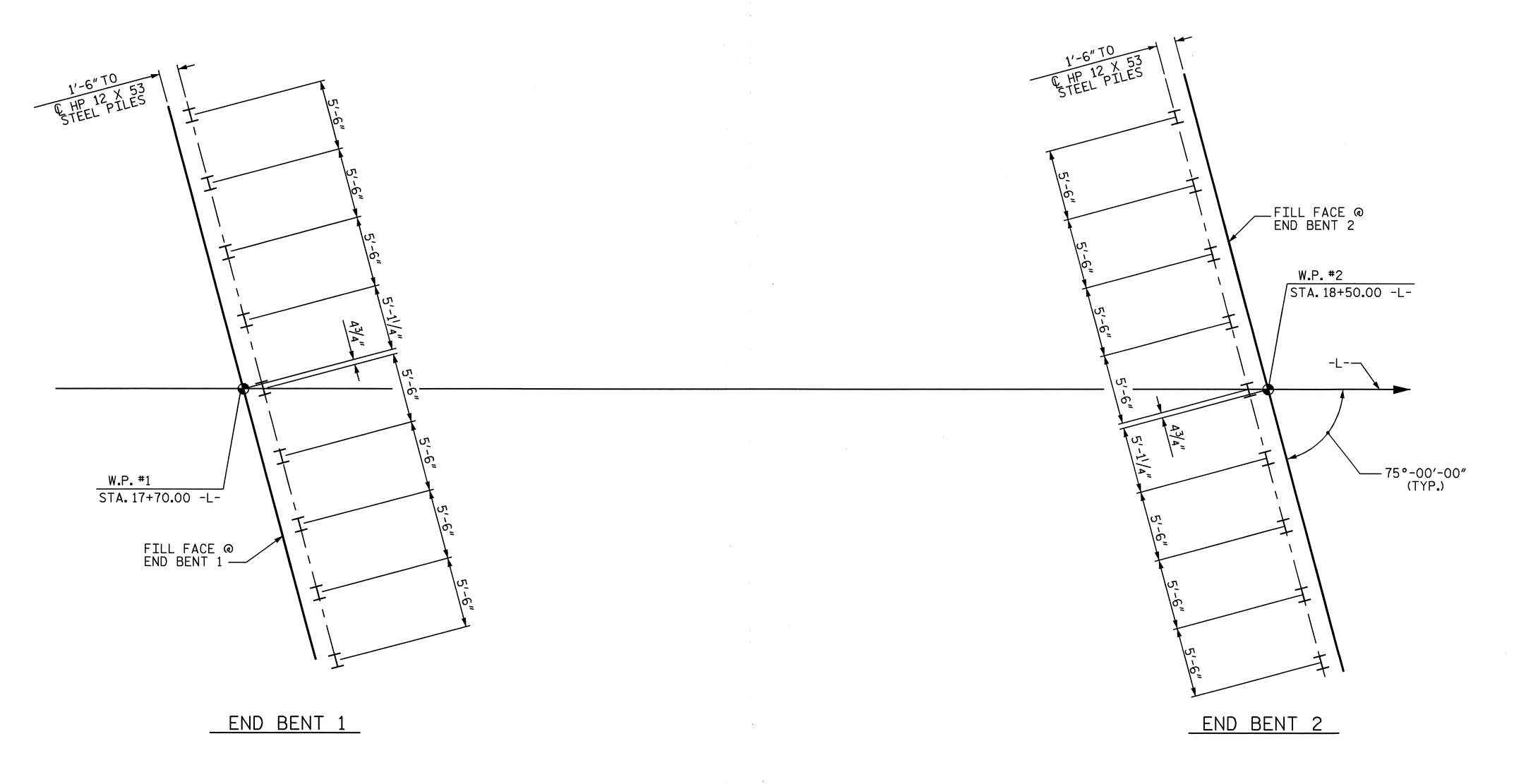
DESIGN DATA

ADT 2007 = 4,320ADT 2027 = 7,970

V = 60 MPH

FUNC. CLASS = RURAL LOCAL





NOTES

DIMENSIONS LOCATING PILES ARE TO THE PILE CENTERLINE

DRIVE PILES AT END BENT 1 & END BENT 2
TO A REQUIRED BEARING CAPACITY OF 120 TONS
PER PILE. THE REQUIRED BEARING CAPACITY
IS EQUAL TO THE ALLOWABLE BEARING CAPACITY
WITH A MINIMUM FACTOR SAFETY OF TWO.

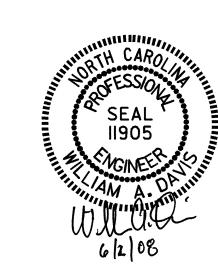
THE ALLOWABLE BEARING CAPACITY FOR PILES AT END BENT NO.1 & NO.2 IS 60 TONS PER PILE.

CONSTRUCTION SEQUENCE:

- DRIVE STEEL PILES FOR END BENT 1 AND END BENT 2. COMPLETE POUR 1 OF END BENTS.
- 2. ONCE CONCRETE HAS ATTAINED THE REQUIRED STRENGTH, INSTALL NUT, WASHER AND SOLE PLATE ON ANCHOR BOLTS. ERECT GIRDERS AND ALIGN SOLE PLATES WITH HOLES IN FLANGES REGARDLESS OF TEMPERATURE AT TIME OF SETTING. SOLE PLATE SHOULD BE WELDED TO THE GIRDER FLANGE BEFORE FALSEWORK IS PLACED. ADJUST LOWER NUT TO SET GIRDER BEARING AT THE PROPER ELEVATION. INSTALL WASHER AND NUT ON TOP OF FLANGES. LEAVE TOP NUT LOOSE TO ALLOW FOR GIRDER END ROTATION AND TRANSLATION DURING DECK POURING SEQUENCE.
- 3. POUR BRIDGE DECK IN ACCORDANCE WITH THE POURING SEQUENCE OUTLINED ON THE "SUPERSTRUCTURE BILL OF MATERIAL" SHEET EXCEPT THE FINAL TWO POURS CONTAINING THE ABUTMENT. NOTE THAT THE FINAL TWO POURS CONTAINING THE WINGWALLS AND ABUTMENT ARE PLACED WITH THE FINAL POURS OF THE BRIDGE DECK.
- 4. TIGHTEN THE TOP NUT 1/4 TURN PAST FINGER TIGHT. COMPLETE FINAL TWO DECK POURS WHICH INCLUDES THE ABUTMENT, DECK AND THE WINGWALLS.
- 5. PLACE THE REINFORCED BRIDGE APPROACH FILL AND BACKFILL IN LIFTS UNTIL THE DESIRED SUBGRADE ELEVATION IS REACHED. CONSTRUCT SLEEPER SLABS.
- 6. POUR THE APPROACH SLABS STARTING AT THE END FURTHEST FROM THE BACKWALL AND PROGRESSING TOWARDS THE END BENT. POURS SHALL BE PERFORMED DURING THE MORNING HOURS TO MINIMIZE PLACING THE APPROACH SLAB IN TENSION FROM BRIDGE THERMAL MOVEMENTS.

FOUNDATION LAYOUT

(ORIENT PILES AS SHOWN)



PROJECT NO. B-4244

RANDOLPH COUNTY

STATION: 18+10.00 -L-

SHEET 2 OF 3

DEPARTMENT OF TRANSPORTATION
RALEIGH

GENERAL DRAWING

FOR BRIDGE OVER
GABRIEL'S CREEK
ON SR 2215 BEWEEN
SR 2217 AND SR 2216

REVISIONS

BY: DATE: NO. BY: DATE:

TOTAL SHEETS
25

DRAWN BY: A.R.CHESSON DATE: 7-07
CHECKED BY: W.A.DAVIS DATE: 3-5-08

BM #2: EL. 607.35, RR SPIKE IN BASE OF POWER POLE (CP&L K794AD) 90.12' RT. OF -L- STA. 19+90.63 (3 NOTE: FOR UTILITY INFORMATION, SEE UTILITY PLANS AND SPECIAL PROVISIONS. WOODS -EXISTING STRUCTURE ♠ BRIDGE /STA.18+10.00 -L---- GUARDRAIL ROADWAY PAY ITEM (TYP.) CLASS II RIP RAP WOODS TO SR 2217 TO SR 2216 المالية දියි දයි - 75°-00'-00" (TYP.) عننحننحنحنحن - CLASS II RIP RAPCOM WOODS —— LOCATION SKETCH ——

HYDRAULIC DATA

DESIGN DISCHARGE FREQUENCY OF DESIGN FLOOD

= 820 CFS. = 25 YRS.

DESIGN HIGH WATER ELEVATION = 599.1 DRAINAGE AREA

= 2.34 SQ. MI.

BASIC DISCHARGE (Q100) BASIC HIGH WATER ELEVATION = 600.1

= 1,300 CFS.

= 604.3

OVERTOPPING FLOOD DATA

OVERTOPPING DISCHARGE $= 3.600 \pm CFS.$ FREQUENCY OF OVERTOPPING FLOOD = 500+YRS.

NOTES

ASSUMED LIVE LOAD = HS 20 OR ALTERNATE LOADING. EXCEPT THE GIRDERS HAVE BEEN DESIGNED FOR HS 25.

FOR OTHER DESIGN DATA AND GENERAL NOTES. SEE SHEET

FOR EROSION CONTROL MEASURES SEE EROSION CONTROL PLANS.

THIS BRIDGE HAS BEEN DESIGNED BY THE STRENGTH DESIGN METHOD AS SPECIFIED IN AASHTO STANDARD SPECIFICATIONS.

ALL STRUCTURAL STEEL SHALL BE AASHTO M270 GRADE 50W AND NO PAINTING OF STRUCTURAL STEEL IS REQUIRED.

THE EXISTING STRUCTURE CONSISTING OF TWO 17'-9" CONTINUOUS SPANS WITH A TIMBER DECK ON I BEAMS. WITH TIMBER PILES AND CONCRETE INCASED CAPS AND PILES AND LOCATED AT THE PROPOSED BRIDGE SITE SHALL BE REMOVED. THE EXISTING BRIDGE IS PRESENTLY POSTED BELOW THE LEGAL LOAD LIMIT. SHOULD THE STRUCTURAL INTEGRITY OF THE BRIDGE FURTHER DETERIORATE. THIS LOAD LIMITATION MAY BE REDUCED AS FOUND NECESSARY.

THE MATERIAL SHOWN IN THE CROSS-HATCHED AREA ON SHEET S-1 SHALL BE EXCAVATED FOR A DISTANCE OF 30 FT. EACH SIDE OF CENTERLINE ROADWAY AS DIRECTED BY THE ENGINEER. THIS WORK WILL BE PAID FOR AT THE CONTRACT LUMP SUM PRICE FOR UNCLASSIFIED STRUCTURE EXCAVATION, SEE SECTION 412 OF STANDARD SPECIFICATIONS.

THE CONTRACTOR SHALL PROVIDE INDEPENDENT ASSURANCE SAMPLES OF REINFORCING STEEL AS FOLLOWS: FOR PROJECTS REQUIRING UP TO 400 TONS OF REINFORCING STEEL, ONE 30 INCH SAMPLE OF EACH SIZE BAR USED, AND FOR PROJECTS REQUIRING OVER 400 TONS OF REINFORCING STEEL. TWO 30 INCH SAMPLES OF EACH SIZE BAR USED. THE BARS FROM WHICH THE SAMPLES ARE TAKEN MUST THEN BE SPLICED WITH REPLACEMENT BARS OF THE SIZE AND LENGTH OF THE SAMPLE, PLUS A MINIMUM LAP SPLICE OF THIRTY BAR DIAMETERS.

FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.

REMOVABLE FORMS MAY BE USED IN LIEU OF METAL STAY-IN-PLACE FORMS IN ACCORDANCE WITH ARTICLE 420-3 OF THE STANDARD SPECIFICATIONS.

REMOVAL OF THE EXISTING BRIDGE SHALL BE PERFORMED SO AS NOT TO ALLOW DEBRIS TO FALL INTO THE WATER. THE CONTRACTOR SHALL REMOVE THE BRIDGE AND SUBMIT PLANS FOR DEMOLITION IN ACCORDANCE WITH ARTICLE 402-2 OF THE STANDARD SPECIFICATIONS.

THIS STRUCTURE HAS BEEN DESIGNED IN ACCORDANCE WITH HEC 18, "EVALUATING SCOUR AT BRIDGES", MAY, 2001.

THE SUBSTRUCTURE OF THE EXISTING BRIDGE INDICATED ON THE PLANS IS FROM THE BEST INFORMATION AVAILABLE. SINCE THIS INFORMATION IS SHOWN FOR THE CONVENIENCE OF THE CONTRACTOR, THE CONTRACTOR SHALL HAVE NO CLAIM WHATSOEVER AGAINST THE DEPARTMENT OF TRANSPORTATION FOR ANY DELAYS OR ADDITIONAL COST INCURRED BASED ON DIFFERENCES BETWEEN THE EXISTING BRIDGE SUBSTRUCTURE SHOWN ON THE PLANS AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

THIS BRIDGE HAS BEEN DESIGNED IN ACCORDANCE WITH THE REQUIREMENTS OF THE AASHTO STANDARD SPECIFICATIONS FOR SEISMIC DESIGN OF HIGHWAY BRIDGES FOR SEISMIC PERFORMANCE CATEGORY A.

INASMUCH AS THE PAINT SYSTEM ON THE EXISTING STRUCTURAL STEEL CONTAINS LEAD, THE CONTRACTOR'S ATTENTION IS DIRECTED TO ARTICLE 107-1 OF THE STANDARD SPECIFICATIONS. ANY COSTS RESULTING FROM COMPLIANCE WITH APPLICABLE STATE OR FEDERAL REGULATIONS PERTAINING TO HANDLING OF MATERIALS CONTAINING LEAD BASED PAINT SHALL BE INCLUDED IN THE BID PRICE FOR "REMOVAL OF EXISTING STRUCTURE AT STATION 18+10.00 -L-."

NEEDLE BEAMS WILL NOT BE ALLOWED UNLESS OTHERWISE CALLED FOR ON THE PLANS OR APPROVED BY THE ENGINEER.

FOR SUBMITTAL OF WORKING DRAWINGS, SEE SPECIAL PROVISIONS.

FOR FALSEWORK AND FORMWORK. SEE SPECIAL PROVISIONS.

FOR CRANE SAFETY, SEE SPECIAL PROVISIONS.

STATION:

SHEET 3 OF 3

FOR CONSTRUCTION SEQUENCE FOR INTEGRAL END BENTS, SEE SHEET 2 OF 3.

PROJECT NO. B-4244

_ COUNTY

18+10.00 -L-

RANDOLPH

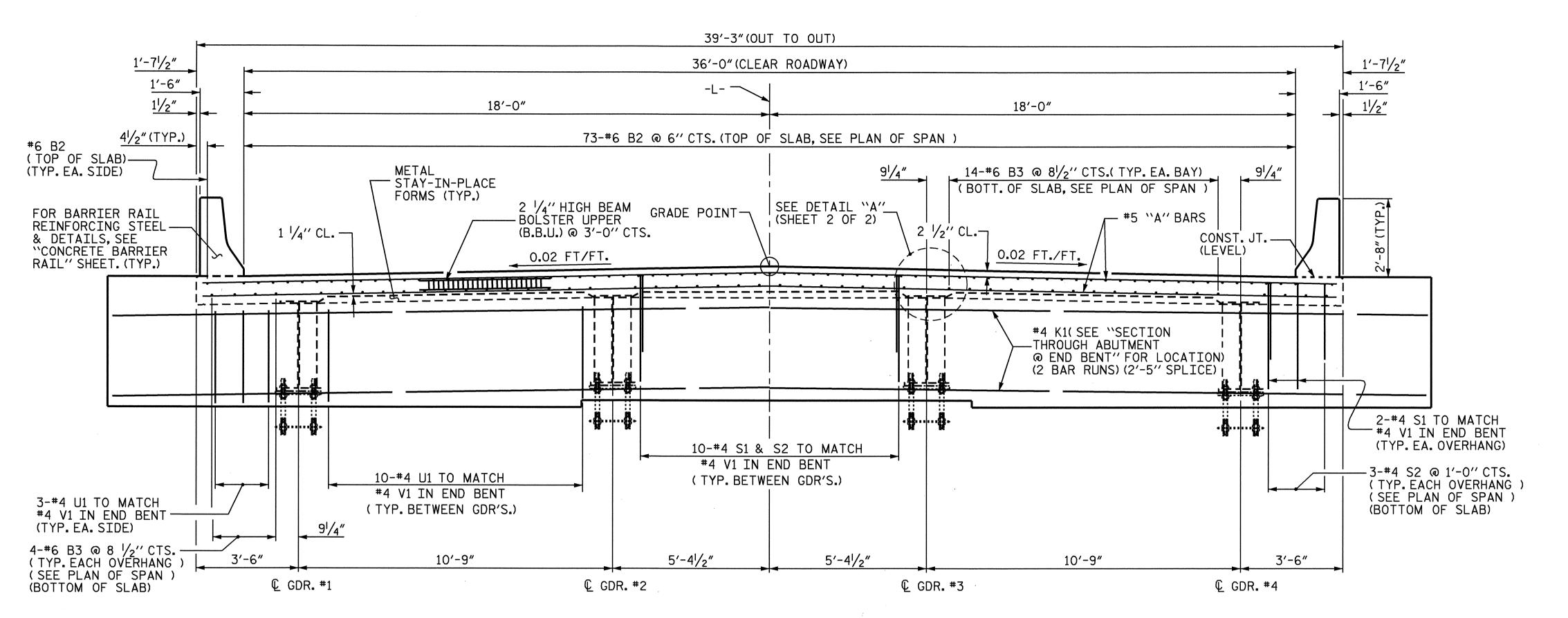
					TOTA	L BIL	L OF	MATER	RI	AL				
	REMOVAL OF EXISTING STRUCTURE	UNCLASSIFIED STRUCTURE EXCAVATION	REINFORCED CONCRETE DECK SLAB	GROOVING BRIDGE FLOORS	CLASS A CONCRETE	BRIDGE APPROACH SLABS	REINFORCING STEEL	APPROX. STRUCTURAL STEEL	HP STEI	12 X 53 EL PILES	CONCRETE BARRIER RAIL	RIP RAP CLASS II	FILTER FABRIC FOR DRAINAGE	EVAZOTE JOINT SEALS
	LUMP SUM	LUMP SUM	SQ.FT.	SQ.FT.	CU.YDS.	LUMP SUM	LBS.	APPROX.LBS.	NO.	LIN.FT.	LIN.FT.	TONS	SQ. YDS.	LUMP SUM
SUPERSTRUCTURE	LUMP SUM	LUMP SUM	3072	3419		LUMP SUM		86500			156.54			LUMP SUM
END BENT 1					18.3		2658	,	9	135		109	121	
END BENT 2					18.3		2676		9	135		152	169	
TOTAL	LUMP SUM	LUMP SUM	3072	3419	36.6	LUMP SUM	5334	86500	18	270	156 . 54	261	290	LUMP SUM

OVERTOPPING FLOOD ELEVATION

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION GENERAL DRAWING FOR BRIDGE OVER SR 2215 BETWEEN

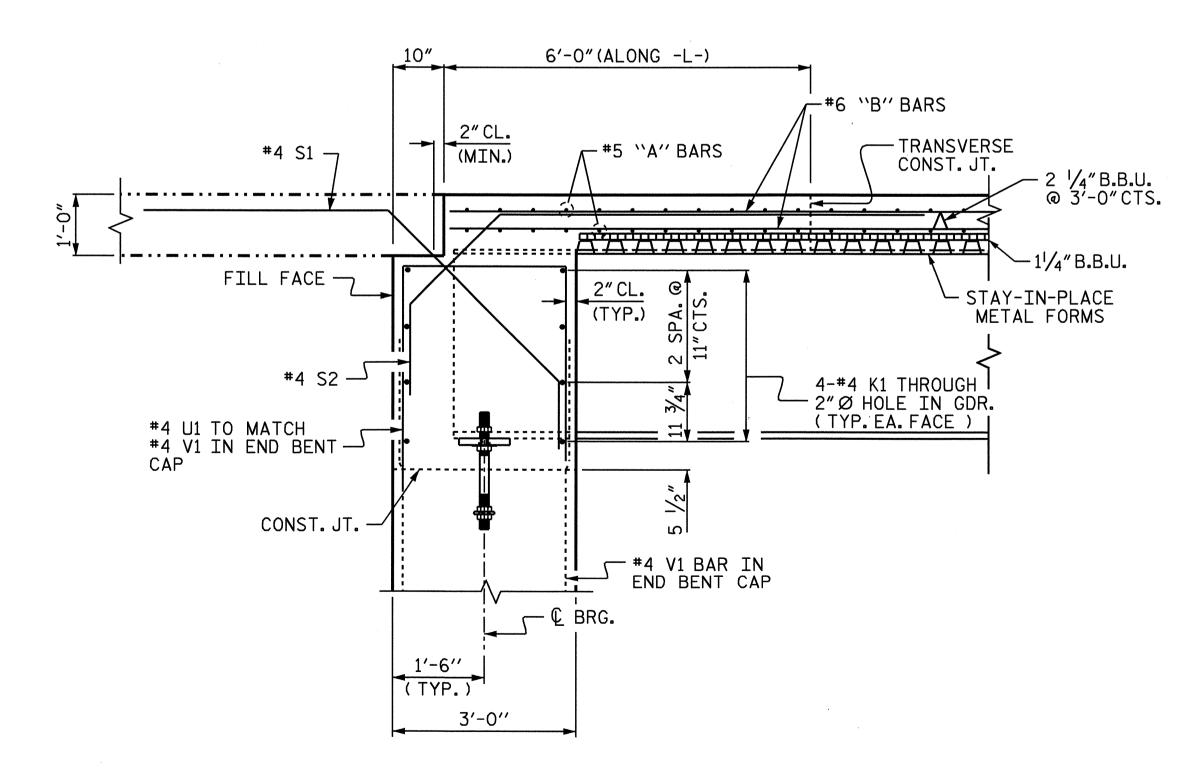
SR 2217 AND SR REVISIONS SHEET NO. S-3 NO. BY: DATE: DATE: BY:

DRAWN BY: A.R.CHESSON DATE: 8-07
CHECKED BY: W.A.DAVIS DATE: 3-5-08



TYPICAL SECTION

(SHOWING ABUTMENT AT END BENT)
(APPROACH SLAB BLOCKOUT & WINGS ARE NOT SHOWN FOR CLARITY)



SECTION THROUGH ABUTMENT @ END BENT

DRAWN BY: R.W. WRIGHT DATE: 1-07
CHECKED BY: A.R. CHESSON DATE: 7-07

NOTES

PROVIDE 11/4"HIGH BEAM BOLSTERS UPPER AT 4'-0"CTS. ATOP THE METAL STAY-IN-PLACE FORMS TO SUPPORT THE BOTTOM MAT OF 'A' BARS. WHEN USING REMOVABLE FORMS, PROVIDE CONTINUOUS HIGH CHAIRS FOR METAL DECK (C.H.C.M.) @ 4'-0"CTS. WITH A HEIGHT TO SUPPORT THE BOTTOM MAT OF 'A' BARS A CLEAR DISTANCE OF 21/2" ABOVE THE TOP OF THE REMOVABLE FORM.

METAL STAY-IN-PLACE FORMS SHALL NOT BE WELDED TO BEAM OR GIRDER FLANGES IN THE ZONES REQUIRING CHARPY V-NOTCH TEST. SEE STRUCTURAL STEEL DETAIL SHEETS.

THE CONTRACTOR MAY, WHEN NECESSARY, PROPOSE A SCHEME FOR AVOIDING INTERFERENCE BETWEEN METAL STAY-IN-PLACE FORM SUPPORTS OR FORMS AND BEAM/GIRDER STIFFENERS OR CONNECTOR PLATES. THE PROPOSAL SHALL BE INDICATED, AS APPROPRIATE, ON EITHER THE STEEL WORKING DRAWINGS OR THE METAL STAY-IN-PLACE FORM WORKING DRAWINGS.

FOR WING ELEVATIONS AND DETAILS, SEE "PLAN OF SPAN DETAILS" SHEET.

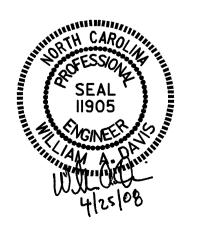
PROJECT NO. B-4244

RANDOLPH COUNTY

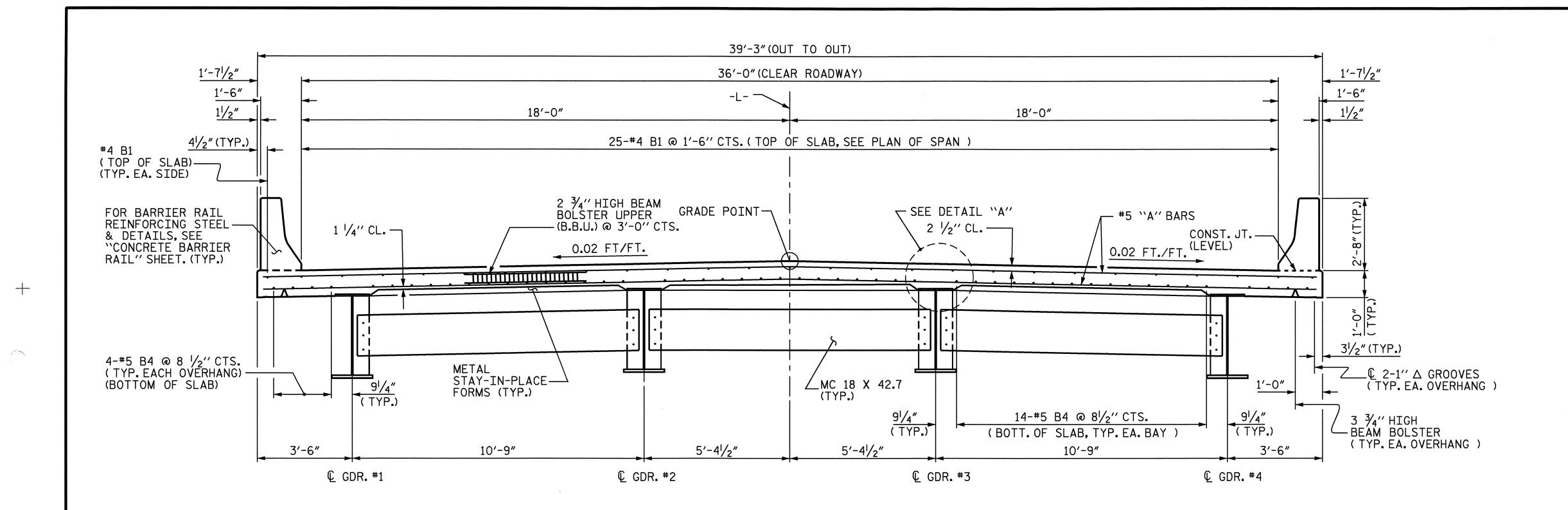
STATION: 18+10.00 -L-

SHEET 1 OF 2

DEPARTMENT OF TRANSPORTATION
RALEIGH
SUPERSTRUCTURE
TYPICAL SECTION

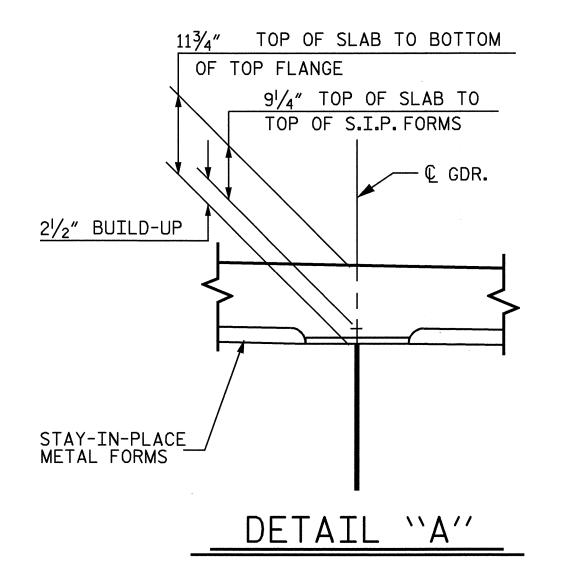


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T			3			TOTAL SHEETS
			4			25



TYPICAL SECTION

(SHOWING INTERMEDIATE DIAPHRAGMS)



PROJECT NO. B-4244

RANDOLPH COUNTY

STATION: 18+10.00 -L-

SHEET 2 OF 2

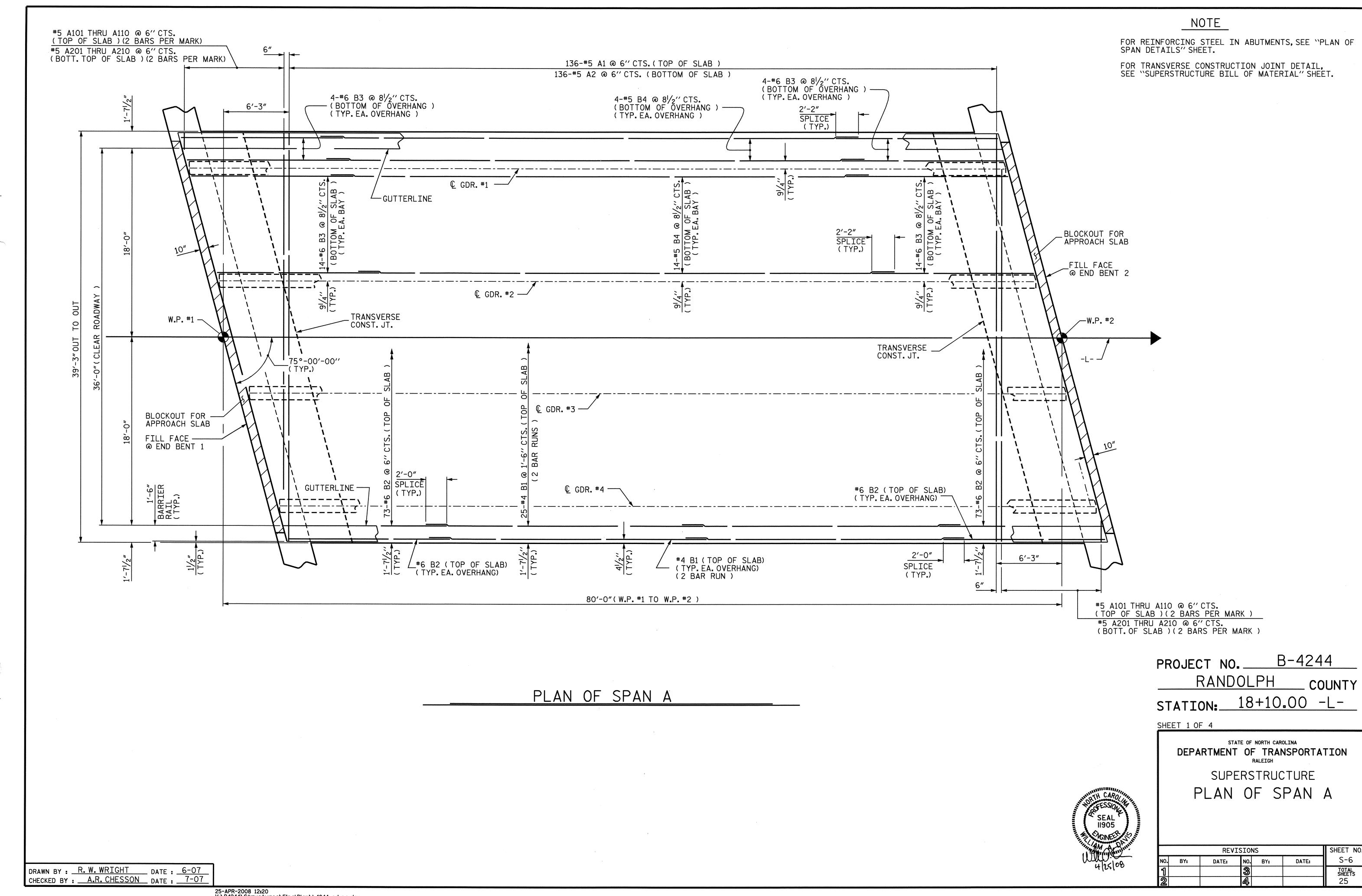
DEPARTMENT OF TRANSPORTATION
RALEIGH
SUPERSTRUCTURE
TYPICAL SECTION



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	3			TOTAL SHEETS
	4			25
		DATE: NO.		DATE: NO. BY: DATE:

DRAWN BY: R.W. WRIGHT DATE: 1-07
CHECKED BY: A.R. CHESSON DATE: 7-07

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12'-101/4'' 12′-101/4″ 3-#4 S2 SPACE TO MATCH #4 V1 BARS IN END BARRIER BENT CAP -RAIL (TYP. EA. SIDE) (TYP.) 3-#4 U1 SPACE TO MATCH #4 V1 BARS IN END BENT (TYP. EA. SIDE) (====-\\-\\ 2-#4 S1 -----SPACE TO MATCH -#4 V1 BARS IN END BENT (TYP. EA. SIDE) € GDR. #1 ---GUTTERLINE (TYP.) 10-#4 S1 SPACE TO MATCH #4 V1 BARS IN END 10-#4 U1 10-#4 S2 \
SPACE TO MATCH BENT (TYP. BETWEEN GDR'S.) SPACED TO MATCH #4 V1 BARS IN END #4 V1 BARS IN END V BENT (TYP. BETWEEN BENT (TYP. BETWEEN GDR'S.) GDR'S.) BLOCKOUT FOR -APPROACH SLAB FILL FACE -@ END BENT 1 € GDR. #2 — TRANSVERSE_CONST.JT. TRANSVERSE CONST. JT. W.P. #1 ─ 75°-00'-00'' BLOCKOUT FOR APPROACH SLAB -----_FILL FACE © GDR. #3 —∕ @ END BENT 2 10-#4 U1 10-#4 S2 ----SPACED TO MATCH SPACE TO MATCH #4 V1 BARS IN END -#4 V1 BARS IN END BENT (TYP. BETWEEN BENT (TYP. BETWEEN 10-#4 S1 GDR'S.) SPACE TO MATCH #4 V1 BARS IN END BENT (TYP. BETWEEN GDR'S.) € GDR. #4 ---3-#4 U1 SPACE TO MATCH ____ #4 V1 BARS IN END — BENT (TYP.EA.SIDE) 2-#4 S1 SPACE TO MATCH #4 V1 BARS IN END BENT (TYP.EA.SIDE) BARRIER RAIL — (TYP.) 1'-0" (TYP.) 3-#4 S2 W2) L GUTTERLINE SPACE TO MATCH —— #4 V1 BARS IN END BENT (TYP.EA.SIDE) (TYP.) 12'-101/4'' 12'-10¹/₄" PLAN OF ABUTMENT AT END BENT 1 PLAN OF ABUTMENT AT END BENT 2

NOTE

FOR REINFORCING STEEL IN ABUTMENTS, SEE "PLAN OF SPAN DETAILS" SHEET.

FOR TRANSVERSE CONSTRUCTION JOINT DETAIL, SEE "SUPERSTRUCTURE BILL OF MATERIAL" SHEET.

PROJECT NO. B-4244

RANDOLPH county

STATION: 18+10.00 -L-

SHEET 2 OF 4

SEAL 11905 STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION

RALEIGH

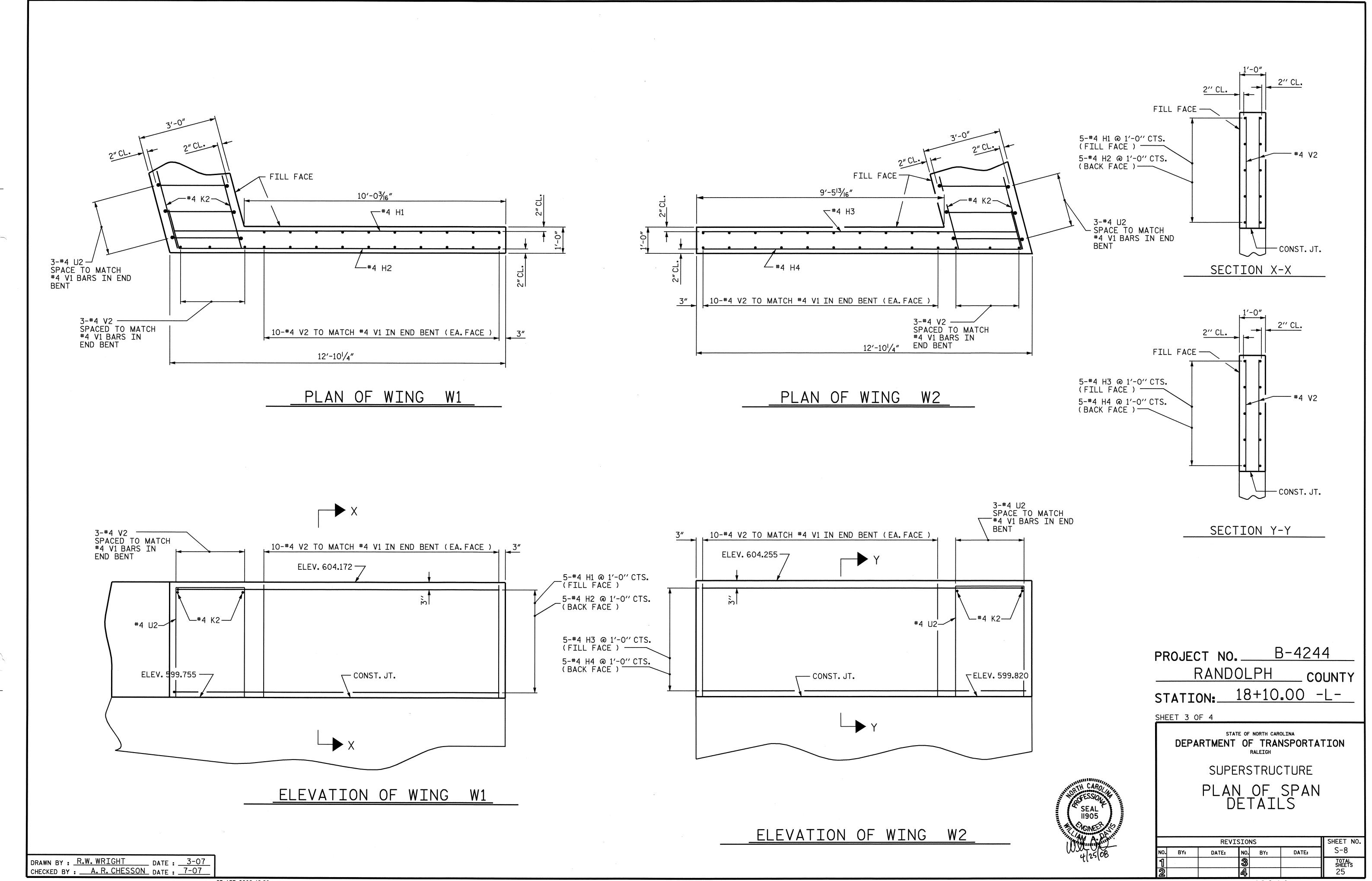
SUPERSTRUCTURE
PLAN OF SPAN
DETAILS

REVISIONS

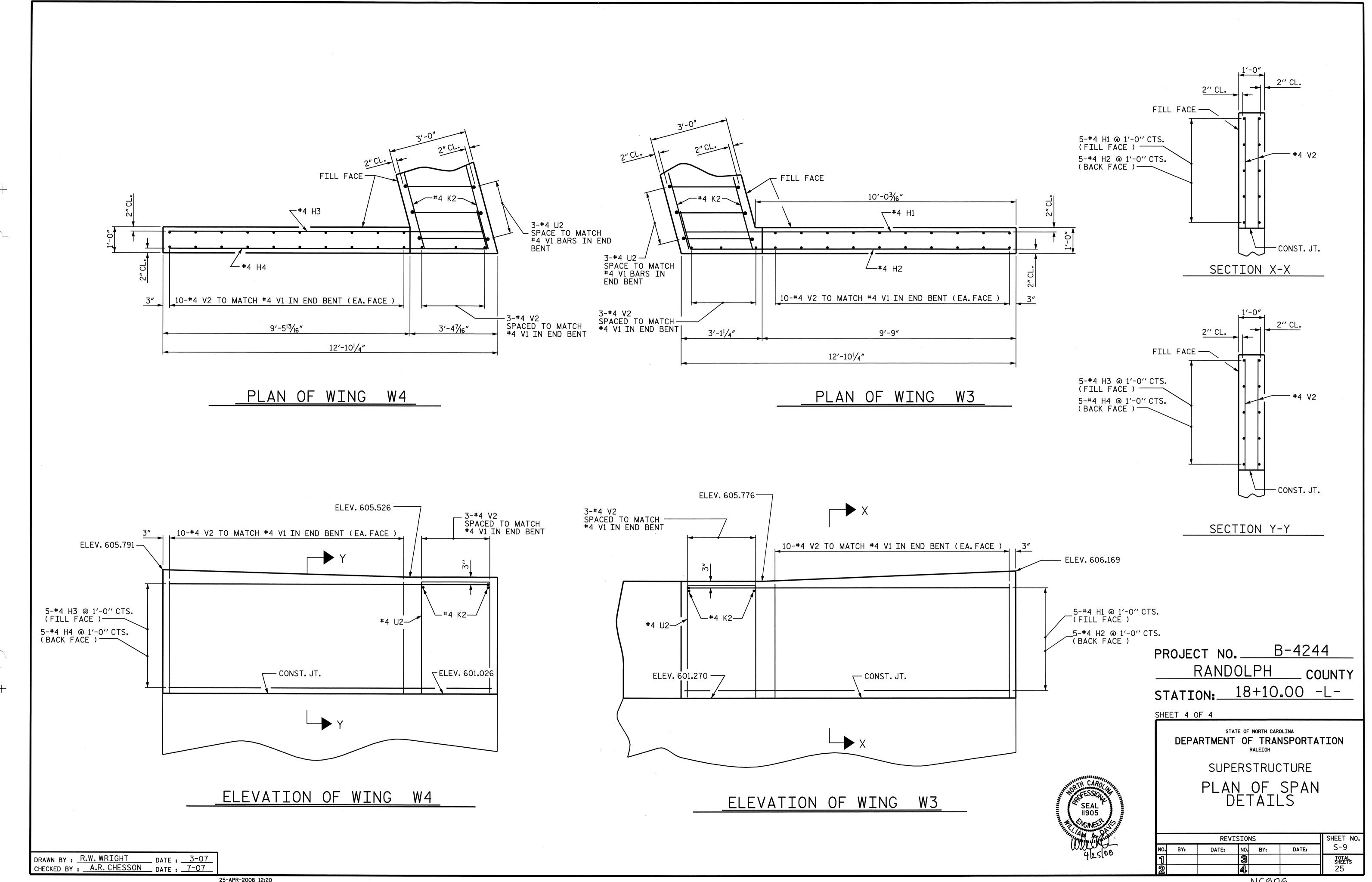
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TOTAL SHEETS
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DRAWN BY: R. W. WRIGHT DATE: 6-07
CHECKED BY: A.R. CHESSON DATE: 7-07

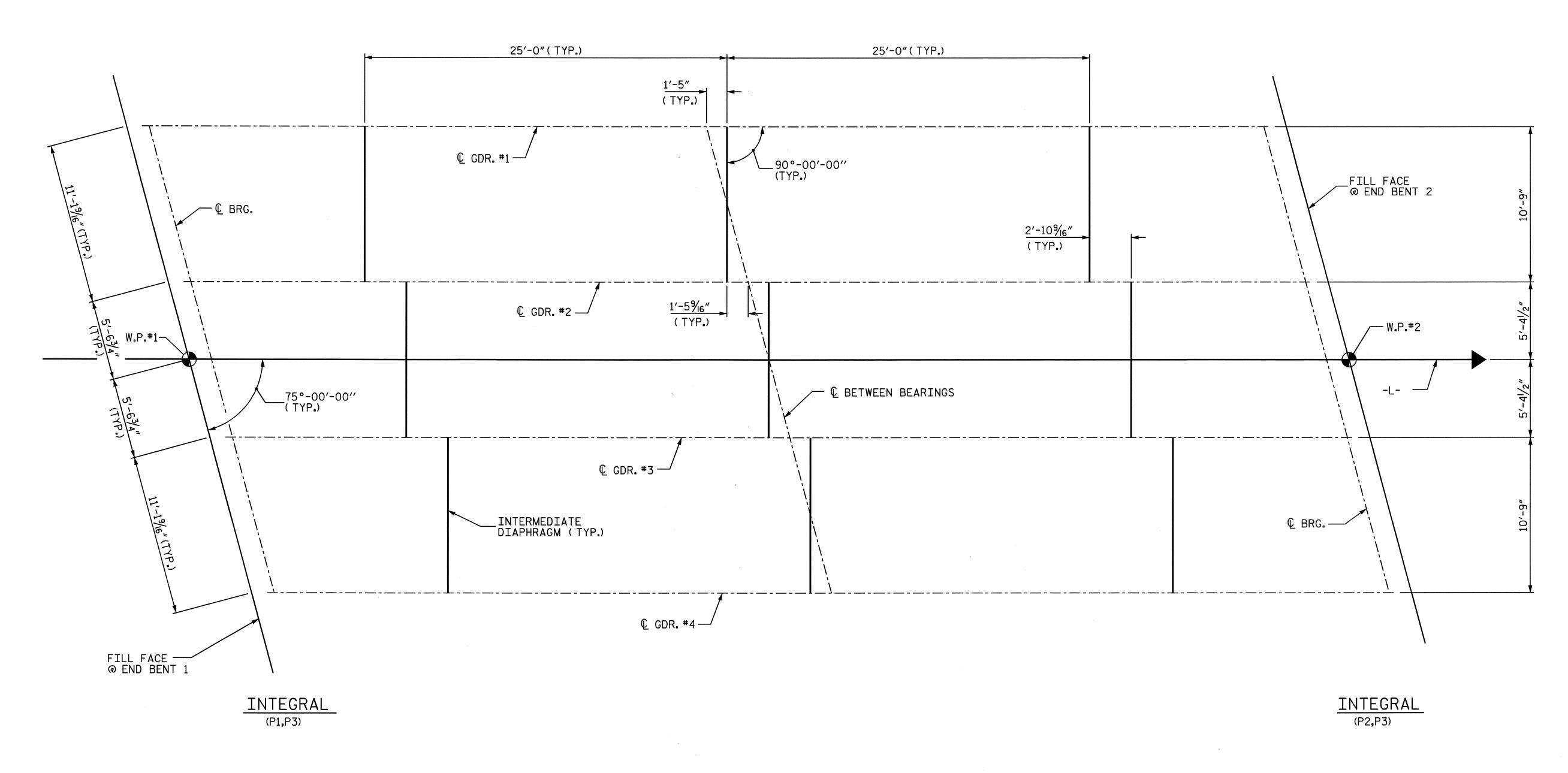


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NCBD5

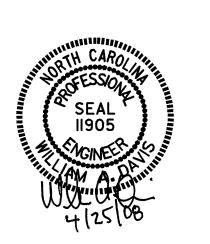


FRAMING PLAN_

PROJECT NO. B-4244

RANDOLPH COUNTY

STATION: 18+10.00 -L-



DEPARTMENT OF TRANSPORTATION
RALEIGH
SUPERSTRUCTURE
FRAMING PLAN

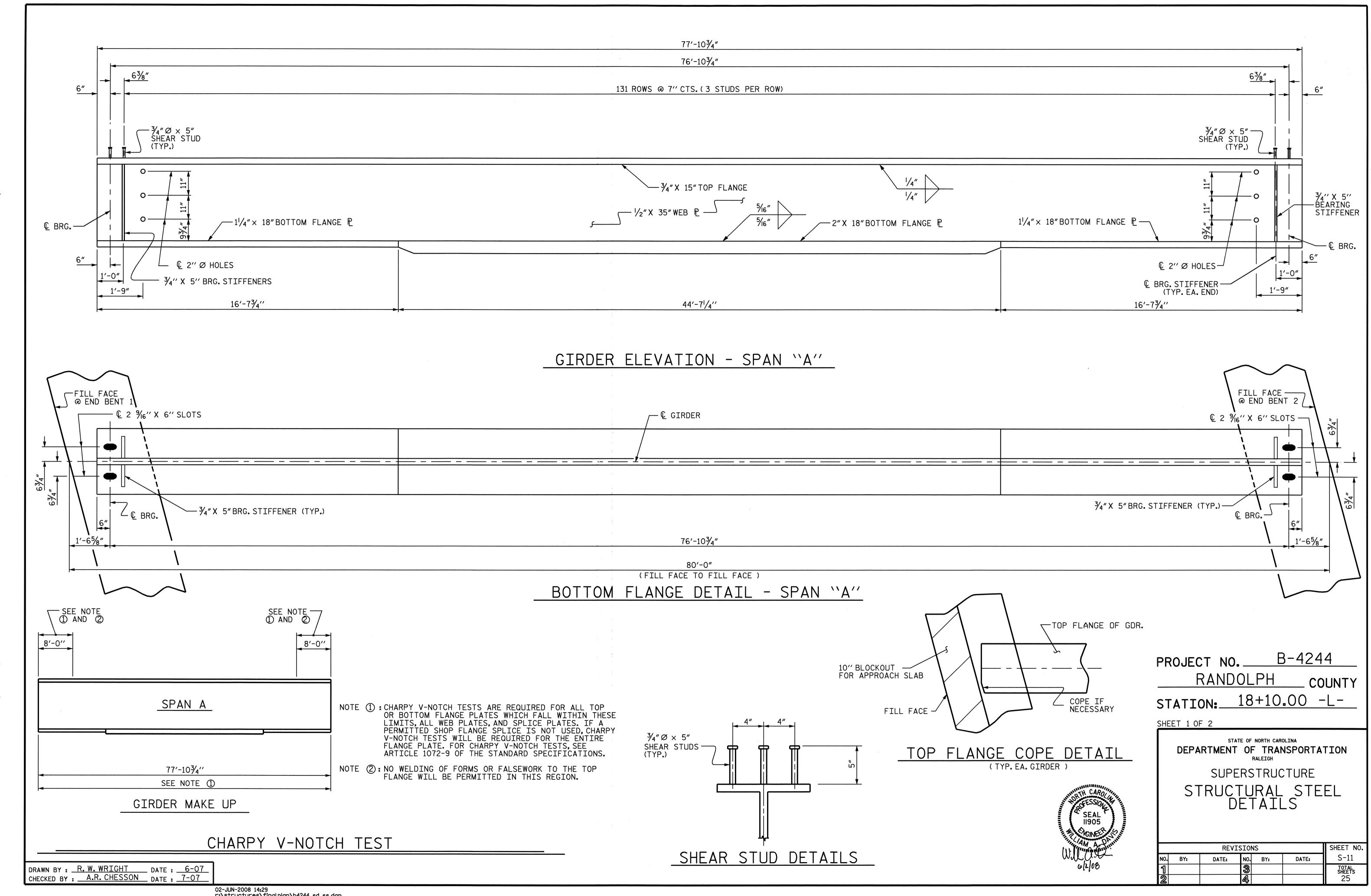
REVISIONS

BY: DATE: NO. BY: DATE: S-10

TOTAL SHEETS
25

DRAWN BY: R. W. WRIGHT DATE: 6-07
CHECKED BY: A.R. CHESSON DATE: 7-07

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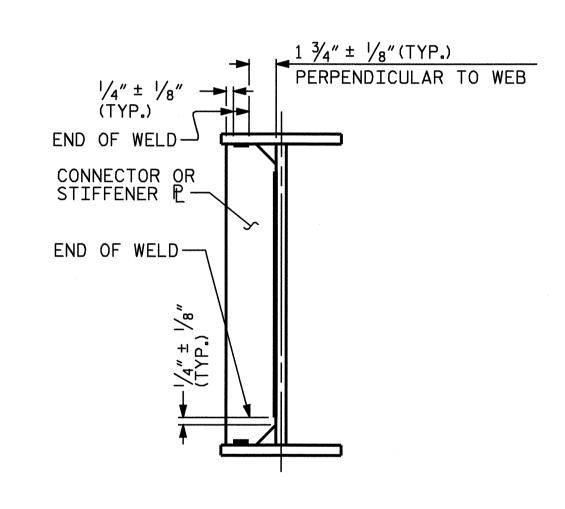
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DEAD LOAD DEF	LECT	ION	TAB	LE F	OR C	IRDI	ERS				
	GIRDERS 1 THRU 4										
TENTH POINTS	0	.1	.2	. 3	.4	. 5	.6	.7	.8	.9	0
DEFLECTION DUE TO WEIGHT OF GIRDER	0.000	0.012	0.023	0.032	0.037	0.039	0.037	0.032	0.023	0.012	0.000
*DEFLECTION DUE TO WEIGHT OF SLAB	0.000	0.039	0.103	0.151	0.182	0.193	0.183	0.152	0.103	0.039	0.000
DEFLECTION DUE TO WEIGHT OF BARRIER RAIL	0.000	0.004	0.008	0.010	0.012	0.012	0.012	0.010	0.008	0.004	0.000
TOTAL DEAD LOAD DEFLECTION	0.000	0.055	0.134	0.193	0.231	0.244	0.231	0.193	0.134	0.055	0.000
VERTICAL CURVE ORDINATE (SAG)	0.000	-0.072	-0.127	-0.167	-0.191	-0.199	-0.191	-0.167	-0.127	-0.072	0.000
							1 3				
REQUIRED CAMBER	0	- 3/16"	1/16′′	5/16′′	1/2"	9/16′′	1/2"	5/16′′	1/16′′	-3/16′′	0

^{*} INCLUDES SLAB, BUILDUPS & STAY-IN-PLACE FORMS.

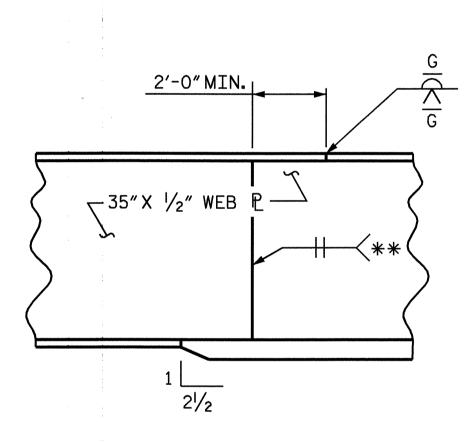
ALL VALUES ARE SHOWN IN FEET (DECIMAL FORM), EXCEPT "REQUIRED CAMBER", WHICH IS GIVEN IN INCHES (FRACTION FORM).

REQUIRED CAMBER VALUES SHOWN WITH "-" ARE DOWNWARD.



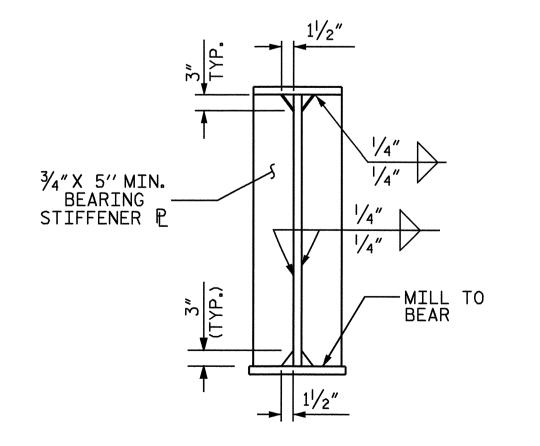
CONNECTOR PLATE CONNECTIONS

WELD TERMINATION DETAILS

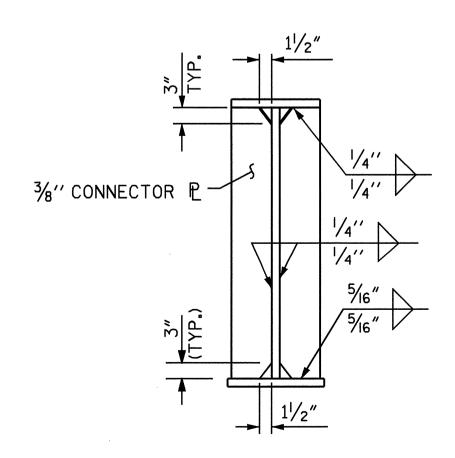


PERMISSIBLE SHOP FLANGE AND WEB SPLICE

** GRIND SMOOTH AND FLUSH ON OUTSIDE OF EXTERIOR GIRDERS



BEARING STIFFENER



CONNECTOR PLATE DETAILS

NOTES

ALL STRUCTURAL STEEL SHALL BE AASHTO M270 GRADE 50W. NO PAINTING OF STRUCTURAL STEEL IS REQUIRED.

ALL DIMENSIONS SHOWN ARE HORIZONTAL OR VERTICAL, UNLESS OTHERWISE NOTED.

ALL FIELD CONNECTIONS TO BE $\frac{7}{8}$ // Ø HIGH STRENGTH BOLTS UNLESS OTHERWISE NOTED.

SHOP SPLICES ARE PERMITTED TO LIMIT THE MAXIMUM REQUIRED FLANGE PIECE LENGTHS TO 60 FEET AND WEB PIECE LENGTHS TO 45 FEET. PERMITTED FLANGE AND WEB SHOP SPLICES SHALL NOT BE LOCATED WITHIN 15 FEET OF MAXIMUM DEAD LOAD DEFLECTION NOR WITHIN 15 FEET OF INTERMEDIATE BEARINGS OF CONTINUOUS UNITS. KEEP 2 FEET MINIMUM BETWEEN WEB AND FLANGE SHOP SPLICES. KEEP 6"MINIMUM BETWEEN CONNECTOR PLATE OR TRANSVERSE STIFFENER WELDS AND WEB OR FLANGE SHOP SPLICES.

STUDS ON GIRDERS MAY BE SHIFTED UP TO 1"IF NECESSARY TO CLEAR FLANGE SPLICE WELD.

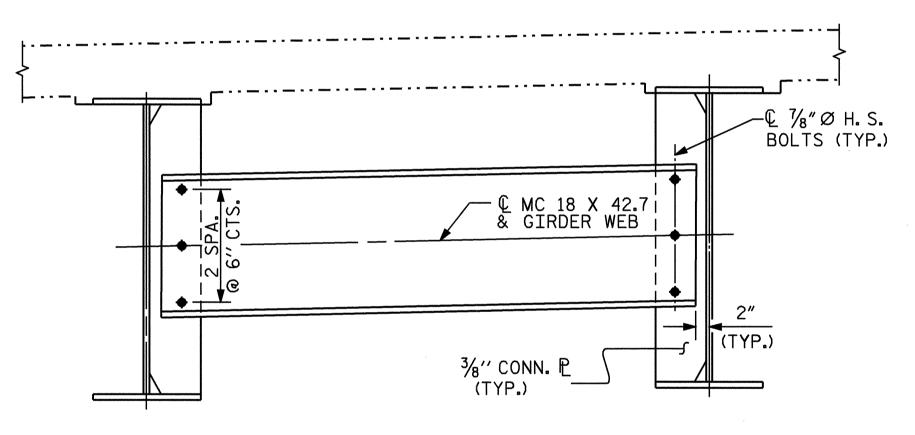
TENSION ON THE AASHTO M164 BOLTS SHALL BE CALIBRATED USING DIRECT TENSION INDICATOR WASHERS IN ACCORDANCE WITH ARTICLE 440-8 OF THE STANDARD SPECIFICATIONS.

BEARING STIFFENERS ARE TO BE PLACED NORMAL TO THE WEB OF THE GIRDER AND SHALL BE PLUMB.

END OF GIRDERS SHALL BE PLUMB.

FOR HIGH STRENGTH BOLTS, SEE SPECIAL PROVISIONS.

FOR SHIPPING STEEL STRUCTURAL MEMBERS, SEE SPECIAL PROVISIONS.



TYPICAL INTERMEDIATE DIAPHRAGM

PROJECT NO. B-4244

RANDOLPH COUNTY

STATION: 18+10.00 -L-

SHEET 2 OF 2

STATE OF NORTH CAROLINA

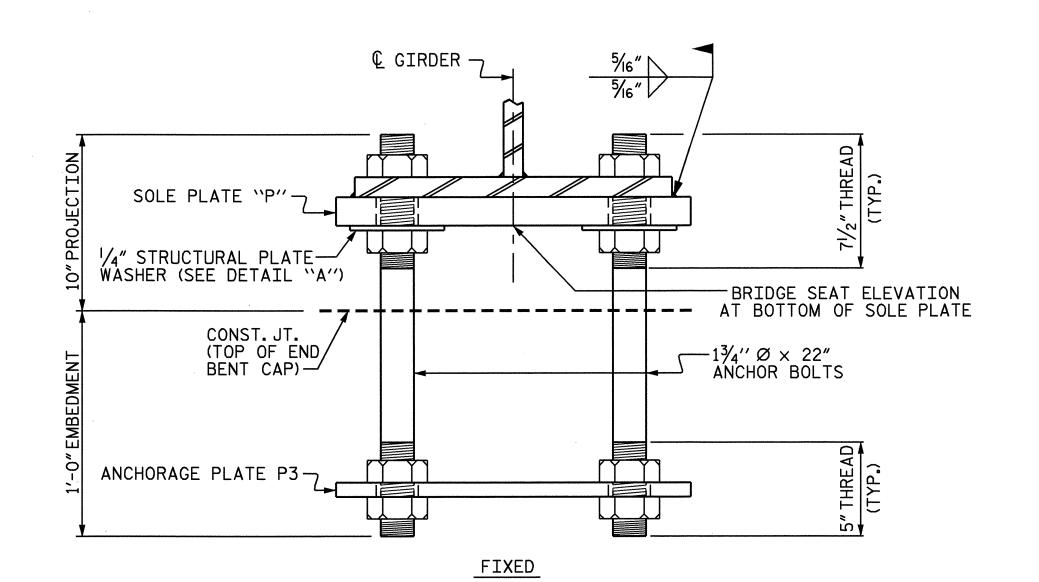
DEPARTMENT OF TRANSPORTATION
RALEIGH

SUPERSTRUCTURE STRUCTURAL STEEL DETAILS

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Y:	DATE:	NO.	BY:	DATE:	S-12
		3			TOTAL SHEETS
		4			25

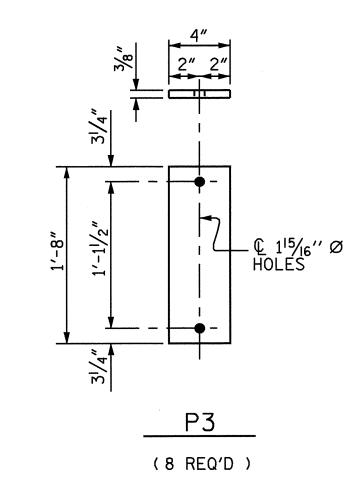
DRAWN BY: R. W. WRIGHT DATE: 6-07
CHECKED BY: A.R. CHESSON DATE: 7-07

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END VIEW 10" 5".5" \$\frac{1}{5}

SOLE PLATE DETAILS



ANCHORAGE PLATE DETAILS

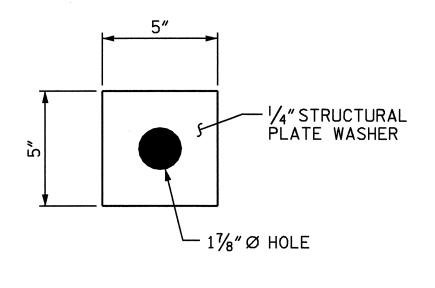


FOR AASHTO M270 GRADE 50W STRUCTURAL STEEL, SOLE PLATE SHALL BE AASHTO M270 GRADE 50W AND SHALL NOT BE GALVANIZED. ANCHOR BOLTS, NUTS AND WASHERS SHALL BE GALVANIZED IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

ANCHOR BOLTS SHALL MEET THE REQUIREMENTS OF ASTM A449. NUTS SHALL MEET THE REQUIREMENTS OF AASHTO M291-DH OR AASHTO M292-2H. WASHERS SHALL MEET THE REQUIREMENTS OF AASHTO M293. SHOP DRAWINGS ARE NOT REQUIRED FOR ANCHOR BOLTS, NUTS AND WASHERS. SHOP INSPECTION IS REQUIRED.

ALL SURFACES OF BEARING PLATES SHALL BE SMOOTH AND STRAIGHT.

STRUCTURAL PLATE WASHER SHALL BE AASHTO M270 GRADE 50W AND SHALL NOT BE GALVANIZED.



__DETAIL A

PROJECT NO. B-4244

RANDOLPH COUNTY

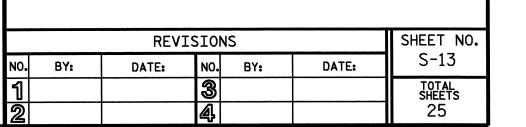
STATION: 18+10.00 -L-

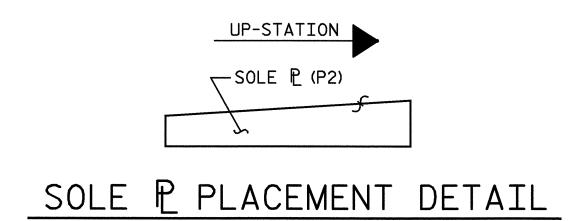
STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION

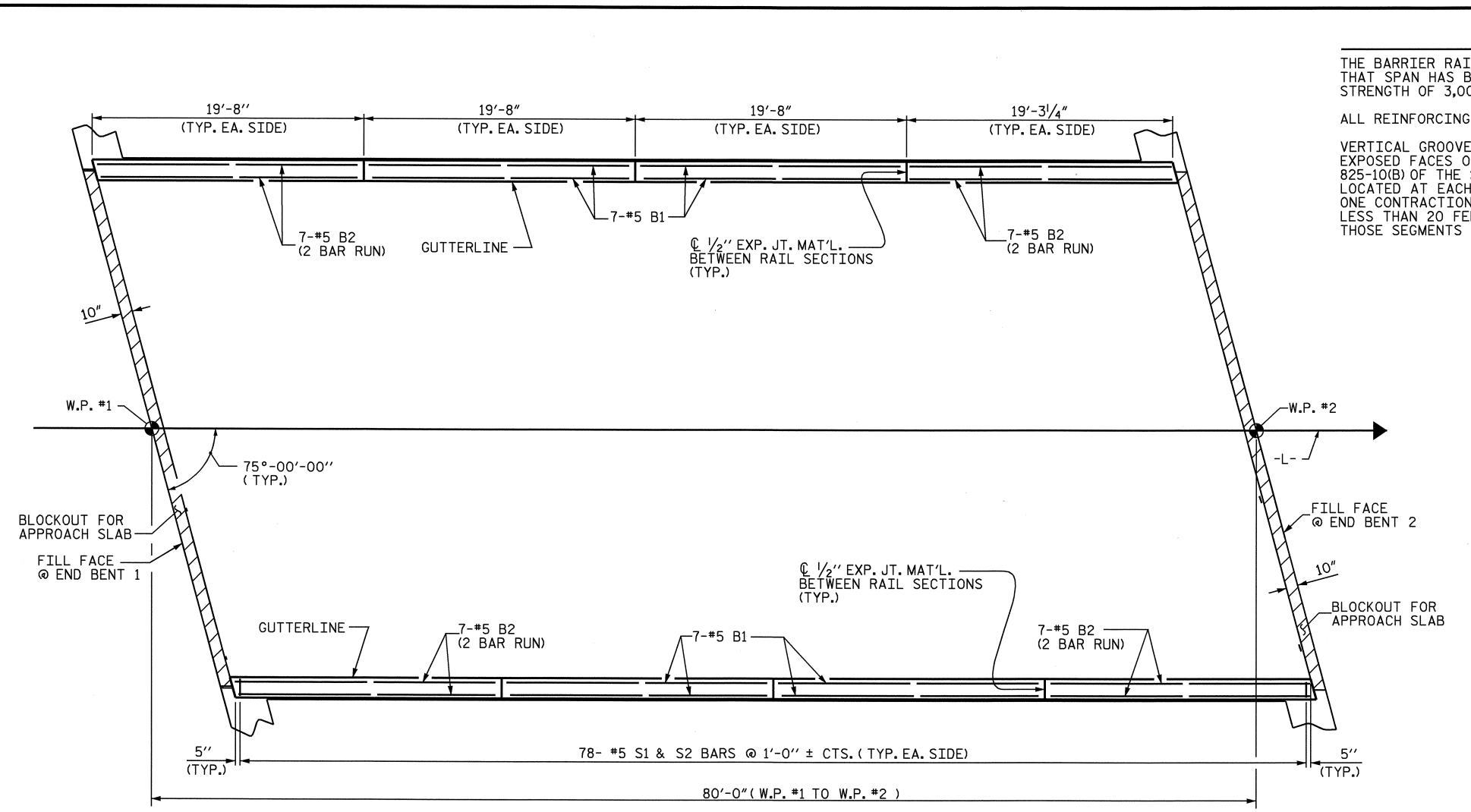
RALEIGH

SUPERSTRUCTURE
BEARING DETAILS





DRAWN BY: R.W.WRIGHT DATE: 6-07
CHECKED BY: A.R. CHESSON DATE: 7-07

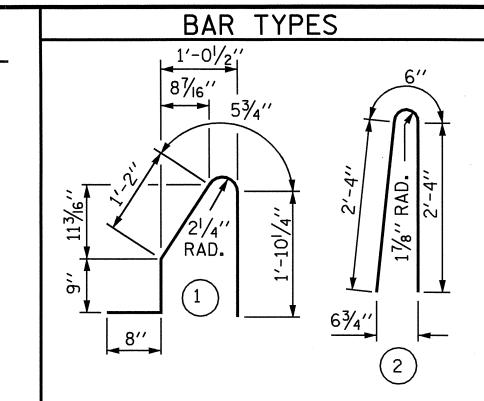


THE BARRIER RAIL SHALL NOT BE CAST UNTIL ALL SLAB CONCRETE IN THAT SPAN HAS BEEN CAST AND HAS REACHED A MINIMUM COMPRESSIVE STRENGTH OF 3,000 PSI.

ALL REINFORCING STEEL IN BARRIER RAILS SHALL BE EPOXY COATED.

VERTICAL GROOVED CONTRACTION JOINTS, $\frac{1}{2}$ " IN DEPTH, SHALL BE TOOLED IN ALL EXPOSED FACES OF THE BARRIER RAIL AND IN ACCORDANCE WITH ARTICLE 825-10(B) OF THE STANDARD SPECIFICATIONS. THE CONTRACTION JOINT SHALL BE LOCATED AT EACH THIRD POINT BETWEEN BARRIER RAIL EXPANSION JOINTS. ONLY ONE CONTRACTION JOINT IS REQUIRED AT MIDPOINT OF BARRIER RAIL SEGMENTS LESS THAN 20 FEET IN LENGTH AND NO CONTRACTION JOINTS ARE REQUIRED FOR THOSE SEGMENTS LESS THAN 10 FEET IN LENGTH.

NOTES



ALL BAR DIMENSIONS ARE OUT TO OUT

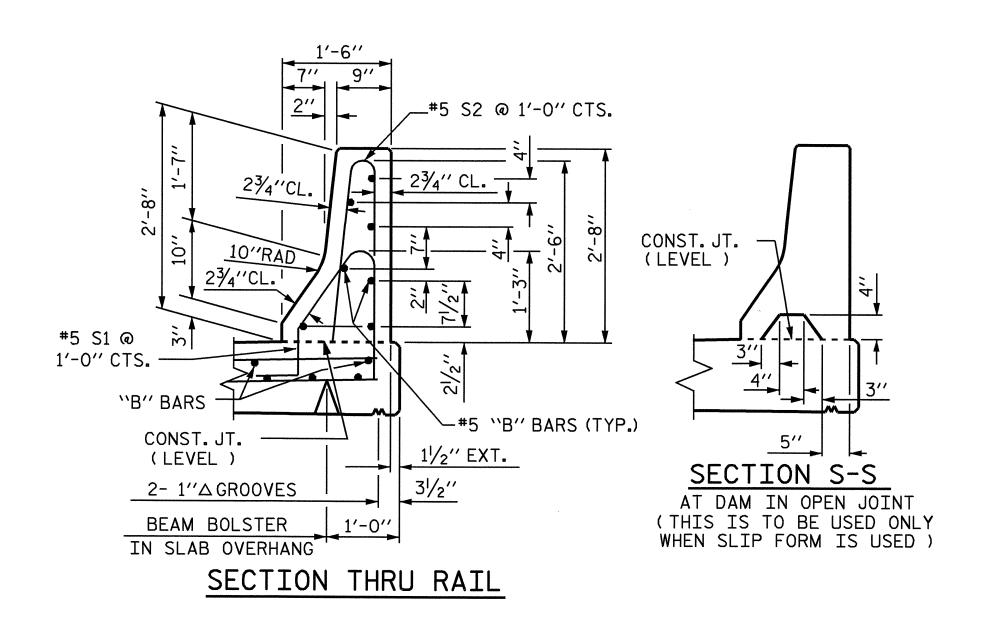
	BIL	L OF	MA	TERIA						
FOR CONCRETE BARRIER RAIL ONLY										
BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT					
· B1	28	#5	STR	19'-4''	565					
· B2	56	#5	STR	11'-4''	662					
S1	156	#5	1	4'-11''	800					
S2	156	#5	2	5'-2''	841					
		·								

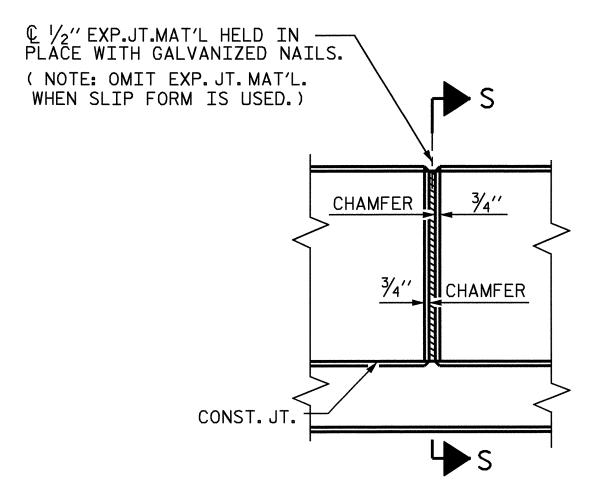
* EPOXY COATED REINFORCING STEEL

CLASS AA CONCRETE 15.7 CU. YDS. CONCRETE BARRIER RAIL 156.54 LIN. FT.

2868 LBS

PLAN OF BARRIER RAIL





ELEVATION AT EXPANSION JOINTS BARRIER RAIL DETAILS

PROJECT NO. B-4244 RANDOLPH COUNTY STATION: 18+10.00 -L-

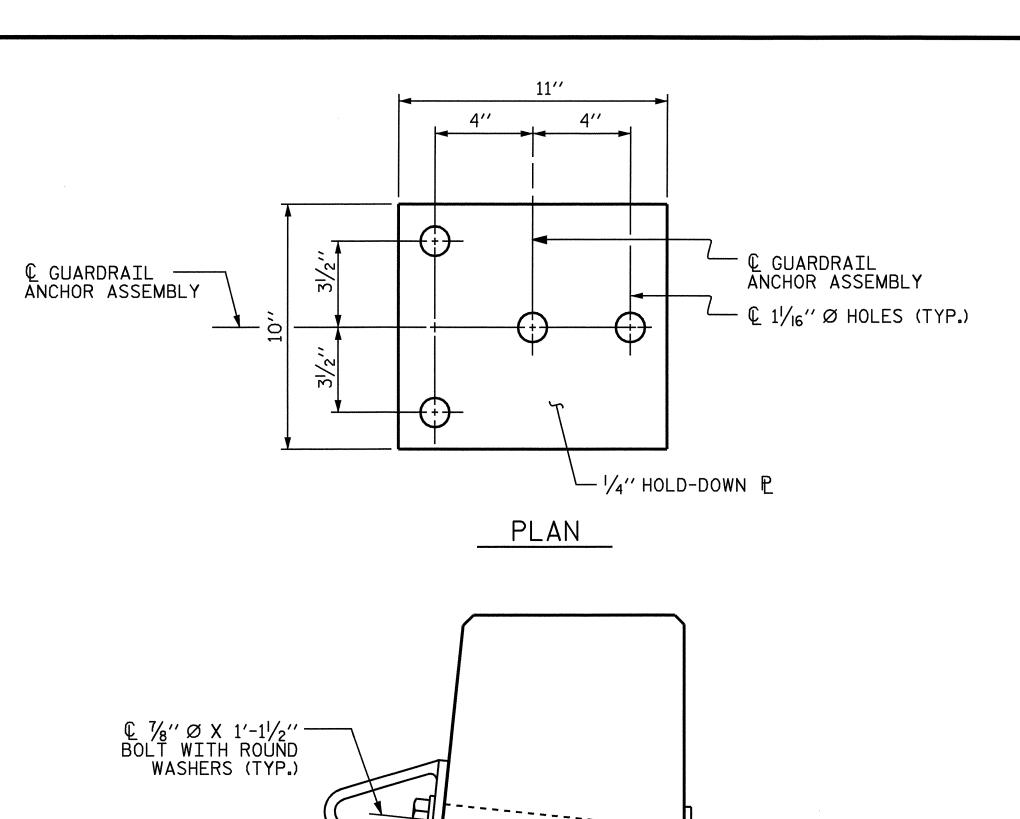
STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION STANDARD CONCRETE BARRIER RAIL

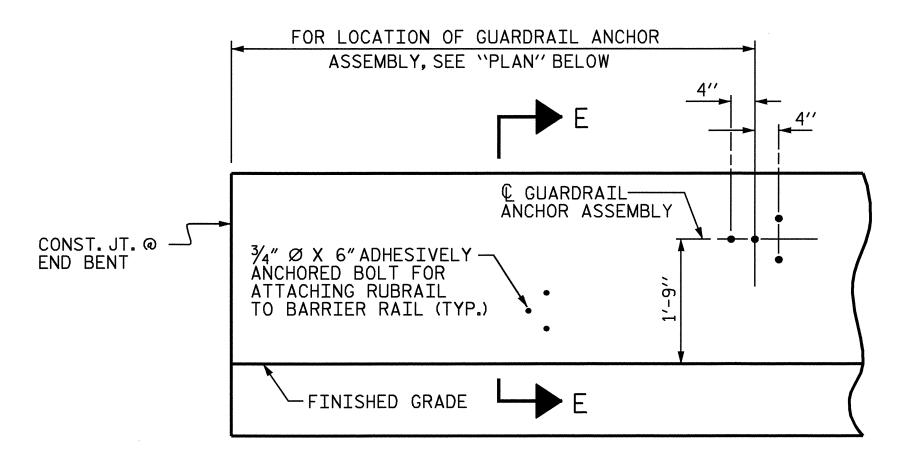
	REVI	SIONS	1		SHEET NO.
BY:	DATE:	NO.	BY:	DATE:	S-14
		3			TOTAL SHEETS
		4			25

ASSEMBLED BY: R. W. WRIGHT DATE: 06-07 CHECKED BY: A.R. CHESSON DATE: 7-07 REV. 10/17/00 REV. 5/7/03R REV. 5/1/06 RWW/LES RWW/JTE TLA/GM DRAWN BY: ARB 5/87 CHECKED BY: SJD 9/87

25-APR-2008 12:20 V:\B4244\Structures\FinalPlan\B4244_sd_BR.dgn

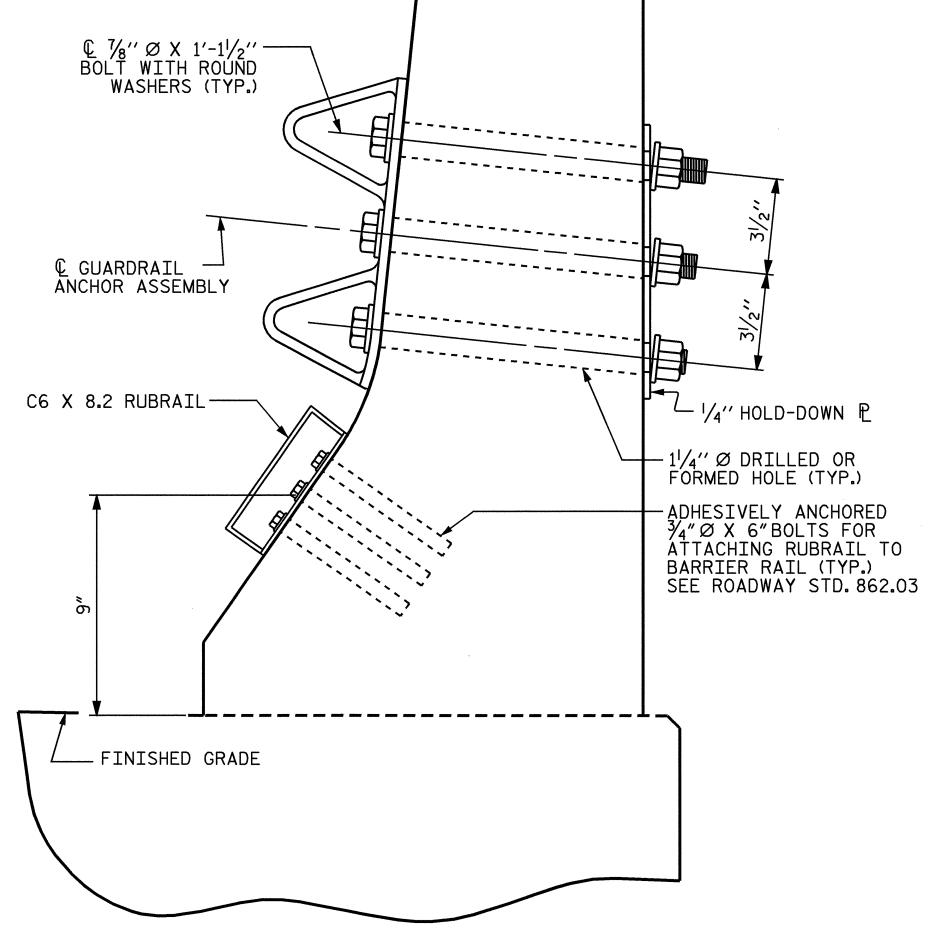
STD. NO. CBR1 (SHT 3)





ELEVATION

FOR LOCATION OF RUBRAIL, SEE ROADWAY STD. 862.03



GUARDRAIL
ANCHOR
ASSEMBLY

6'-7¾"

CONST. JT. @
GUARDRAIL
A"

GUARDRAIL
A"

GUARDRAIL
A"

ANCHOR
ANCHOR
ANCHOR
ASSEMBLY

PLAN

LOCATION OF ANCHORS FOR GUARDRAIL

END BENT #1 SHOWN, END BENT #2 SIMILAR.

NOTES

THE GUARDRAIL ANCHOR ASSEMBLY SHALL CONSIST OF A $\frac{1}{4}$ " HOLD DOWN PLATE AND 4 - $\frac{7}{8}$ " Ø BOLTS WITH NUTS AND WASHERS, RUBRAIL, AND ADHESIVELY ANCHORED BOLTS.

THE HOLD-DOWN PLATE SHALL CONFORM TO AASHTO M270 GRADE 36.AFTER FABRICATION, THE HOLD-DOWN PLATE SHALL BE HOT-DIP GALVANIZED IN ACCORDANCE WITH AASHTO M111.

BOLTS SHALL CONFORM TO THE REQUIREMENTS OF ASTM A307 AND NUTS SHALL CONFORM TO THE REQUIREMENTS OF AASHTO M291. BOLTS, NUTS AND WASHERS SHALL BE GALVANIZED. (AT THE CONTRACTOR'S OPTION, STAINLESS STEEL BOLTS, NUTS AND WASHERS MAY BE USED AS AN ALTERNATE FOR THE 1/8" Ø GALVANIZED BOLTS, NUTS AND WASHERS. THEY SHALL CONFORM TO OR EXCEED THE MECHANICAL REQUIREMENTS OF ASTM A307. THE USE OF THIS ALTERNATE SHALL BE APPROVED BY THE ENGINEER.)

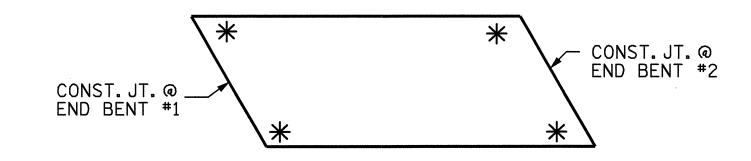
THE GUARDRAIL ANCHOR ASSEMBLY IS REQUIRED AT ALL POINTS WHERE APPROACH GUARDRAIL IS TO BE ATTACHED TO THE END OF BARRIER RAIL. FOR POINTS OF ATTACHMENT, SEE SKETCH.

AFTER INSTALLATION, THE EXPOSED THREAD OF THE BOLT SHALL BE BURRED WITH A SHARP POINTED TOOL.

THE COST OF THE GUARDRAIL ANCHOR ASSEMBLY SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR CONCRETE BARRIER RAIL.

THE 1 $\frac{1}{4}$ $^{\prime\prime}$ Ø HOLES SHALL BE FORMED OR DRILLED WITH A CORE BIT. IMPACT TOOLS WILL NOT BE PERMITTED. ANY CONCRETE DAMAGED BY THIS WORK SHALL BE REPAIRED TO THE SATISFACTION OF THE ENGINEER.

THE C6 X 8.2 RUBRAIL IS TO BE ADHESIVELY ANCHORED TO THE RAIL USING THREE 3/4" Ø X 6"BOLTS WITH WASHERS. LEVEL ONE FIELD TESTING IS REQUIRED, AND THE YIELD LOAD OF THE 3/4" Ø BOLT IS 12 KIPS. FOR ADHESIVELY ANCHORED ANCHOR BOLTS OR DOWELS, SEE SPECIAL PROVISIONS. SEE ROADWAY STANDARD 862.03 FOR DETAILS AND LOCATION OF THE RUBRAIL.



SKETCH SHOWING POINTS OF ATTACHMENTS

* DENOTES GUARDRAIL ANCHOR ASSEMBLY

PROJECT NO. B-4244

RANDOLPH COUNTY

STATION: 18+10.00 -L-

DEPARTMENT OF TRANSPORTATION
RALEIGH
STANDARD

F

GUARDRAIL ANCHORAGE FOR BARRIER RAIL

REVISIONS

BY: DATE: NO. BY: DATE: S-15

TOTAL SHEETS
25

ASSEMBLED BY: R. W. WRIGHT DATE: 4-Ø7 CHECKED BY: A.R. CHESSON DATE: 7-Ø7

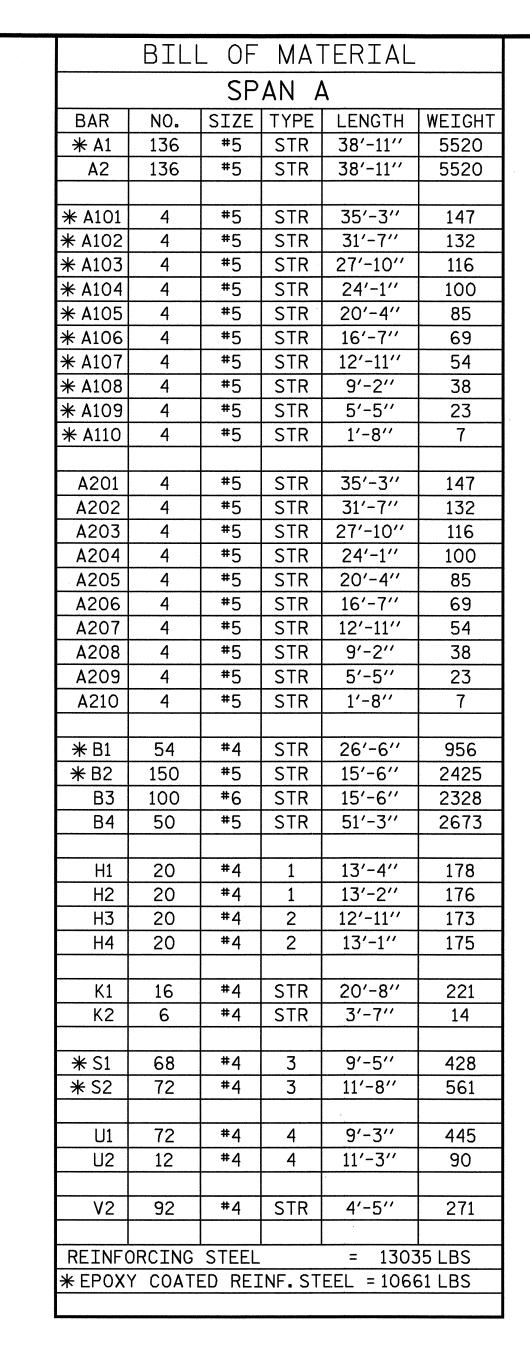
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CHECKED BY: GM 5/06

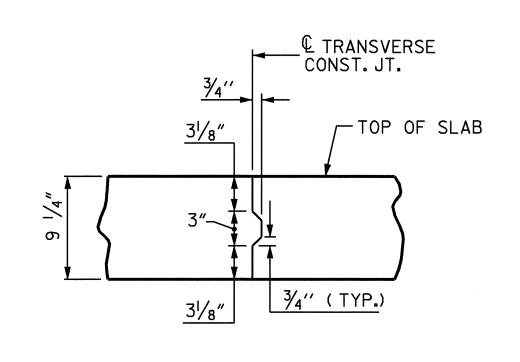
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SECTION E-E

GUARDRAIL ANCHOR ASSEMBLY DETAILS

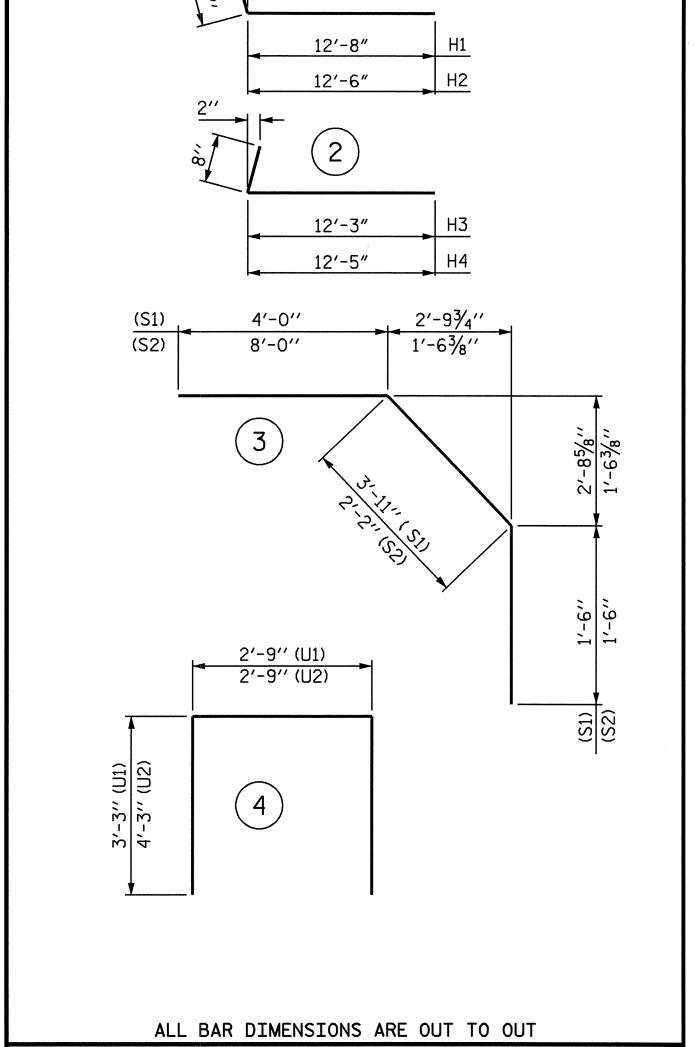
(SHT 3) STD. NO. GRA2





TRANSVERSE CONSTRUCTION JOINT

NOTE: REINFORCING STEEL IN SLAB NOT SHOWN.
LONGITUDINAL REINFORCING STEEL SHALL BE
CONTINUOUS THRU JOINT



-BAR TYPES

— SUP	ERSTRUCT	URE BILL OF	MATERIAL —
	CLASS AA CONCRETE	REINFORCING STEEL	EPOXY COATED REINFORCING STEEL
	(CU.YDS.)	(LBS.)	(LBS.)
SPAN "A"		13035	10661
POUR #1	95.2		
POUR #2	56.5		
			,
TOTALS**	151.7	13035	10661

**QUANTITIES FOR BARRIER RAIL ARE NOT INCLUDED

	GROOVING	BRIDGE	FL	00RS
	APPROACH SLABS			SQ.FT.
-	BRIDGE DECK		2572	SQ.FT.
	TOTAL		3419	SQ.FT.

PROJECT NO. B-4244

RANDOLPH COUNTY

STATION: 18+10.00 -L-

STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION

RALEIGH

STANDARD

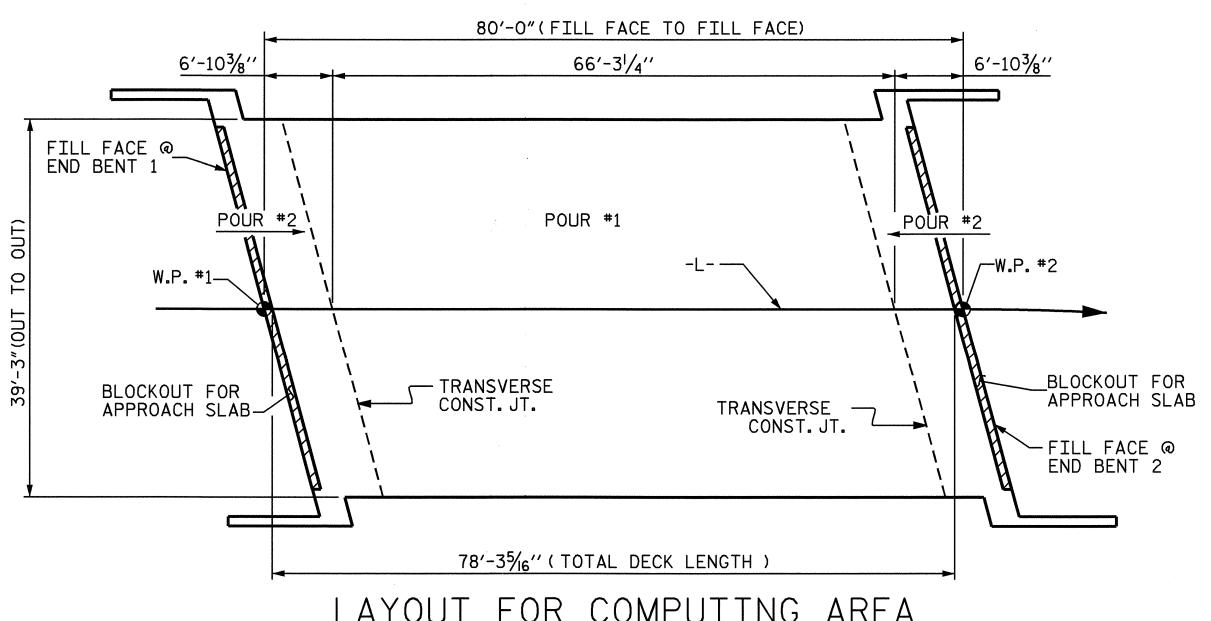
SUPERSTRUCTURE BILL OF MATERIAL

		SHEET NO.				
0.	BY:	DATE:	NO.	BY:	DATE:	S-16
			3			TOTAL SHEETS
2			4			25

							1
							FI EN
		JCTURE S ARE			STEEL E	Ĺ .	
FOLL	OWING	MININ	MUM SF	PLICE L	ENGTHS	OUT)	
BAR SIZE	SUPERSTF EXCEPT A SLABS, PA AND BARR	APPROACH ARAPET,	APPROAC	H SLABS	PARAPET AND BARRIER	, (OUT TO	
	EPOXY COATED	UNCOATED	EPOXY COATED	UNCOATED	RAIL	39′-3″ (TUO	
#4	2'-0"	1'-9"	2'-0"	1'-9"	2'-9"		
#5	2'-6"	2'-2"	2′-6″	2'-2"	3′-5″		
#6	3′-0″	2'-7"	3′-10″	2'-7"	4'-4"	-	<u> </u>
#7	5′-3″	3′-6″					
#8	6'-10"	4'-7"					

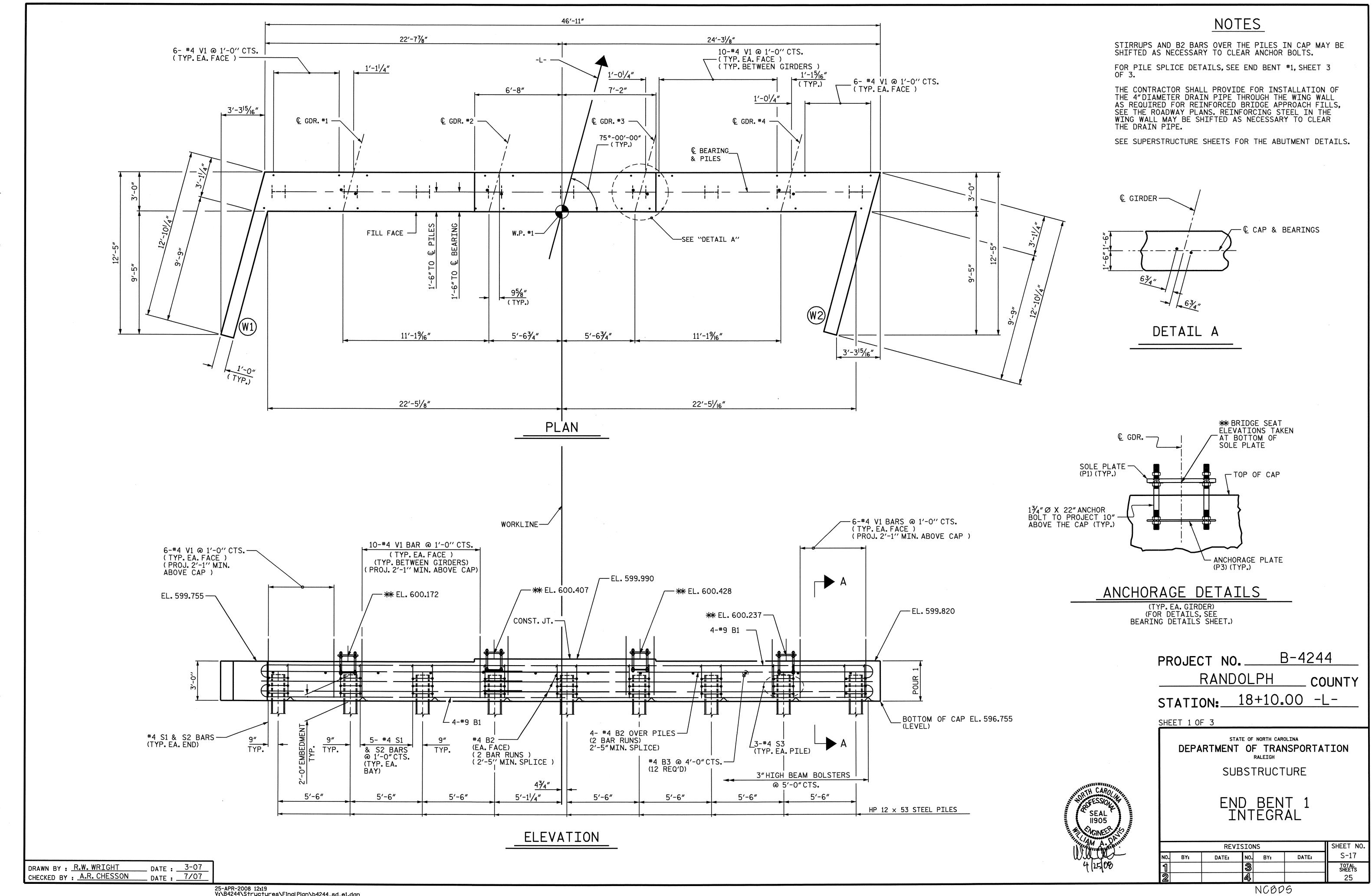
ASSEMBLED BY: R. W. WRIGHT DATE: 6-07
CHECKED BY: A.R. CHESSON DATE: 7-07

DRAWN BY: JMB 5/87
CHECKED BY: SJD 9/87
REV. 6/1/94
REV. 8/16/99
REV. 5/1/06
REV. 5/1/06

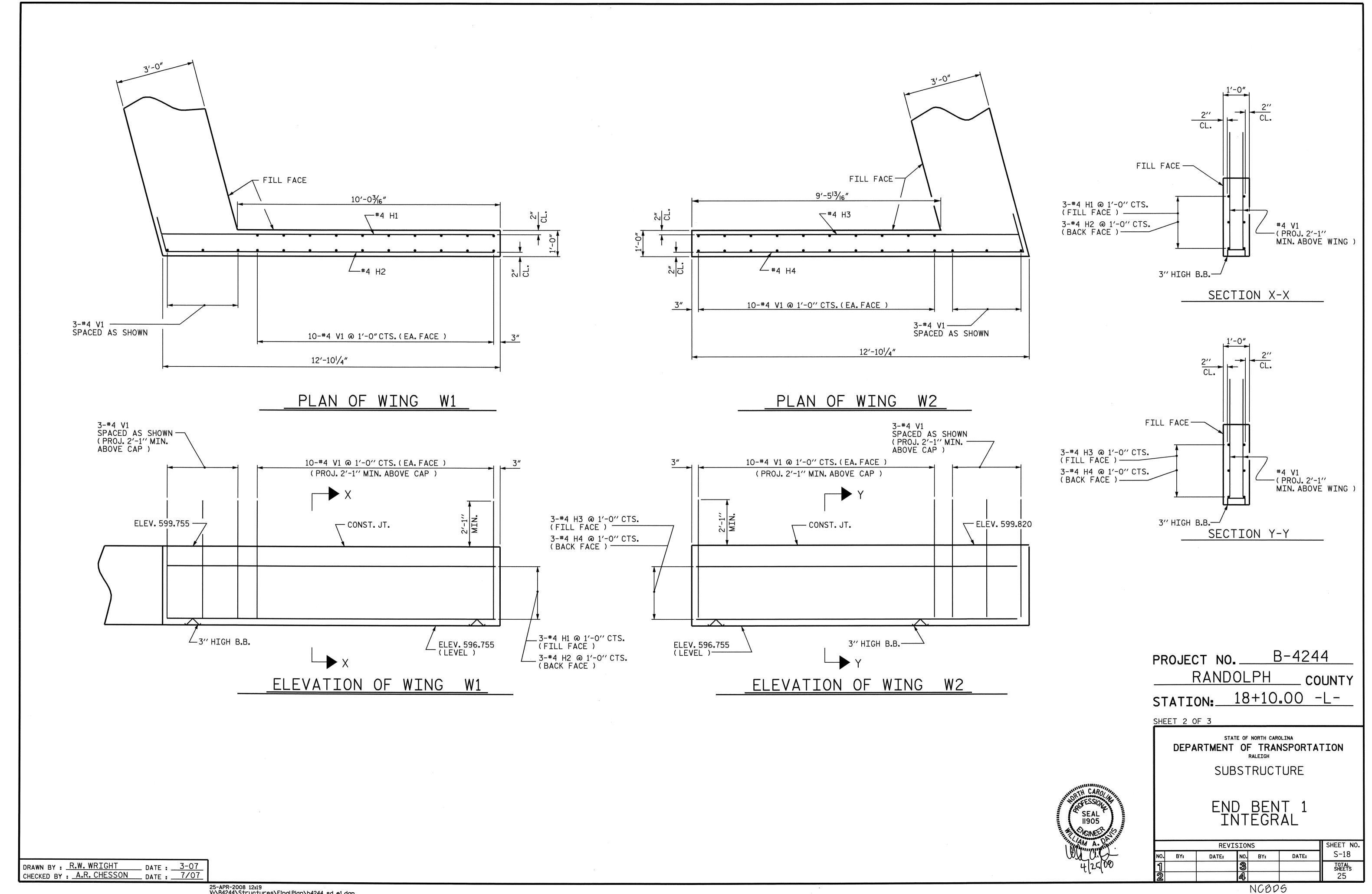


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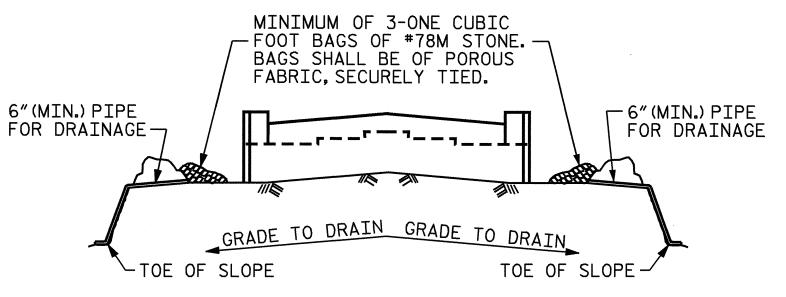
STD. NO. BOM1



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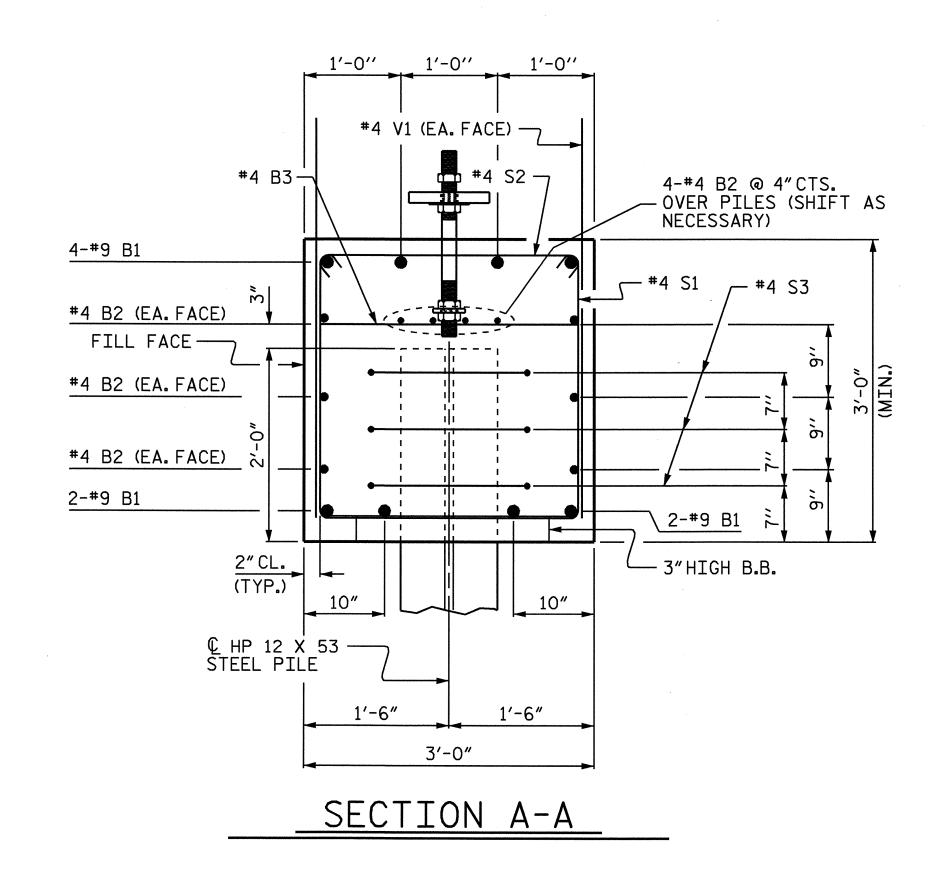


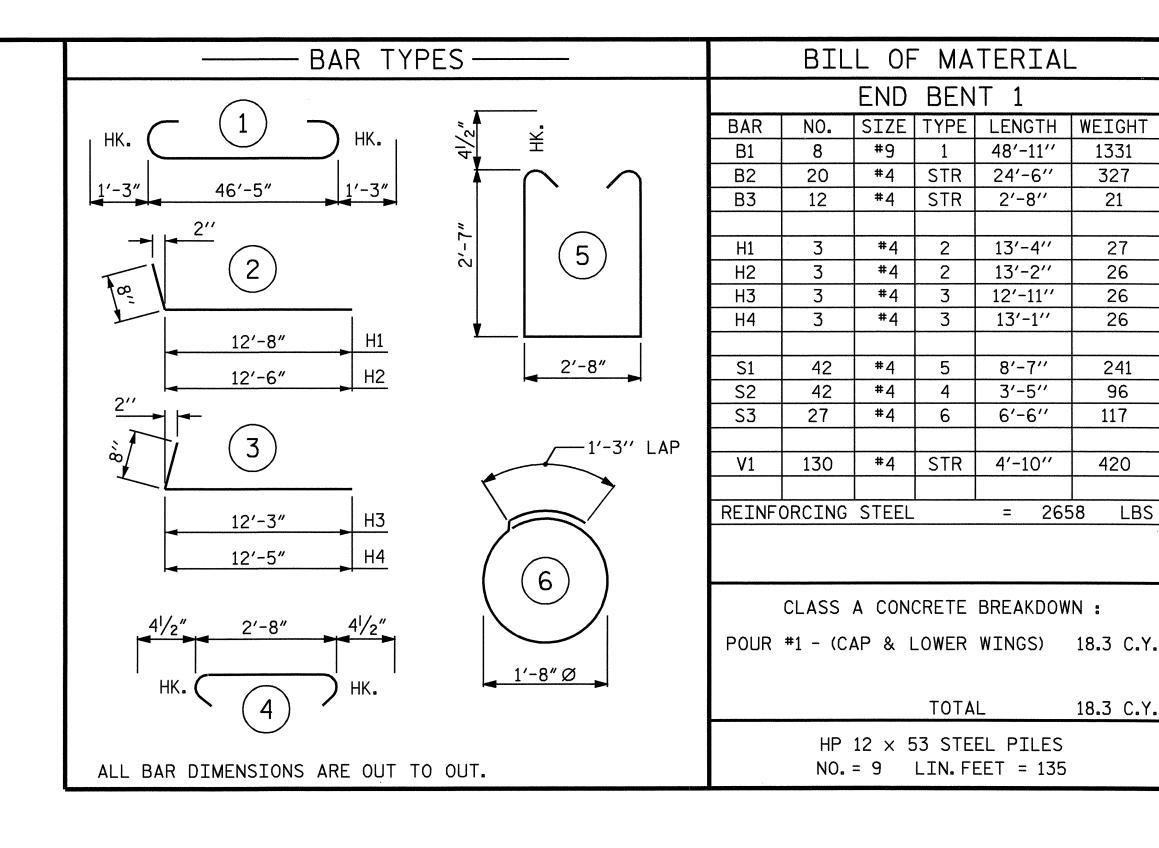
BAGGED STONE AND PIPE SHALL BE PLACED IMMEDIATELY AFTER COMPLETION OF END BENT EXCAVATION. PIPE MAY BE EITHER CONCRETE, CORRUGATED STEEL, CORRUGATED ALUMINUM ALLOY, OR CORRUGATED PLASTIC. PERFORATED PIPE WILL NOT BE ALLOWED.

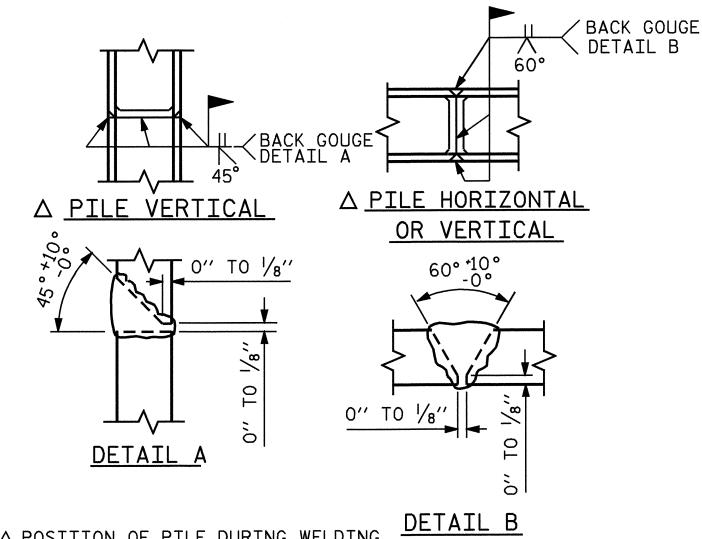
BAGGED STONE SHALL REMAIN IN PLACE UNTIL THE ENGINEER DIRECTS THAT IT BE REMOVED. THE CONTRACTOR SHALL REMOVE AND DISPOSE OF SILT ACCUMULATIONS AT BAGGED STONE WHEN SO DIRECTED BY THE ENGINEER. BAGS SHALL BE REMOVED AND REPLACED WHENEVER THE ENGINEER DETER-MINES THAT THEY HAVE DETERIORATED AND LOST THEIR EFFECTIVENESS.

NO SEPARATE PAYMENT WILL BE MADE FOR THIS WORK AND THE ENTIRE COST OF THIS WORK SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR THE SEVERAL PAY ITEMS.

TEMPORARY DRAINAGE AT END BENT







Δ POSITION OF PILE DURING WELDING. DETAIL B

PILE SPLICE DETAILS

B-4244 PROJECT NO. ____ RANDOLPH COUNTY 18+10.00 -L-STATION:_

327

21

27

26

26

26

241

96

117

18.3 C.Y.

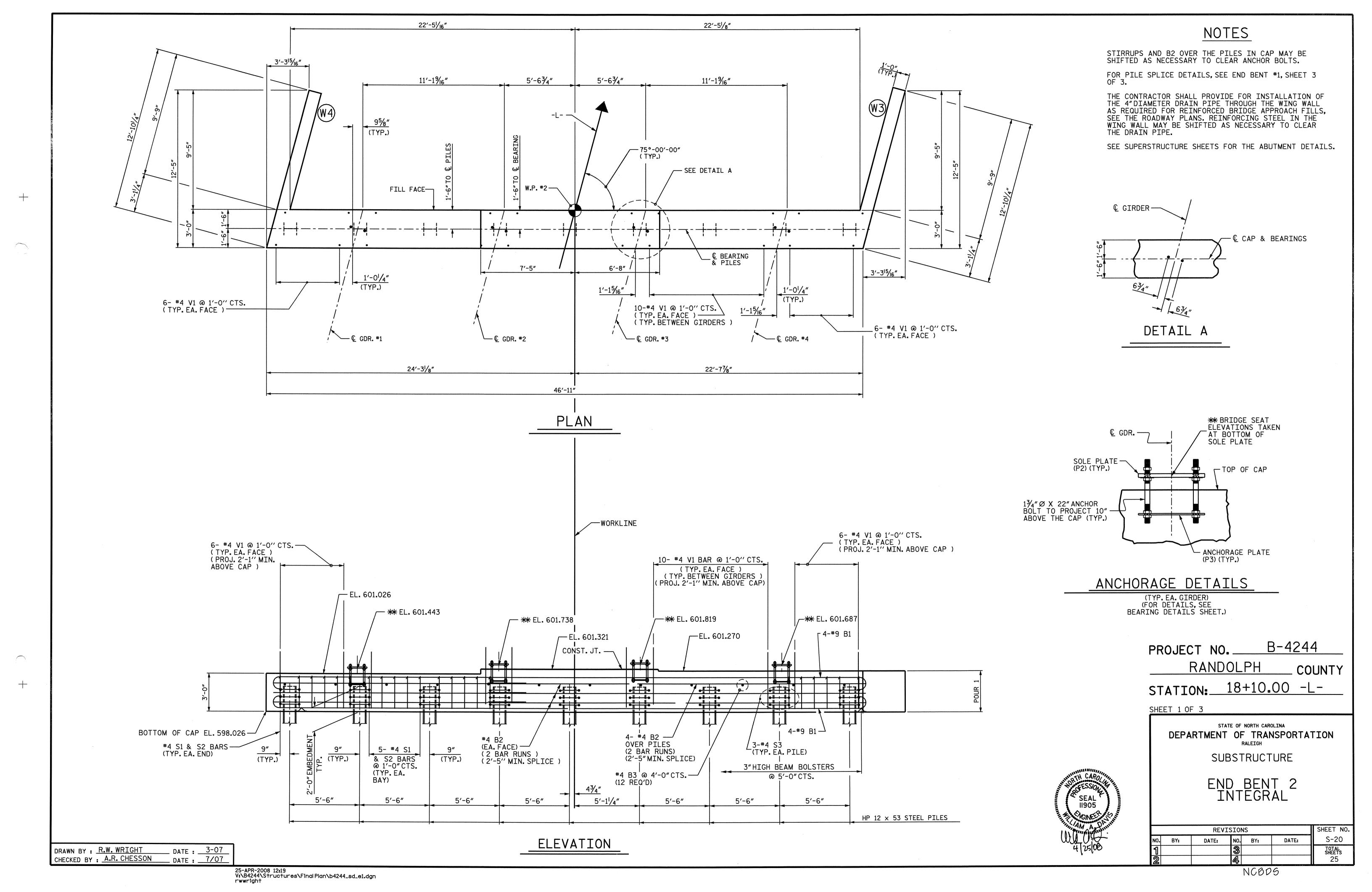
SHEET 3 OF 3

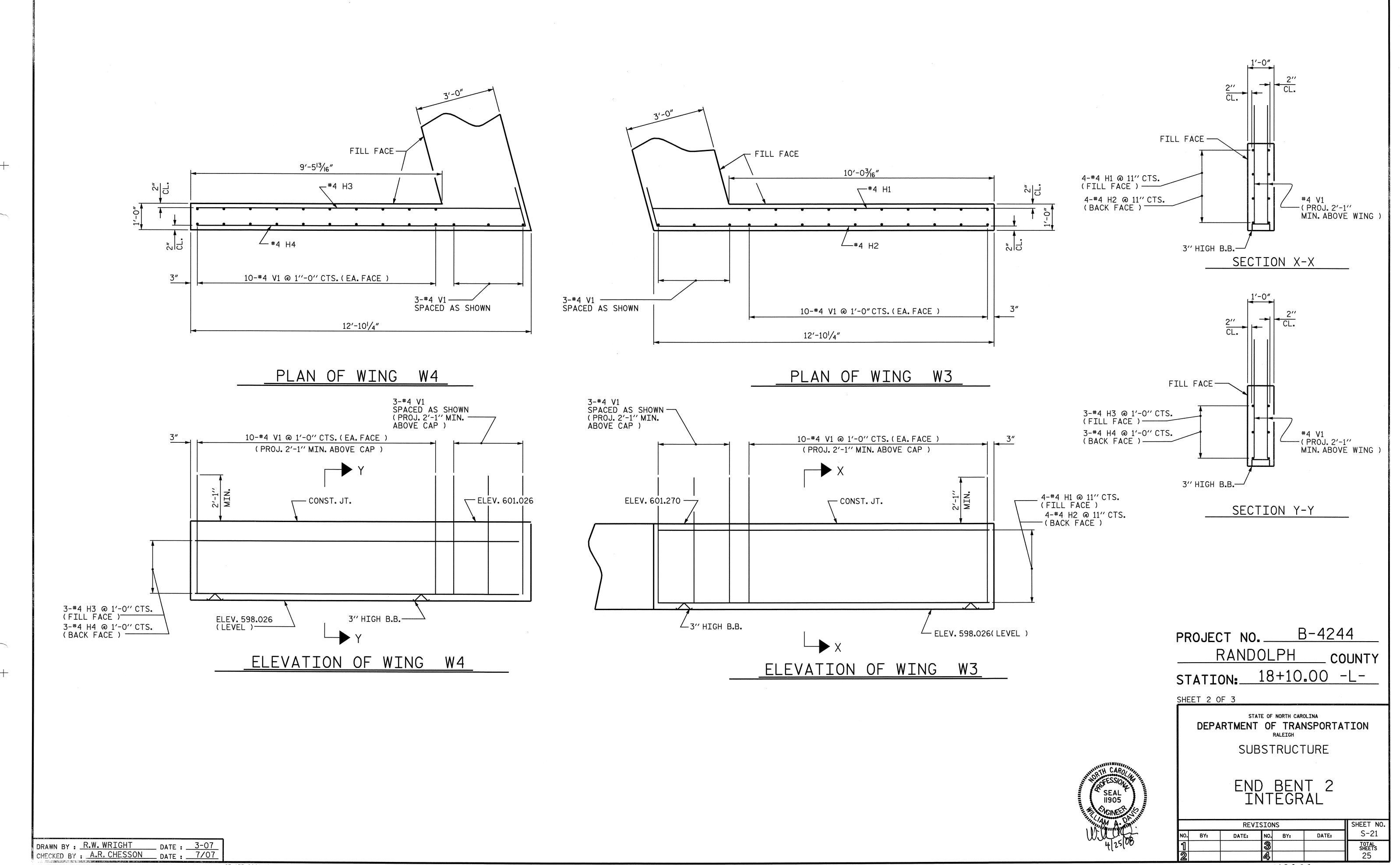
STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

> SUBSTRUCTURE END BENT 1 INTEGRAL

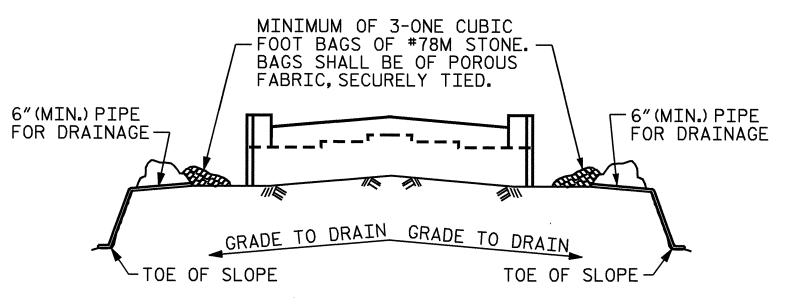
SHEET NO. REVISIONS S-19 NO. BY: DATE: DATE: BY: TOTAL SHEETS 25

DRAWN BY: R.W. WRIGHT DATE: 03/07
CHECKED BY: A.R. CHESSON DATE: 7/07





25-APR-2008 12:19 V:\B4244\Structures\FinalPlan\b4244_sd_e1.dgn rwwright NCBDS

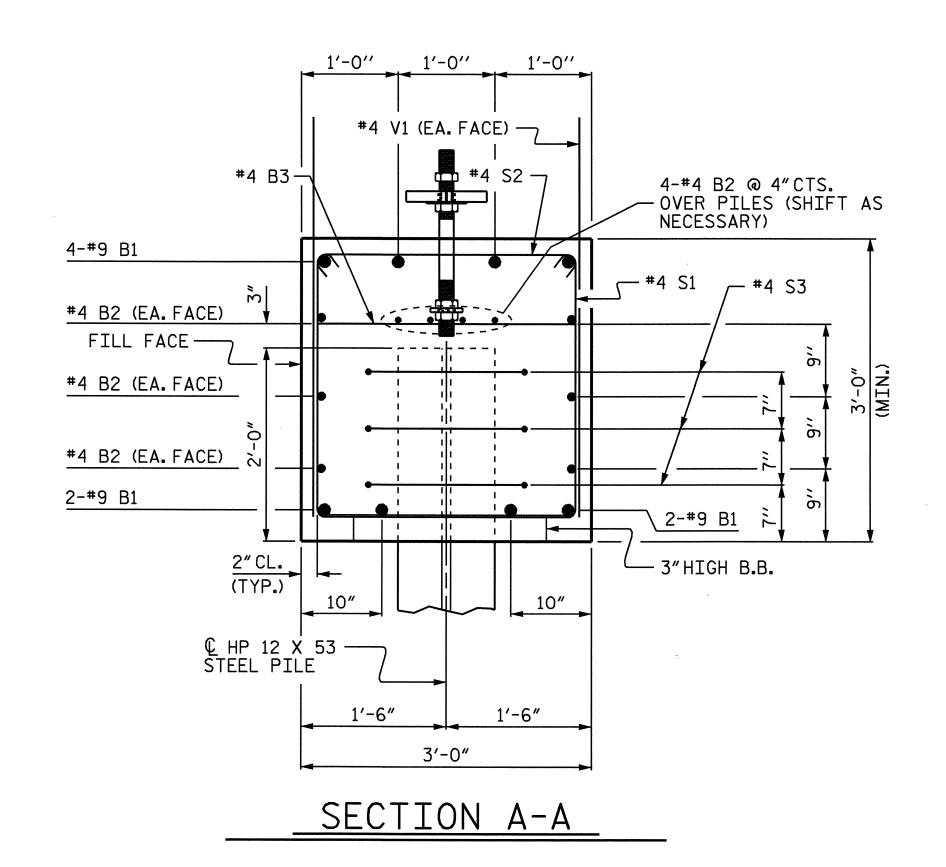


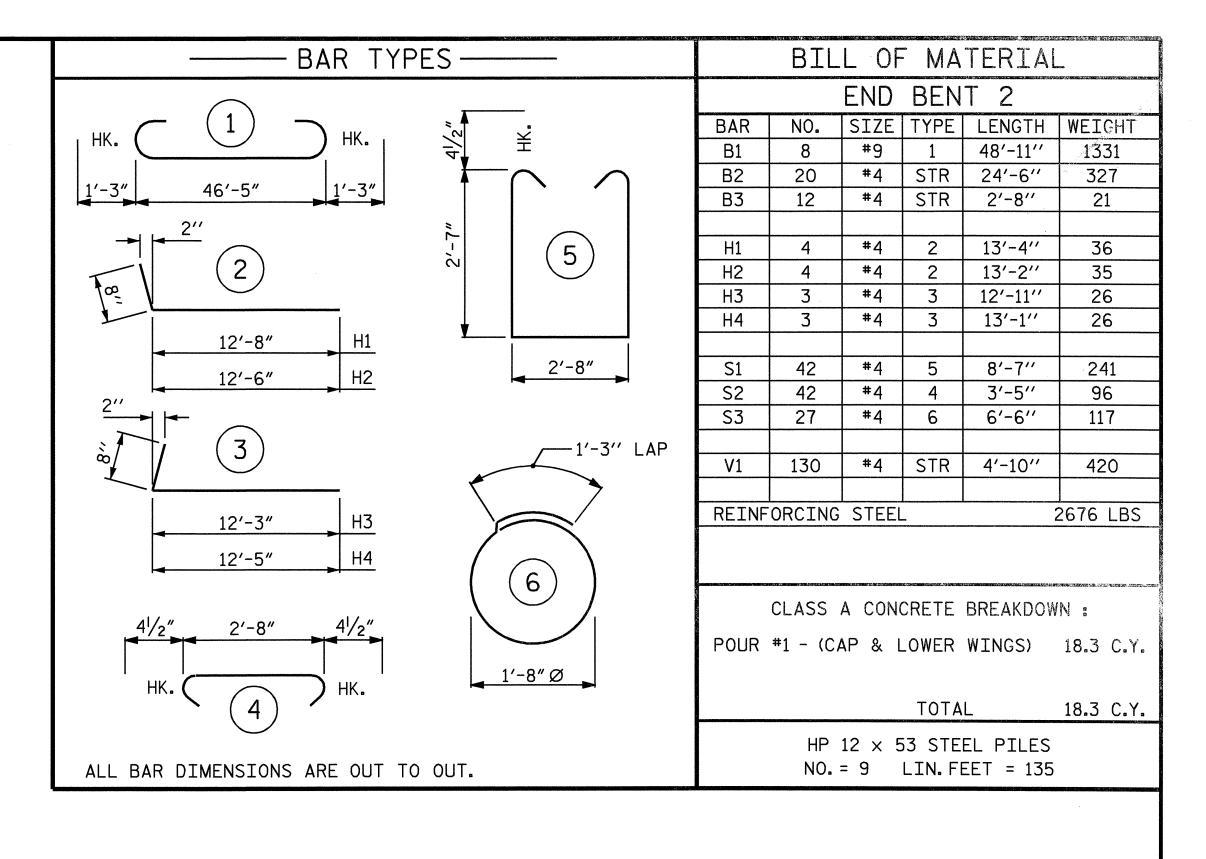
BAGGED STONE AND PIPE SHALL BE PLACED IMMEDIATELY AFTER COMPLETION OF END BENT EXCAVATION. PIPE MAY BE EITHER CONCRETE, CORRUGATED STEEL, CORRUGATED ALUMINUM ALLOY, OR CORRUGATED PLASTIC. PERFORATED PIPE WILL NOT BE ALLOWED.

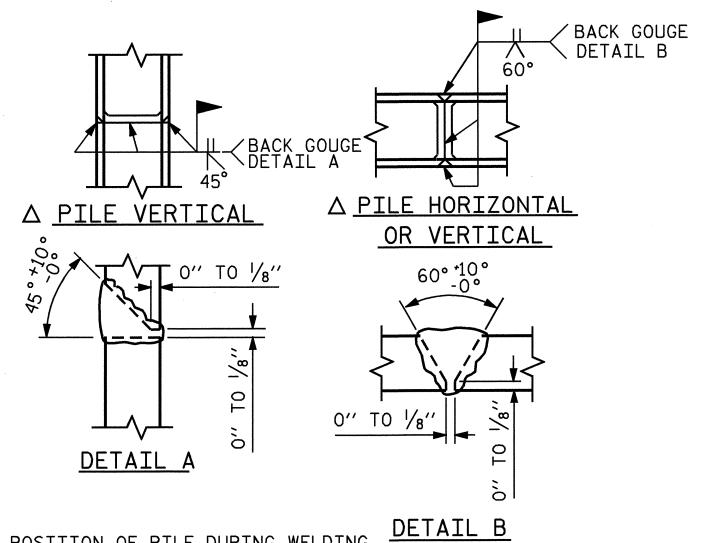
BAGGED STONE SHALL REMAIN IN PLACE UNTIL THE ENGINEER DIRECTS THAT IT BE REMOVED. THE CONTRACTOR SHALL REMOVE AND DISPOSE OF SILT ACCUMULATIONS AT BAGGED STONE WHEN SO DIRECTED BY THE ENGINEER. BAGS SHALL BE REMOVED AND REPLACED WHENEVER THE ENGINEER DETER-MINES THAT THEY HAVE DETERIORATED AND LOST THEIR EFFECTIVENESS.

NO SEPARATE PAYMENT WILL BE MADE FOR THIS WORK AND THE ENTIRE COST OF THIS WORK SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR THE SEVERAL PAY ITEMS.

TEMPORARY DRAINAGE AT END BENT







A POSITION OF PILE DURING WELDING. DETAIL B

PILE SPLICE DETAILS

PROJECT NO. B-4244

RANDOLPH COUNTY

STATION: 18+10.00 -L-

SHEET 3 OF 3

STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION
RALEIGH

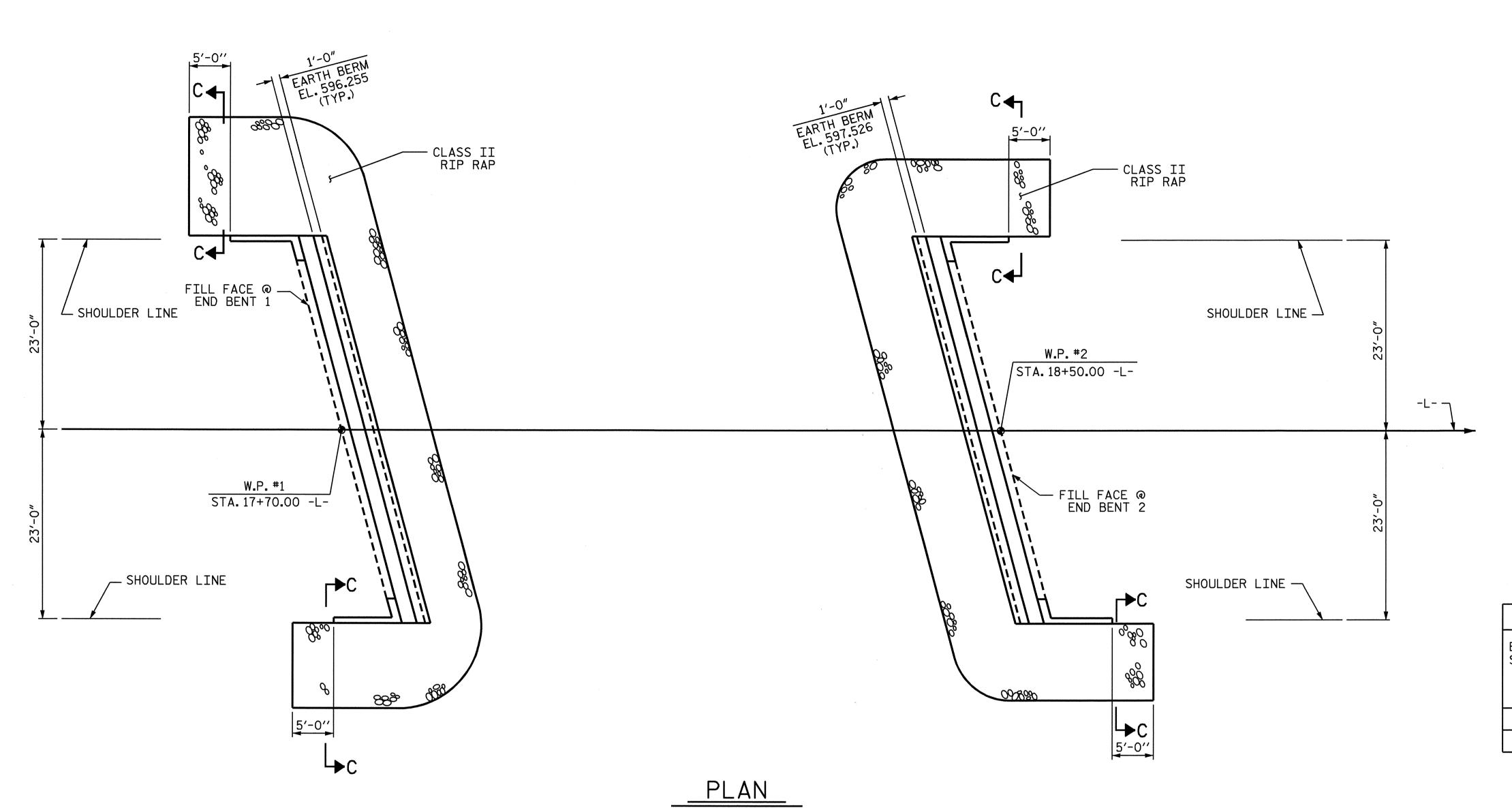
SUBSTRUCTURE
END BENT 2
INTEGRAL

REVISIONS

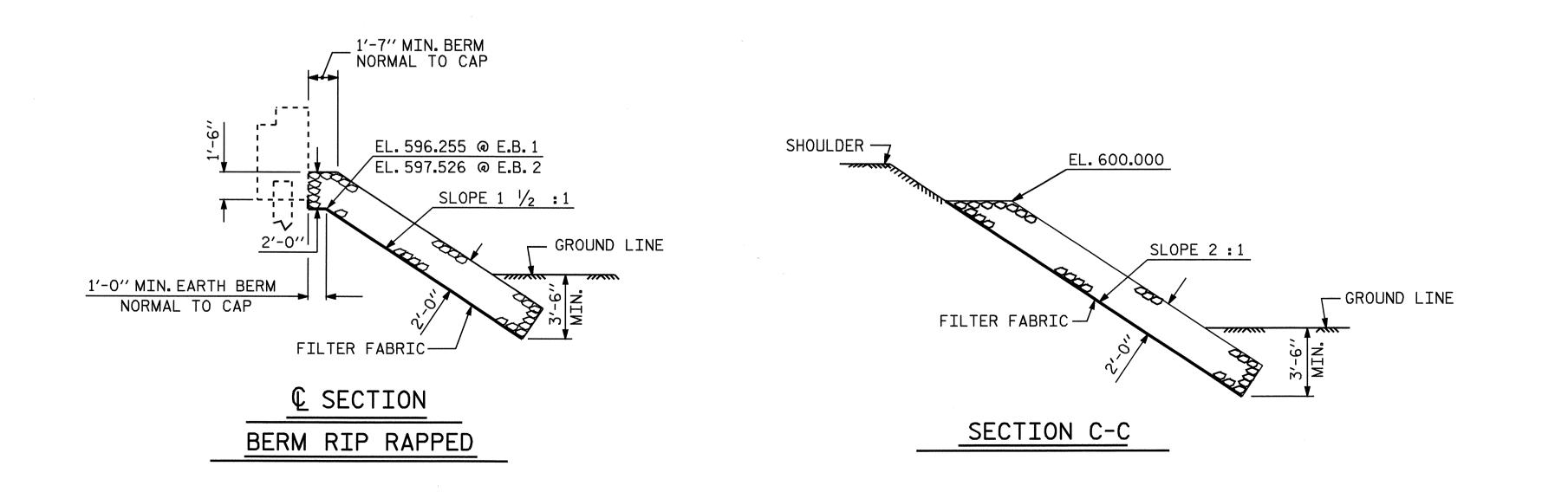
NO. BY: DATE: NO. BY: DATE: S-22

1 3 TOTAL SHEETS
25

DRAWN BY: R.W. WRIGHT DATE: 03/07
CHECKED BY: A.R. CHESSON DATE: 7/07



ESTIMATED QUANTITIES								
BRIDGE @ STA.18+10.00 -L-	RIP RAP CLASS II	FILTER FABRIC FOR DRAINAGE						
	TONS	SQUARE YARDS						
END BENT 1	109	121						
END BENT 2	152	169						



PROJECT NO. B-4244

RANDOLPH COUNTY

STATION: 18+10.00 -L-

STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION
RALEIGH

STANDARD

-RIP RAP DETAILS-

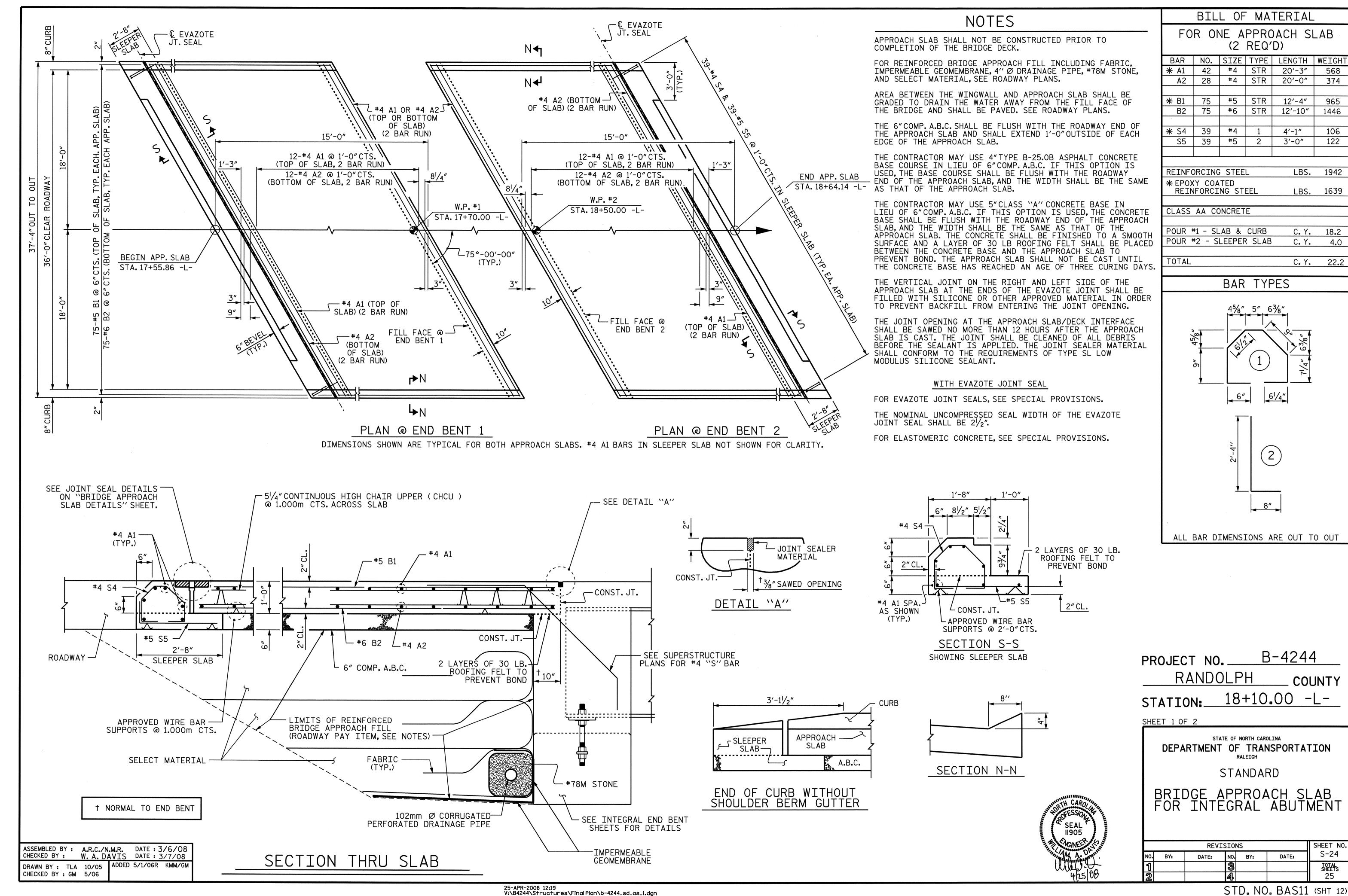
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			3			TOTAL SHEETS
2			4			25

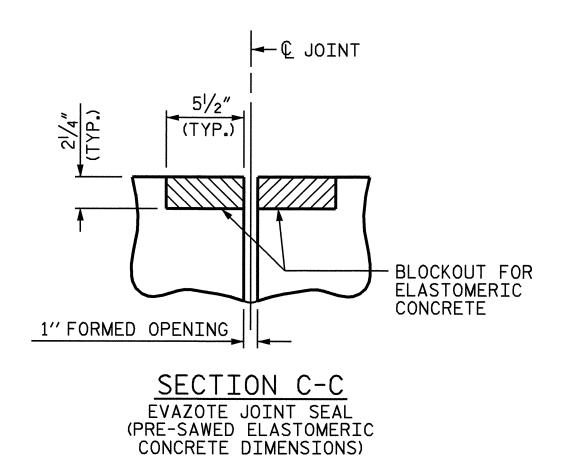
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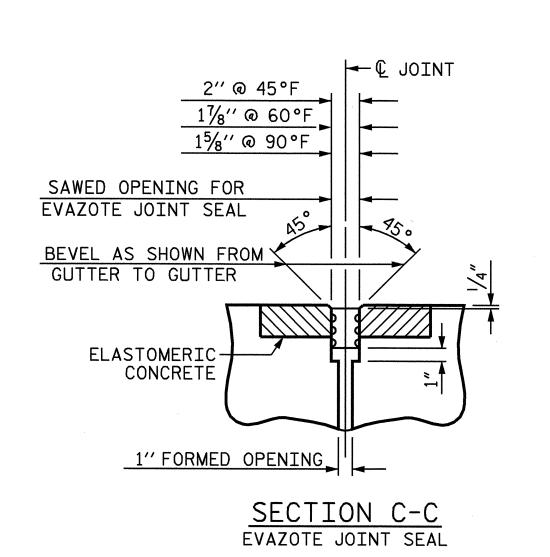
ASSEMBLED BY: Noil M. Ruffin DATE: 3/6/08 CHECKED BY: W. A. DAVIS DATE: 3/7/08

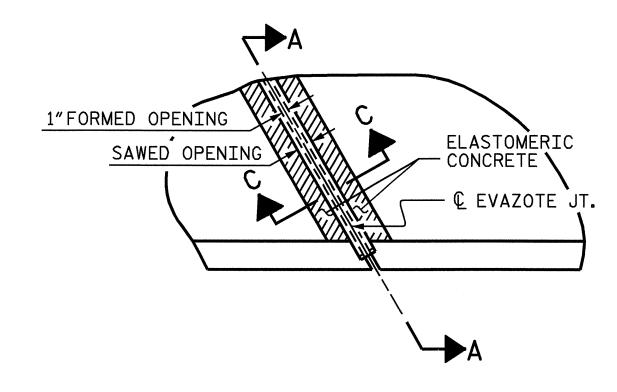
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REV. 8/16/99 REV. 10/17/00 REV. 5/1/06 RWW/LES RWW/LES TLA/GM

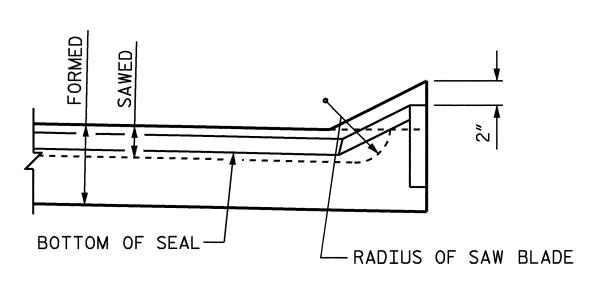




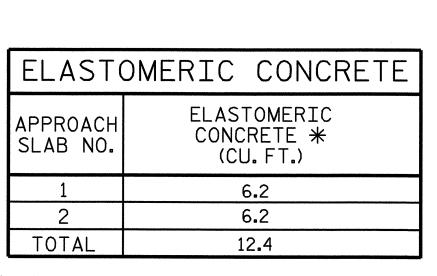




PLAN

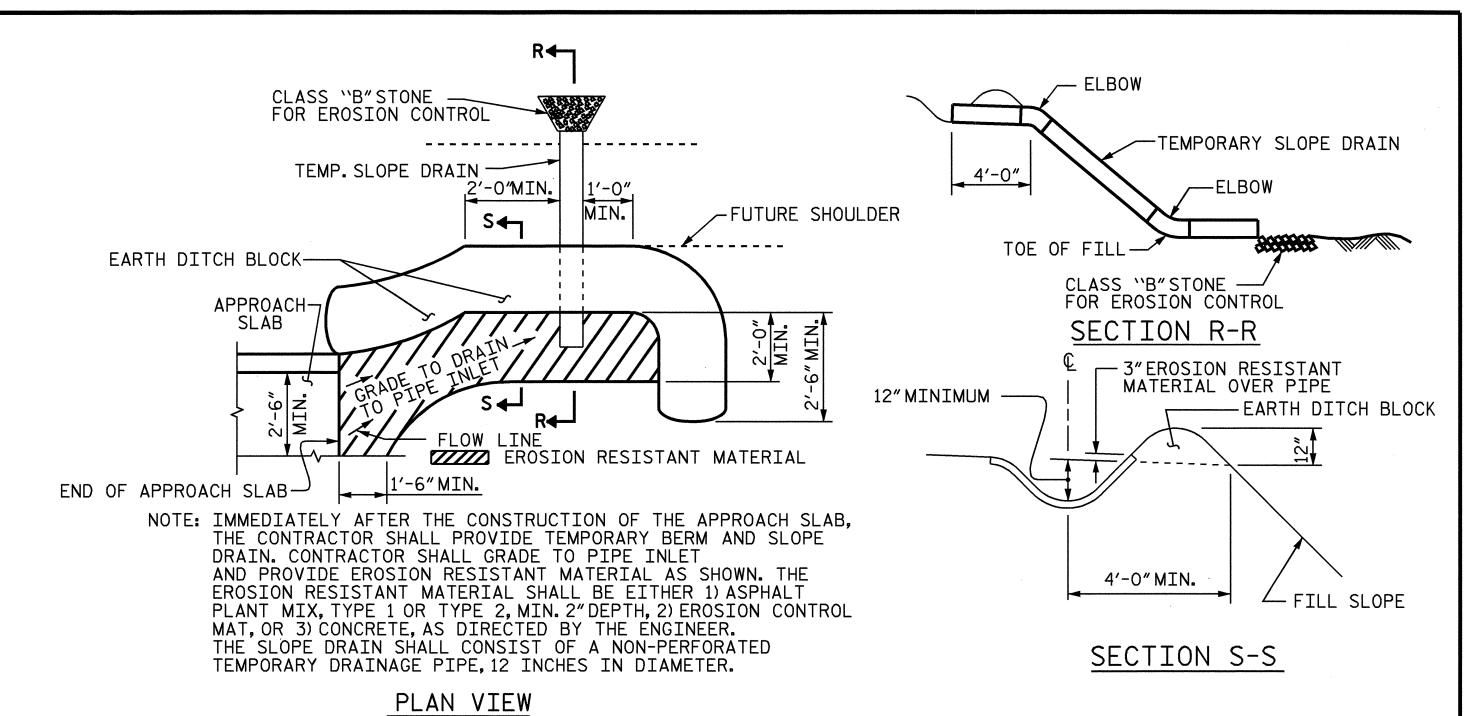


SECTION A-A



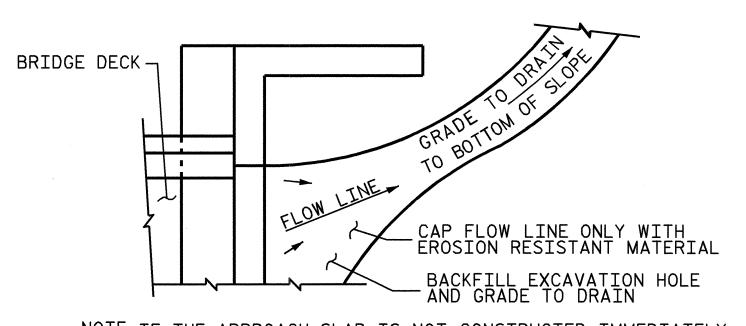
* BASED ON THE MINIMUM BLOCKOUT SHOWN.

ASSEMBLED BY : A.F CHECKED BY :	R.C./N.M.R. W.A.DAVIS	DATE : DATE :	
DRAWN BY: FCJ CHECKED BY: ARB	11/88 REV. 11/88 REV. REV.	10/17/00 5/7/03 5/1/06R	RWW/LES RWW/JTE MAA/KMM



TEMPORARY BERM AND SLOPE DRAIN DETAILS

(TO BE USED WHEN SHOULDER BERM GUTTER IS REQUIRED)



NOTE: IF THE APPROACH SLAB IS NOT CONSTRUCTED IMMEDIATELY AFTER THE BACKFILLING OF THE END BENT EXCAVATION, GRADE TO DRAIN TO THE BOTTOM OF THE SLOPE AND PROVIDE EROSION RESISTANT MATERIAL, SUCH AS FIBERGLASS ROVING OR AS DIRECTED BY THE ENGINEER TO PREVENT SOIL EROSION AND TO PROTECT THE AREA ADJACENT TO THE STRUCTURE. THE CONTRACTOR WILL BE REQUIRED TO REMOVE THESE MATERIALS PRIOR TO CONSTRUCTION OF THE APPROACH SLAB.

TEMPORARY DRAINAGE DETAIL

PROJECT NO. ____B-4244 ____RANDOLPH ___ COUNTY STATION: __18+10.00 -L-

SHEET 2 OF 2

STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION

RALEIGH

STANDARD

BRIDGE APPROACH SLAB DETAILS

	SHEET NO.				
BY:	DATE:	NO.	BY:	DATE:	S-25
		3			TOTAL SHEETS
		4			25

STD. NO. BAS10

STANDARD NOTES

DESIGN DATA:

---- A.A.S.H.T.O. (CURRENT) SPECIFICATIONS LIVE LOAD ---- SEE A.A.S.H.T.O. IMPACT ALLOWANCE STRESS IN EXTREME FIBER OF STRUCTURAL STEEL - AASHTO M270 GRADE 36 - 20,000 LBS. PER SQ. IN. - AASHTO M270 GRADE 50W - 27,000 LBS. PER SQ. IN. - AASHTO M270 GRADE 50 - 27.000 LBS. PER SQ. IN. REINFORCING STEEL IN TENSION GRADE 60 -- 24.000 LBS. PER SQ. IN. 1.200 LBS. PER SQ. IN. CONCRETE IN COMPRESSION ---- SEE A.A.S.H.T.O. CONCRETE IN SHEAR STRUCTURAL TIMBER - TREATED OR 1,800 LBS. PER SQ. IN. UNTREATED - EXTREME FIBER STRESS COMPRESSION PERPENDICULAR TO GRAIN 375 LBS. PER SQ. IN. OF TIMBER ----30 LBS. PER CU. FT. EQUIVALENT FLUID PRESSURE OF EARTH

MATERIAL AND WORKMANSHIP:

EXCEPT AS MAY OTHERWISE BE SPECIFIED ON PLANS OR IN THE SPECIAL PROVISIONS, ALL MATERIAL AND WORKMANSHIP SHALL BE IN ACCORDANCE WITH THE 2002 STANDARD SPECIFICATIONS "FOR ROADS AND STRUCTURES" OF THE N. C. DEPARTMENT OF TRANSPORTATION.

(MINIMUM)

STEEL SHEET PILING FOR PERMANENT OR TEMPORARY APPLICATIONS SHALL BE HOT ROLLED.

CONCRETE:

UNLESS OTHERWISE REQUIRED ON PLANS, CLASS A CONCRETE SHALL BE USED FOR ALL PORTIONS OF ALL STRUCTURES WITH THE EXCEPTION THAT: CLASS AA CONCRETE SHALL BE USED IN BRIDGE SUPERSTRUCTURES, ABUTMENT BACKWALLS, AND APPROACH SLABS; CLASS B CONCRETE SHALL BE USED FOR SLOPE PROTECTION AND RIP RAP; AND CLASS S SHALL BE USED FOR UNDERWATER FOOTING SEALS.

CONCRETE CHAMFERS:

UNLESS OTHERWISE NOTED ON THE PLANS, ALL EXPOSED CORNERS ON STRUCTURES SHALL BE CHAMFERED 3/4"WITH THE FOLLOWING EXCEPTIONS: TOP CORNERS OF CURBS MAY BE ROUNDED TO 1-1/2"RADIUS WHICH IS BUILT INTO CURB FORMS; CORNERS OF TRANSVERSE FLOOR EXPANSION JOINTS SHALL BE ROUNDED WITH A 1/4"FINISHING TOOL UNLESS OTHERWISE REQUIRED ON PLANS; AND CORNERS OF EXPANSION JOINTS IN THE ROADWAY FACES AND TOPS OF CURBS AND SIDEWALKS SHALL BE ROUNDED TO A 1/4"RADIUS WITH A FINISHING STONE OR TOOL UNLESS OTHERWISE REQUIRED ON PLANS.

DOWELS:

DOWELS WHEN INDICATED ON PLANS AS FOR CULVERT EXTENSIONS, SHALL BE EMBEDDED AT LEAST 12" INTO THE OLD CONCRETE AND GROUTED INTO PLACE WITH 1:2 CEMENT MORTAR.

ALLOWANCE FOR DEAD LOAD DEFLECTION, SETTLEMENT, ETC. IN CASTING SUPERSTRUCTURES:

BRIDGES SHALL BE BUILT ON THE GRADE OR VERTICAL CURVE SHOWN ON PLANS. SLABS, CURBS AND PARAPETS SHALL CONFORM TO THE GRADE OR CURVE.

ALL DIMENSIONS WHICH ARE GIVEN IN SECTION AND ARE AFFECTED BY DEAD LOAD DEFLECTIONS ARE DIMENSIONS AT CENTER LINE OF BEARING UNLESS OTHERWISE NOTED ON PLANS. IN SETTING FORMS FOR STEEL BEAM BRIDGES AND PRESTRESSED CONCRETE GIRDER BRIDGES, ADJUSTMENTS SHALL BE MADE DUE TO THE DEAD LOAD DEFLECTIONS FOR THE ELEVATIONS SHOWN. WHERE BLOCKS ARE SHOWN OVER BEAMS FOR BUILDING UP TO THE SLAB, THE VERTICAL DIMENSIONS OF THE BLOCKS SHALL BE ADJUSTED BETWEEN BEARINGS TO COMPENSATE FOR DEAD LOAD DEFLECTIONS, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER. WHERE BOTTOM OF SLAB IS IN LINE WITH BOTTOM OF TOP FLANGES, DEPTH OF SLAB BETWEEN BEARINGS SHALL BE ADJUSTED TO COMPENSATE FOR DEAD LOAD DEFLECTION, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER.

IN SETTING FALSEWORK AND FORMS FOR REINFORCED CONCRETE SPANS, AN ALLOWANCE SHALL BE MADE FOR DEAD LOAD DEFLECTIONS, SETTLEMENT OF FALSEWORK, AND PERMANENT CAMBER WHICH SHALL BE PROVIDED FOR IN ADDITION TO THE ELEVATIONS SHOWN. AFTER REMOVAL OF THE FALSEWORK, THE FINISHED STRUCTURES SHALL CONFORM TO THE PROFILE AND ELEVATIONS SHOWN ON THE PLANS AND CONSTRUCTION ELEVATIONS ELEVATIONS FURNISHED BY THE ENCINEER.

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DETAILED DRAWINGS FOR FALSEWORK OR FORMS FOR BRIDGE SUPERSTRUCTURE
AND ANY STRUCTURE OR PARTS OF A STRUCTURE AS NOTED ON THE PLANS SHALL
BE SUBMITTED TO THE ENGINEER FOR APPROVAL BEFORE CONSTRUCTION OF THE
FALSEWORK OR FORMS IS STARTED.

REINFORCING STEEL:

ALL REINFORCING STEEL SHALL BE DEFORMED WITH THE EXCEPTION OF #2
BARS WHICH MAY BE FABRICATED FROM COLD DRAWN STEEL WIRE. DIMENSIONS
RELATIVE TO PLACEMENT OF REINFORCING ARE TO CENTERS OF BARS UNLESS OTHERWISE
INDICATED IN THE PLANS. DIMENSIONS ON BAR DETAILS ARE TO CENTERS OF BARS
OR ARE OUT TO OUT AS INDICATED ON PLANS.

WIRE BAR SUPPORTS SHALL BE PROVIDED FOR REINFORCING STEEL WHERE
INDICATED ON THE PLANS. WHEN BAR SUPPORT PIECES ARE PLACED IN CONTINUOUS
LINES, THEY SHALL BE SO PLACED THAT THE ENDS OF THE SUPPORTING WIRES SHALL
BE LAPPED TO LOCK LEGS ON ADJOINING PIECES.

STRUCTURAL STEEL:

AT THE CONTRACTOR'S OPTION, HE MAY SUBSTITUTE 7/8%% SHEAR STUDS FOR THE 3/4%% STUDS SPECIFIED ON THE PLANS. THIS SUBSTITUTION SHALL BE MADE AT THE RATE OF 3-7/8%% STUDS FOR 4-3/4%% STUDS, AND STUD SPACING CHANGES SHALL BE MADE AS NECESSARY TO PROVIDE THE SAME EQUIVALENT NUMBER OF 7/8%% STUDS ALONG THE BEAM AS SHOWN FOR 3/4%% STUDS BASED ON THE RATIO OF 3-7/8%% STUDS FOR 4-3/4%% STUDS. STUDS OF THE LENGTH SPECIFIED ON THE PLANS MUST BE PROVIDED. THE MAXIMUM SPACING SHALL BE 2'-0%

EXCEPT AT THE INTERIOR SUPPORTS OF CONTINUOUS BEAMS WHERE THE COVER PLATE IS IN CONTACT WITH BEARING PLATE, THE CONTRACTOR MAY, AT HIS OPTION, SUBSTITUTE FOR THE COVER PLATES DESIGNATED ON THE PLANS COVER PLATES OF THE EQUIVALENT AREA PROVIDED THESE PLATES ARE AT LEAST 5/16"IN THICKNESS AND DO NOT EXCEED A WIDTH EQUAL TO THE FLANGE WIDTH LESS 2"OR A THICKNESS EQUAL TO 2 TIMES THE FLANGE THICKNESS. THE SIZE OF FILLET WELDS SHALL CONFORM TO THE REQUIREMENTS OF THE CURRENT ANSI/AASHTO/AWS "BRIDGE WELDING CODE". ELECTROSLAG WELDING WILL NOT BE PERMITTED.

PLACEMENT OF BEAM OR GIRDER MEMBERS ON TRUCKS FOR HAULING SHALL
BE DONE IN COMPLIANCE WITH LIMITS SHOWN ON SKETCHES PROVIDED TO THE MATERIALS
AND TEST UNIT APPROVED BY THE STRUCTURE DESIGN UNIT DATED MAY 8, 1991.
THESE SKETCHES PRIMARILY LIMIT THE UNSUPPORTED CANTILEVER LENGTH OF MEMBERS.
WHEN THE CONTRACTOR WISHES TO PLACE MEMBERS ON TRUCKS NOT IN ACCORDANCE
WITH THESE LIMITS, TO SHIP BY RAIL, TO ATTACH SHIPPING RESTRAINTS TO THE
MEMBERS OR TO INVERT MEMBERS, HE SHALL SUBMIT A SKETCH FOR APPROVAL
PRIOR TO SHIPPING. SEE ALSO ARTICLE 1072-11.

WITH THE SOLE EXCEPTION OF EDGES AT SURFACES WHICH BEAR ON OTHER SURFACES, ALL SHARP EDGES AND ENDS OF SHAPES AND PLATES SHALL BE SLIGHTLY ROUNDED BY SUITABLE MEANS TO A RADIUS OF APPROXIMATELY 1/16 INCH OR EQUIVALENT FLAT SURFACE AT A SUITABLE ANGLE PRIOR TO PAINTING, GALVANIZING, OR METALLIZING.

HANDRAILS AND POSTS:

METAL STANDARDS AND FACES OF THE CONCRETE END POSTS FOR THE METAL RAIL SHALL BE SET NORMAL TO THE GRADE OF THE CURB, UNLESS OTHERWISE SHOWN ON PLANS. THE METAL RAIL AND TOPS OF CONCRETE POSTS USED WITH THE ALUMINUM RAIL SHALL BE BUILT PARALLEL TO THE GRADE OF THE CURB.

METAL HANDRAILS SHALL BE IN ACCORDANCE WITH THE PLANS. RAILS SHALL BE AS MANUFACTURED FOR BRIDGE RAILING. CASTINGS SHALL BE OF A UNIFORM APPEARANCE. FINS AND OTHER DEFORMATIONS RESULTING FROM CASTING OR OTHERWISE SHALL BE REMOVED IN A MANNER SO THAT A UNIFORM COLORING OF THE COMPLETED CASTING SHALL BE OBTAINED. CASTINGS WITH DISCOLORATIONS OR OF NON-UNIFORM COLORING WILL NOT BE ACCEPTED. CERTIFIED MILL REPORTS ARE REQUIRED FOR METAL RAILS AND POSTS.

SPECIAL NOTES:

GENERALLY, IN CASE OF DISCREPANCY, THIS STANDARD SHEET OF NOTES SHALL GOVERN OVER THE SPECIFICATIONS, BUT THE REMAINDER OF THE PLANS SHALL GOVERN OVER NOTES HEREON, AND SPECIAL PROVISIONS SHALL GOVERN OVER ALL. SEE SPECIFICATIONS ARTICLE 105-4.

ENGLISH