

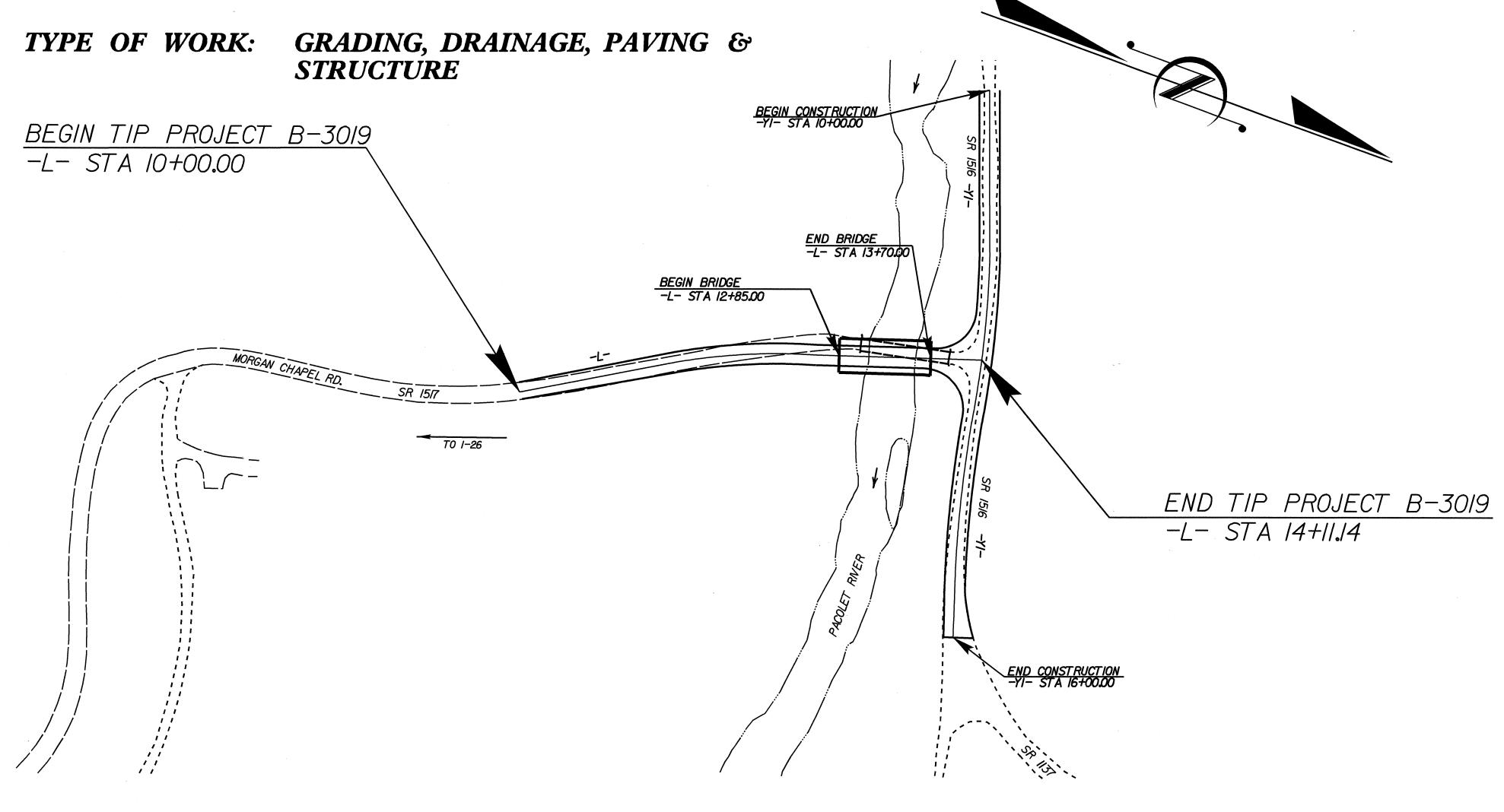
# STATE OF NORTH CAROLINA DIVISION OF HIGHWAYS

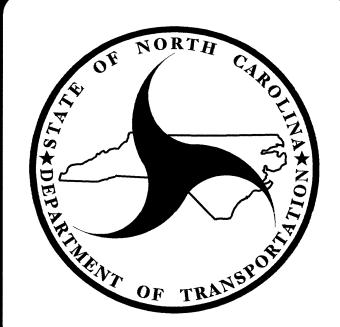
# POLK COUNTY

B-3019 BRZ-1517(1) 32815.1.1 BRZ-1517(1) 32815.3.1 ROW, UTIL CONST. 32815.2.2 BRZ-1517(1)

**LOCATION:** BRIDGE NO. 19 OVER PACOLET RIVER ON SR 1517

(MORGAN CHAPEL RD.)





### DESIGN DATA

ADT 2008 = 200

ADT 2030 = 400DHV = 10 %

D = 60 %

V = 40 MPH\* TTST 1% DUAL 1%

### PROJECT LENGTH

LENGTH ROADWAY TIP PROJECT B-3019 = 0.062 MI LENGTH STRUCTURE TIP PROJECT B-3019 = 0.016 MI TOTAL LENGTH TIP PROJECT B-3019 =

2006 STANDARDS SPECIFICATION

LETTING DATE: MARCH 18, 2008

### Prepared in the Office of:

**DEPARTMENT OF TRANSPORTATION DIVISION OF HIGHWAYS** 1000 Birch Ridge Drive Raleigh, N.C. 27610

> B. S. COX, P.E. PROJECT ENGINEER

D. E. PETREY, P.E. PROJECT DESIGN ENGINEER

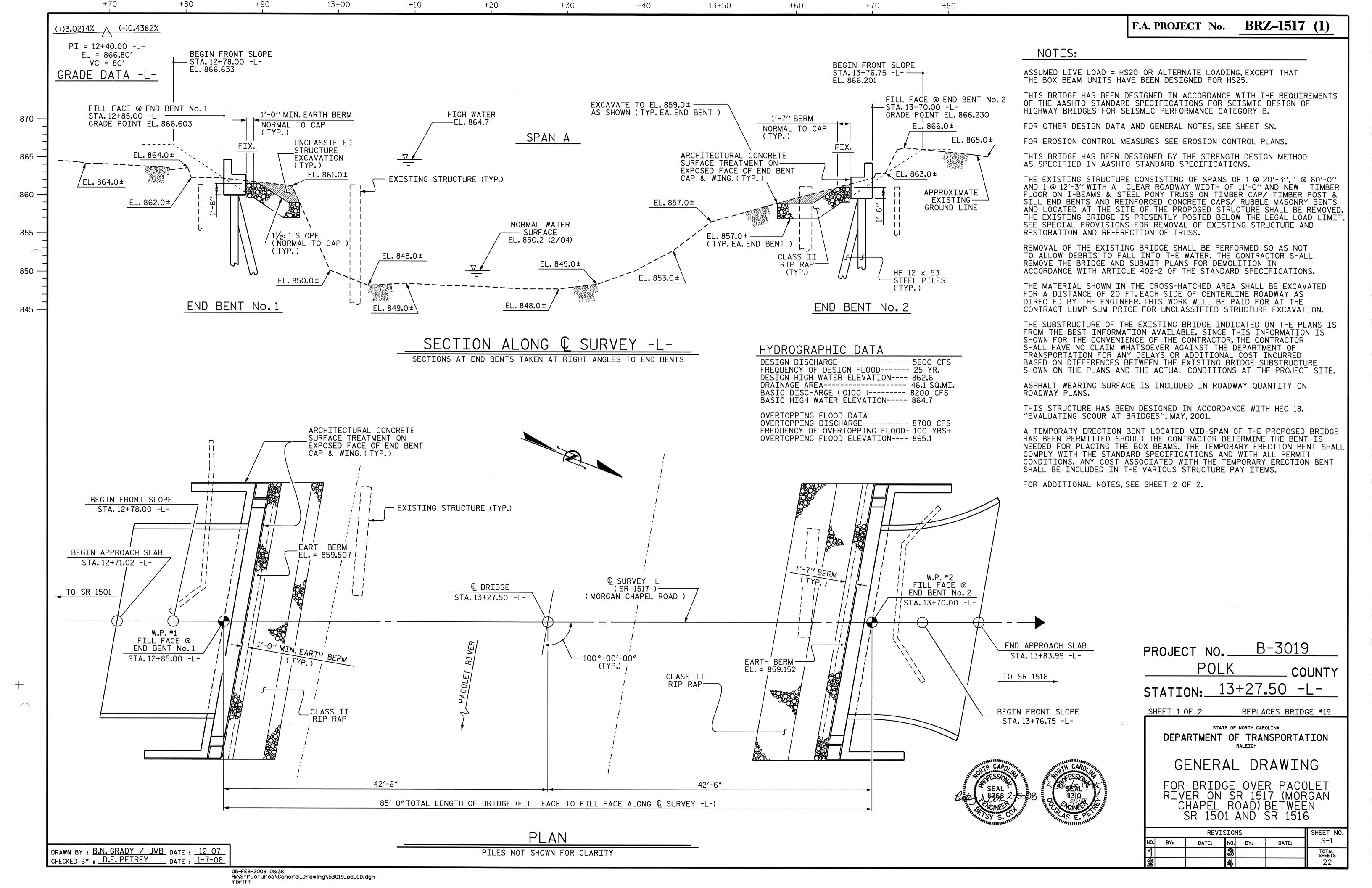
### STRUCTURE DESIGN UNIT

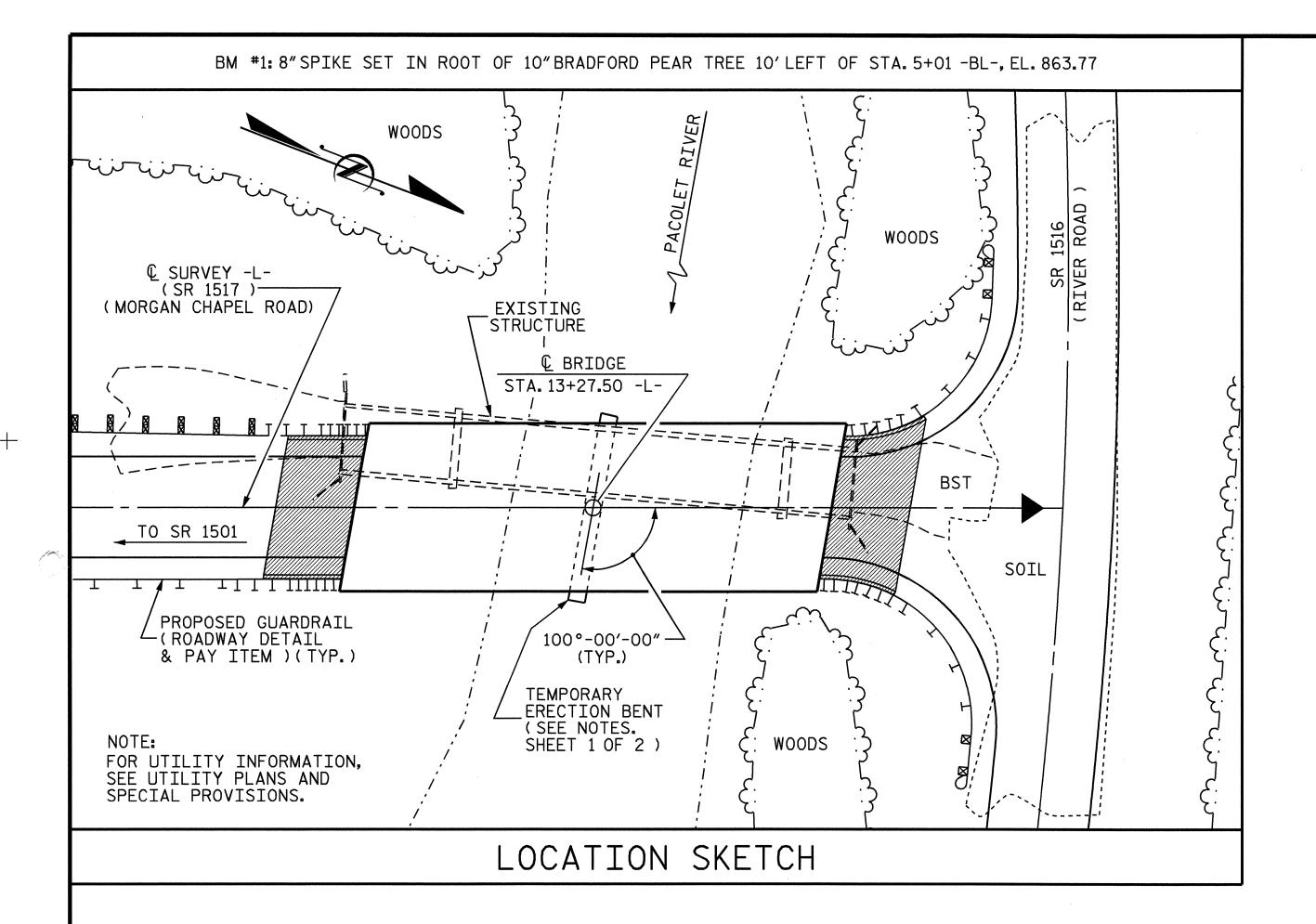
DIVISION OF HIGHWAYS STATE OF NORTH CAROLINA

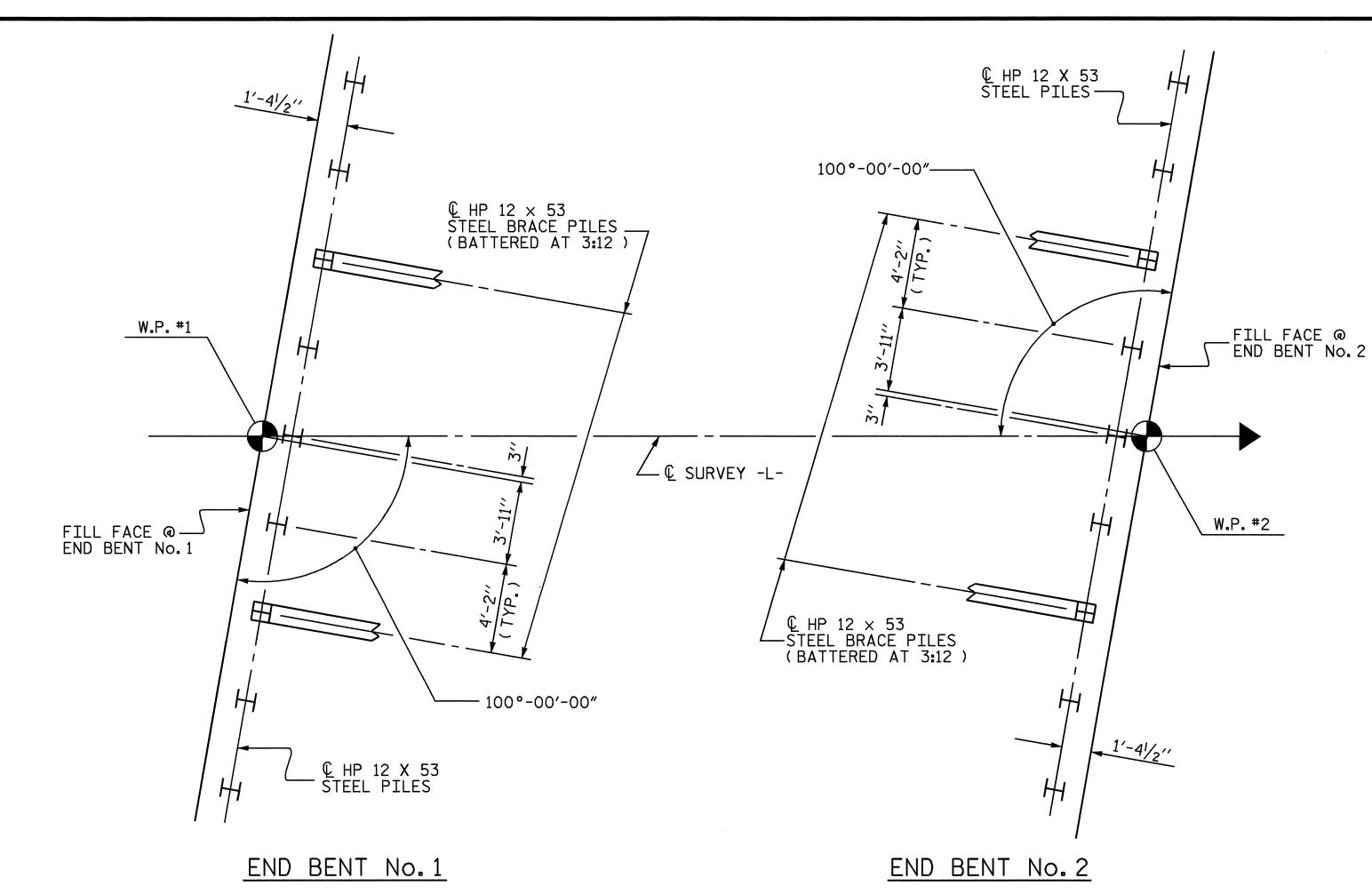
DEPARTMENT OF TRANSPORTATION FEDERAL HIGHWAY ADMINISTRATION

<u>APPROVED FOR</u> DIVISION ADMINISTRATOR

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mbritt







FOUNDATION LAYOUT

(DIMENSIONS LOCATING PILES ARE TO THE CENTERLINE OF THE PILE AT THE BOTTOM OF THE CAP)

NOTES : (CONTINUED FROM SHEET 1 OF 2)

THE CONTRACTOR SHALL PROVIDE INDEPENDENT ASSURANCE SAMPLES OF REINFORCING STEEL AS FOLLOWS: FOR PROJECTS REQUIRING UP TO 400 TONS OF REINFORCING STEEL, ONE 30 INCH SAMPLE OF EACH SIZE BAR USED, AND FOR PROJECTS REQUIRING OVER 400 TONS OF REINFORCING STEEL, TWO 30 INCH SAMPLES OF EACH SIZE BAR USED. THE BARS FROM WHICH THE SAMPLES ARE TAKEN MUST THEN BE SPLICED WITH REPLACEMENT BARS OF THE SIZE AND LENGTH OF THE SAMPLE, PLUS A MINIMUM LAP SPLICE OF THIRTY BAR DIAMETERS.

INASMUCH AS THE PAINT SYSTEM ON THE EXISTING STRUCTURAL STEEL CONTAINS LEAD, THE CONTRACTOR'S ATTENTION IS DIRECTED TO ARTICLE 107-1 OF THE STANDARD SPECIFICATIONS. ANY COSTS RESULTING FROM COMPLIANCE WITH APPLICABLE STATE OR FEDERAL REGULATIONS PERTAINING TO HANDLING OF MATERIALS CONTAINING LEAD BASED PAINT SHALL BE INCLUDED IN THE BID PRICE FOR "REMOVAL OF EXISTING STRUCTURE AT STATION 13+27.50 -L-".

FOR SUBMITTAL OF WORKING DRAWINGS, SEE SPECIAL PROVISIONS.

FOR FALSEWORK AND FORMWORK, SEE SPECIAL PROVISIONS.

FOR CRANE SAFETY, SEE SPECIAL PROVISIONS.

FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.

DRIVE PILES AT END BENT No.1 AND END BENT No.2 TO A BEARING CAPACITY OF 120 TONS PER PILE. THE REQUIRED BEARING CAPACITY IS EQUAL TO THE ALLOWABLE BEARING CAPACITY WITH A MINIMUM FACTOR OF SAFETY OF TWO.

THE ALLOWABLE BEARING CAPACITY FOR PILES AT END BENT No.1 AND END BENT No.2 IS 60 TONS PER PILE.

FOR ADHESIVELY ANCHORED ANCHOR BOLTS OR DOWELS, SEE SPECIAL PROVISIONS.

FOR PRESTRESSED CONCRETE MEMBERS, SEE SPECIAL PROVISIONS.

FOR RESTORATION AND RE-ERECTION OF TRUSS, SEE SPECIAL PROVISIONS.

FOR ARCHITECTURAL CONCRETE SURFACE TREATMENT, SEE SPECIAL PROVISIONS.

FOR CONCRETE PENETRATING STAIN, SEE SPECIAL PROVISIONS.

CONTRACTOR SHALL SUBMIT IN ONE PACKAGE TO THE ENGINEER FOR HIS APPROVAL THE ANODIZING COLOR(S) FOR THE 2 BAR METAL RAILS AND THE MATCHING PAINT COLOR(S) PROPOSED FOR THE RESTORED TRUSS SIDE MEMBERS, THE TRUSS CONNECTIONS TO THE CONCRETE PEDESTALS, ANODIZED RAIL CONNECTION PAINT, THE ROADWAY PAINTED GALVANIZED STEEL BEAM GUARDRAIL AND THE CONCRETE PENETRATING STAIN COLOR(S). SEE RESTORATION AND RE-ERECTION OF TRUSS SPECIAL PROVISION AND ROADWAY PAINTED GALVANIZED STEEL BEAM GUARDRAIL SPECIAL PROVISION.

THE CONTRACTOR, AT HIS OPTION, MAY SUBMIT TO THE ENGINEER FOR HIS APPROVAL PLANS AND DETAILS FOR PROVIDING NEW TRUSS SIDE MEMBERS IN LIEU OF RESTORING THE EXISTING TRUSS SIDE MEMBERS. SEE RESTORATION AND RE-ERECTION OF TRUSS SPECIAL PROVISION.



PROJECT NO. \_

SHEET 2 OF 2

POLK

STATION: 13+27.50 -L-

DEPARTMENT OF TRANSPORTATION
RALEIGH

GENERAL DRAWING

FOR BRIDGE OVER PACOLET RIVER ON SR 1517 (MORGAN CHAPEL ROAD) BETWEEN

B-3019

COUNTY

				PARTICIPATION OF THE PARTICIPA	TOTAL	$\boldsymbol{B}$	ILL	OF M	ATERIA	L					······
	REMOVAL OF EXISTING STRUCTURE	UNCLASSIFIED STRUCTURE EXCAVATION	CLASS A CONCRETE	BRIDGE APPROACH SLABS	REINFORCING STEEL	HP 1 STEE	2 x 53 L PILES	TWO BAR METAL RAIL	1'-6'' × 2'-10'' CONCRETE PARAPET	RIP RAP CLASS II (2'-0" THICK)	BEARINGS	PRES	" x 3'-3" STRESSED CRETE BEAMS	ARCHITECTURAL CONCRETE SURFACE TREATMENT	RESTORATION AND RE-ERECTION OF TRUSS
	LUMP SUM	LUMP SUM	CU. YDS.	LUMP SUM	LBS.	No.	LIN.FT.	LIN. FT.	LIN. FT.	TONS	LUMP SUM	No.	LIN.FT.	LUMP SUM	LUMP SUM
SUPERSTRUCTURE								149.23	165.42			10	827.08		
				,											
END BENT No. 1			18.7		2,531	9	180			35					
END BENT No. 2			18.7		2,531	9	250			40					
												·			
TOTAL	LUMP SUM	LUMP SUM	37.4	LUMP SUM	5,062	18	430	149.23	165.42	75	LUMP SUM	10	827.08	LUMP SUM	LUMP SUM

DRAWN BY: B.N. GRADY / JMB DATE: 12-07
CHECKED BY: D.E. PETREY DATE: 1-7-08

SR 1501 AND SR 1516

REVISIONS

NO. BY: DATE: NO. BY: DATE: S-2

1 3 TOTAL SHEETS
2 22

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# NOTES

ALL PRESTRESSING STRANDS SHALL BE 7-WIRE LOW RELAXATION GRADE 270 STRANDS AND SHALL CONFORM TO AASHTO M203 EXCEPT FOR SAMPLING REQUIREMENTS WHICH SHALL BE IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

ALL REINFORCING STEEL CAST WITH THE BOX BEAM SECTIONS SHALL BE GRADE 60 AND SHALL BE INCLUDED IN THE UNIT PRICE BID FOR PRESTRESSED CONCRETE BOX BEAMS.

RECESSES FOR TRANSVERSE STRANDS SHALL BE GROUTED AFTER THE TENSIONING OF THE STRANDS.

THE 21/2" Ø DOWEL HOLES AT FIXED ENDS OF BOX BEAM SECTIONS SHALL BE FILLED WITH GROUT.

THE 2"Ø BACKER ROD SHALL CONFORM TO THE REQUIREMENTS OF TYPE M BOND BREAKER. SEE SECTION 1028 OF THE STANDARD SPECIFICATIONS.

THE TRANSFER OF LOAD FROM THE ANCHORAGES TO THE BOX BEAM UNIT SHALL BE DONE WHEN THE CONCRETE HAS REACHED A COMPRESSIVE STRENGTH OF NOT LESS THAN 4800 PSI.

ALL REINFORCING STEEL IN PARAPETS & PEDESTALS SHALL BE EPOXY COATED.

PRESTRESSING STRANDS SHALL BE CUT FLUSH WITH THE BOX BEAM UNIT

APPLY EPOXY PROTECTIVE COATING TO BOX BEAM UNIT ENDS.

THE LOCATION OF THE VOID DRAINS MAY BE SHIFTED SLIGHTLY WHERE NECESSARY TO CLEAR PRESTRESSING STRANDS OR TRANSVERSE REINFORCING STEEL.

FOR PRESTRESSED CONCRETE MEMBERS, SEE SPECIAL PROVISIONS.

FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.

BEARING PAD SHALL BE 60 DUROMETER HARDNESS.

FOR CONCRETE PENETRATING STAIN, SEE SPECIAL PROVISONS.

-FIELD APPLY CONCRETE PENETRATING STAIN TO FACE AND EXPOSED TOP OF EXTERIOR BOX BEAMS AND EXPOSED FACES OF CONCRETE PEDESTALS NOT RECEIVING ARCHITECTURAL CONCRETE SURFACE

TREATMENT.

RESET TRUSS SIDE

- MEMBERS FROM

EXISTING BRIDGE

FOR TRUSS LAYOUT

SEE, "TRUSS RE-

ERECTION" SHEET.

(TYP.)

2"MAX. FORM

LINER RELIEF

(TYPICAL VERTICAL EXTERIOR FACE OF

PEDESTAL )

1'-7"

• •

**`**~~~~~~~~

3'-0"

(TYP.)

GRADE PT.-

HALF-SECTION AT INTERMEDIATE DIAPHRAGMS

15'-0"

11'-11"

\_0.02

1'-7" 1'-6"

• 🔖

...........

CONCRETE -

PEDESTAL FOR

RESETTING TRUSS SIDE MEMBERS

FROM EXISTING

BRIDGE (TYP.) (SEE PLAN OF

SPAN A )

ASSEMBLED BY: N. PIERCE CHECKED BY: J.M. BRITT

DRAWN BY: TLA 5/05

CHECKED BY: GM 6/05

DATE: 05-07 DATE: 07-07

ADDED 7/II/05R REV. 5/I/06 TLA/GM

(TYP.)

2 BAR ANODIZED

METAL RAIL (TYP.)

-CONST.JT.

(TYP.)

(TYP.)

0.6"Ø H.S. TRANSVERSE

IN 2"\Ø HOLE

POST-TENSIONED STRAND -

—3¾″@ @ BRG.

HALF-SECTION AT VOIDS

SHEAR KEYS TO BE FILLED WITH GROUT AFTER-

ALL ERECTION HAS BEEN COMPLETED AND AFTER

FINAL TENSIONING OF TRANSVERSE STRANDS

15'-0"

2"MAX.FORM LINER RELIEF

(TYP.EA.FACE)

11'-11"

- ASPHALT WEARING SURFACE

(SEE ROADWAY PLANS)

1'-6"CONCRETE PARAPET WITH ARCHITECTURAL —

CONCRETE SURFACE TREATMENT (TYP.)

0.02

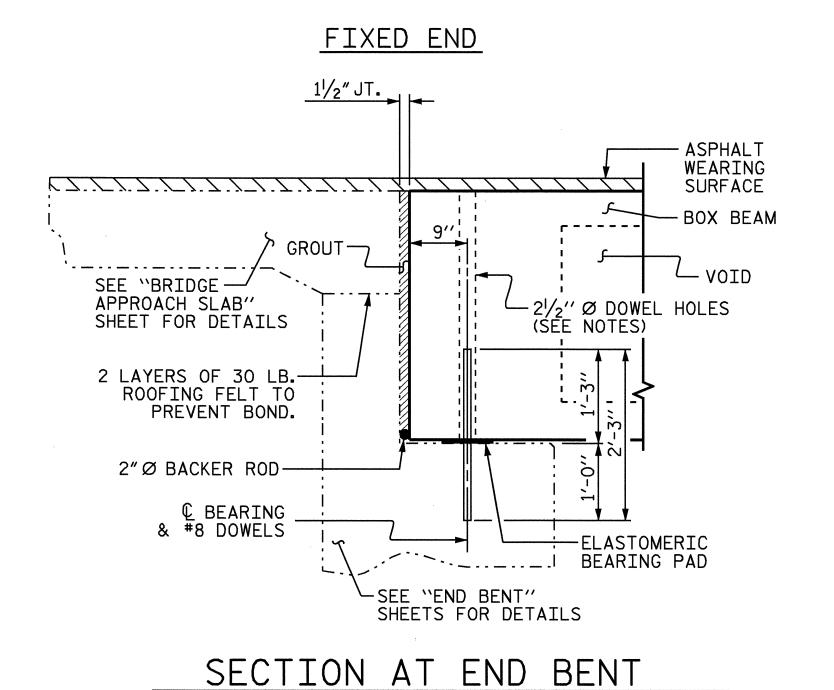
# TYPICAL SECTION

10- 3'-0" X 3'-3" PRESTRESSED CONCRETE BOX BEAM UNITS = 30'-0"

30'-0"(OUT TO OUT)

23'-10" (CLEAR ROADWAY)

67⁄8″@ ⊈ BRG.−



— € BEARING PAD — € 1 1/4" Ø HOLES BEARING PAD
- TYPE I -

FIXED END (TYPE I - 20 REQ'D)

# ELASTOMERIC BEARING DETAILS

BEARING PAD SHALL BE 60 DUROMETER HARDNESS

B-3019 PROJECT NO.\_

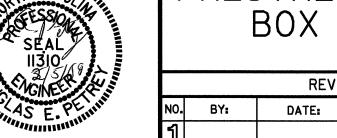
POLK \_ COUNTY

13+27.50-L-STATION:

SHEET 1 OF 4

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION STANDARD

3'-0" X 3'-3" PRESTRESSED CONCRETE BOX BEAM UNIT

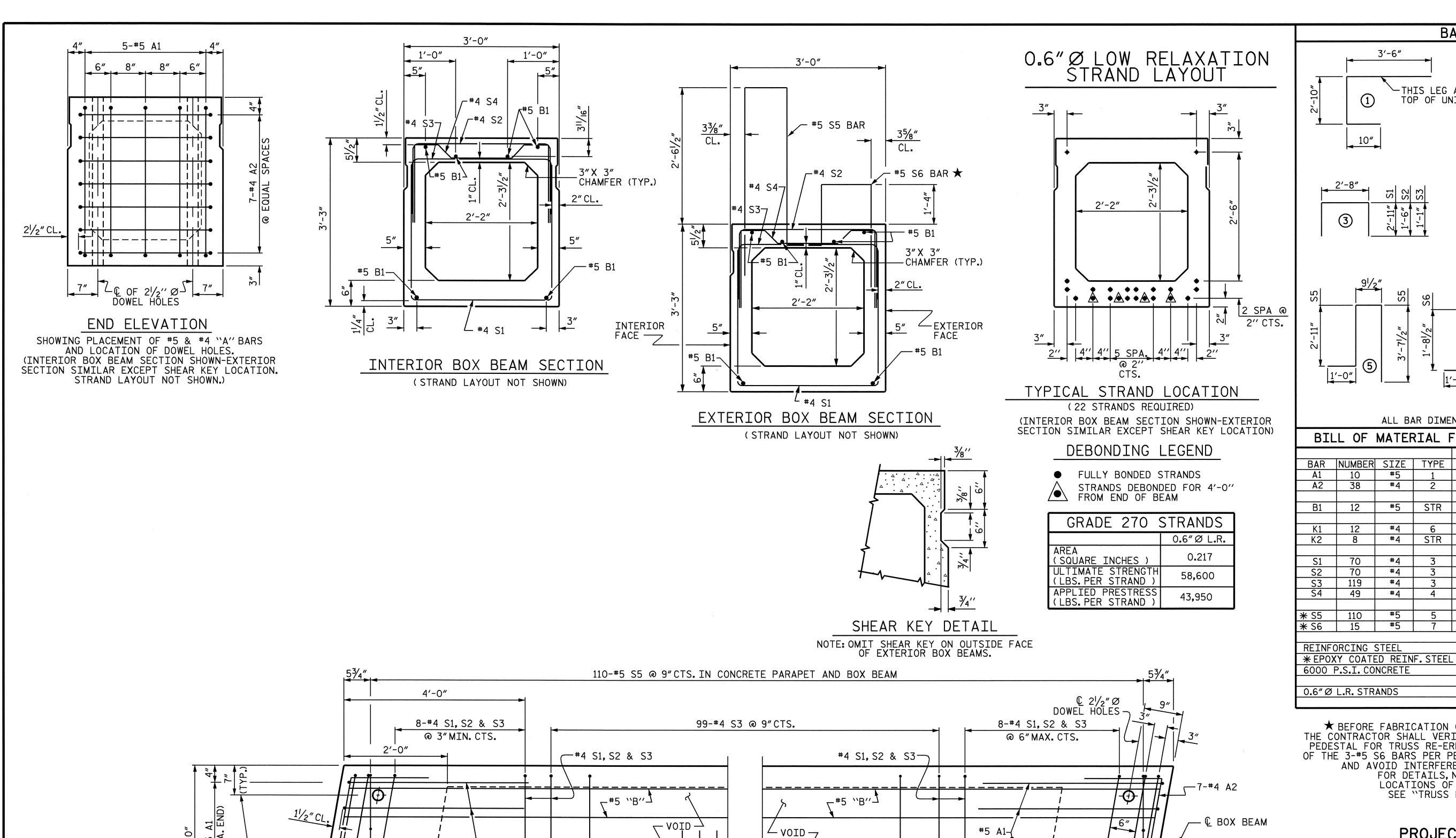


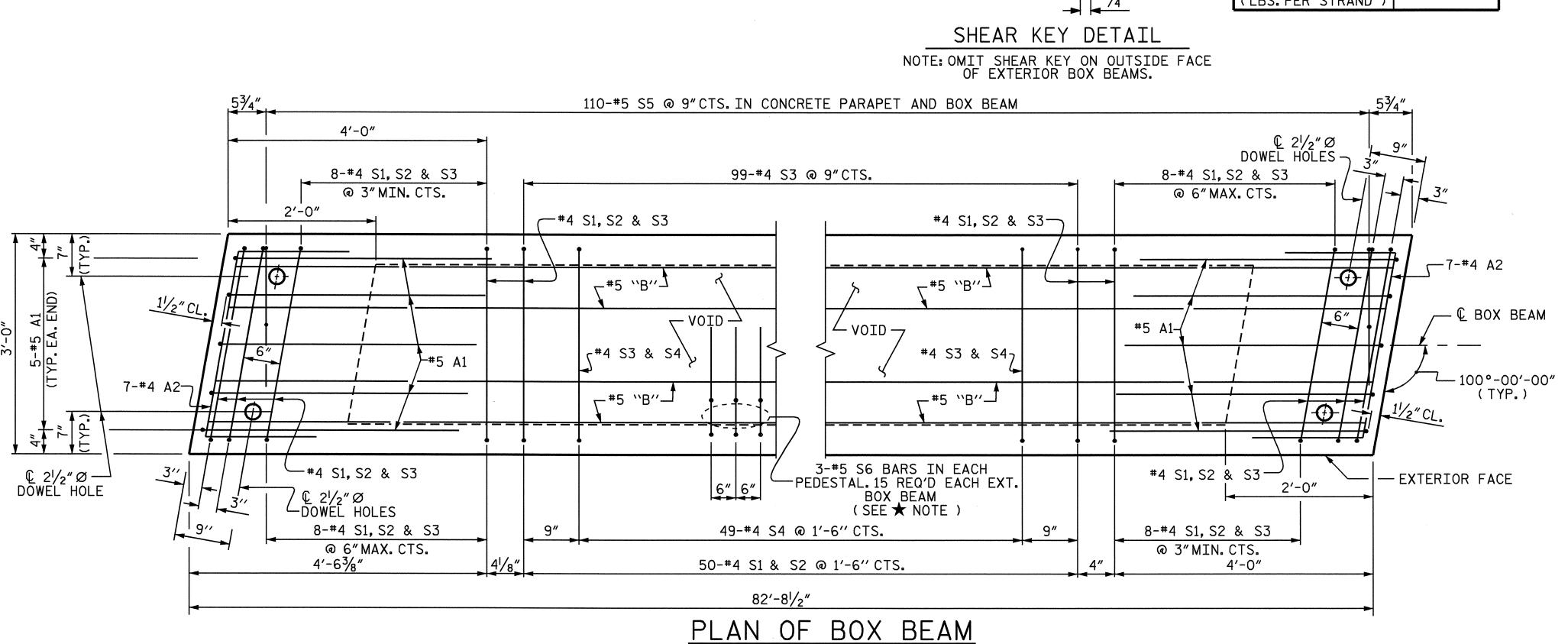


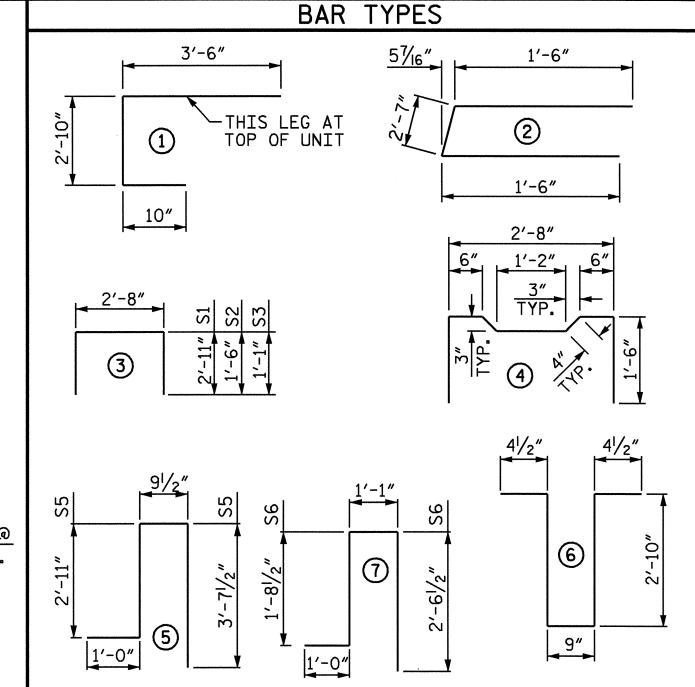
SHEET NO. **REVISIONS** S-3 BY: TOTAL SHEETS

★ CONCRETE PEDESTALS MAY NEED TO BE SHIFTED TO MATCH LAYOUT SHOWN ON "TRUSS RE-ERECTION" SHEET. CONTRACTOR TO VERIFY LOCATION OF PEDESTALS PRIOR TO FABRICATION OF EXTERIOR BOX BEAM UNITS. SEE TRUSS RE-ERECTION SHEET. 85'-0"(W.P. #1 TO W.P. #2) 82'-81/2" ✓ © ½"EXP.JT.MAT'L IN PARAPET (TYP.) 27'-51/2" 27'-81/2" 27'-61/2" **★** 12'-1<sup>1</sup>/<sub>4</sub>" **★** 15′-0″ **★**15′-0″ ★ 12'-1<sup>1</sup>/<sub>4</sub>" **★** 13′-6″ **★** 15′-0″ (TYP.EA.SIDE) (TYP.EA.SIDE) 1'-6" (TYP.EA.SIDE) (TYP.EA.SIDE) (TYP.EA.SIDE) (TYP.EA.SIDE) (TYP.) END POST CONCRETE PEDESTAL -CONCRETE PEDESTAL (TYP.) FOR TRUSS (TYP.) (TYP.) /-GUTTERLINE FOR TRUSS (TYP.) ) JOINT @ END BENT No. 2 - 6-#5 B1 IN BOX BEAM (2 BAR RUNS )
(3'-0"SPLICE)(SEE BOX BEAM SECTION
VIEWS FOR LOCATION)(TYP.) GUTTERLINE-FILL FACE @ — END BENT No. 1 - W.P. #2 W.P. #1-— € SURVEY -L-U JOINT @ —— END BENT No.1 ~100°-00′-00" (TYP.) FILL FACE @ END BENT No. 2 2"MAX. \_\_\_ FORM LINER GUTTERLINE-RELIEF GUTTERLINE — € 2"Ø HOLES FOR 0.6"Ø
-H.S. TRANSVERSE POST-TENSIONING
STRANDS (TYP.) 53/4" 53/4" ∠8-#5 B2 IN EACH PARAPET SECTION (TYP.) -2"MAX.FORM LINER RELIEF (TYP.EXTERIOR FACE) L 2"MAX. FORM LINER RELIEF 7'-11¾"(TYP.) 22'-3"(TYP.) 7′-11<sup>3</sup>⁄<sub>4</sub>″(TYP.) 22'-3"(TYP.) 22'-3"(TYP.) 110-#5 S5 @ 9"CTS. IN BOX BEAM AND PARAPET (TYP.EA.SIDE)  $27'-8\frac{1}{2}''$ 27'-61/2" 27'-5<sup>1</sup>/<sub>2</sub>" 82'-81/2" FIXED <u>FIXED</u> B-3019 PLAN OF SPAN A PROJECT NO. POLK COUNTY 13+27.50-L-STATION: © 2"Ø HOLES FOR 0.6"Ø H.S. TRANSVERSE POST TENSIONING STRANDS (TYP.) 21'-31/8" SHEET 2 OF 4 (TYP.) STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION 4<sup>1</sup>/<sub>16</sub>" (TYP.) 2'-13/4" 5′-55⁄<sub>16</sub>″ 5′-5<sup>5</sup>/<sub>16</sub>" 2'-0" SUPERSTRUCTURE 7′-11¾″ 7'-113/4" 22'-3" 22'-3" 22'-3" PLAN OF SPAN A  $82'-8\frac{1}{2}''$ PLAN OF BOX BEAM UNIT **REVISIONS** SHEET NO. NO. BY: S-4 NO. BY: DATE: DATE: N. PIERCE J.M. BRITT \_\_ DATE : 05-07 \_\_ DATE : 07-07 DRAWN BY : TOTAL SHEETS

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BIL	L OF	MATER	RIAL F	FOR ONE	BOX BE	EAM SEC	TION				
	EXTERIOR UNIT   INTERIOR UNIT										
BAR	NUMBER	SIZE	TYPE	LENGTH	WEIGHT	LENGTH	WEIGHT				
A1	10	#5	1	7′-2″	75	7'-2"	75				
A2	38	#4	2	5′-7″	142	5′-7″	142				
B1	12	#5	STR	42'-9"	535	42'-9"	535				
						-					
K1	12	#4	6	7′-2″	57	7′-2″	57				
K2	8	#4	STR	2'-7"	14	2′-7″	14				
S1	70	#4	3	8′-6″	397	8'-6"	397				
S2	70	#4	3	5′-8″	265	5′-8″	265				
S3	119	#4	3	4'-10"	384	4'-10"	384				
S4	49	#4	4	5′-10″	191	5′-10″	191				
* S5	110	#5	5	8'-4"	956						
* S6	15	#5	7	6′-4″	99						
REINFO	ORCING :	STEEL		2	060 LBS.		2060 LBS.				

ALL BAR DIMENSIONS ARE OUT TO OUT

NOTE: \*\*BEFORE FABRICATION OF EXTERIOR BOX BEAM UNITS,
THE CONTRACTOR SHALL VERIFY THE LOCATION OF THE CONCRETE
PEDESTAL FOR TRUSS RE-ERECTION AND LOCATE THE POSITION
OF THE 3-#5 S6 BARS PER PEDESTAL TO MAINTAIN 2"MIN. COVER
AND AVOID INTERFERENCE WITH ATTACHMENT BOLTS.
FOR DETAILS, NOTES & APPROXIMATE
LOCATIONS OF CONCRETE PEDESTALS,
SEE "TRUSS RE-ERECTION" SHEET.

No. 22

B-3019 PROJECT NO. POLK COUNTY 13+27.50-L-STATION:

1055 LBS.

16.2 CU. YDS.

16.2 CU. YDS

SHEET NO.

S-5

TOTAL SHEETS

22

DATE:

No. 22

SHEET 3 OF 4 STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION STANDARD PRESTRESSED CONCRETE

REVISIONS BY: BY: DATE:

RIGHT EXTERIOR UNIT SHOWN, LEFT EXTERIOR UNIT SIMILAR EXCEPT LOCATIONS OF #5 S6 AND #5 S5, INTERIOR UNIT SIMILAR EXCEPT OMIT #5 S5 AND S6 BARS. FOR LOCATION OF DIAPHRAGMS, SEE PLAN OF SPANS. FOR REINFORCING STEEL IN DIAPHRAGMS, SEE DIAPHRAGM DETAILS.

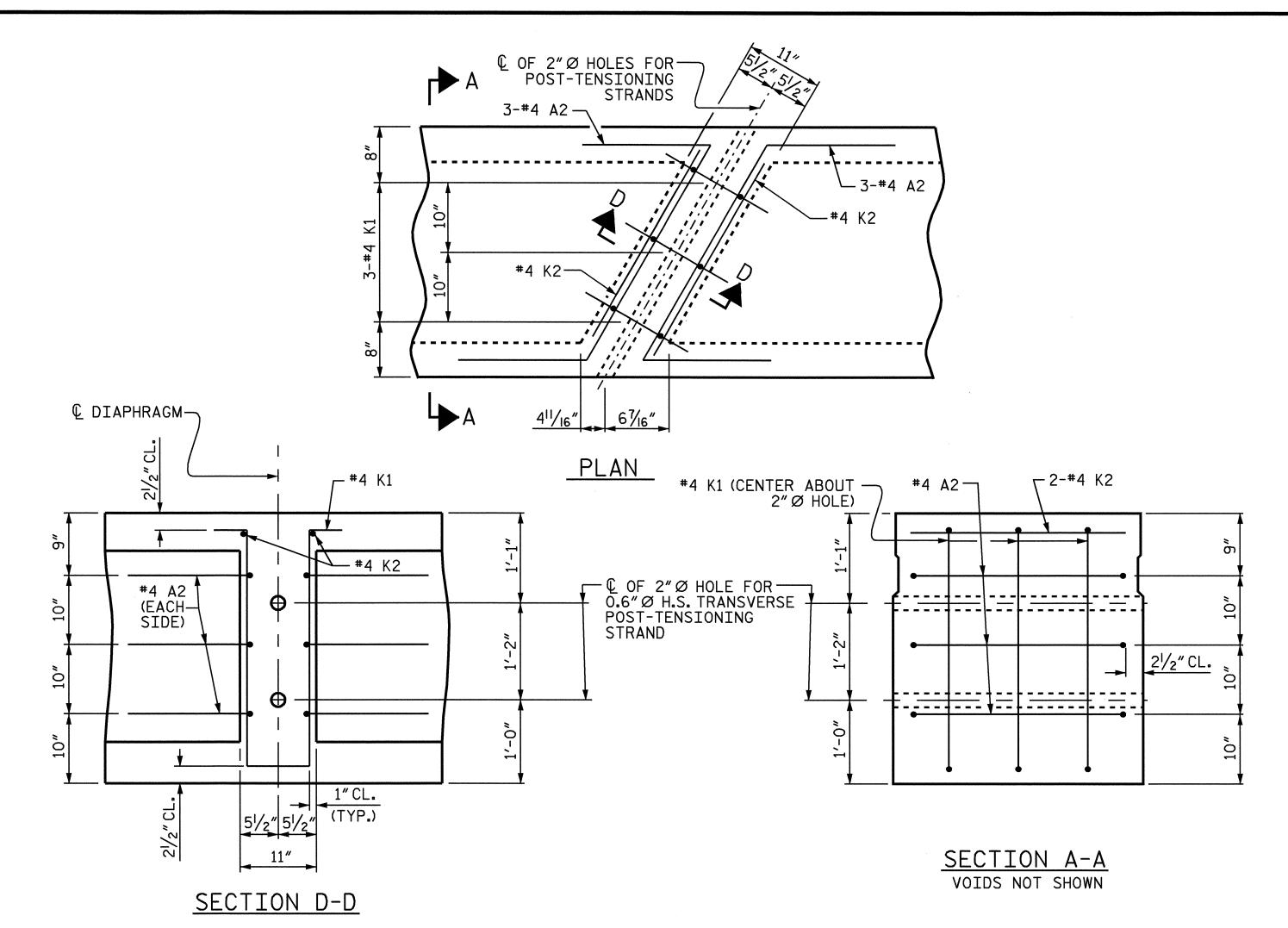
DATE: 05-07

DATE: 08-07

ASSEMBLED BY: N. PIERCE CHECKED BY: J.M. BRITT

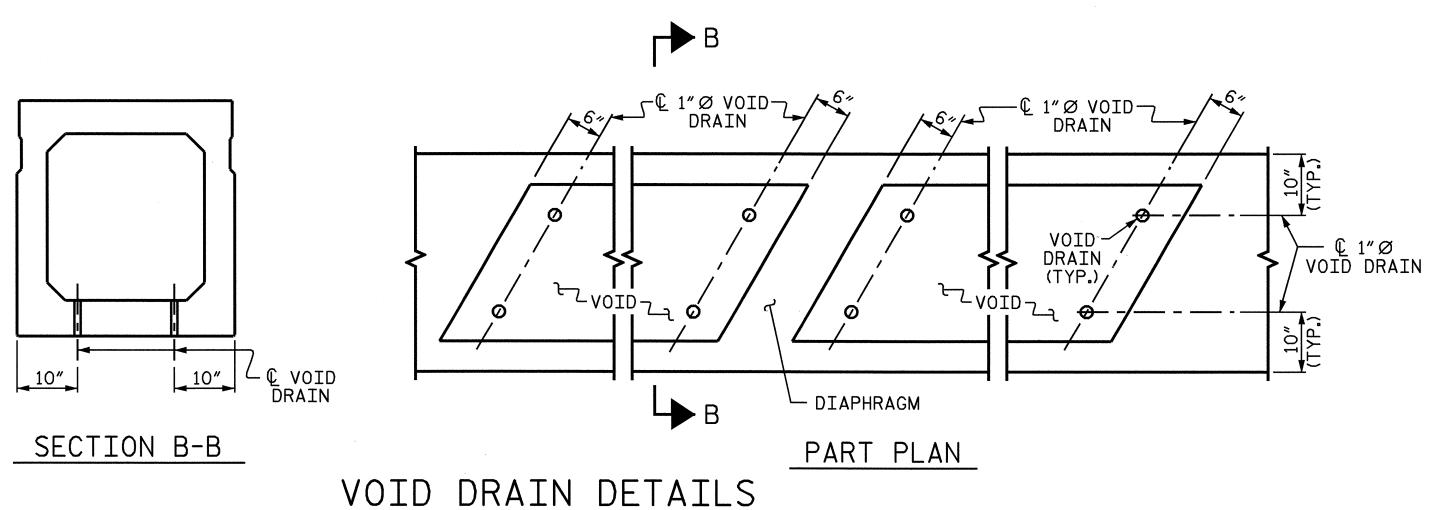
CHECKED BY : GM 6/05

DRAWN BY: TLA 5/05 ADDED 7/II/05
CHECKED BY: GM 6/05 REV. 5/I/06 TLA/GM

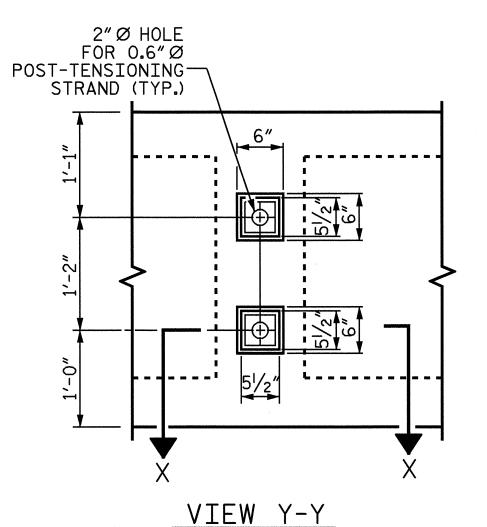


# DOUBLE DIAPHRAGM DETAILS

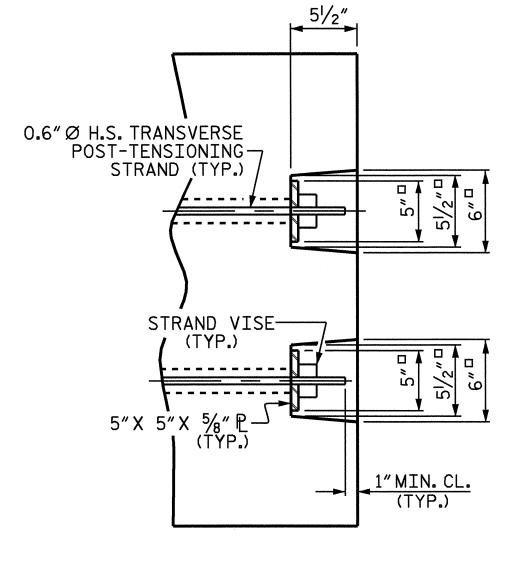
#4 "S" BARS NOT SHOWN. #4 "S" BARS MAY BE SHIFTED SLIGHTLY TO CLEAR 2" Ø HOLE.



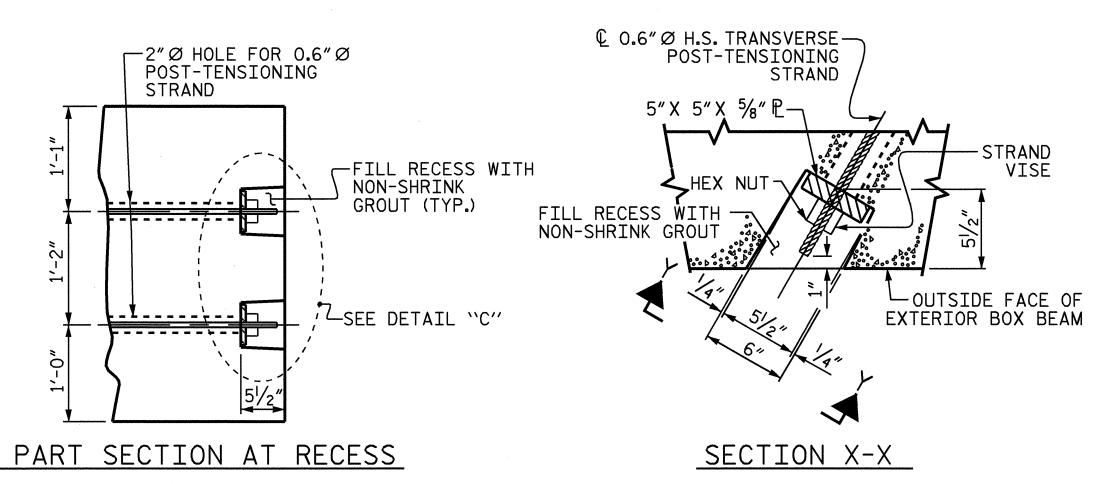
[	BOX BEAMS	REQUIRED	
	NUMBER	LENGTH	TOTAL LENGTH
SPAN A			
EXTERIOR B.B.	2	82'-8 <sup>l</sup> / <sub>2</sub> "	165′-5″
INTERIOR B.B.	8	82'-8 <sup>1</sup> / <sub>2</sub> "	661′-8″
TOTAL	10		927 09/



SHOWING ELEVATION VIEW OF GROUTED RECESS



DETAIL "C"



# GROUTED RECESS DETAIL AT END OF POST-TENSIONED STRANDS OF EXTERIOR BOX BEAM

DEAD LOAD DEFLECTION	AND CAMBER
	3'-0"× 3'-3"
	0.6"Ø L.R. STRAND
	SPAN A
CAMBER (BEAM ALONE IN PLACE)	23/4"
DEFLECTION DUE TO SUPERIMPOSED DEAD LOAD **	3/4"
FINAL CAMBER	2"

\*\* INCLUDES FUTURE WEARING SURFACE

PROJECT NO. B-3019
POLK COUNTY
STATION: 13+27.50-L-

SHEET 4 OF 4

DEPARTMENT OF TRANSPORTATION

RALEIGH

STANDARD

3'-0" X 3'-3"
PRESTRESSED CONCRETE
BOX BEAM UNIT

	REV:	ISION	S		SHEET NO.
BY:	DATE:	NO.	BY:	DATE:	S-6
		3			TOTAL SHEETS
		4			22

ASSEMBLED BY: N. PIERCE DATE: 05-07 CHECKED BY: J.M. BRITT DATE: 08-07

DRAWN BY: TLA 5/05 CHECKED BY: GM 6/05

DRAWN BY: GM 6/05

DRAWN BY: GM 6/05

DRAWN BY: GM 6/05

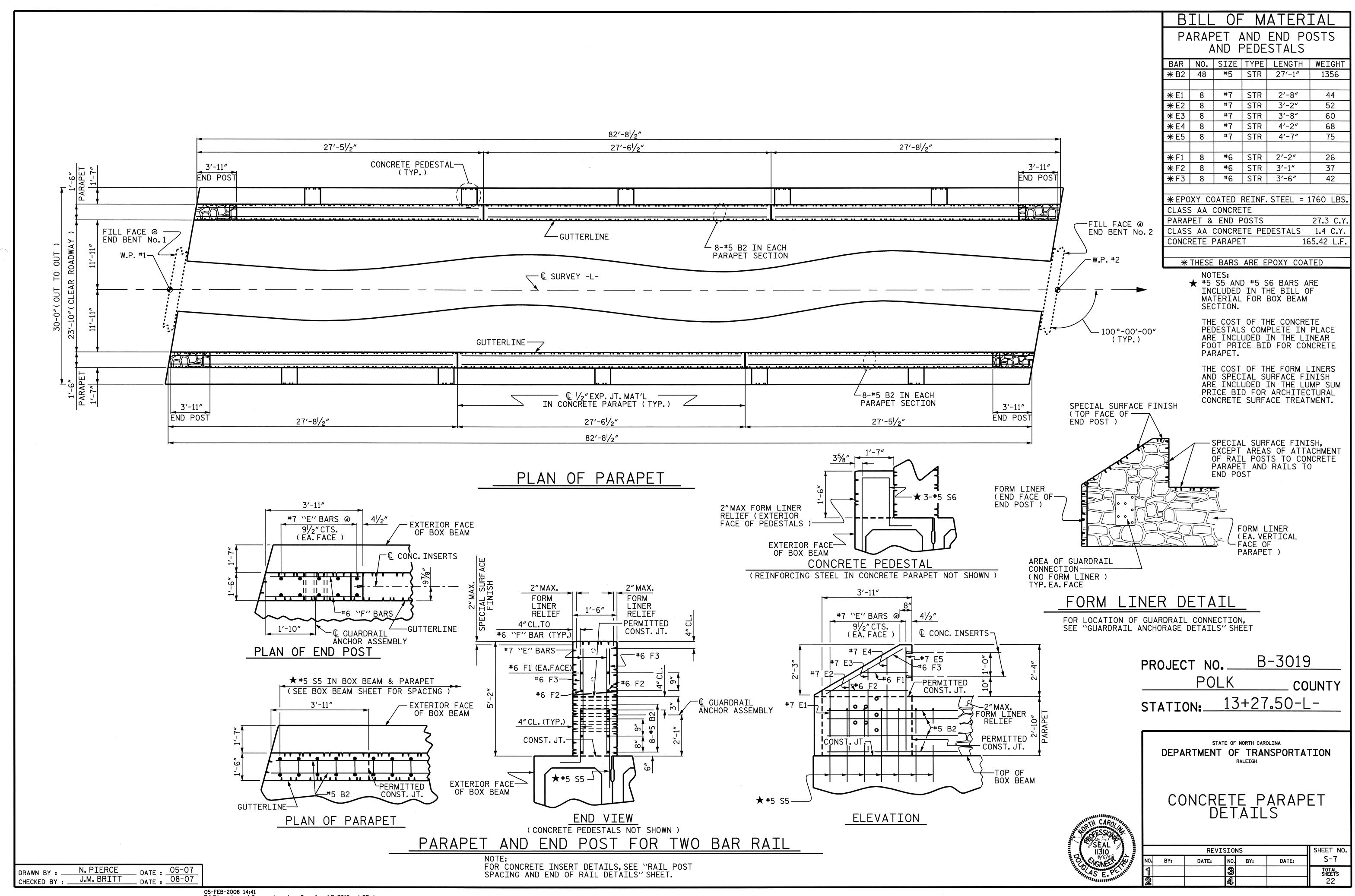
DRAWN BY: TLA 5/05 CHECKED BY: GM 6/05

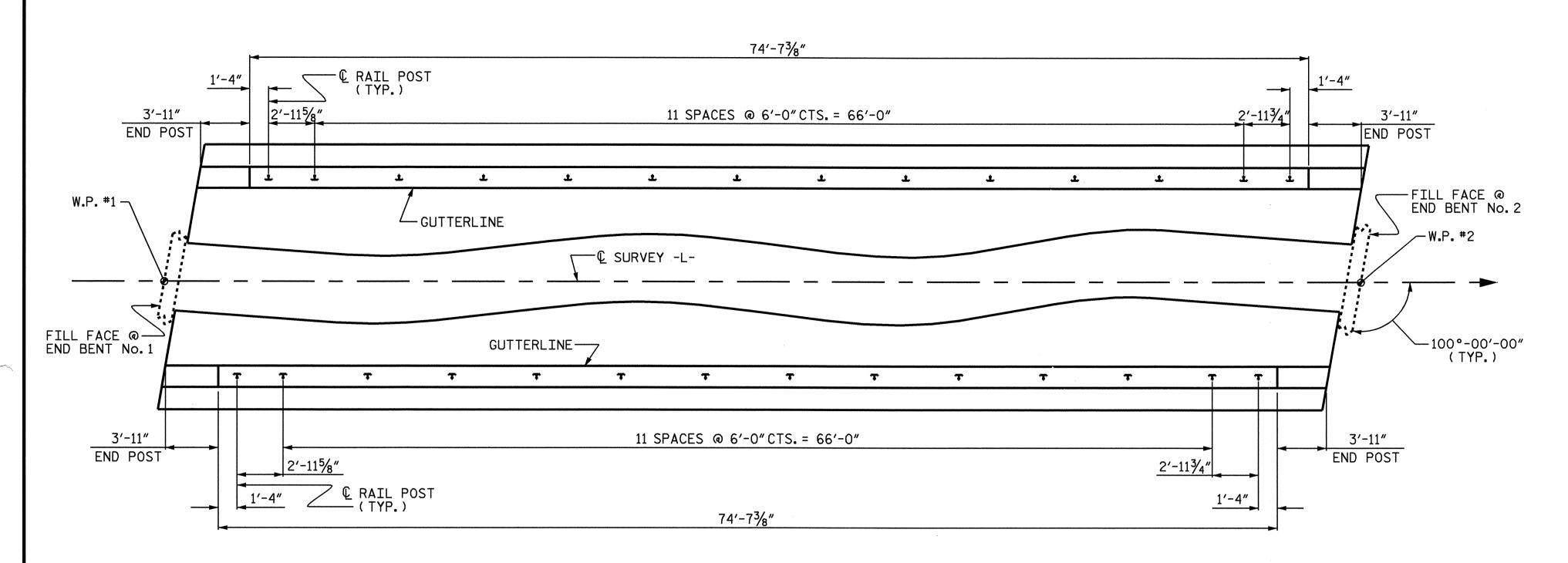
DRAWN BY: TLA 5/05 REV. 5/1/06

REV. 5/1/06

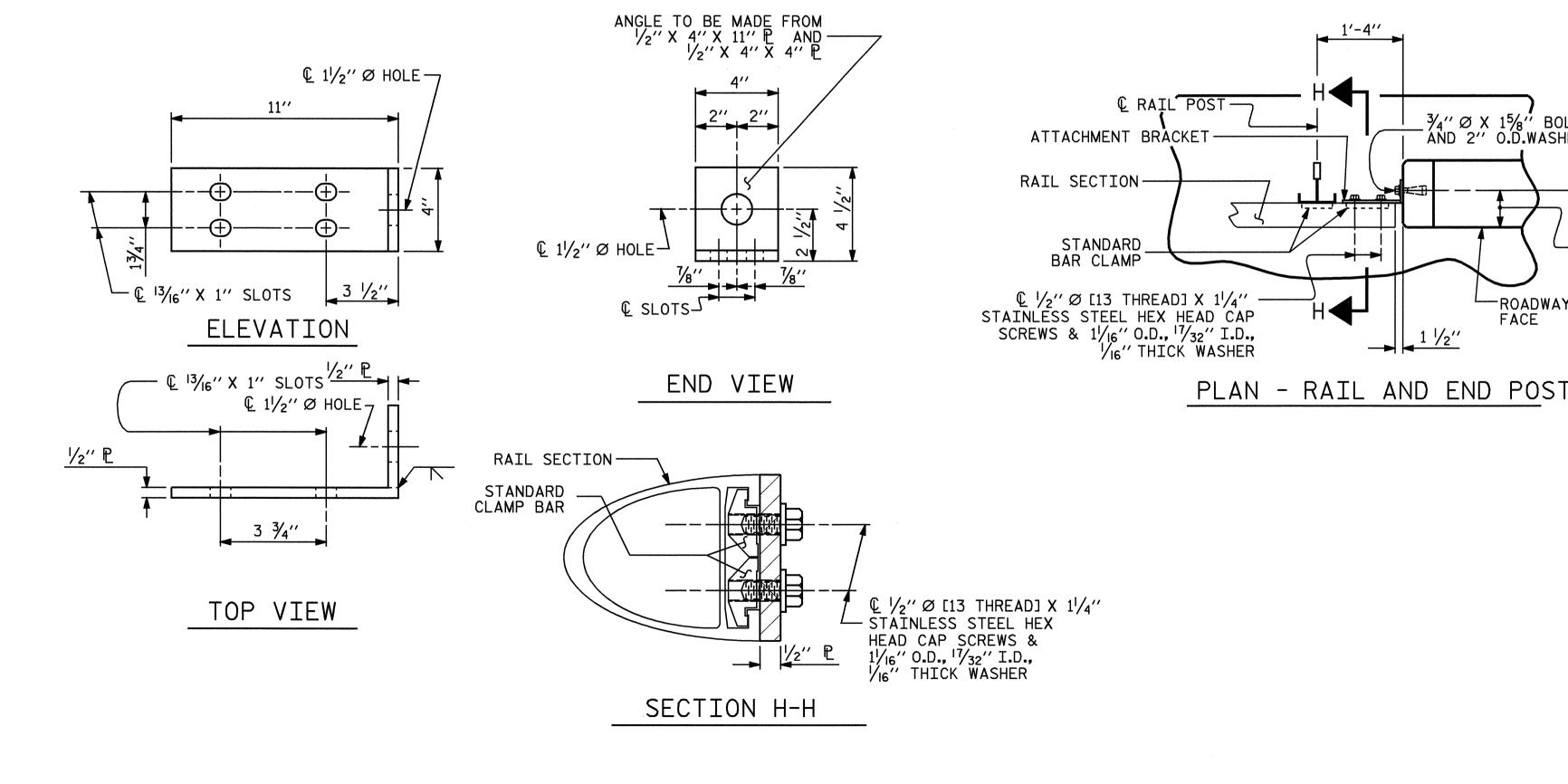
TLA/GM

(DIMENSIONS SHOWN ARE TYPICAL FOR EACH VOID)





# PLAN OF RAIL POST SPACINGS



# DETAILS FOR ATTACHING METAL RAIL TO END POST

ASSEMBLED BY: N. PIERCE CHECKED BY: J.M. BRITT DATE: 05-07 DATE: 08-07 REV. 10/17/00 REV. 5/7/03 REV. 5/1/06 LES/RDR DRAWN BY: FCJ 1/88 RWW/JTE CHECKED BY : CRK 3/89 TLA/GM

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### NOTES

### STRUCTURAL CONCRETE INSERT

- THE STRUCTURAL CONCRETE INSERT ASSEMBLY SHALL CONSIST OF THE FOLLOWING COMPONENTS:
- A. FERRULES SHALL BE MADE FROM STEEL MEETING THE REQUIREMENTS OF AASHTO M169. GRADE 12L14 AND SHALL HAVE A MINIMUM LENGTH OF THREADS OF 11/2".
- B. 1  $\frac{3}{4}$ " Ø X  $1\frac{5}{8}$ " BOLT WITH WASHER. BOLT SHALL CONFORM TO THE REQUIREMENTS OF ASTM A307. BOLT AND WASHER SHALL BE GALVANIZED. (AT THE CONTRACTOR'S OPTION, STAINLESS STEEL BOLT AND WASHER MAY BE USED AS AN ALTERNATE FOR THE  $\frac{3}{4}$ "  $\varnothing$  X  $1\frac{5}{8}$ " GALVANIZED BOLT AND WASHER. THEY SHALL CONFORM TO OR EXCEED THE MECHANICAL REQUIREMENTS OF ASTM A307. THE USE OF THIS ALTERNATE SHALL BE APPROVED BY THE ENGINEER.)
- C. WIRE STRUT SHOWN IN THE CONCRETE INSERT ASSEMBLY DETAIL IS THE MINIMUM ALLOWABLE SIZE AND SHALL HAVE A MINIMUM TENSILE STRENGTH OF 100,000 PSI. AS AN OPTION, A  $\gamma_6$ " Ø WIRE STRUT WITH A MINIMUM TENSILE STRENGTH OF 90,000 PSI IS ACCEPTABLE.

### NOTES

### METAL RAIL TO END POST CONNECTION

THE METAL RAIL TO END POST CONNECTION SHALL CONSIST OF THE FOLLOWING COMPONENTS:

- A.  $\frac{1}{2}$ " PLATES SHALL CONFORM TO AASHTO M270 GRADE 36 AND SHALL BE GALVANIZED AFTER FABRICATION.
- B. 3/4" STRUCTURAL CONCRETE INSERT SHALL HAVE A WORKING LOAD SHEAR CAPACITY OF 4800 LBS. THE FERRULES SHALL ENGAGE A 34"Ø X 15%" BOLT WITH 2" O.D. WASHER IN PLACE. THE 34"Ø X 15%" BOLT SHALL HAVE N. C. THREADS.
- C. CAP SCREWS FOR RAIL ATTACHMENT TO ANGLE SHALL CONFORM TO THE REQUIREMENTS OF ASTM F593 ALLOY 305 STAINLESS STEEL. CAP SCREWS TO BE CENTERED IN SLOTS AT 60°F.
- D. STANDARD CLAMP BARS (SEE METAL RAIL SHEET ).

¾"∅ X 15%" BOLT AND 2" O.D.WASHER

ROADWAY

E.  $\frac{1}{2}$ " Ø PIPE SLEEVES (IF REQUIRED) TO BE GALVANIZED.

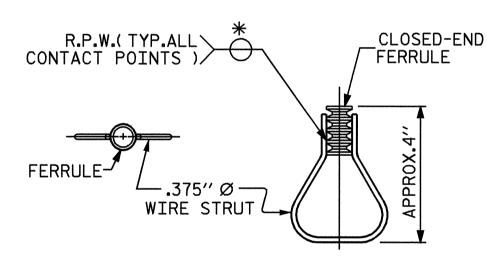
THE COST OF THE STANDARD CLAMP BARS AND CAP SCREWS USED IN THE METAL RAIL TO END POST CONNECTION SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR LINEAR FEET OF 1 OR 2 BAR METAL RAILS.

THE  $\frac{3}{4}$ " STRUCTURAL CONCRETE INSERT WITH BOLT SHALL BE ASSEMBLED IN THE SHOP.

© ¾″ STRUCTURAL -CONCRETE INSERT

THE COST OF THE  $\frac{3}{4}$ " STRUCTURAL CONCRETE INSERT ASSEMBLY, AND THE  $\frac{1}{2}$ " PLATES COMPLETE IN PLACE SHALL BE INCLUDED IN THE VARIOUS PAY ITEMS.

THE CONTRACTOR, AT HIS OPTION, MAY USE AN ADHESIVE BONDING SYSTEM IN LIEU OF THE STRUCTURAL CONCRETE INSERT EMBEDDED IN THE END POST. IF THE ADHESIVE BONDING SYSTEM IS USED, THE  $\frac{3}{4}$ "  $\frac{3}{4}$ BOLT WITH WASHER SHALL BE REPLACED WITH A  $\frac{3}{4}$ "Ø X  $\frac{6}{2}$ " BOLT AND 2" O.D. WASHER. ALL SPECIFICATIONS THAT APPLY TO THE  $\frac{3}{4}$ "Ø X  $\frac{15}{8}$ " BOLT SHALL APPLY TO THE  $\frac{3}{4}$ "Ø X 6  $\frac{1}{2}$ " BOLT. FIELD TESTING OF THE ADHESIVE BONDING SYSTEM IS NOT REQUIRED.



PLAN

ELEVATION

STRUCTURAL CONCRETE

\* EACH WELDED ATTACHMENT OF WIRE TO FERRULE SHALL DEVELOP THE TENSILE STRENGTH OF THE WIRE.

B-3019 PROJECT NO.

POLK

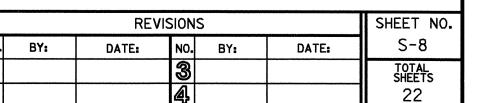
COUNTY

STATION: 13+27.50-L-

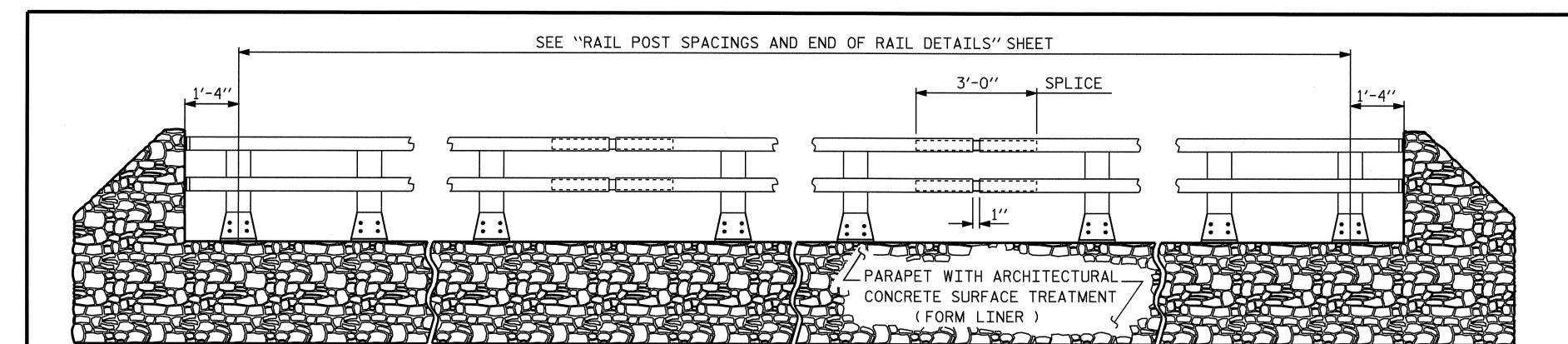
STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION STANDARD

RAIL POST SPACINGS

END OF RAIL DETAILS FOR TWO BAR METAL RAIL

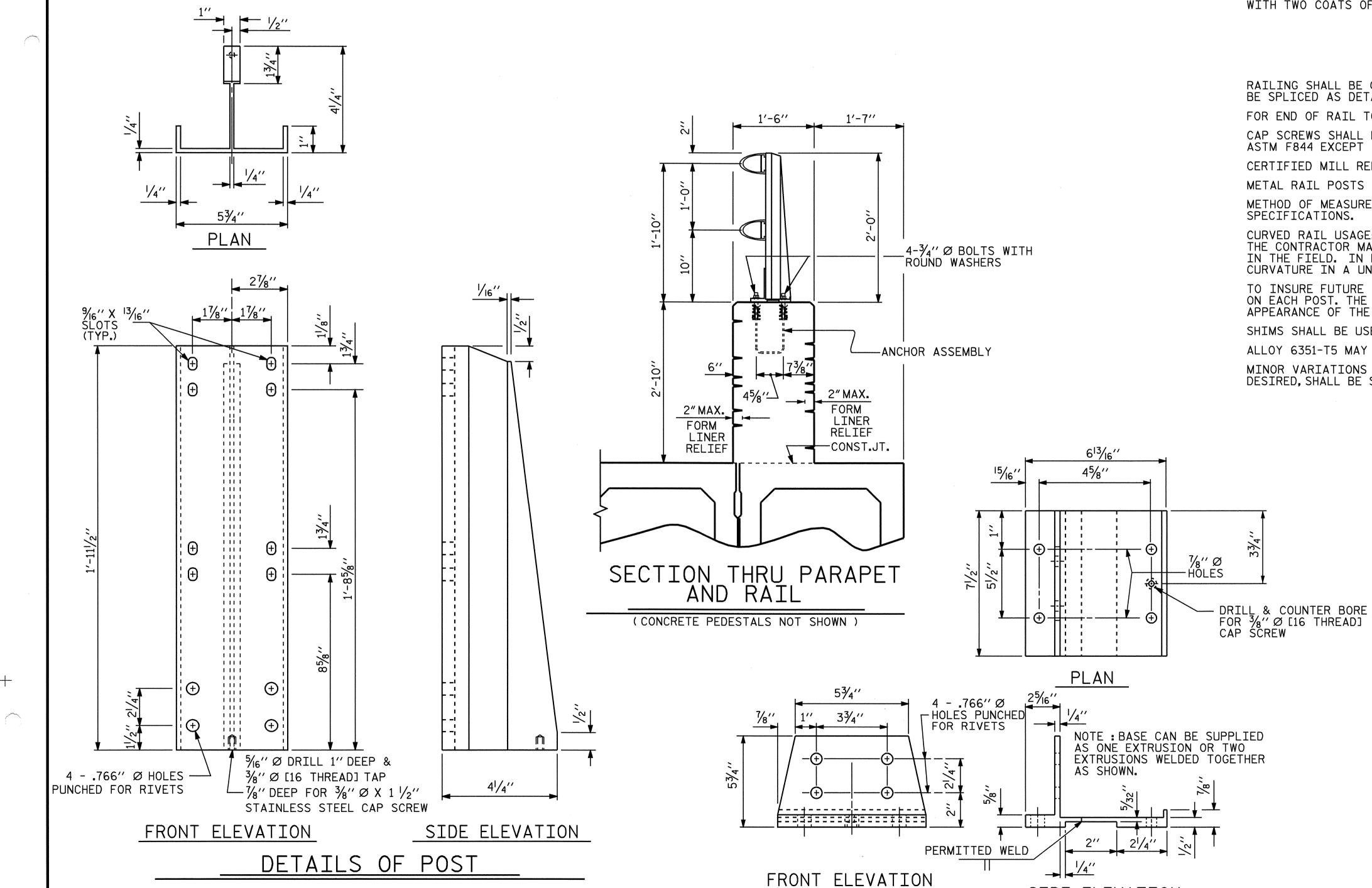


STD. NO. BMR2



## ELEVATION

NOTE : FOR ATTACHMENT OF METAL RAIL TO END POST. SEE STANDARD NO. BMR2.



DATE: 05-07 DATE: 08-07

> LES/RDR RWW/JTE

REV. 10/17/00 REV. 5/7/03R REV. 5/1/06

ASSEMBLED BY: N. PIERCE CHECKED BY: J.M. BRITT

DRAWN BY: EEM 6/94 CHECKED BY: RGW 6/94

# NOTES

### ALUMINUM RAILS

MATERIAL FOR POSTS, BASES AND RAILS, EXPANSION BARS AND CLAMP BARS SHALL BE ASTM B-221 ALLOY 6061-T6. MATERIAL FOR RIVETS SHALL BE ASTM B316 ALLOY 6061-T6. RIVETS SHALL BE STANDARD BUTTON HEAD AND CONE POINT COLD DRIVEN AS PER DRAWING.

THE BASE OF RAIL POSTS, OR ANY OTHER ALUMINUM SURFACE IN CONTACT WITH CONCRETE SHALL BE THOROUGHLY COATED WITH AN ALUMINUM IMPREGNATED CAULKING COMPOUND OF APPROVED QUALITY.

MATERIAL FOR SHIMS TO BE ASTM B209 ALLOY 6061-T6.

ALUMINUM FOR POSTS, BASES, RAILS, EXPANSION BARS, CLAMP BARS, WASHERS, RIVETS, CAPS AND SHIMS SHALL BE ANODIZED. THE CONTRACTOR SHALL SUBMIT THREE (3) SETS OF ASTM B-221 6061-T6 ALUMINUM SAMPLES ANODIZED LIGHT BRONZE, MEDIUM BRONZE AND DARK BRONZE TO THE ENGINEER. THE ENGINEER SHALL SELECT FROM THE SAMPLES FURNISHED BY THE CONTRACTOR THE COLOR WHICH MOST CLOSELY MATCHES THE PAINT COLOR ON THE RE-ERECTED TRUSS AND THE GUARDRAIL ATTACHED TO THE BRIDGE.

AFTER A SHADE OF BRONZE HAS BEEN SELECTED FOR THE RAILING, THE CONTRACTOR SHALL SUBMIT A SAMPLE OF A COMPATIBLE EXTERIOR ACRYLIC HOUSE PAINT TO THE ENGINEER. THIS PAINT SHALL MATCH THE ANODIZED RAIL COLOR AS CLOSELY AS POSSIBLE. AFTER ERECTION OF THE ANODIZED ALUMINUM RAILING, ALL EXPOSED ANCHOR BOLTS, NUTS, WASHERS, MACHINE SCREWS, CAP SCREWS, BOLTS AND BUILT UP ANGLES SHALL BE COATED WITH TWO COATS OF THIS PAINT.

### GENERAL NOTES

RAILING SHALL BE CONTINUOUS FROM END POST TO END POST OF BRIDGE. EACH JOINT IN RAIL LENGTH SHALL BE SPLICED AS DETAILED. PANEL LENGTHS OF RAIL SHALL BE ATTACHED TO A MINIMUM OF THREE POSTS.

FOR END OF RAIL TO CLEAR FACE OF CONCRETE END POST DIMENSION, SEE STANDARD NO. BMR2.

CAP SCREWS SHALL BE ASTM F593 ALLOY 305 STAINLESS STEEL. WASHERS SHALL MEET THE REQUIREMENTS OF ASTM F844 EXCEPT THEY SHALL BE MADE FROM ALLOY 304 STAINLESS STEEL.

CERTIFIED MILL REPORTS ARE REQUIRED FOR RAILS AND POSTS. SHOP INSPECTION IS NOT REQUIRED.

METAL RAIL POSTS SHALL BE SET NORMAL TO CURB GRADE.

METHOD OF MEASUREMENT FOR METAL RAILS: FOR LENGTH OF METAL RAILS TO BE PAID FOR, SEE THE STANDARD SPECIFICATIONS.

CURVED RAIL USAGE: WHERE RAILS ARE TO BE USED ON BRIDGES ON HORIZONTAL AND/OR VERTICAL CURVATURE THE CONTRACTOR MAY, AT HIS OPTION, HAVE THE REQUIRED CURVATURE IN THE RAIL FORMED IN THE SHOP OR IN THE FIELD. IN EITHER EVENT, THE RAIL SHALL CONFORM WITHOUT BUCKLING OR KINKING TO THE REQUIRED CURVATURE IN A UNIFORM MANNER ACCEPTABLE TO THE ENGINEER.

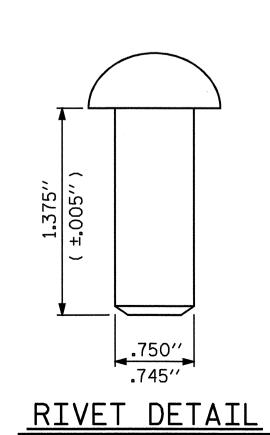
TO INSURE FUTURE IDENTIFICATION OF THE FABRICATOR, A PERMANENT IDENTIFYING MARK SHALL BE PLACED ON EACH POST. THE METHOD OF MARKING AND LOCATION SHALL BE SUCH THAT IT DOES NOT DETRACT FROM THE APPEARANCE OF THE POST, BUT REMAINS VISIBLE AFTER RAIL PLACEMENT.

SHIMS SHALL BE USED AS NECESSARY FOR POST ALIGNMENT.

ALLOY 6351-T5 MAY BE SUBSTITUTED FOR ALLOY 6061-T6 WHERE APPLICABLE.

MINOR VARIATIONS IN DETAILS OF METAL RAIL WILL BE CONSIDERED. DETAILS OF SUCH VARIATIONS, IF DESIRED, SHALL BE SUBMITTED FOR APPROVAL.

PAY LENGTH = 149.23 LIN. FT.



SHEET 1 OF 2

STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION
RALEIGH

STANDARD

2 BAR METAL RAIL

REVISIONS

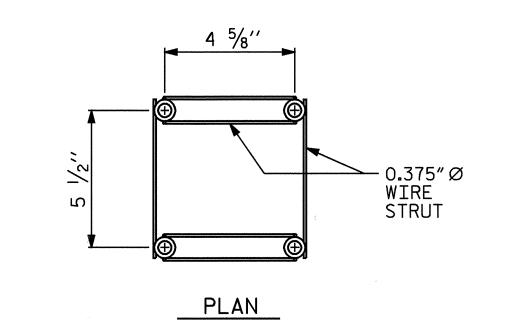
NO. BY: DATE: NO. BY: DATE: S-9

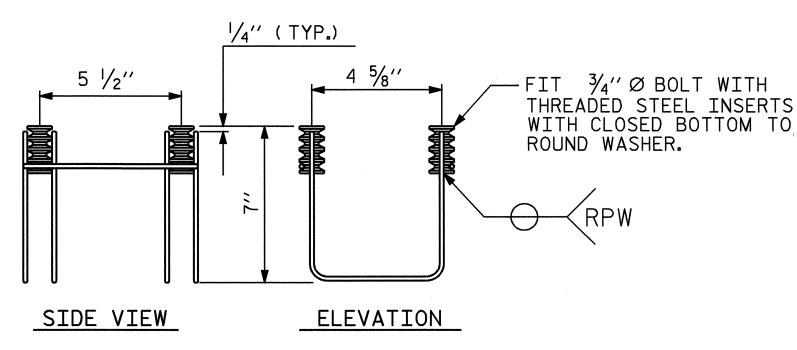
1 3 TOTAL SHEETS
2 4 2 22

18-JAN-2008 09:03
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POST BASE DETAILS

SIDE ELEVATION





MINIMUM LENGTH OF THREADS IN INSERT (FERRULE): 13/4"

# METAL RAIL ANCHOR ASSEMBLY

(28 ASSEMBLIES REQUIRED)

### NOTES

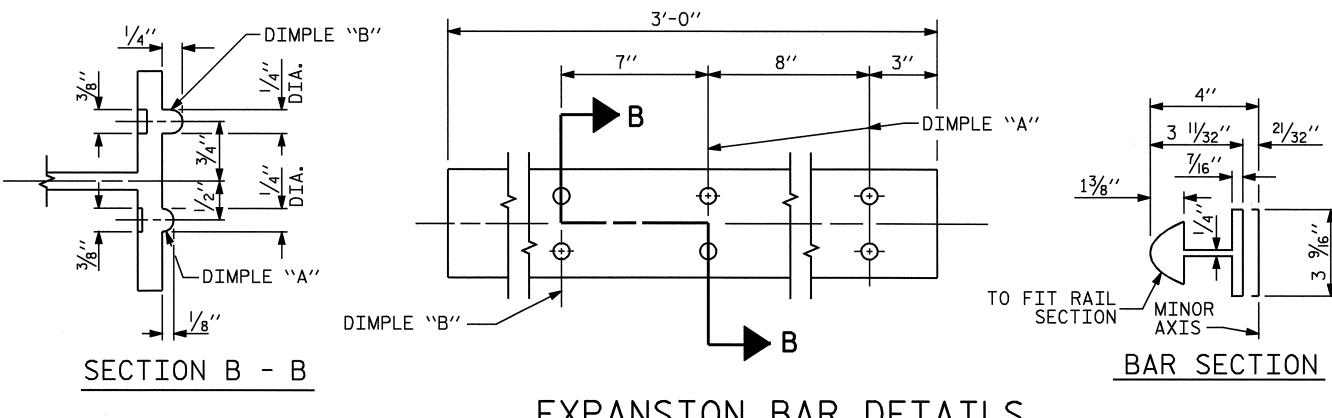
### STRUCTURAL CONCRETE ANCHOR ASSEMBLY

THE STRUCTURAL CONCRETE ANCHOR ASSEMBLY SHALL CONSIST OF THE FOLLOWING COMPONENTS:

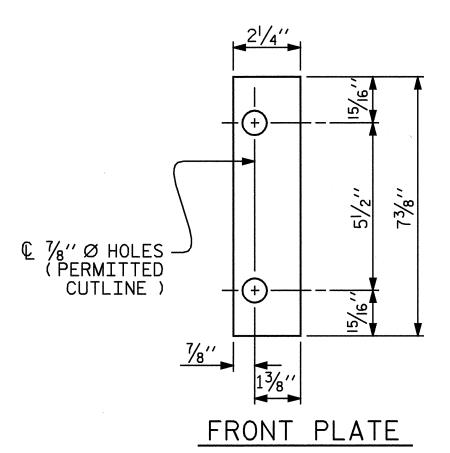
- A. FERRULES SHALL BE MADE FROM STEEL MEETING THE REQUIREMENTS OF AASHTO M169, GRADE 12L14 AND SHALL HAVE A MINIMUM LENGTH OF THREADS OF 2" FOR 3/4" FERRULES.
- B. 4 3/4" Ø X 21/2" BOLTS WITH WASHERS.BOLTS SHALL CONFORM TO THE REQUIREMENTS OF ASTM A307. BOLTS AND WASHERS SHALL BE GALVANIZED. AT THE CONTRACTOR'S OPTION, STAINLESS STEEL BOLTS AND WASHERS MAY BE USED AS AN ALTERNATE FOR THE 3/4" Ø X 21/2" GALVANIZED BOLTS AND WASHERS. THEY SHALL CONFORM TO OR EXCEED THE MECHANICAL REQUIREMENTS OF ASTM A307. THE USE OF THIS ALTERNATE SHALL BE APPROVED BY THE ENGINEER.
- C. WIRE STRUT SHOWN IN THE CONCRETE ANCHOR ASSEMBLY DETAIL IS THE MINIMUM ALLOWABLE SIZE AND SHALL HAVE A MINIMUM TENSILE STRENGTH OF 100,000 PSI. AS AN OPTION, A %6'' Ø WIRE STRUT WITH A MINIMUM TENSILE STRENGTH OF 90,000 PSI IS ACCEPTABLE.
- D. THE METAL RAIL ANCHOR ASSEMBLIES TO BE HOT DIPPED GALVANIZED TO CONFORM TO REQUIREMENTS OF AASHTO M111.
- E. THE COST OF THE METAL RAIL ANCHOR ASSEMBLY WITH BOLTS AND WASHERS COMPLETE IN PLACE SHALL BE INCLUDED IN THE PRICE BID FOR LINEAR FEET OF METAL RAIL.
- F. BOLTS TO BE TIGHTENED ONE-HALF TURN WITH A WRENCH FROM A FINGER-TIGHT POSITION.

THE CONTRACTOR, AT HIS OPTION, MAY USE ADHESIVELY ANCHORED ANCHOR BOLTS IN LIEU OF THE METAL RAIL ANCHOR ASSEMBLY. THE YIELD LOAD OF THE  $\frac{3}{4}$ " Ø BOLT IS 10 KIPS. FIELD TESTING OF THE ADHESIVE BONDING SYSTEM IS REQUIRED. FOR ADHESIVELY ANCHORED BOLTS OR DOWELS, SEE SPECIAL PROVISIONS.

WHEN ADHESIVELY ANCHORED ANCHOR BOLTS ARE USED, BOLTS SHALL MEET THE REQUIREMENTS OF ASTM F593 ALLOY 304 STAINLESS STEEL WITH MINIMUM 75,000 PSI ULTIMATE STRENGTH. NUTS SHALL MEET THE REQUIREMENTS OF ASTM F594 ALLOY 304 STAINLESS STEEL AND WASHERS SHALL MEET THE REQUIREMENTS OF ASTM F844 EXCEPT THEY SHALL BE MADE FROM ALLOY 304 STAINLESS STEEL.



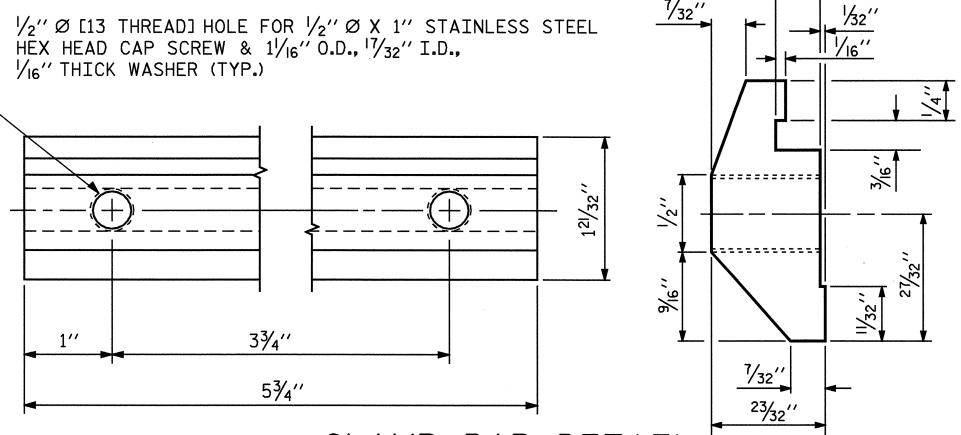




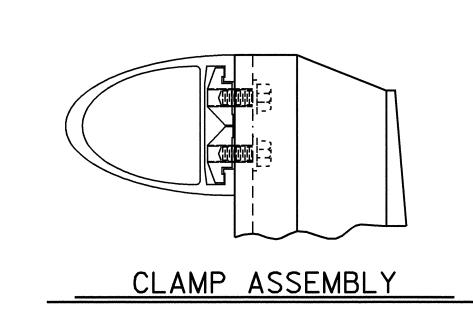
# ← © 1/8" Ø HOLES (PERMITTED CUTLINE) REAR PLATE

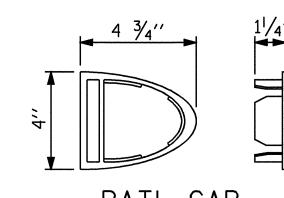
# SHIM DETAILS

NOTE: SHIMS MAY BE CUT ALONG PERMITTED CUTLINE OR SLOTTED TO EDGE OF PLATE TO FACILITATE PLACEMENT.

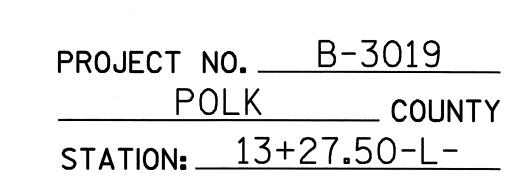


CLAMP BAR DETAIL (4 REQUIRED PER POST





RAIL CAP



MINOR AXIS

RAIL SECTION

- SEMI-ELLIPSE

MAJOR

SHEET 2 OF 2

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

STANDARD

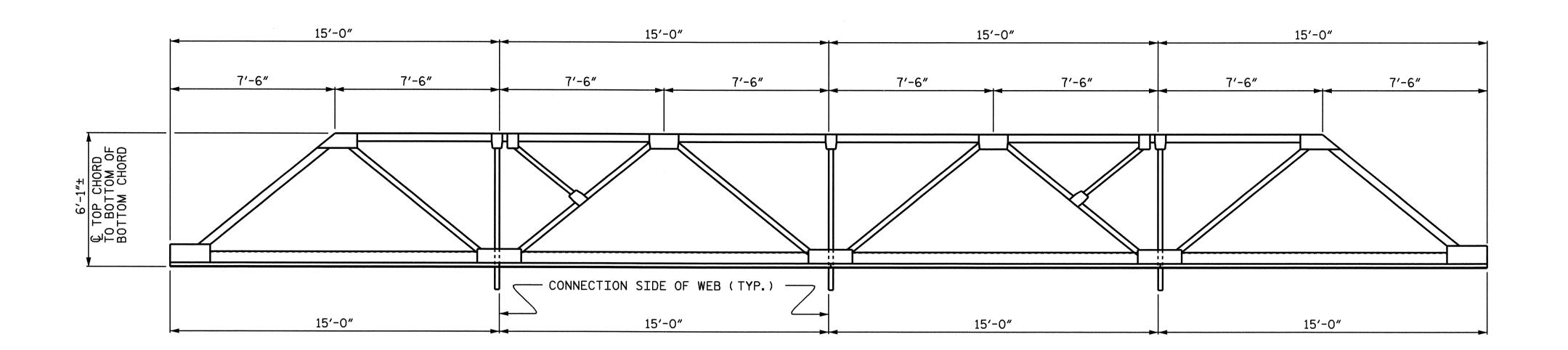


2	BAR	METAL	RAIL
	*		

	REVI	SIONS	<b>;</b>		SHEET NO.
BY:	DATE:	NO.	BY:	DATE:	S-10
		3			TOTAL SHEETS
		4			22

ASSEMBLED BY: N. PIERCE CHECKED BY: J.M. BRITT DATE: 05-07 DATE: 08-07 DRAWN BY : EEM 6/94 REV. 2/6/97 EEM/RGW REV. 8/16/99 MAB/LES REV. 5/7/03 RWW/JTE

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# EXISTING TRUSS SIDE MEMBERS ELEVATION

(RIGHT SIDE SHOWN, LEFT SIDE SIMILAR ) (ALL DIMENSIONS ARE APPROXIMATE AND MUST BE FIELD VERIFIED, SEE ★ NOTE )

### NOTES:

DIMENSIONS SHOWN FOR THE EXISTING TRUSS SIDE MEMBERS INDICATED ON THESE PLANS ARE FROM THE BEST INFORMATION AVAILABLE. SINCE THIS INFORMATION IS SHOWN FOR THE CONVENIENCE OF THE CONTRACTOR, THE CONTRACTOR SHALL HAVE NO CLAIM WHATSOEVER AGAINST THE DEPARTMENT OF TRANSPORTATION FOR ANY DELAYS OR ADDITIONAL COST INCURRED BASED ON DIFFERENCES BETWEEN THE EXISTING TRUSS SIDE MEMBERS SHOWN ON THE PLANS AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

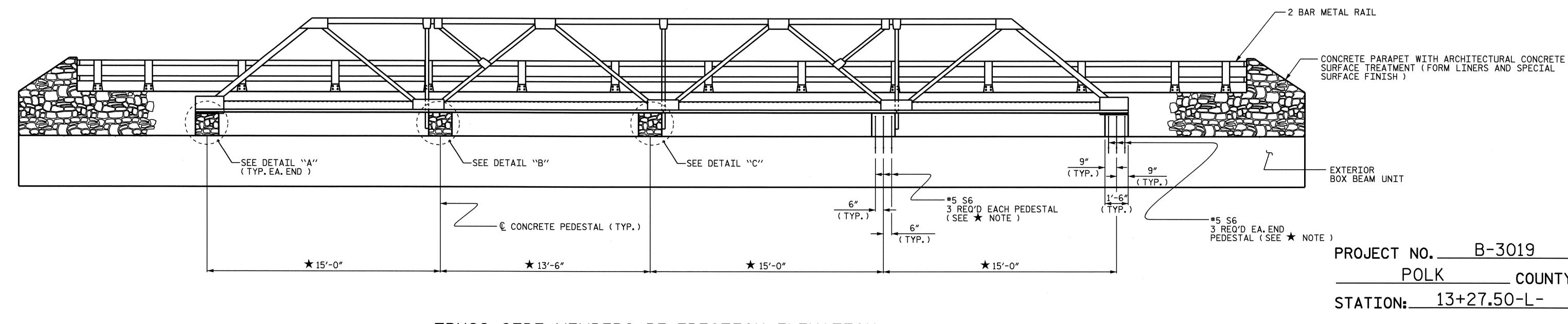
\* EXISTING TRUSS DIMENSIONS MUST BE FIELD VERIFIED AND DETERMINATION OF CONNECTION SIDE OF EXISTING VERTICAL TRUSS CHORD TO CONCRETE PEDESTAL MUST BE DETERMINED PRIOR TO CASTING OF BOX BEAMS SO REINFORCING STEEL IN BOX BEAMS AND CONCRETE PEDESTALS CAN BE LOCATED TO MATCH ACTUAL FIELD CONDITIONS AND AVOID INTERFERENCE WITH ATTACHMENT BOLTS.

THE CONTRACTOR SHALL SUBMIT DETAILED PLANS FOR LOCATIONS OF PEDESTALS, LOCATIONS OF 3-#5 S6 BARS IN BOX BEAMS AND PEDESTALS, AND CONNECTION OF TRUSS SIDE MEMBERS TO PEDESTALS TO THE ENGINEER FOR REVIEW AND APPROVAL PRIOR TO FABRICATION OF THE BOX BEAM UNITS.

CONTRACTOR TO REPLACE THE PORTIONS OF EXISTING TRUSS PLATES. CHANNELS AND I-SECTIONS WHERE REQUIRED DUE TO DETERIORATION AND SECTION LOSS AS DIRECTED BY THE ENGINEER. SEE SPECIAL PROVISIONS FOR RESTORATION AND RE-ERECTION OF TRUSS.

ARCHITECTURAL CONCRETE SURFACE TREATMENT (FORM LINERS) SHALL BE USED ON THE EXTERIOR FACE OF THE PEDESTALS. APPLY CONCRETE PENETRATING STAIN TO ALL OTHER EXPOSED CONCRETE SURFACES OF THE CONCRETE PEDESTALS. SEE ARCHITECTURAL CONCRETE SURFACE TREATMENT AND CONCRETE PENETRATING STAIN SPECIAL PROVISIONS.

FOR DETAILS "A," "B," AND "C" SEE "TRUSS RE-ERECTION CONNECTIONS" SHEET.



TRUSS SIDE MEMBERS RE-ERECTION ELEVATION

(RIGHT SIDE SHOWN, LEFT SIDE SIMILAR)

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

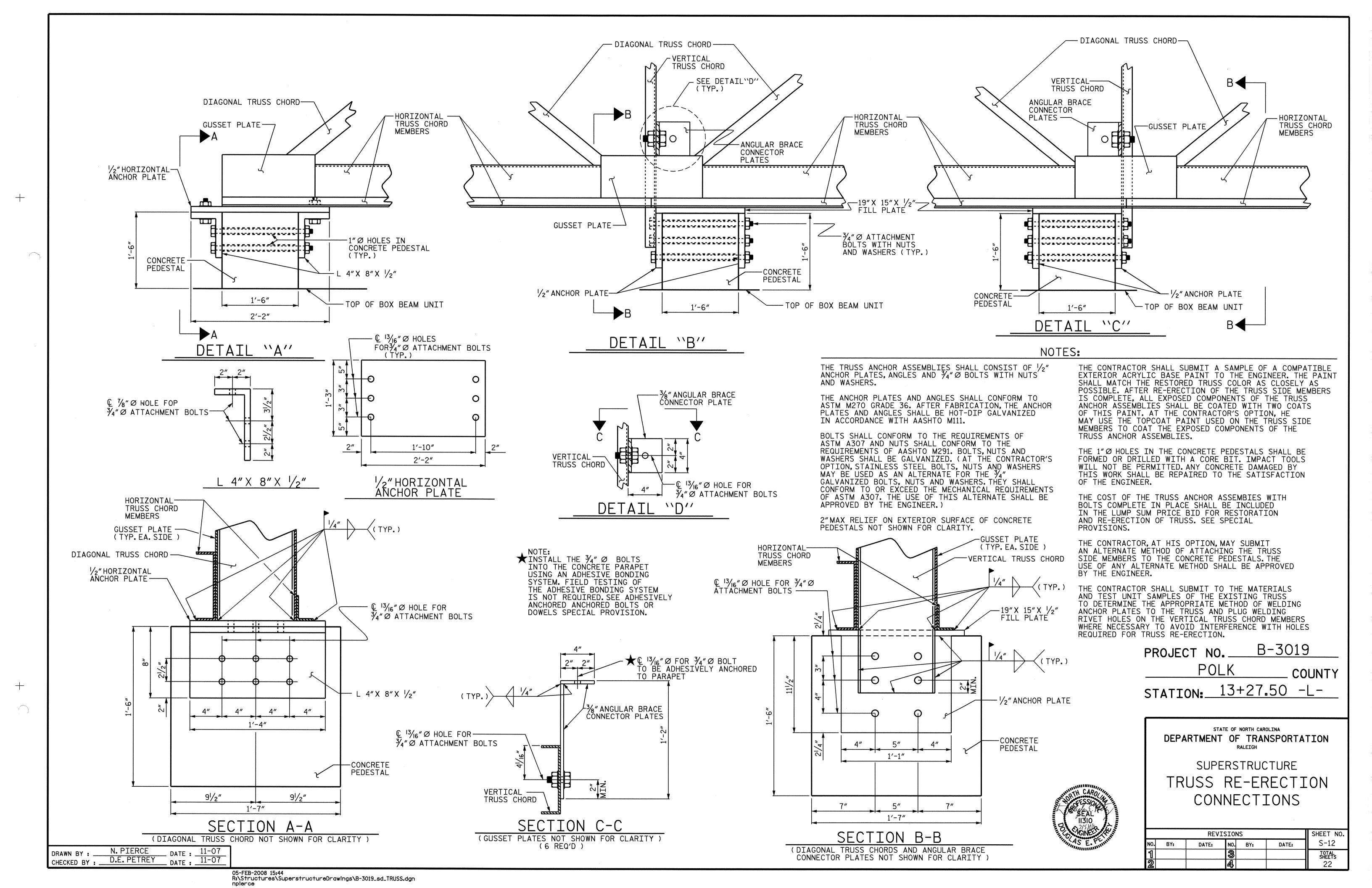
COUNTY

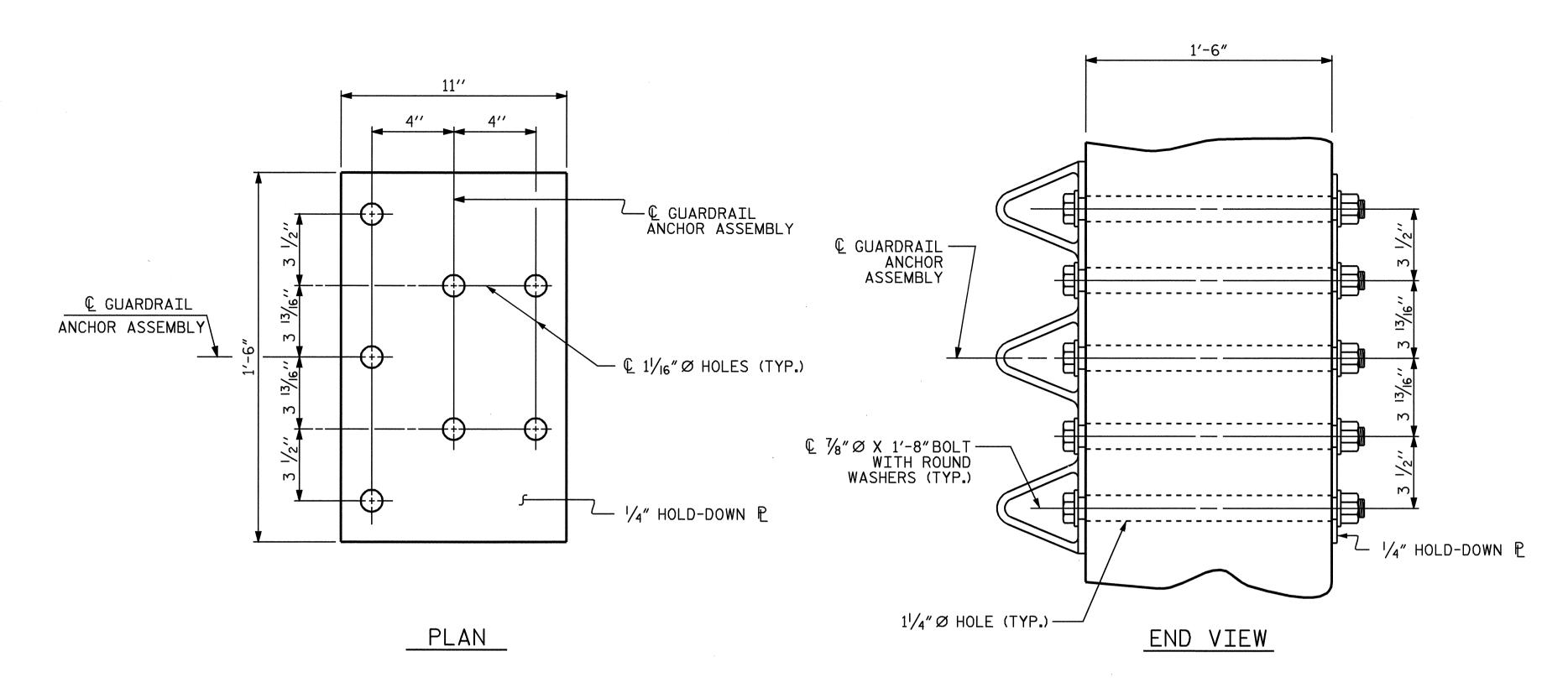
SUPERSTRUCTURE TRUSS RE-ERECTION



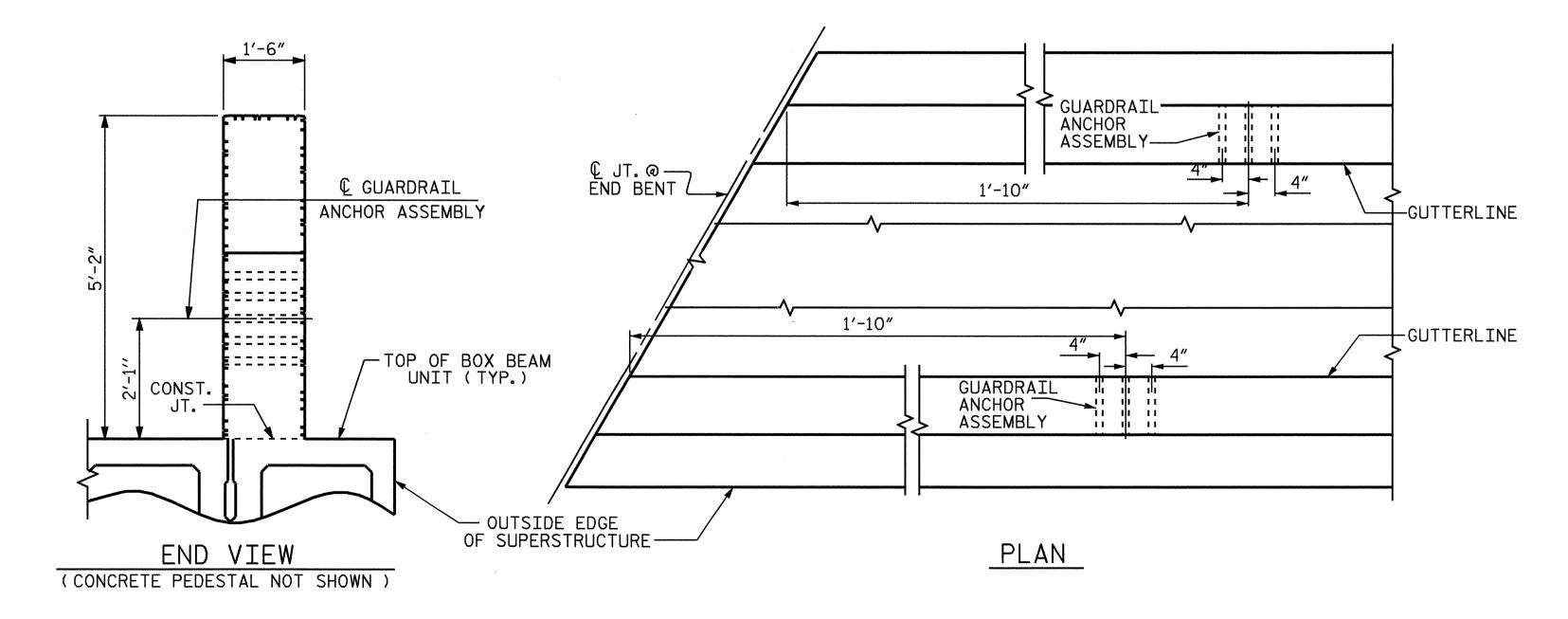
	SHEET NO.					
NO.	BY:	DATE:	NO.	BY:	DATE:	S-11
1			3			TOTAL SHEETS
2			4			22

\_\_ DATE : 05-07 \_\_ DATE : 08-07 DRAWN BY : \_ J.M. BRITT





# GUARDRAIL ANCHOR ASSEMBLY DETAILS



ASSEMBLED BY: N. PIERCE DATE: 05-07 CHECKED BY: J.M. BRITT DATE: 08-07

DRAWN BY: EEM 6/94 REV. 10/17/00 RWW/LES RWW/JTE TLA/GM

LOCATION OF GUARDRAIL ANCHOR AT END POST

NOTES

THE GUARDRAIL ANCHOR ASSEMBLY SHALL CONSIST OF A  $1/4^{\prime\prime}$  HOLD DOWN PLATE AND 7 -  $1/8^{\prime\prime}$  Ø BOLTS WITH NUTS AND WASHERS.

THE HOLD-DOWN PLATE SHALL CONFORM TO AASHTO M270 GRADE 36. AFTER FABRICATION, THE HOLD-DOWN PLATE SHALL BE HOT-DIP GALVANIZED IN ACCORDANCE WITH AASHTO M111.

BOLTS SHALL CONFORM TO THE REQUIREMENTS OF ASTM A307 AND NUTS SHALL CONFORM TO THE REQUIREMENTS OF AASHTO M291. BOLTS, NUTS AND WASHERS SHALL BE GALVANIZED. AT THE CONTRACTOR'S OPTION, STAINLESS STEEL BOLTS, NUTS AND WASHERS MAY BE USED AS AN ALTERNATE FOR THE ½" Ø GALVANIZED BOLTS, NUTS AND WASHERS. THEY SHALL CONFORM TO OR EXCEED THE MECHANICAL REQUIREMENTS OF ASTM A307. THE USE OF THIS ALTERNATE SHALL BE APPROVED BY THE ENGINEER.

AFTER INSTALLATION, THE EXPOSED THREAD OF THE BOLT SHALL BE BURRED WITH A SHARP POINTED TOOL.

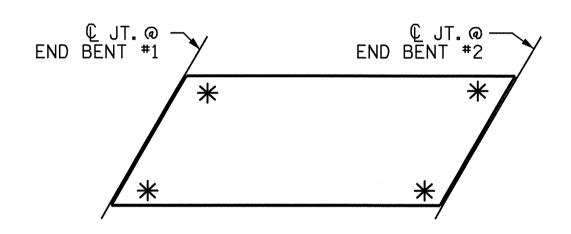
THE COST OF THE GUARDRAIL ANCHOR ASSEMBLIES WITH BOLTS, NUTS AND WASHERS COMPLETE IN PLACE, SHALL BE INCLUDED IN THE VARIOUS PAY ITEMS.

THE VERTICAL REINFORCING BARS MAY BE SHIFTED SLIGHTLY IN THE END POST TO CLEAR ASSEMBLY BOLTS.

THE 1  $\frac{1}{4}$ " Ø HOLES SHALL BE FORMED OR DRILLED WITH A CORE BIT. IMPACT TOOLS WILL NOT BE PERMITTED. ANY CONCRETE DAMAGED BY THIS WORK SHALL BE REPAIRED TO THE SATISFACTION OF THE ENGINEER.

NO ARCHITECTURAL CONCRETE SURFACE TREATMENT (FORM LINERS ) SHALL BE USED IN THE AREAS OF GUARDRAIL CONNECTION & HOLD-DOWN P. THESE AREAS SHALL BE SMOOTH AND FLUSH WITH OUTSIDE OF PARAPET.

AFTER ATTACHMENT OF THE GUARDRAIL, ALL EXPOSED PORTIONS OF THE GUARDRAIL ANCHOR ASSEMBLY SHALL BE COATED WITH TWO COATS OF THE TOPCOAT PAINT USED TO PAINT THE GUARDRAIL.



SKETCH SHOWING POINTS OF ATTACHMENT

\*LOCATION OF GUARDRAIL ATTACHMENT

DEPARTMENT OF TRANSPORTATION

STANDARD

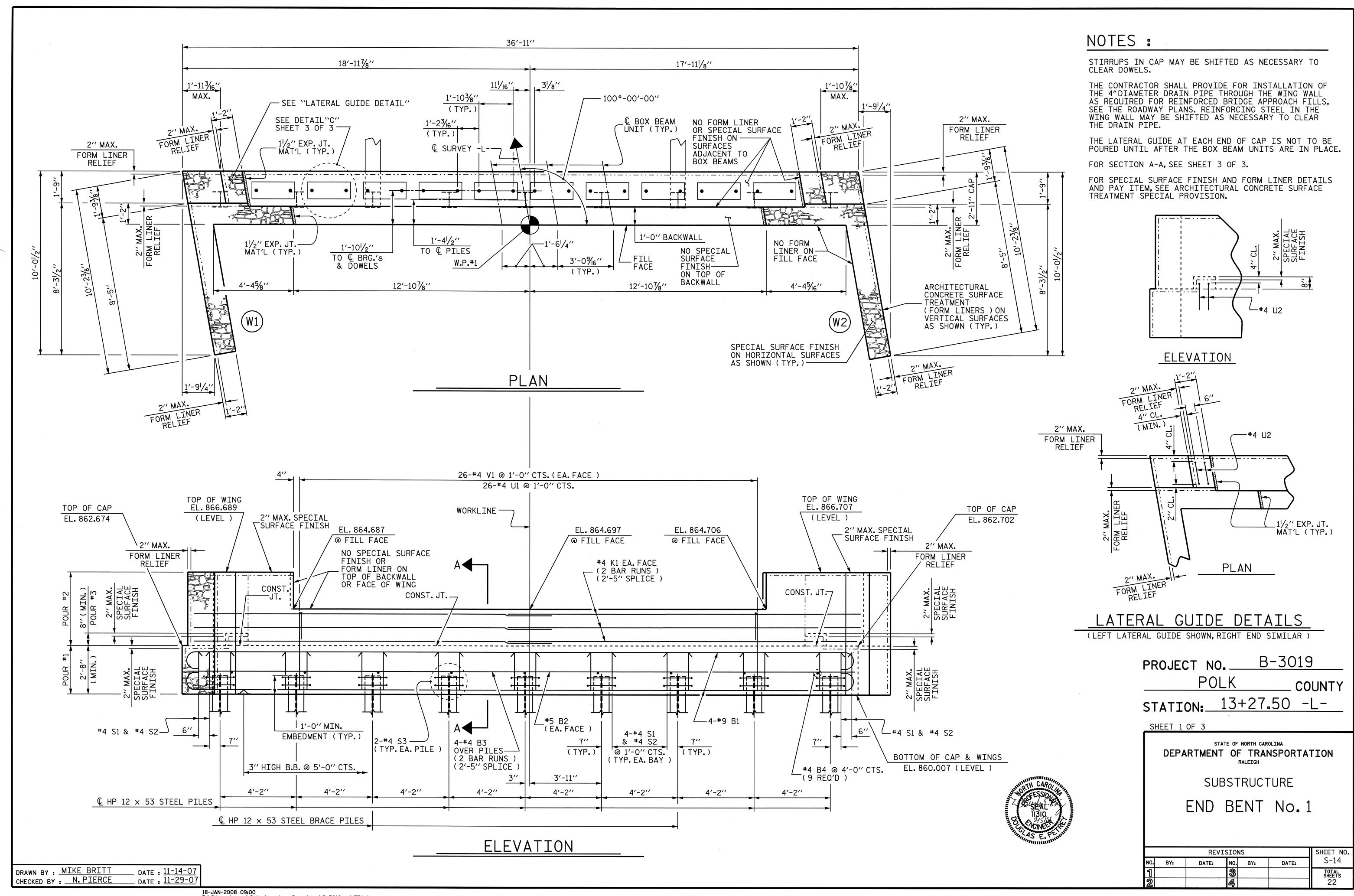
GUARDRAIL ANCHORAGE

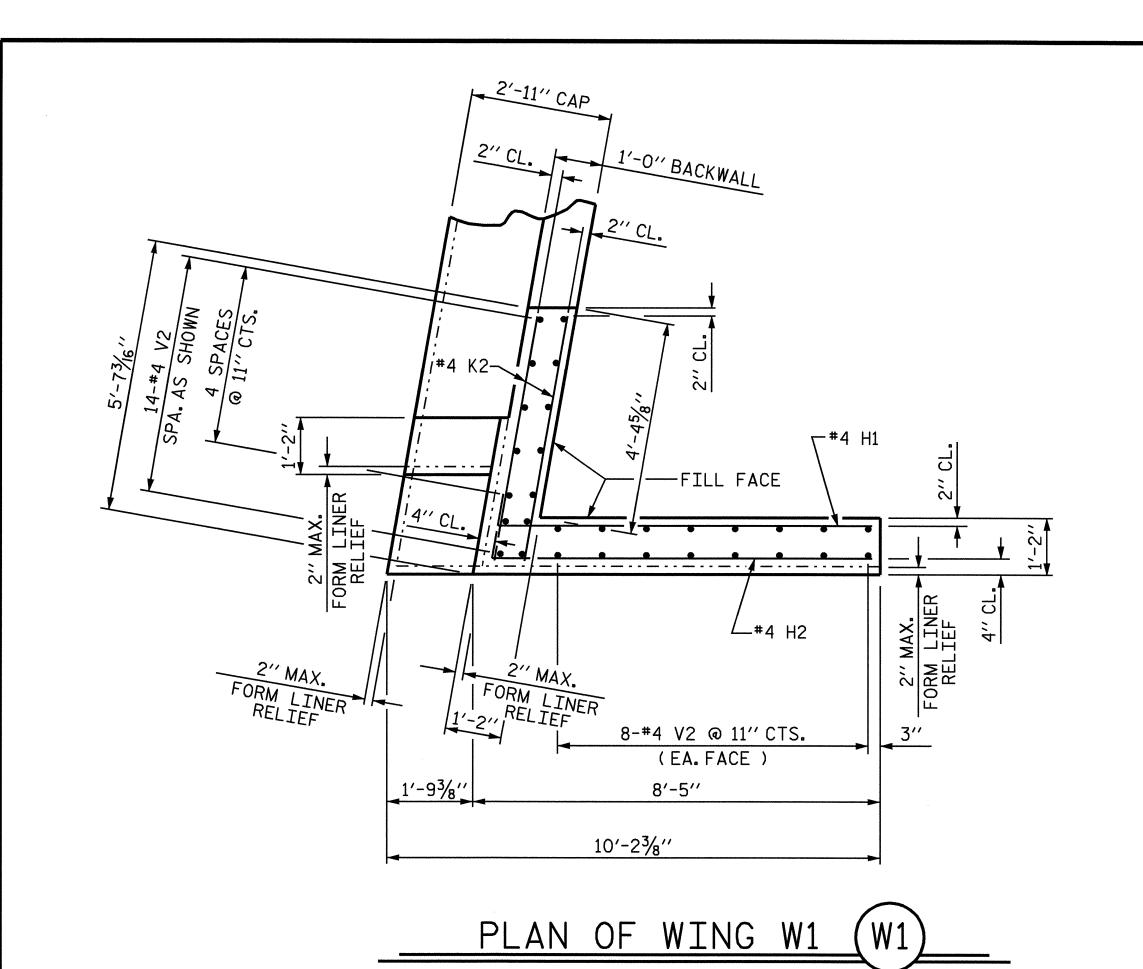
DETAILS

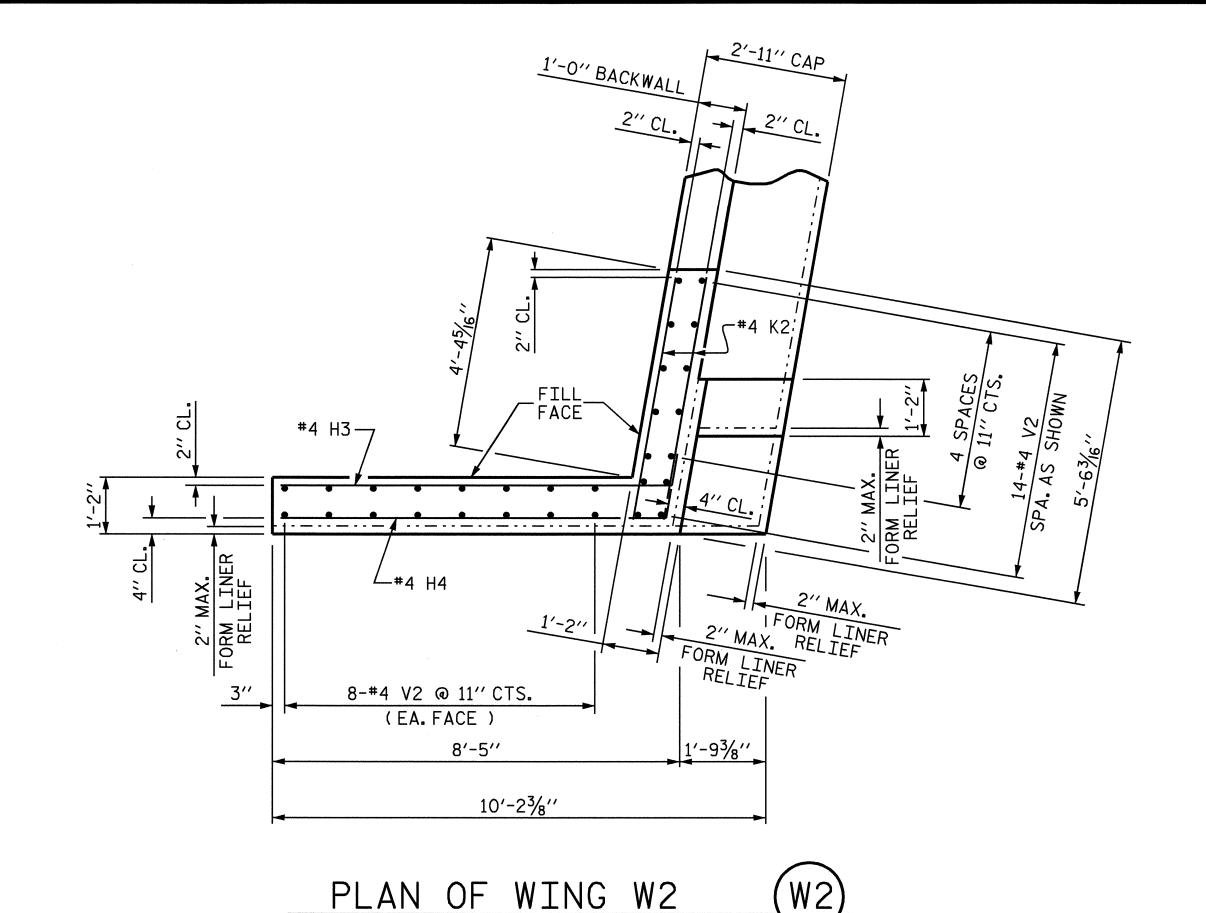
FOR METAL RAILS

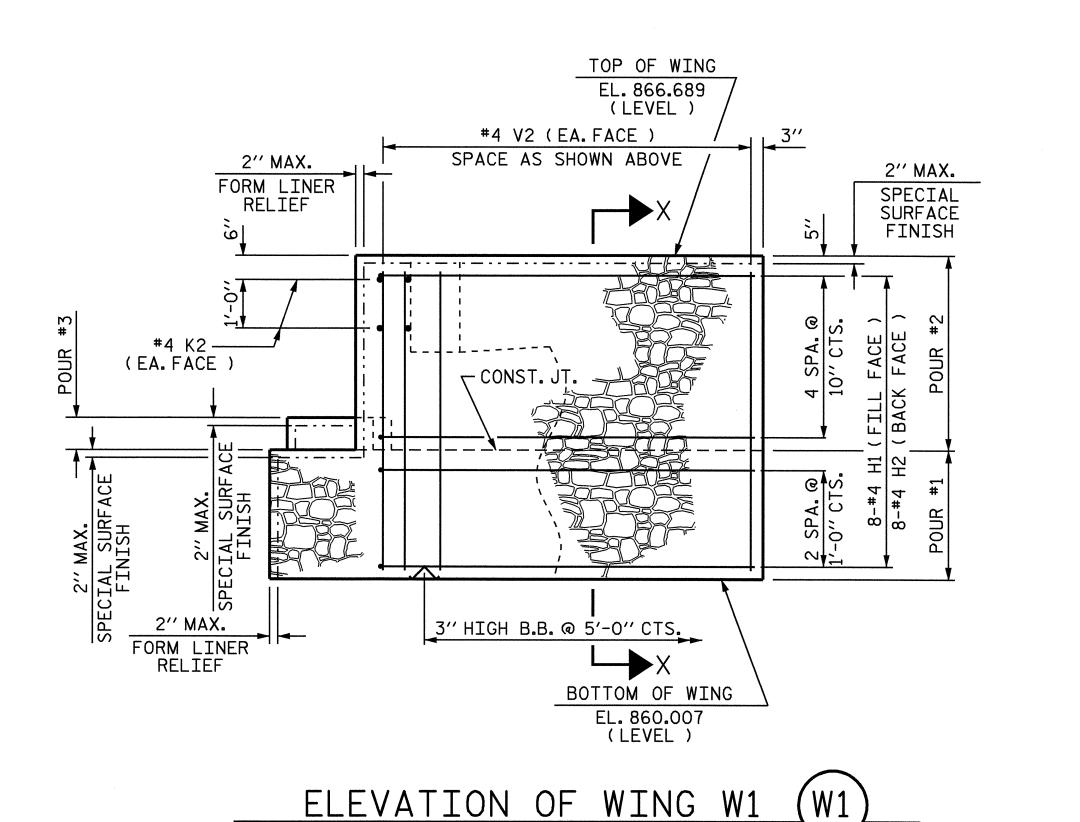


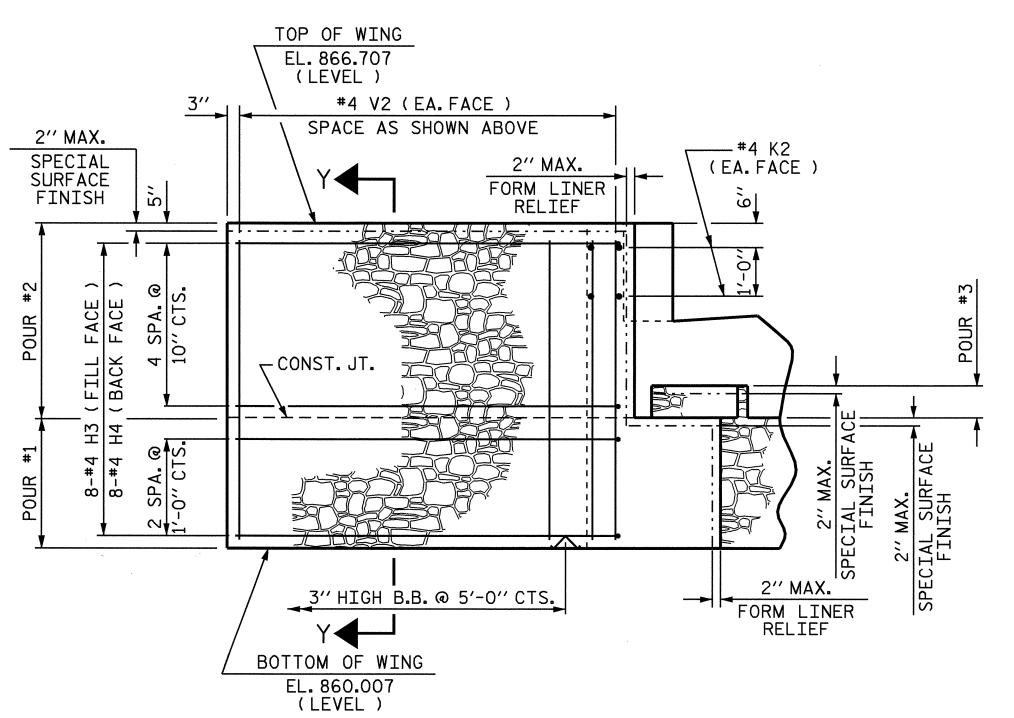
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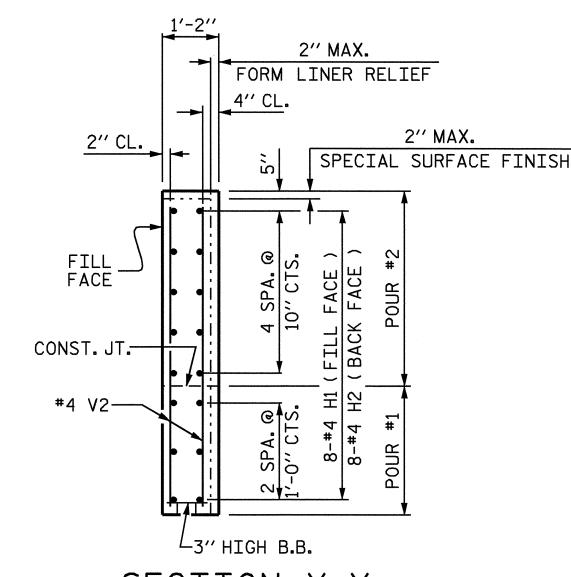




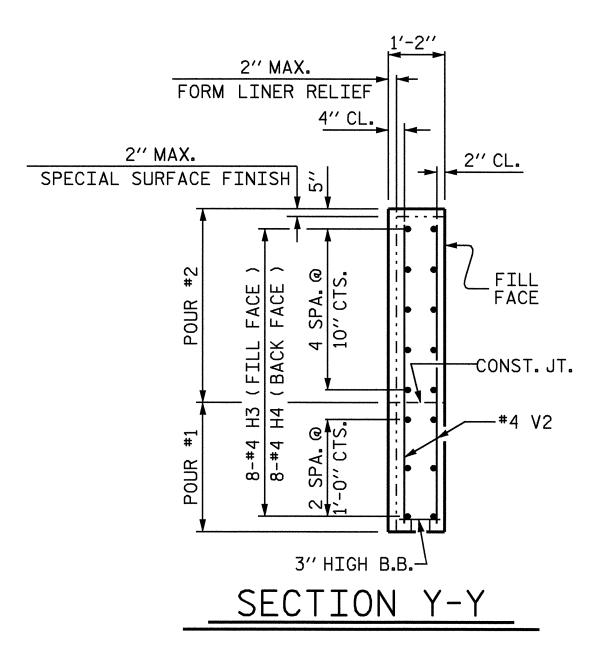


BOTTOM OF WING
EL. 860.007
(LEVEL)

ELEVATION OF WING W2



SECTION X-X



B-3019 PROJECT NO. \_\_\_ POLK COUNTY STATION: 13+27.50 -L-

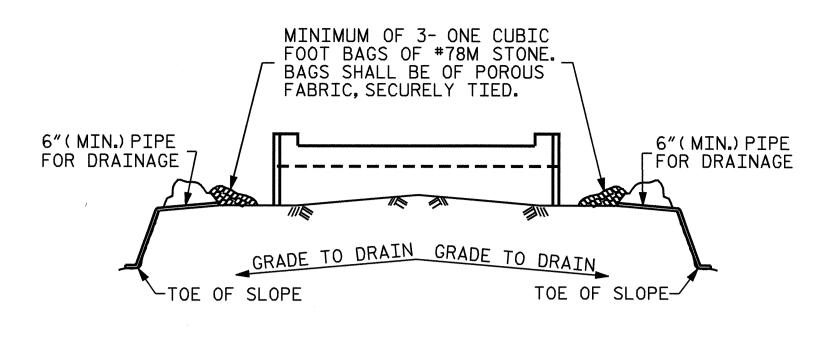
SHEET 2 OF 3

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION SUBSTRUCTURE

END BENT No. 1

REVISIONS SHEET NO. S-15 NO. BY: TOTAL SHEETS 22

DRAWN BY : MIKE BRITT DRAWN BY: MIKE BRITT DATE: 11-15-07
CHECKED BY: N. PIERCE DATE: 11-29-07

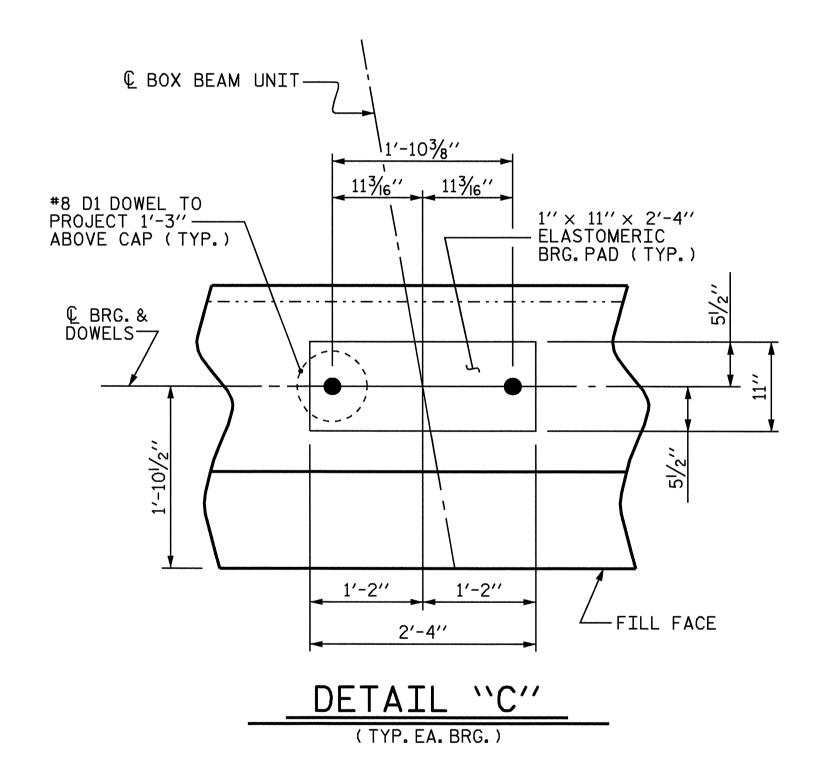


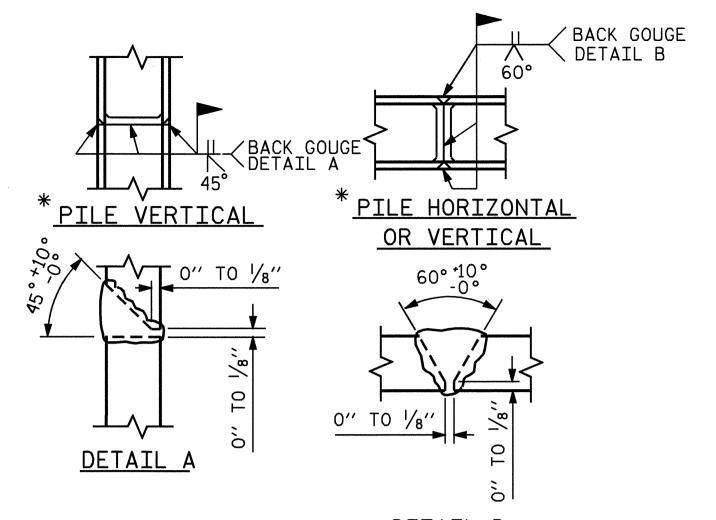
BAGGED STONE AND PIPE SHALL BE PLACED IMMEDIATELY AFTER COMPLETION OF END BENT EXCAVATION. PIPE MAY BE EITHER CONCRETE, CORRUGATED STEEL, CORRUGATED ALUMINUM ALLOY, OR CORRUGATED PLASTIC. PERFORATED PIPE WILL NOT BE ALLOWED.

BAGGED STONE SHALL REMAIN IN PLACE UNTIL THE ENGINEER DIRECTS THAT IT BE REMOVED. THE CONTRACTOR SHALL REMOVE AND DISPOSE OF SILT ACCUMULATIONS AT BAGGED STONE WHEN SO DIRECTED BY THE ENGINEER. BAGS SHALL BE REMOVED AND REPLACED WHENEVER THE ENGINEER DETER-MINES THAT THEY HAVE DETERIORATED AND LOST THEIR EFFECTIVENESS.

NO SEPARATE PAYMENT WILL BE MADE FOR THIS WORK AND THE ENTIRE COST OF THIS WORK SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR THE SEVERAL PAY ITEMS.

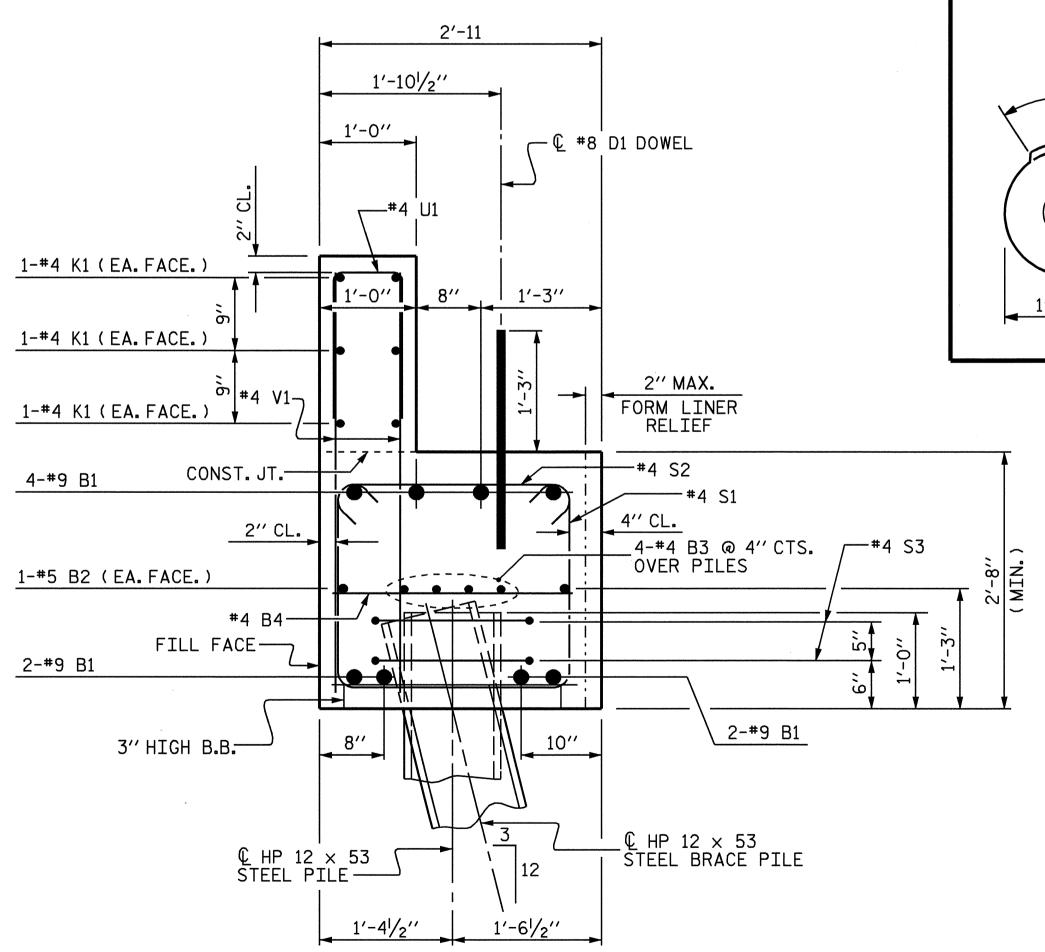
# TEMPORARY DRAINAGE AT END BENT

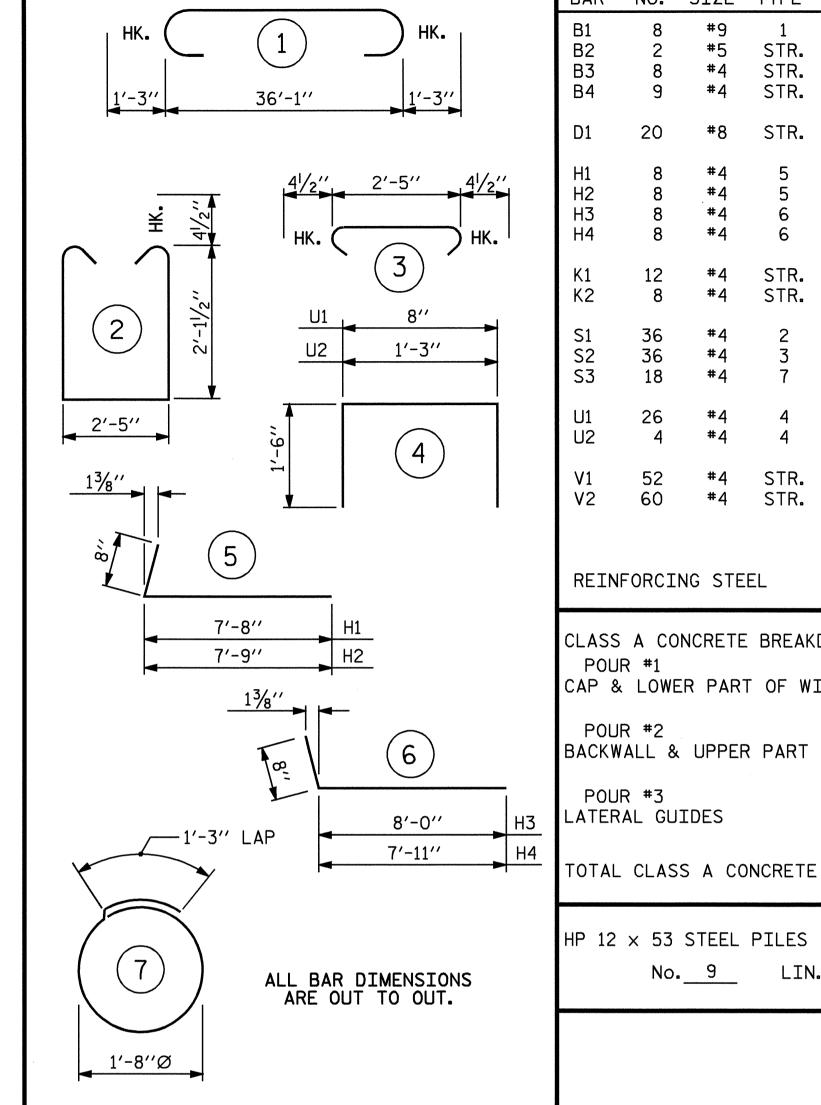




\* POSITION OF PILE DURING WELDING.

PILE SPLICE DETAILS





BAR TYPES

END BENT No. 1 BAR No. SIZE TYPE LENGTH WEIGHT #5 STR. 36′-3′′ STR. 19'-4'' 103 2'-5'' B4 STR. 2'-3'' 120 8'-5" Н3 8'-8" 19'-4'' 155 K2 STR. 5'-0" S1 S2 3'-2" **S**3 6'-6'' 78 4'-3'' STR. 247 REINFORCING STEEL 2,531 LBS. CLASS A CONCRETE BREAKDOWN POUR #1 CAP & LOWER PART OF WINGS = 12.4 C.Y.POUR #2 BACKWALL & UPPER PART OF WINGS = 6.2 C.Y.POUR #3 LATERAL GUIDES = 0.1 C.Y.

No. 9 LIN.FT. 180

= 18.7 C.Y.

— BILL OF MATERIAL ——

B-3019 PROJECT NO. \_ POLK COUNTY STATION: 13+27.50 -L-

SHEET 3 OF 3

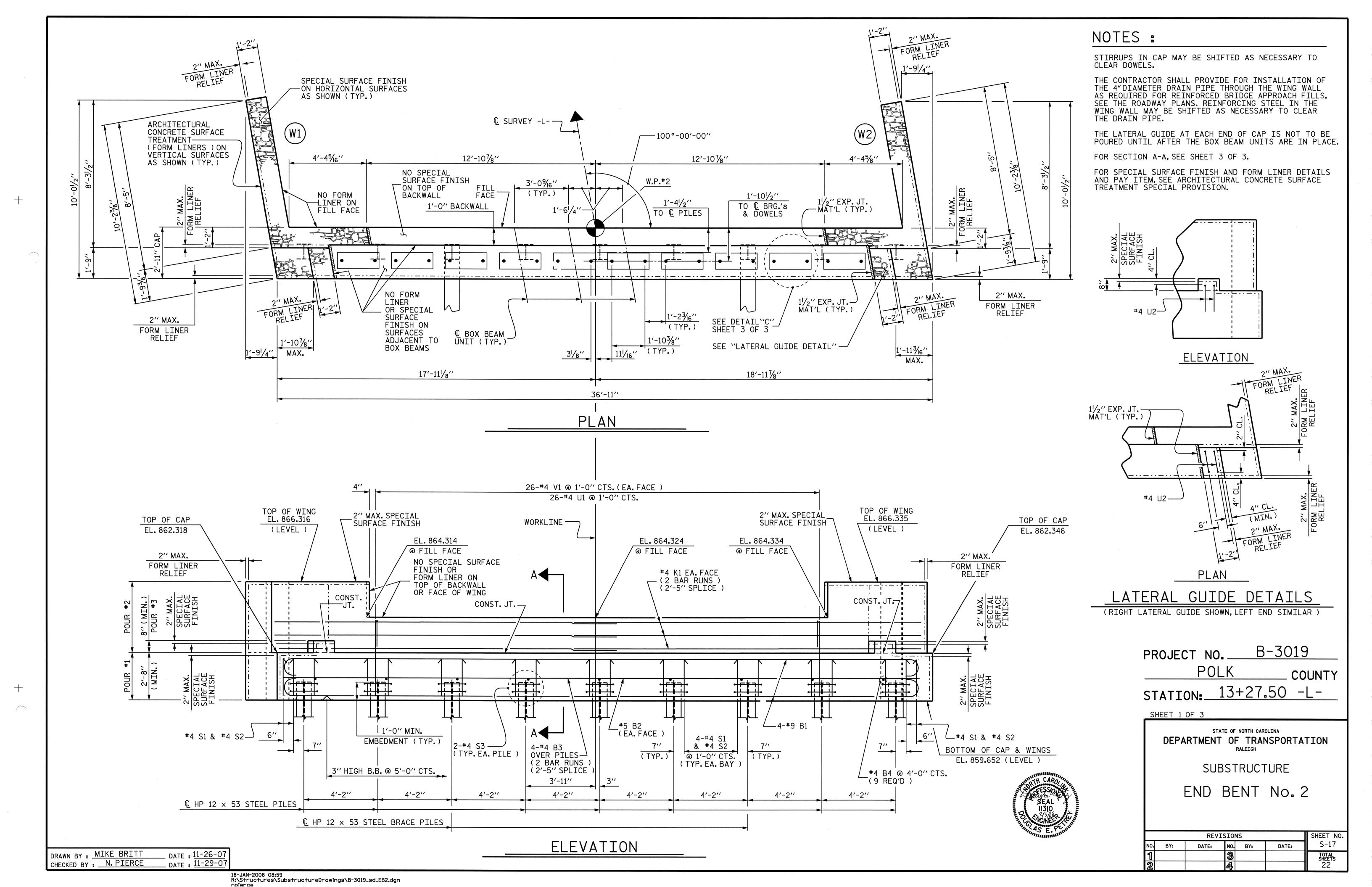
STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

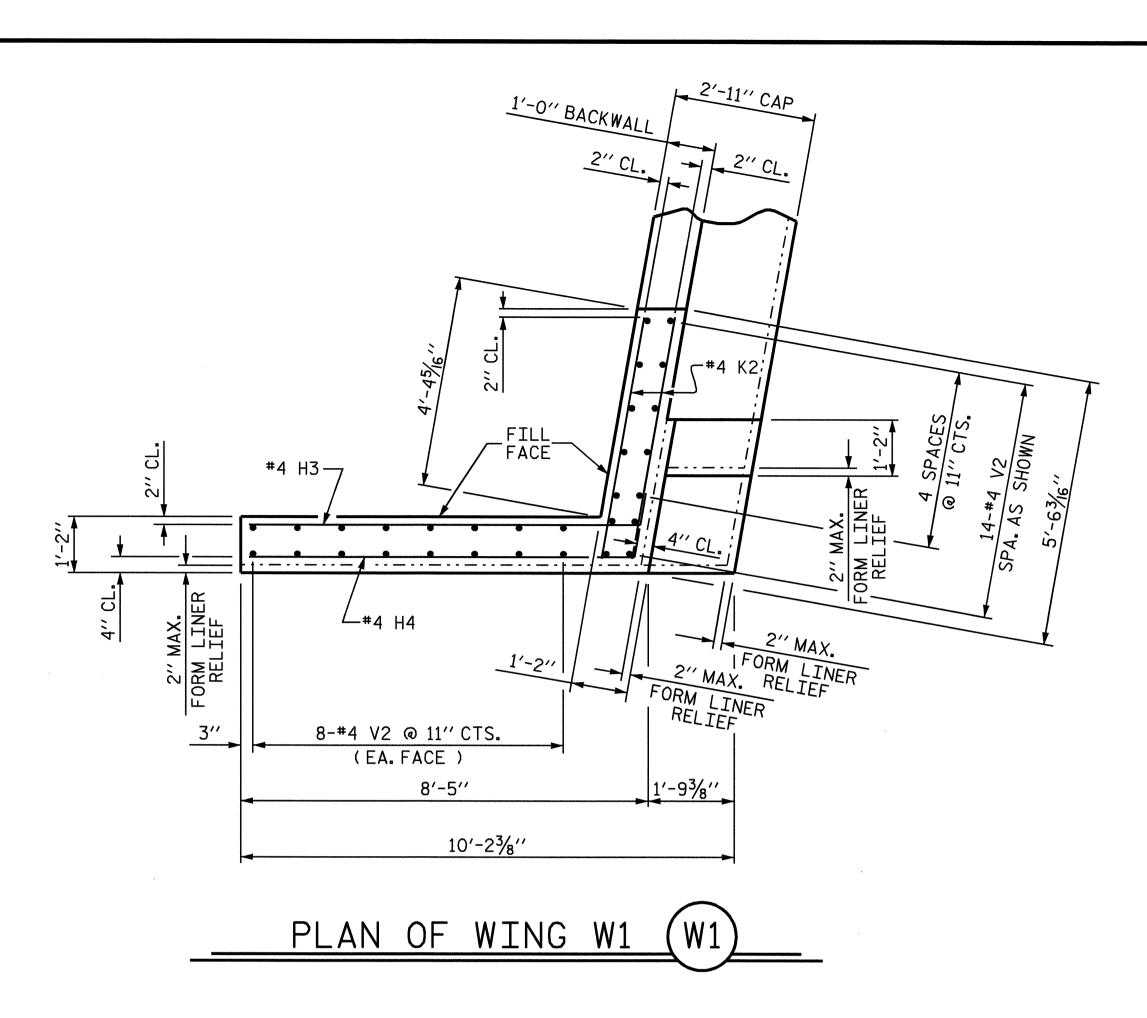
SUBSTRUCTURE

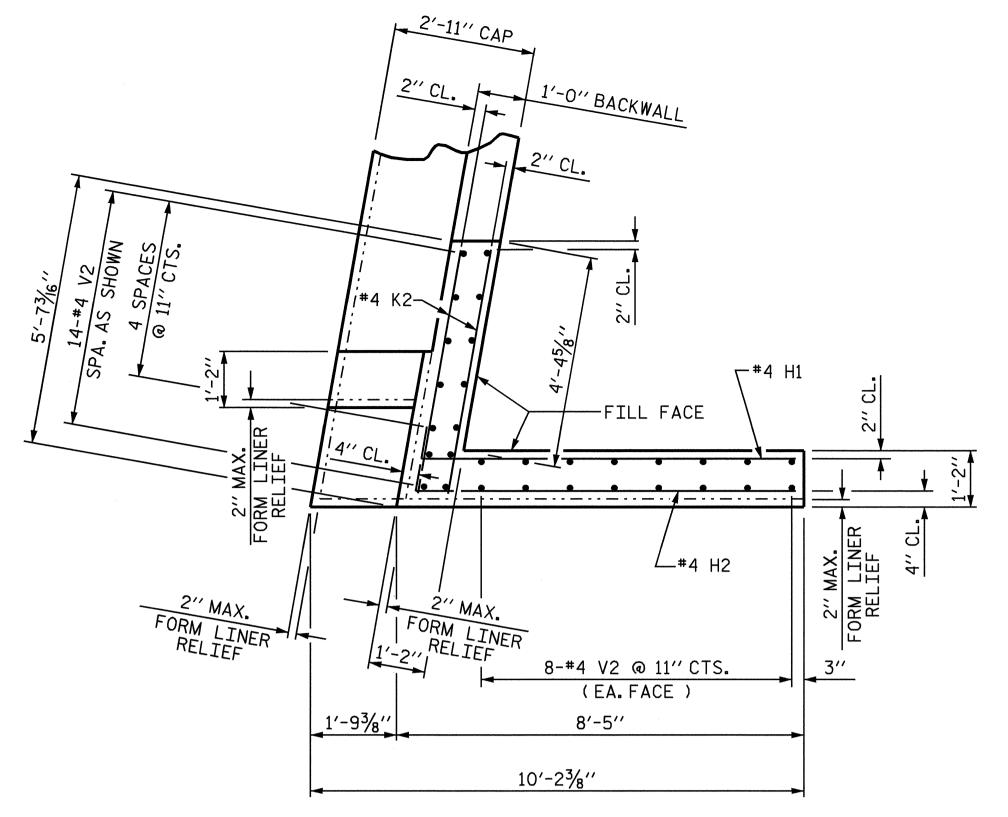
END BENT No. 1

SHEET NO REVISIONS S-16 NO. BY: DATE:

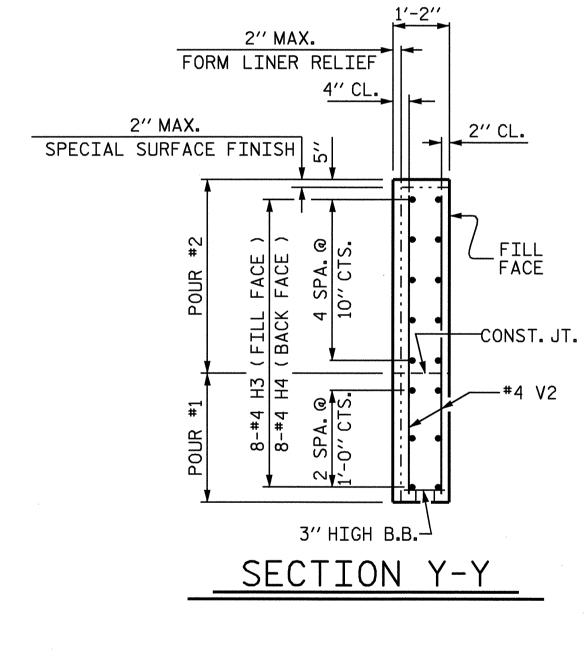
SECTION A-A







PLAN OF WING W2



TOP OF WING

EL. 866.335
(LEVEL)

\*4 V2 (EA. FACE )

SPACE AS SHOWN ABOVE

FORM LINER

RELIEF

CONST. JT.

CONST. JT.

SPECIAL
SURFACE
FINISH

A SUPERACE
FINISH

CONST. JT.

SPECIAL
SURFACE
FINISH

A SUPERACE
FINISH

A SUPERACE
FINISH

SPECIAL
SURFACE
FINISH

A SUPERACE
FINISH

BOTTOM OF WING

EL. 869.652
(LEVEL)

A SPORT AS SHOWN ABOVE

SPECIAL
SURFACE
FINISH

A SPACE AS SHOWN ABOVE

SPECIAL
SURFACE
FINISH

A SPACE AS SHOWN ABOVE

EL. 866.335

CLEVEL

SPECIAL
SURFACE
FINISH

A SPACE AS SHOWN ABOVE

SPECIAL
SURFACE
FINISH

A SPACE AS SHOWN ABOVE

SPECIAL
SURFACE
FINISH

A SPACE AS SHOWN ABOVE

CLEVEL

SPECIAL
SURFACE
FINISH

A SPACE AS SHOWN ABOVE

CLEVEL

SPACE AS SHOWN ABOVE

CLEVEL AS

ELEVATION OF WING W2 (W2)

•

PROJECT NO. B-3019
POLK COUNTY
STATION: 13+27.50 -L-

2" MAX.
FORM LINER RELIEF

2" MAX.

SPECIAL SURFACE FINISH

4" CL.

L<sub>3"</sub> HIGH B.B.

SECTION X-X

2" CL.

FILL FACE

CONST. JT.

#4 V2-

SHEET 2 OF 3

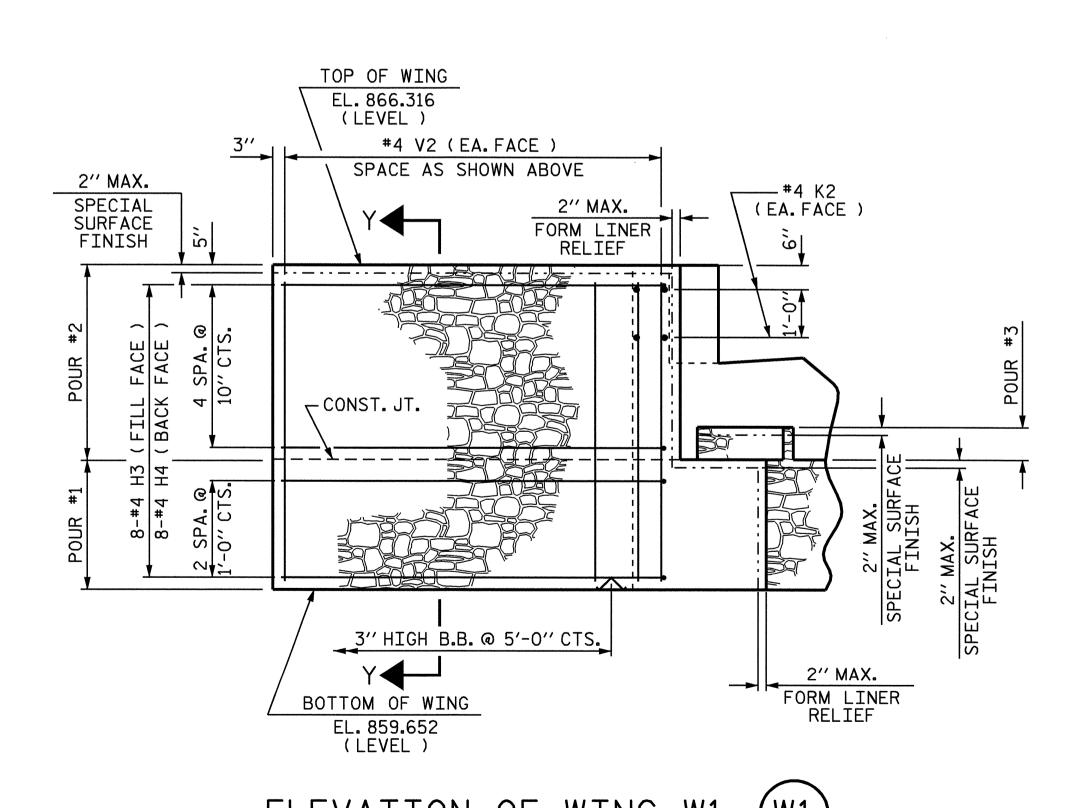
STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION
RALEIGH

SUBSTRUCTURE

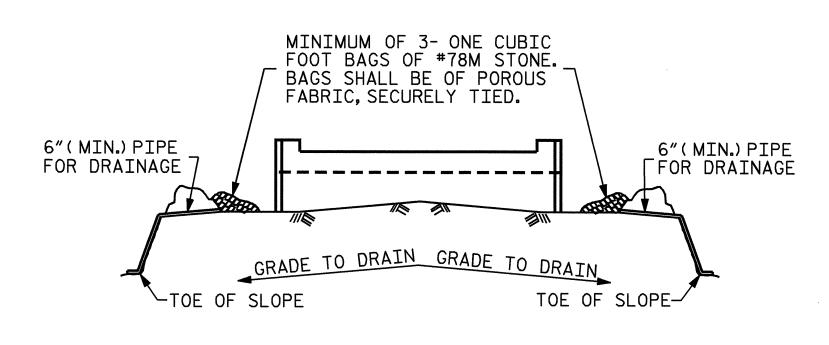
END BENT No. 2

		SHEET NO.				
).	BY:	DATE:	NO.	BY:	DATE:	S-18
			3			TOTAL SHEETS
)			4			22



ELEVATION OF WING W1 (W1)

DRAWN BY: MIKE BRITT DATE: 11-26-07
CHECKED BY: N. PIERCE DATE: 11-29-07

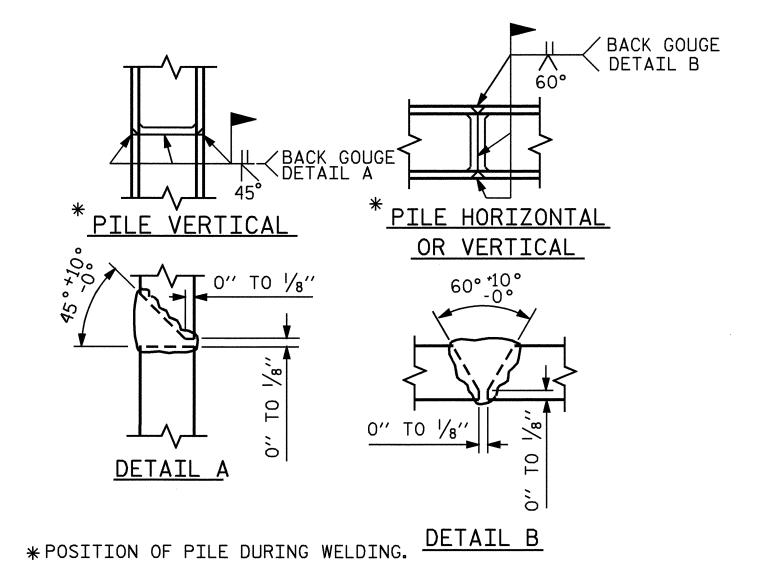


BAGGED STONE AND PIPE SHALL BE PLACED IMMEDIATELY AFTER COMPLETION OF END BENT EXCAVATION. PIPE MAY BE EITHER CONCRETE, CORRUGATED STEEL, CORRUGATED ALUMINUM ALLOY, OR CORRUGATED PLASTIC. PERFORATED PIPE WILL NOT BE ALLOWED.

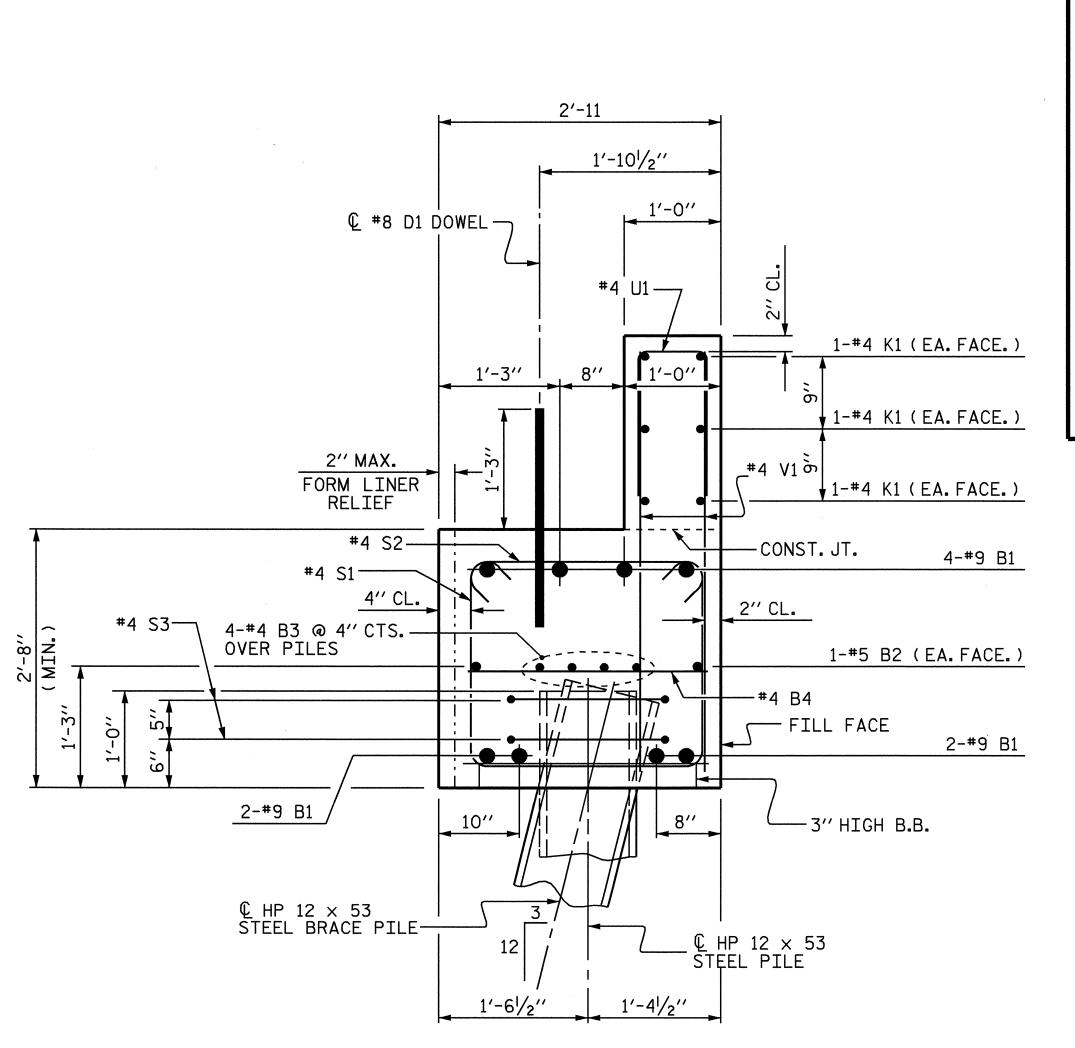
BAGGED STONE SHALL REMAIN IN PLACE UNTIL THE ENGINEER DIRECTS THAT IT BE REMOVED. THE CONTRACTOR SHALL REMOVE AND DISPOSE OF SILT ACCUMULATIONS AT BAGGED STONE WHEN SO DIRECTED BY THE ENGINEER. BAGS SHALL BE REMOVED AND REPLACED WHENEVER THE ENGINEER DETER-MINES THAT THEY HAVE DETERIORATED AND LOST THEIR EFFECTIVENESS.

NO SEPARATE PAYMENT WILL BE MADE FOR THIS WORK AND THE ENTIRE COST OF THIS WORK SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR THE SEVERAL PAY ITEMS.

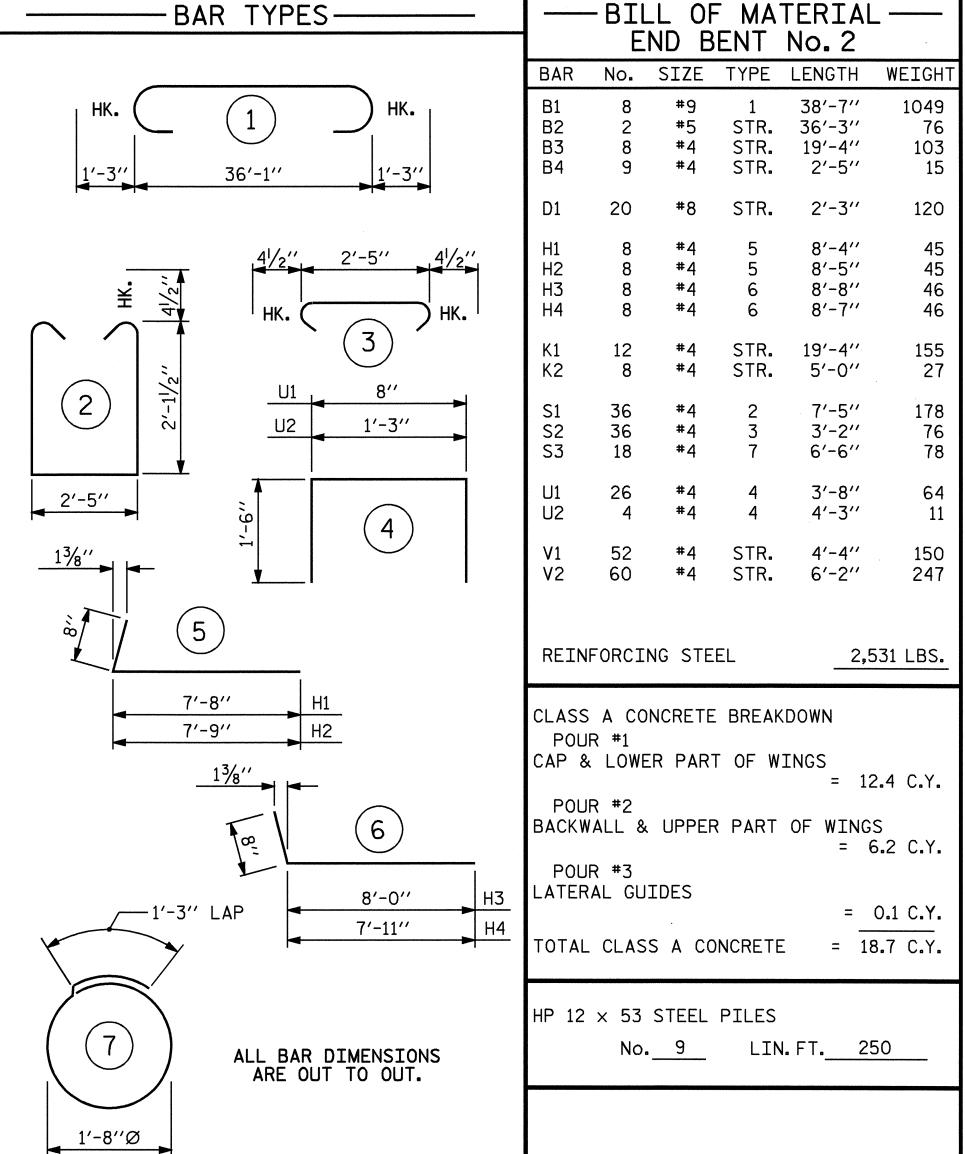
# TEMPORARY DRAINAGE AT END BENT



PILE SPLICE DETAILS



SECTION A-A



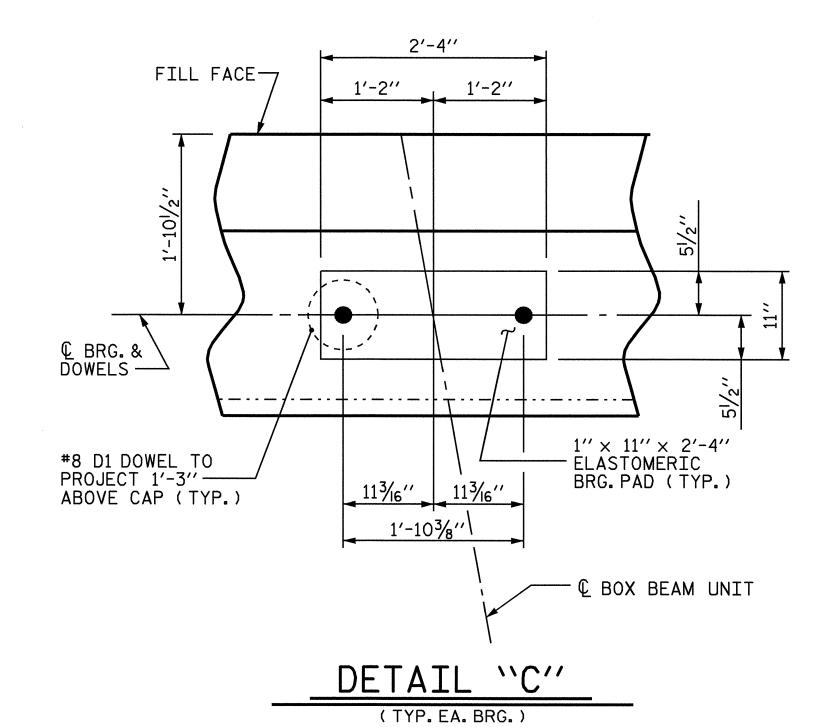
B-3019 PROJECT NO. \_\_ POLK COUNTY 13+27.50 -L-STATION:\_

SHEET 3 OF 3

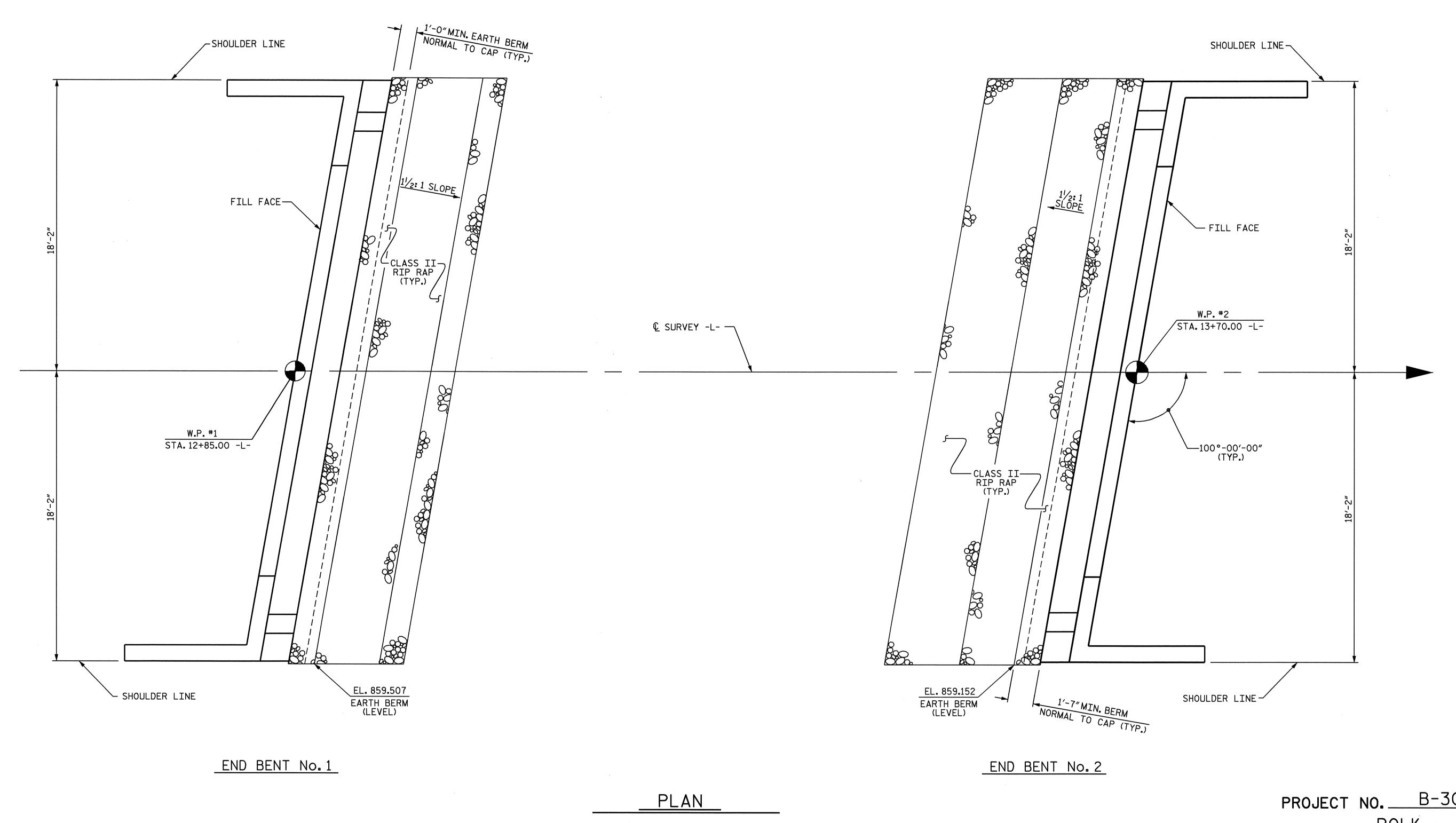
STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

> SUBSTRUCTURE END BENT No. 2

SHEET NO. REVISIONS S-19 DATE: NO. BY:



\_ DATE : <u>11-26-07</u> \_ DATE : <u>11-29-07</u> DRAWN BY : MIKE BRITT CHECKED BY : N. PIERCE

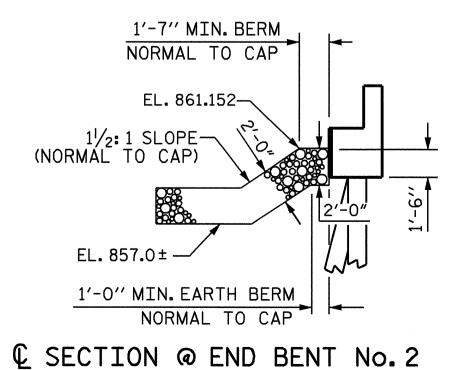


ESTIMATED	QUANTITIES
BRIDGE @ STA.13+27.50 -L-	RIP RAP CLASS II
	TONS
END BENT 1	35
END BENT 2	40

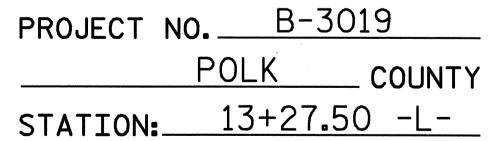
1'-7" MIN. BERM NORMAL TO CAP — EL. 861.507 -11/2:1 SLOPE (NORMAL TO CAP) ► EL. 857.0± 1'-0" MIN. EARTH BERM NORMAL TO CAP

BERM RIP RAPPING

NOTE: CONTRACTOR SHALL PROVIDE STONE FOR RIP RAP IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS AND SHALL PROVIDE RIP RAP WITH COLORATION TO BLEND WITH THE ARCHITECTURAL CONCRETE SURFACE TREATMENT COLORATION USED ON THE VARIOUS STRUCTURE MEMBERS AS APPROVED BY THE ENGINEER.



BERM RIP RAPPING



STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

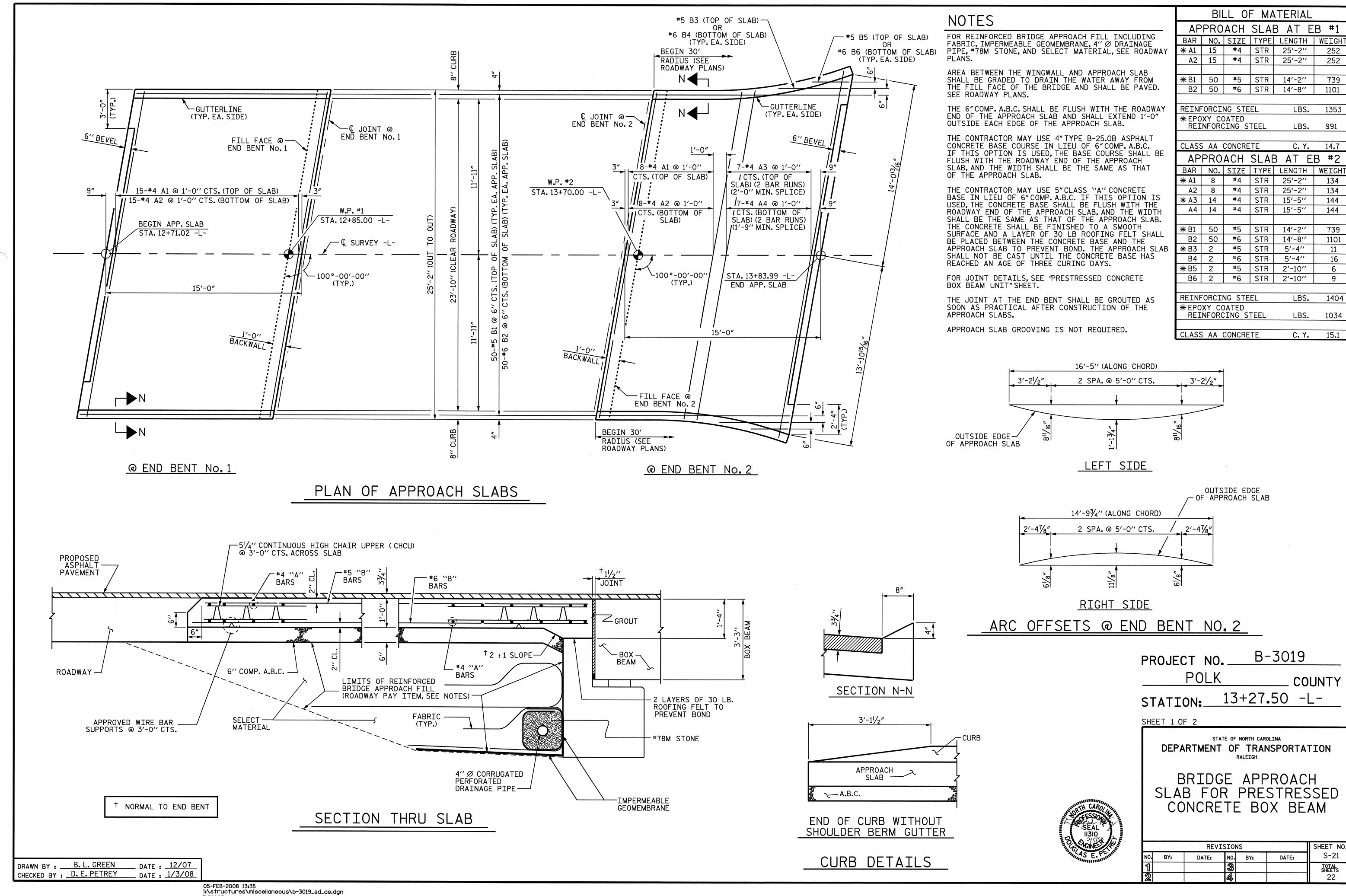
RIP RAP DETAILS

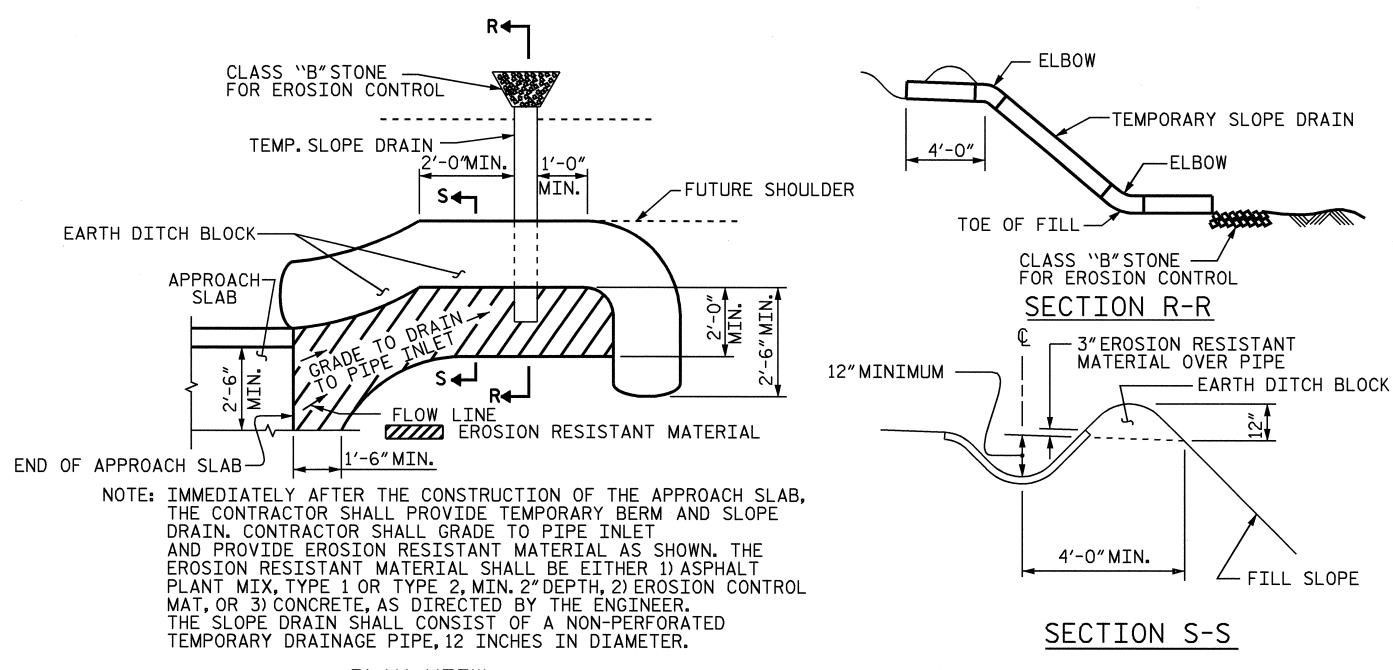


		SHEET NO.				
NO.	BY:	DATE:	NO.	BY:	DATE:	S-20
1			3			TOTAL SHEETS
2			4			22

DRAWN BY: A. V. ROYAL
CHECKED BY: D. E. PETREY DATE: 12/07
DATE: 12/07

18-JAN-2008 08:58
R:\Structures\Miscellaneous\B3019\_sd\_RR.dgn
npierce

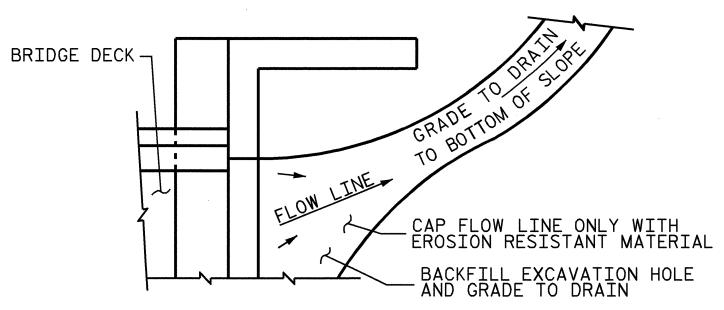




PLAN VIEW

# TEMPORARY BERM AND SLOPE DRAIN DETAILS

(TO BE USED WHEN SHOULDER BERM GUTTER IS REQUIRED)



NOTE: IF THE APPROACH SLAB IS NOT CONSTRUCTED IMMEDIATELY AFTER THE BACKFILLING OF THE END BENT EXCAVATION, GRADE TO DRAIN TO THE BOTTOM OF THE SLOPE AND PROVIDE EROSION RESISTANT MATERIAL, SUCH AS FIBERGLASS ROVING OR AS DIRECTED BY THE ENGINEER TO PREVENT SOIL EROSION AND TO PROTECT THE AREA ADJACENT TO THE STRUCTURE. THE CONTRACTOR WILL BE REQUIRED TO REMOVE THESE MATERIALS PRIOR TO CONSTRUCTION OF THE APPROACH SLAB.

TEMPORARY DRAINAGE DETAIL

PROJECT NO. B-3019
POLK COUNTY
STATION: 13+27.50 -L-

SHEET 2 OF 2

STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION

RALEIGH

BRIDGE APPROACH SLAB DETAILS

REVISIONS

BY: DATE: NO. BY: DATE: S-22

3 TOTAL SHEETS
22

SEAL HISTORY CAROLAND SEAL HISTORY CAROLAND

ASSEMBLED BY: B.L.GREEN DATE: 12/07 CHECKED BY: D.E.PETREY DATE: 1/3/08

DRAWN BY: FCJ II/88 REV. IO/IT/00 RWW/LES RWW/JTE REV. 5/7/03 REV. 5/1/06R

# STANDARD NOTES

### DESIGN DATA:

CONCRETE IN SHEAR

STRUCTURAL TIMBER - TREATED OR

UNTREATED - EXTREME FIBER STRESS ---- 1,800 LBS. PER SQ. IN.

---- SEE A.A.S.H.T.O.

and the same of the same

COMPRESSION PERPENDICULAR TO GRAIN

OF TIMBER ----

375 LBS. PER SQ. IN.

EQUIVALENT FLUID PRESSURE OF EARTH

30 LBS. PER CU. FT.

(MINIMUM)

### MATERIAL AND WORKMANSHIP:

EXCEPT AS MAY OTHERWISE BE SPECIFIED ON PLANS OR IN THE SPECIAL PROVISIONS, ALL MATERIAL AND WORKMANSHIP SHALL BE IN ACCORDANCE WITH THE 2002 STANDARD SPECIFICATIONS "FOR ROADS AND STRUCTURES" OF THE N. C. DEPARTMENT OF TRANSPORTATION.

STEEL SHEET PILING FOR PERMANENT OR TEMPORARY APPLICATIONS SHALL BE HOT ROLLED.

### CONCRETE:

UNLESS OTHERWISE REQUIRED ON PLANS, CLASS A CONCRETE SHALL BE USED FOR ALL PORTIONS OF ALL STRUCTURES WITH THE EXCEPTION THAT: CLASS AA CONCRETE SHALL BE USED IN BRIDGE SUPERSTRUCTURES, ABUTMENT BACKWALLS, AND APPROACH SLABS; CLASS B CONCRETE SHALL BE USED FOR SLOPE PROTECTION AND RIP RAP; AND CLASS S SHALL BE USED FOR UNDERWATER FOOTING SEALS.

### CONCRETE CHAMFERS:

UNLESS OTHERWISE NOTED ON THE PLANS, ALL EXPOSED CORNERS ON STRUCTURES SHALL BE CHAMFERED 3/4" WITH THE FOLLOWING EXCEPTIONS: TOP CORNERS OF CURBS MAY BE ROUNDED TO 1-1/2" RADIUS WHICH IS BUILT INTO CURB FORMS; CORNERS OF TRANSVERSE FLOOR EXPANSION JOINTS SHALL BE ROUNDED WITH A 1/4" FINISHING TOOL UNLESS OTHERWISE REQUIRED ON PLANS; AND CORNERS OF EXPANSION JOINTS IN THE ROADWAY FACES AND TOPS OF CURBS AND SIDEWALKS SHALL BE ROUNDED TO A 1/4" RADIUS WITH A FINISHING STONE OR TOOL UNLESS OTHERWISE REQUIRED ON PLANS.

### DOWELS:

DOWELS WHEN INDICATED ON PLANS AS FOR CULVERT EXTENSIONS, SHALL BE EMBEDDED AT LEAST 12" INTO THE OLD CONCRETE AND GROUTED INTO PLACE WITH 1:2 CEMENT MORTAR.

# ALLOWANCE FOR DEAD LOAD DEFLECTION, SETTLEMENT, ETC. IN CASTING SUPERSTRUCTURES:

BRIDGES SHALL BE BUILT ON THE GRADE OR VERTICAL CURVE SHOWN ON PLANS. SLABS, CURBS AND PARAPETS SHALL CONFORM TO THE GRADE OR CURVE.

ALL DIMENSIONS WHICH ARE GIVEN IN SECTION AND ARE AFFECTED BY DEAD LOAD DEFLECTIONS ARE DIMENSIONS AT CENTER LINE OF BEARING UNLESS OTHERWISE NOTED ON PLANS. IN SETTING FORMS FOR STEEL BEAM BRIDGES AND PRESTRESSED CONCRETE GIRDER BRIDGES, ADJUSTMENTS SHALL BE MADE DUE TO THE DEAD LOAD DEFLECTIONS FOR THE ELEVATIONS SHOWN. WHERE BLOCKS ARE SHOWN OVER BEAMS FOR BUILDING UP TO THE SLAB, THE VERTICAL DIMENSIONS OF THE BLOCKS SHALL BE ADJUSTED BETWEEN BEARINGS TO COMPENSATE FOR DEAD LOAD DEFLECTIONS, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER. WHERE BOTTOM OF SLAB IS IN LINE WITH BOTTOM OF TOP FLANGES, DEPTH OF SLAB BETWEEN BEARINGS SHALL BE ADJUSTED TO COMPENSATE FOR DEAD LOAD DEFLECTION, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER.

IN SETTING FALSEWORK AND FORMS FOR REINFORCED CONCRETE SPANS, AN ALLOWANCE SHALL BE MADE FOR DEAD LOAD DEFLECTIONS, SETTLEMENT OF FALSEWORK, AND PERMANENT CAMBER WHICH SHALL BE PROVIDED FOR IN ADDITION TO THE ELEVATIONS SHOWN. AFTER REMOVAL OF THE FALSEWORK, THE FINISHED STRUCTURES SHALL CONFORM TO THE PROFILE AND ELEVATIONS SHOWN ON THE PLANS AND CONSTRUCTION ELEVATIONS FURNISHED BY THE ENGINEER.

DETAILED DRAWINGS FOR FALSEWORK OR FORMS FOR BRIDGE SUPERSTRUCTURE AND ANY STRUCTURE OR PARTS OF A STRUCTURE AS NOTED ON THE PLANS SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL BEFORE CONSTRUCTION OF THE FALSEWORK OR FORMS IS STARTED.

### REINFORCING STEEL:

ALL REINFORCING STEEL SHALL BE DEFORMED WITH THE EXCEPTION OF #2
BARS WHICH MAY BE FABRICATED FROM COLD DRAWN STEEL WIRE. DIMENSIONS
RELATIVE TO PLACEMENT OF REINFORCING ARE TO CENTERS OF BARS UNLESS OTHERWISE
INDICATED IN THE PLANS. DIMENSIONS ON BAR DETAILS ARE TO CENTERS OF BARS
OR ARE OUT TO OUT AS INDICATED ON PLANS.

WIRE BAR SUPPORTS SHALL BE PROVIDED FOR REINFORCING STEEL WHERE INDICATED ON THE PLANS. WHEN BAR SUPPORT PIECES ARE PLACED IN CONTINUOUS LINES, THEY SHALL BE SO PLACED THAT THE ENDS OF THE SUPPORTING WIRES SHALL BE LAPPED TO LOCK LEGS ON ADJOINING PIECES.

### STRUCTURAL STEEL:

AT THE CONTRACTOR'S OPTION, HE MAY SUBSTITUTE 7/8" Ø SHEAR STUDS FOR THE 3/4" Ø STUDS SPECIFIED ON THE PLANS. THIS SUBSTITUTION SHALL BE MADE AT THE RATE OF 3 - 7/8" Ø STUDS FOR 4 - 3/4" Ø STUDS, AND STUD SPACING CHANGES SHALL BE MADE AS NECESSARY TO PROVIDE THE SAME EQUIVALENT NUMBER OF 7/8" Ø STUDS ALONG THE BEAM AS SHOWN FOR 3/4" Ø STUDS BASED ON THE RATIO OF 3 - 7/8" Ø STUDS FOR 4 - 3/4" Ø STUDS. STUDS OF THE LENGTH SPECIFIED ON THE PLANS MUST BE PROVIDED. THE MAXIMUM SPACING SHALL BE 2'-0".

EXCEPT AT THE INTERIOR SUPPORTS OF CONTINUOUS BEAMS WHERE THE COVER PLATE IS IN CONTACT WITH BEARING PLATE, THE CONTRACTOR MAY, AT HIS OPTION, SUBSTITUTE FOR THE COVER PLATES DESIGNATED ON THE PLANS COVER PLATES OF THE EQUIVALENT AREA PROVIDED THESE PLATES ARE AT LEAST 5/16"IN THICKNESS AND DO NOT EXCEED A WIDTH EQUAL TO THE FLANGE WIDTH LESS 2"OR A THICKNESS EQUAL TO 2 TIMES THE FLANGE THICKNESS. THE SIZE OF FILLET WELDS SHALL CONFORM TO THE REQUIREMENTS OF THE CURRENT ANSI/AASHTO/AWS "BRIDGE WELDING CODE". ELECTROSLAG WELDING WILL NOT BE PERMITTED.

PLACEMENT OF BEAM OR GIRDER MEMBERS ON TRUCKS FOR HAULING SHALL
BE DONE IN COMPLIANCE WITH LIMITS SHOWN ON SKETCHES PROVIDED TO THE MATERIALS
AND TEST UNIT APPROVED BY THE STRUCTURE DESIGN UNIT DATED MAY 8,1991.
THESE SKETCHES PRIMARILY LIMIT THE UNSUPPORTED CANTILEVER LENGTH OF MEMBERS.
WHEN THE CONTRACTOR WISHES TO PLACE MEMBERS ON TRUCKS NOT IN ACCORDANCE
WITH THESE LIMITS, TO SHIP BY RAIL, TO ATTACH SHIPPING RESTRAINTS TO THE
MEMBERS OR TO INVERT MEMBERS, HE SHALL SUBMIT A SKETCH FOR APPROVAL
PRIOR TO SHIPPING. SEE ALSO ARTICLE 1072-11.

WITH THE SOLE EXCEPTION OF EDGES AT SURFACES WHICH BEAR ON OTHER SURFACES, ALL SHARP EDGES AND ENDS OF SHAPES AND PLATES SHALL BE SLIGHTLY ROUNDED BY SUITABLE MEANS TO A RADIUS OF APPROXIMATELY 1/16 INCH OR EQUIVALENT FLAT SURFACE AT A SUITABLE ANGLE PRIOR TO PAINTING, GALVANIZING, OR METALLIZING.

### HANDRAILS AND POSTS:

METAL STANDARDS AND FACES OF THE CONCRETE END POSTS FOR THE METAL RAIL SHALL BE SET NORMAL TO THE GRADE OF THE CURB, UNLESS OTHERWISE SHOWN ON PLANS. THE METAL RAIL AND TOPS OF CONCRETE POSTS USED WITH THE ALUMINUM RAIL SHALL BE BUILT PARALLEL TO THE GRADE OF THE CURB.

METAL HANDRAILS SHALL BE IN ACCORDANCE WITH THE PLANS. RAILS SHALL BE AS MANUFACTURED FOR BRIDGE RAILING. CASTINGS SHALL BE OF A UNIFORM APPEARANCE. FINS AND OTHER DEFORMATIONS RESULTING FROM CASTING OR OTHERWISE SHALL BE REMOVED IN A MANNER SO THAT A UNIFORM COLORING OF THE COMPLETED CASTING SHALL BE OBTAINED. CASTINGS WITH DISCOLORATIONS OR OF NON-UNIFORM COLORING WILL

NOT BE ACCEPTED. CERTIFIED MILL REPORTS ARE REQUIRED FOR METAL RAILS AND POSTS.

### SPECIAL NOTES:

GENERALLY, IN CASE OF DISCREPANCY, THIS STANDARD SHEET OF NOTES SHALL GOVERN OVER THE SPECIFICATIONS, BUT THE REMAINDER OF THE PLANS SHALL GOVERN OVER NOTES HEREON, AND SPECIAL PROVISIONS SHALL GOVERN OVER ALL. SEE SPECIFICATIONS ARTICLE 105-4.

ENGLISH