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BORE LOG & CORE REPORTS

STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION

DIVISION OF HIGHWAYS GEOTECHNICAL UNIT

STRUCTURE SUBSURFACE INVESTIGATION

STATE PR	OJECT <u>33278</u>	<u> </u>	NO. B-3	3826
F.A. PROJE	ECT <u>BRZ-13</u>	31(5)		
COUNTY	CHEROI	KEE		
	DESCRIPTION _			
SITE DESC	RIPTION			

STATE	STATE PR	OJECT REFERENCE NO.	SHEET NO.	TOTAL
N.C.	3343	3.1.1 B-3826	1	9
STATE	PROJ. NO.	F. A. PROJ. NO.	DESCRIP	TION
			P.E.	
i			COME	T

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INVESTIGATED BY C A DUNNAGAN PERSONNEL T B DANIEL

CHECKED BY W D FRYE, Jr

C J COFFEY

SUBMITTED BY W D FRYE, Jr

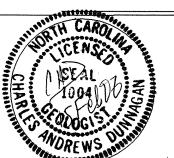
R D CHILDERS

FEBRUARY 2006

NOTE - THE INFORMATION CONTAINED HEREIN IS NOT IMPLIED OR GUARANTEED BY THE N. C. DEPARTMENT OF TRANSPORTATION AS BEING ACCURATE NOR IT IS CONSIDERED TO BE PART OF THE PLANS, SPECIFICATIONS, OR CONTRACT FOR THE PROJECT.

NOTE - BY HAVING REQUESTED THIS INFORMATION THE CONTRACTOR SPECIFICALLY WAIVES ANY CLAIMS FOR INCREASED COMPENSATION OR EXTENSION OF TIME BASED ON DIFFERENCES BETWEEN THE CONDITIONS INDICATED HEREIN AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.





DRAWN BY: C A DUNNAGAN

PROJECT REFERENCE NO. 33278.I.I B-3826 SHEET NO. 2 **of 9**

DIVISION OF HIGHWAYS

GEOTECHNICAL ENGINEERING UNIT

SUBSURFACE INVESTIGATION

	SOIL AND ROO	CK LEGEND, TERM	S, SYMBOLS	, AND ABBREVI	ATIONS			
SOIL DESCRIPTION	GRADATION			ROCK (DESCRIPTION	TERMS AND DEFINITIONS		
SOIL IS CONSIDERED TO BE THE UNCONSOLIDATED, SEMI-CONSOLIDATED, OR WEATHERED EARTH MATERIALS	<u>WELL GRADED</u> - INDICATES A GOOD REPRESENTATION OF PARTICLE SIZES FI <u>UNIFORM</u> - INDICATES THAT SOIL PARTICLES ARE ALL APPROXIMATELY THE	FROM FINE TO COARSE. E SAME SIZE.(ALSO	ROCK LINE INDICA	ATES THE LEVEL AT WHICH NON-C	T IF TESTED, WOULD YIELD SPT REFUSAL. AN INFERRED COASTAL PLAIN MATERIAL WOULD YIELD SPT REFUSAL.	ALLUVIUM (ALLUV.) - SOILS THAT HAVE BEEN TRANSPORTED BY WATER.		
THAT CAN BE PENETRATED WITH A CONTINUOUS FLIGHT POWER AUGER, AND YIELD LESS THAN 100 BLOWS PER FOOT ACCORDING TO STANDARD PENETRATION TEST (AASHTO T206, ASTM D-1586). SOIL	POORLY GRADED) GAP-GRADED - INDICATES A MIXTURE OF UNIFORM PARTICLES OF TWO OR M	10RE SIZES.	SPT REFUSAL IS F	PENETRATION BY A SPLIT SPOON	SAMPLER EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS. ON BETWEEN SOIL AND ROCK IS OFTEN REPRESENTED BY A ZONE	AOUIFER - A WATER BEARING FORMATION OR STRATA.		
CLASSIFICATION IS BASED ON THE AASHTO SYSTEM. BASIC DESCRIPTIONS GENERALLY SHALL INCLUDE:	ANGULARITY OF GRAINS		OF WEATHERED RO	OCK. ARE TYPICALLY DIVIDED AS FOLL		ARENACEOUS - APPLIED TO ROCKS THAT HAVE BEEN DERIVED FROM SAND OR THAT CONTAIN SAND.		
AS MINERALOGICAL COMPOSITION, ANGULARITY, STRUCTURE, PLASTICITY, ETC. EXAMPLE:	THE ANGULARITY OR ROUNDNESS OF SOIL GRAINS IS DESIGNATED BY THE	TERMS: ANGULAR,	WEATHERED			ARGILLACEOUS - APPLIED TO ALL ROCKS OR SUBSTANCES COMPOSED OF CLAY MINERALS, OR HAVING A NOTABLE PROPORTION OF CLAY IN THEIR COMPOSITION, AS SHALE, SLATE, ETC.		
VERY STIFF, GRAY, SILTY CLAY, MOIST WITH INTERBEDOED FINE SAND LAYERS, HIGHLY PLASTIC, A-7-6	SUBANGULAR, SUBROUNDED, OR ROUNDED.	NA I	ROCK (WR)	BLOWS PER FOO	LAIN MATERIAL THAT WOULD YIELD SPT N VALUES > 100	ARTESIAN - GROUND WATER THAT IS UNDER SUFFICIENT PRESSURE TO RISE ABOVE THE LEVEL		
SOIL LEGEND AND AASHTO CLASSIFICATION GENERAL GRANULAR MATERIALS SILT-CLAY MATERIALS COCANIO MATERIALS	MINERALOGICAL COMPOSITIO MINERAL NAMES SUCH AS QUARTZ, FELDSPAR, MICA, TALC, KAOLIN, ETC. ARE U		CRYSTALLINE	FINE TO COARSE	GRAIN IGNEOUS AND METAMORPHIC ROCK THAT	AT WHICH IT IS ENCOUNTERED, BUT WHICH DOES NOT NECESSARILY RISE TO OR ABOVE THE GROUND SURFACE.		
GENERAL GRANULAR MATERIALS SILT-CLAY MATERIALS ORGANIC MATERIALS CLASS. (≤ 35% PASSING #200) (> 35% PASSING #200) ORGANIC MATERIALS	WHENEVER THEY ARE CONSIDERED OF SIGNIFICANCE.	N DESCRIPTIONS	ROCK (CR)	GNEISS, GABBRO,		CALCAREOUS (CALC.) - SOILS THAT CONTAIN APPRECIABLE AMOUNTS OF CALCIUM CARBONATE.		
GROUP A-1 A-3 A-2 A-4 A-5 A-6 A-7 A-1, A-2 A-4, A-5	CUMPRESSIBILITY INVICENTIALLING I			SEDIMENTARY RO	GRAIN METAMORPHIC AND NON-COASTAL PLAIN OCK THAT WOULD YEILD SPT REFUSAL IF TESTED. ROCK TYPE	COLLUVIUM - ROCK FRAGMENTS MIXED WITH SOIL DEPOSITED BY GRAVITY ON SLOPE OR AT BOTTOM		
CLASS. A-1-a A-1-b A-2-4 A-2-5 A-2-6 A-2-7 A-7-5 A-3 A-6, A-7	SLIGHTLY COMPRESSIBLE LIQUID LIMIT MODERATELY COMPRESSIBLE LIQUID LIMIT	LESS THAN 31 EQUAL TO 31-50	ROCK (NCR)	INCLUDES PHYLL	.ITE, SLATE, SANDSTONE, ETC. SEDIMENTS CEMENTED INTO ROCK, BUT MAY NOT YIELD	OF SLOPE.		
SYMBOL 000000000000000000000000000000000000		GREATER THAN 50	SEDIMENTARY ROCK	SPT REFUSAL RO	OCK TYPE INCLUDES LIMESTONE SANDSTONE CEMENTED	CORE RECOVERY (REC.) - TOTAL LENGTH OF ALL MATERIAL RECOVERED IN THE CORE BARREL DIVIDED BY TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE.		
7. PASSING GRANULAR SILT- MUCK,	PERCENTAGE OF MATERIAL GRANULAR SILT - CLAY	L	1017	SHELL BEDS, ETC	ATHERING	DIKE - A TABULAR BODY OF IGNEOUS ROCK THAT CUTS ACROSS THE STRUCTURE OF ADJACENT		
# 40 30 MX 50 MX 51 MN SOILS CLAY PEAT	URGANIC MATERIAL SOILS SOILS	OTHER MATERIAL	FRESH ROCK		OINTS MAY SHOW SLIGHT STAINING, ROCK RINGS UNDER	ROCKS OR CUTS MASSIVE ROCK.		
" 200 15 MX 25 MX 10 MX 35 MX 35 MX 35 MX 35 MX 36 MN	TRACE OF ORGANIC MATTER 2 - 3% 3 - 5% TRACE LITTLE ORGANIC MATTER 3 - 5% 5 - 12% LITTLE		HAMME	ER IF CRYSTALLINE.	OTHER SHOW SELECT STREETING, HOLK MINGS CHOER	<u>DIP</u> - THE ANGLE AT WHICH A STRATUM OR ANY PLANAR FEATURE IS INCLINED FROM THE HORIZONTAL.		
LIQUID LIMIT	MODERATELY ORGANIC 5 - 10% 12 - 20% SOM	1E 20 - 35%			NED, SOME JOINTS MAY SHOW THIN CLAY COATINGS IF OPEN, CE SHINE BRIGHTLY, ROCK RINGS UNDER HAMMER BLOWS IF	DIP DIRECTION (DIP AZIMUTH) - THE DIRECTION OR BEARING OF THE HORIZONTAL TRACE OF		
GROUP INDEX 0 0 0 0 4 MX 8 MX 12 MX 16 MX No MX MODERATE ORGANI	HIGHLY ORGANIC >10% >20% HIGH GROUND WATER	HLY 35% AND ABOVE	OF A	CRYSTALLINE NATURE.		THE LINE OF DIP, MEASURED CLOCKWISE FROM NORTH.		
AMOUNTS OF SOILS	₩ATER LEVEL IN BORE HOLE IMMEDIATELY AFTER C	DRILLING	SLIGHT ROCK (SLI.) 1 INCH	GENERALLY FRESH, JOINTS STAIN	KED AND DISCOLORATION EXTENDS INTO ROCK UP TO AY. IN GRANITOID ROCKS SOME OCCASIONAL FELDSPAR	FAULT - A FRACTURE OR FRACTURE ZONE ALONG WHICH THERE HAS BEEN DISPLACEMENT OF THE SIDES RELATIVE TO ONE ANOTHER PARALLEL TO THE FRACTURE.		
OSOME TIPES STORE FRANCE FINE SILTY OR CLAYEY SILTY CLAYEY ORGANIC OF MAJOR GRAYEL, AND SAND GRAVEL AND SAND SOILS SOILS MATTER MATERIALS SAND GRAYEL AND SAND SOILS SOILS	STATIC WATER LEVEL AFTER 24 HOURS	billetillo	CRYST	TALS ARE DULL AND DISCOLORED.	CRYSTALLINE ROCKS RING UNDER HAMMER BLOWS.	FISSILE - A PROPERTY OF SPLITTING ALONG CLOSELY SPACED PARALLEL PLANES.		
GEN, RATING FAIR TO DOOD FAIR TO DOOD	720		MODERATE SIGNIF	FICANT PORTIONS OF ROCK SHOW	DISCOLORATION AND WEATHERING EFFECTS. IN RE DULL AND DISCOLORED, SOME SHOW CLAY. ROCK HAS	FLOAT - ROCK FRAGMENTS ON SURFACE NEAR THEIR ORIGINAL POSITION AND DISLODGED FROM		
AS A EXCELLENT TO GOOD FAIR TO POOR POOR UNSUITABLE SUBGRADE	E	ING STRATA	DULL :	SOUND UNDER HAMMER BLOWS AND	D SHOWS SIGNIFICANT LOSS OF STRENGTH AS COMPARED	PARENT MATERIAL.		
PI OF A-7-5 SUBGROUP IS ≤ LL - 30; PI OF A-7-6 SUBGROUP IS > LL - 30	SPRING OR SEEP		1	FRESH ROCK.	OR STAINED, IN GRANITOID ROCKS, ALL FELDSPARS DULL	FLOOD PLAIN (FP) - LAND BORDERING A STREAM, BUILT OF SEDIMENTS DEPOSITED BY THE STREAM.		
CONSISTENCY OR DENSENESS	MISCELLANEOUS SYMBOLS		SEVERE AND DI	DISCOLORED AND A MAJORITY SHOW	W KAOLINIZATION, ROCK SHOWS SEVERE LOSS OF STRENGTH	FORMATION (FM.) - A MAPPABLE GEOLOGIC UNIT THAT CAN BE RECOGNIZED AND TRACED IN		
PRIMARY SOIL TYPE COMPACTNESS OR CONSISTENCY PENETRATION RESISTENCE COMPRESSIVE STRENGTH	ROADWAY EMBANKMENT (RE) WITH SOIL DESCRIPTION SPT CFT OFF OHT TEST BORIN	NG SAMPLE DESIGNATIONS	(MOD. SEV.) AND C	CAN BE EXCAVATED WITH A GEOLD ISTED, WOULD YIELD SPT REFUSAL	OGIST'S PICK. ROCK GIVES 'CLUNK' SOUND WHEN STRUCK.	JOINT - FRACTURE IN ROCK ALONG WHICH NO APPRECIABLE MOVEMENT HAS OCCURRED.		
(N=VALUE) (TUN37F)=)	→ ₩	S - BULK SAMPLE	SEVERE ALL R	ROCK EXCEPT QUARTZ DISCOLOREC	O OR STAINED. ROCK FABRIC CLEAR AND EVIDENT BUT REDUCED	LEDGE - A SHELF-LIKE RIDGE OR PROJECTION OF ROCK WHOSE THICKNESS IS SMALL COMPARED TO		
GENERALLY LOOSE 4 TO 10	SOIL SYMBOL AUGER BORING	SS - SPLIT SPOON	(SEV.) IN STR	RENGTH TO STRONG SOIL. IN GRA NT. SOME FRAGMENTS OF STRONG	ANITOID ROCKS ALL FELDSPARS ARE KAOLINIZED TO SOME ROCK USUALLY REMAIN,	ITS LATERAL EXTENT.		
MATERIAL DENSE 10 TO 30	ARTIFICIAL FILL (AF) OTHER THAN ROADWAY EMBANKMENT - CORE BORING	SAMPLE	IF TE:	STED, YIELDS SPT N VALUES > 10	<u>00 BPF</u>	LENS - A BODY OF SOIL OR ROCK THAT THINS OUT IN ONE OR MORE DIRECTIONS.		
(NON-COHESIVE) VERY DENSE >50	INFERRED SOIL BOUNDARY	ST - SHELBY TUBE SAMPLE	VERY SEVERE ALL RI	OCK EXCEPT QUARTZ DISCOLORED 4ASS IS EFFECTIVELY REDUCED T	O OR STAINED. ROCK FABRIC ELEMENTS ARE DISCERNIBLE BUT O SOIL STATUS, WITH ONLY FRAGMENTS OF STRONG ROCK	MOTILED (MOT.) - IRREGULARLY MARKED WITH SPOTS OF DIFFERENT COLORS, MOTTLING IN SOILS USUALLY INDICATES POOR AERATION AND LACK OF GOOD DRAINAGE.		
VERY SOFT	₩ MONITORING WEL		REMAIN	REMAINING, SAPROLITE IS AN EXAMPLE OF ROCK WEATHERED TO A DEGREE SUCH THAT ONLY MINOR VESTIGES OF THE ORIGINAL ROCK FABRIC REMAIN. IF TESTED, YIELDS SPT N VALUES < 100 BPF		PERCHED WATER - WATER MAINTAINED ABOVE THE NORMAL GROUND WATER LEVEL BY THE PRESENCE OF AN		
SILT-CLAY MEDIUM STIFF 4 TO 8 0.5 TO 1.0	THE INFERRED ROCK LINE PIEZOMETER	SAMPLE			NOT DISCERNIBLE, OR DISCERNIBLE ONLY IN SMALL AND	INTERVENING IMPERVIOUS STRATUM, RESIDUAL (RES.) SOIL - SOIL FORMED IN PLACE BY THE WEATHERING OF ROCK,		
MATERIAL STIFF 8 TO 15 1 TO 2 (COHESIVE) VERY STIFF 15 TO 30 2 TO 4	TTTTT ALLUVIAL SOIL BOUNDARY INSTALLATION	RS - ROCK SAMPLE	SCATTE	ERED CONCENTRATIONS. QUARTZ M	MAY BE PRESENT AS DIKES OR STRINGERS. SAPROLITE IS	ROCK QUALITY DESIGNATION (RQD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF		
HARD >30 >4	25/025 DIP & DIP DIRECTION OF SLOPE INDICATOR INSTALLATION	RT - RECOMPACTED TRIAXIAL	ALSO A	AN EXAMPLE.	LIADDNECC	ROCK SEGMENTS EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF CORE RUN AND		
TEXTURE OR GRAIN SIZE	ROCK STRUCTURES — SPT N-VALUE	SAMPLE			HARDNESS	EXPRESSED AS A PERCENTAGE. SAPROLITE (SAP.) - RESIDUAL SOIL THAT RETAINS THE RELIC STRUCTURE OR FABRIC OF THE		
U.S. STD. SIEVE SIZE 4 10 40 60 200 270	SOUNDING ROD REF SPT REFUSAL	CBR - CALIFORNIA BEARING RATIO SAMPLE		NOT BE SCHATCHED BY KNIFE OR ERAL HARD BLOWS OF THE GEOLOG	SHARP PICK. BREAKING OF HAND SPECIMENS REQUIRES GIST'S PICK.	PARENT ROCK.		
OPENING (MM) 4.76 2.00 0.42 0.25 0.075 0.053	ABBREVIATIONS				K ONLY WITH DIFFICULTY, HARD HAMMER BLOWS REQUIRED	SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, THAT HAS BEEN EMPLACED PARALLEL		
BOULDER COBBLE GRAVEL SAND SAND SILT CLAY (BLDR.) (COB.) (GR.) (CCC.)	AR - AUGER REFUSAL HI HIGHLY	₩ - MOISTURE CONTENT		DETACH HAND SPECIMEN.	K. GOUGES OR GROOVES TO 0.25 INCHES DEEP CAN BE	TO THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS.		
(USE, SU.) (F SU.)	BT - BORING TERMINATED MED MEDIUM CL CLAY MICA MICACEOUS	V - VERY VST - VANE SHEAR TEST	HARD EXCA	AVATED BY HARD BLOW OF A GEOL	LOGIST'S PICK. HAND SPECIMENS CAN BE DETACHED	SLICKENSIDE - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT OR		
GRAIN MM 305 75 2.0 0.25 0.005 0.005 SIZE IN. 12 3	CPT - CONE PENETRATION TEST MOD MODERATELY	WEA WEATHERED		MODERATE BLOWS,	CHEC DEED BY FIRM DEFOCUE OF MARKET OF DIGIT DOWN	STANDARD PENETRATION TEST (PENETRATION RESISTANCE) (SPT) - NUMBER OF BLOWS (N OR BPF) OF		
SOIL MOISTURE - CORRELATION OF TERMS	CSE COARSE NP - NON PLASTIC DMT - DILATOMETER TEST ORG ORGANIC	γ - UNIT WEIGHT γ - DRY UNIT WEIGHT	HARD CAN	BE EXCAVATED IN SMALL CHIPS	CHES DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT. TO PEICES 1 INCH MAXIMUM SIZE BY HARD BLOWS OF THE	A 140 LB. HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF 1 FOOT INTO SOIL WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER, SPT REFUSAL IS PENETRATION EQUAL TO OR LESS		
SOIL MOISTURE SCALE FIELD MOISTURE CHIDE FOR FIELD MOISTURE DESCRIPTION	DPT - DYNAMIC PENETRATION TEST PMT - PRESSUREMETER TEST e - VOID RATIO SAP SAPROLITIC	,	l	NT OF A GEOLOGIST'S PICK.	BY KNIFE OR PICK. CAN BE EXCAVATED IN FRAGMENTS	THAN 0.1 FOOT PER 60 BLOWS.		
(ATTERBERG LIMITS) DESCRIPTION GOIDE FOR FIELD MOISTORE DESCRIPTION	F - FINE SD SAND, SANDY		FROM	M CHIPS TO SEVERAL INCHES IN S	SIZE BY MODERATE BLOWS OF A PICK POINT. SMALL, THIN	STRATA CORE RECOVERY (SREC.) - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY TOTAL LENGTH OF STRATUM AND EXPRESSED AS A PERCENTAGE.		
- SATURATED - USUALLY LIQUID; VERY WET, USUALLY (SAT.) FROM BELOW THE GROUND WATER TABLE	FOSS FOSSILIFEROUS SL SILT, SILTY FRAC FRACTURED, FRACTURES SLI SLIGHTLY		1	CES CAN BE BROKEN BY FINGER PI	RESSURE. EXCAVATED READILY WITH POINT OF PICK, PIECES 1 INCH	STRATA ROCK QUALITY DESIGNATION (SROD) - A MEASURE OF ROCK QUALITY DESCRIBED BY		
LL_ LIQUID LIMIT	FRAGS FRAGMENTS TCR - TRICONE REFUSAL		SOFT OR MI	MORE IN THICKNESS CAN BE BROKE	EN BY FINGER PRESSURE. CAN BE SCRATCHED READILY BY	TOTAL LENGTH OF ROCK SEGMENTS WITHIN A STRATUM EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF STRATA AND EXPRESSED AS A PERCENTAGE.		
PLASTIC SEMISOLID; REQUIRES DRYING TO	EQUIPMENT USED ON SUBJECT P	PO IECT		ERNAIL.	DEDDING	TOPSQIL (TS.) - SURFACE SOILS USUALLY CONTAINING ORGANIC MATTER.		
(PI) PLASTIC LIMIT ATTAIN OPTIMUM MOISTURE		HAMMER TYPE:	IERM	URE SPACING SPACING	BEDDING IERM IHICKNESS	RENCH MARK. #2 - RR SPIKE IN 30'HEMLOCK		
ON OPTIMUM MOISTURE - MOIST - (M) SOLID; AT OR NEAR OPTIMUM MOISTURE	DRILL UNITS: ADVANCING TOOLS:	HAMMER TYPE: X AUTOMATIC MANUAL	VERY WIDE	MORE THAN 10 FEET	VERY THICKLY BEDDED > 4 FEET	RENCH WARK: 2 - UU SLIVE IN SA DEWFORK		
OM OPTIMUM MOISTURE - MOIST - (M) SULIU; AT UR NEAR OPTIMUM MOISTURE SL SHRINKAGE LIMIT	MOBILE B- CLAY BITS	A ROTOLINITE LINIORE	WIDE MODERATELY CLO	3 TO 10 FEET OSE 1 TO 3 FEET	THICKLY BEDDED 1.5 - 4 FEET THINLY BEDDED 0.16 - 1.5 FEET	ELEVATION: 1700.58 FT.		
REQUIRES ADDITIONAL WATER TO	6º CONTINUOUS FLIGHT AUGER	CORE SIZE:	CLOSE	Ø.16 TO 1 FEET	VERY THINLY BEDDED 0.03 - 0.16 FEET THICKLY LAMINATED 0.008 - 0.03 FEET	NOTES:		
- DRY - (D) ATTAIN OPTIMUM MOISTURE	BK-51 X 8' HOLLOW AUGERS	-8	VERY CLOSE	LESS THAN 0.16 FEET	THINLY LAMINATED < 0.008 FEET			
PLASTICITY	CME-45C HARD FACED FINGER BITS				URATION	-		
PLASTICITY INDEX (PI) DRY STRENGTH	TUNGCARBIDE INSERTS	П-н	FUR SEDIMENTARY RO		NING OF THE MATERIAL BY CEMENTING, HEAT, PRESSURE, ETC.			
. NONPLASTIC	X CME-550 CASING W/ ADVANCER	I	FRIABLE		WITH FINGER FREES NUMEROUS GRAINS; BLOW BY HAMMER DISINTEGRATES SAMPLE.			
MED. PLASTICITY 16-25 MEDIUM HIGH PLASTICITY 26 OR MORE HIGH	PORTABLE HOIST TRICONE STEEL TEETH	HAND TOOLS: POST HOLE DIGGER	MODERATE		CAN BE SEPARATED FROM SAMPLE WITH STEEL PROBE;			
COLOR	TRICONE 'TUNG -CARR	HAND AUGER	TIODEN ATEL		EASILY WHEN HIT WITH HAMMER.			
DESCRIPTIONS MAY INCLUDE COLOR OR COLOR COMBINATIONS (TAN, RED, YELLOW-BROWN, BLUE-GRAY).	OTHER CORE BIT	SOUNDING ROD	INDURATED		ARE DIFFICULT TO SEPARATE WITH STEEL PROBE; LT TO BREAK WITH HAMMER,			
MODIFIERS SUCH AS LIGHT, DARK, STREAKED, ETC. ARE USED TO DESCRIBE APPEARANCE.	OTHEROTHER	VANE SHEAR TEST	EXTREME		HAMMER BLOWS REQUIRED TO BREAK SAMPLE:			
		OTHER	LATHERI		BREAKS ACROSS GRAINS.			



STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

MICHAEL F. EASLEY
GOVERNOR

LYNDO TIPPETT SECRETARY

February 13, 2006

STATE PROJECT:

33278.1.1 (B-3826)

F. A. PROJECT:

BRZ-1331(5)

COUNTY:

Cherokee

DESCRIPTION:

Bridge No. 166 on SR-1331 over Bates Creek

SUBJECT:

Geotechnical Report – Foundation Investigation

Introduction

This project is in central Cherokee County, approximately 2.5 miles north of Murphy. The proposed construction will replace the existing bridge with a single-span structure. The length will be 30.0 feet and the skew will be 75 degrees.

The subsurface investigation was conducted using a CME-550 drill machine and 8-inch hollow stem augers. Standard Penetration Tests were conducted at intervals of five feet using an automatic drive hammer. Soil samples were collected and submitted for testing of quality. The sample test results were not available at the time of this writing.

Foundation Materials

End Bent One

A single boring, at EB1-B, was drilled on this bent location. Roadway embankment is present from the surface. This material is comprised of 5.0 feet of silty sand and sandy silt. Below the embankment is alluvium. This horizon consists of 4.3 feet of coarse sand and gravel. A mere 0.10 foot of weathered rock separates the alluvium and rock. Hollow auger refusal on rock occurred at 9.4 feet (elevation 1692.82). Static groundwater was measured at 6.6 feet (elevation 1695.62).

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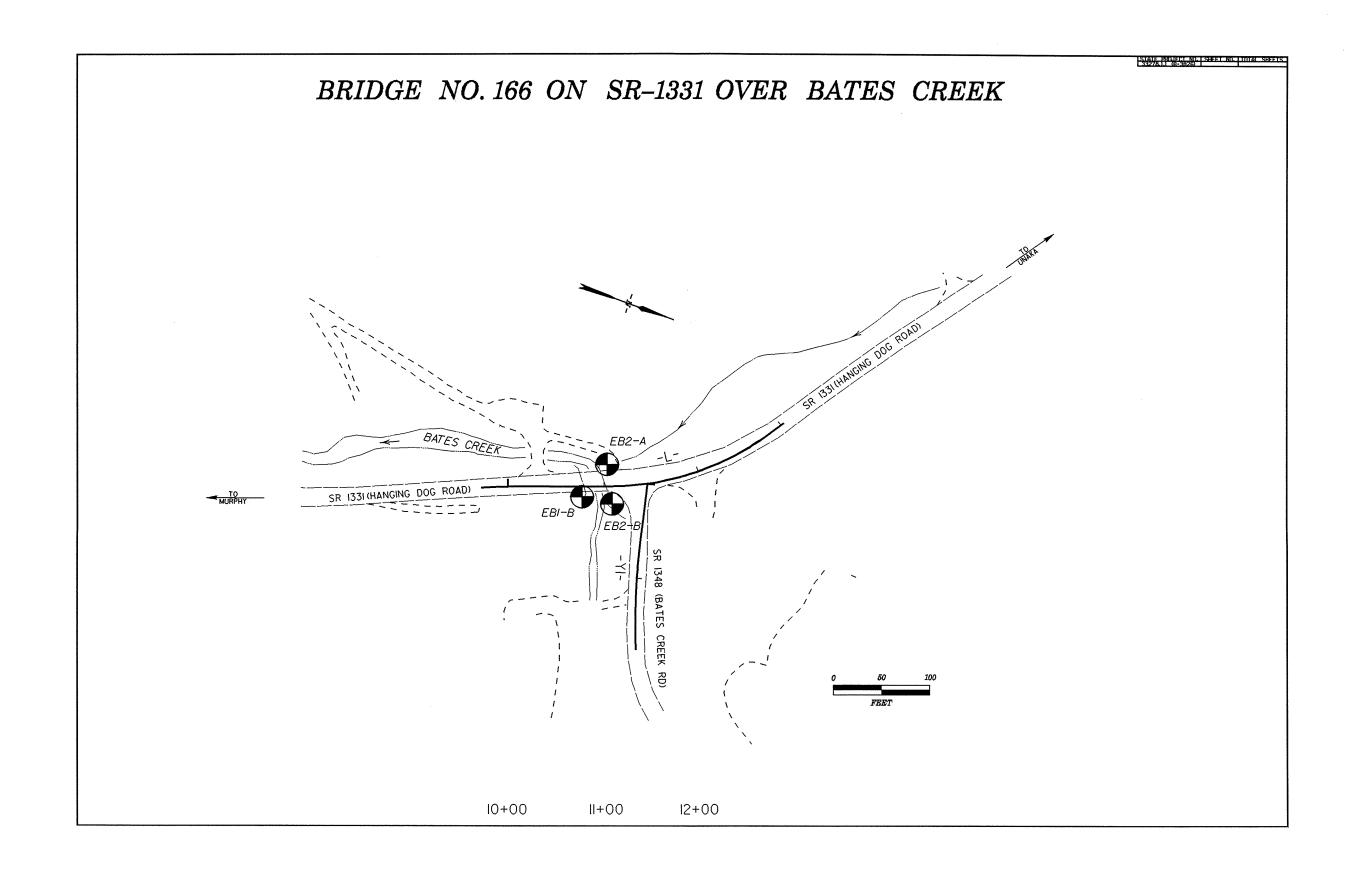
LOCATION: CENTURY CENTER COMPLEX BUILDING B 1020 BIRCH RIDGE DRIVE RALEIGH NC 27610

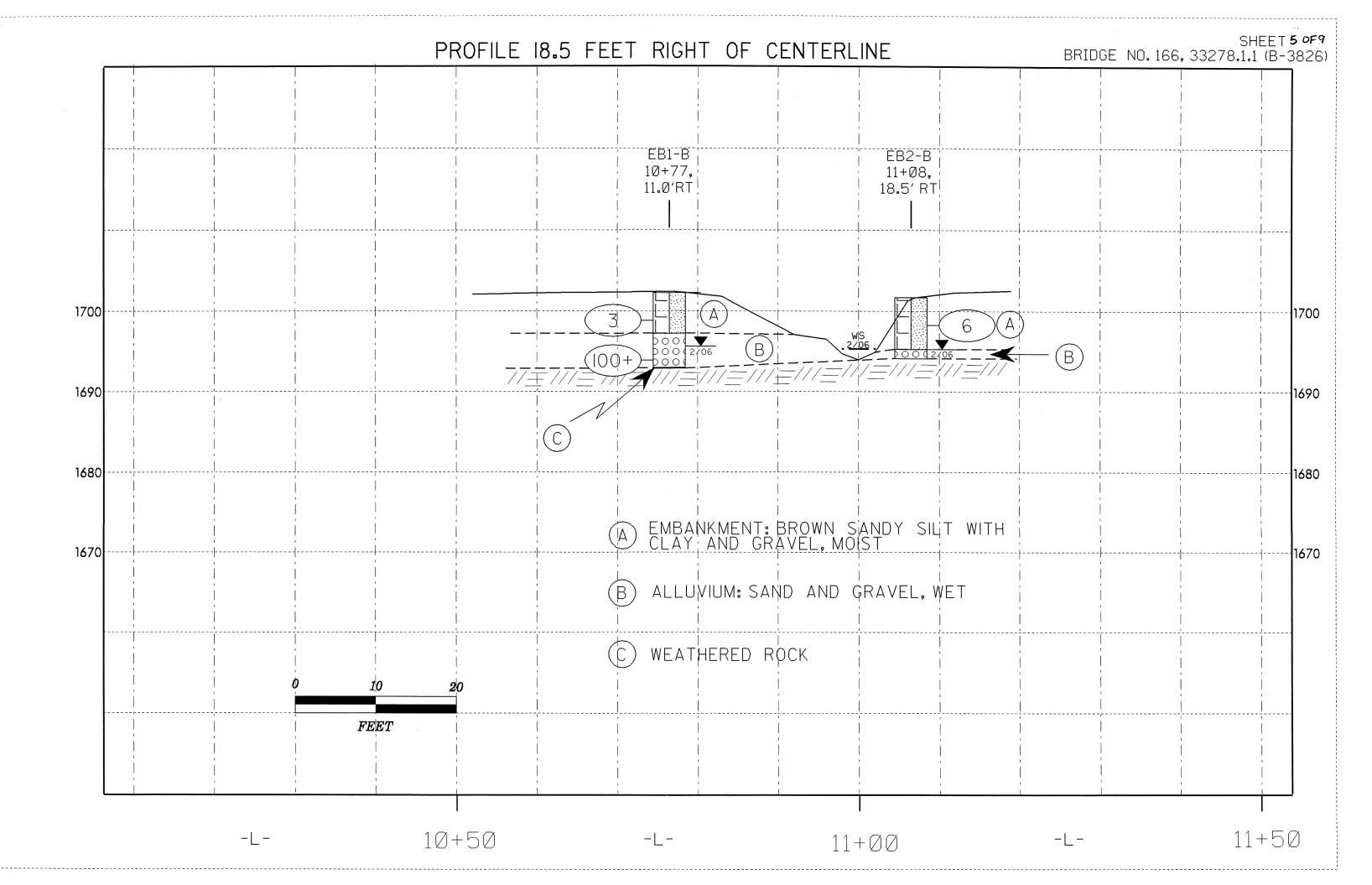
End Bent Two

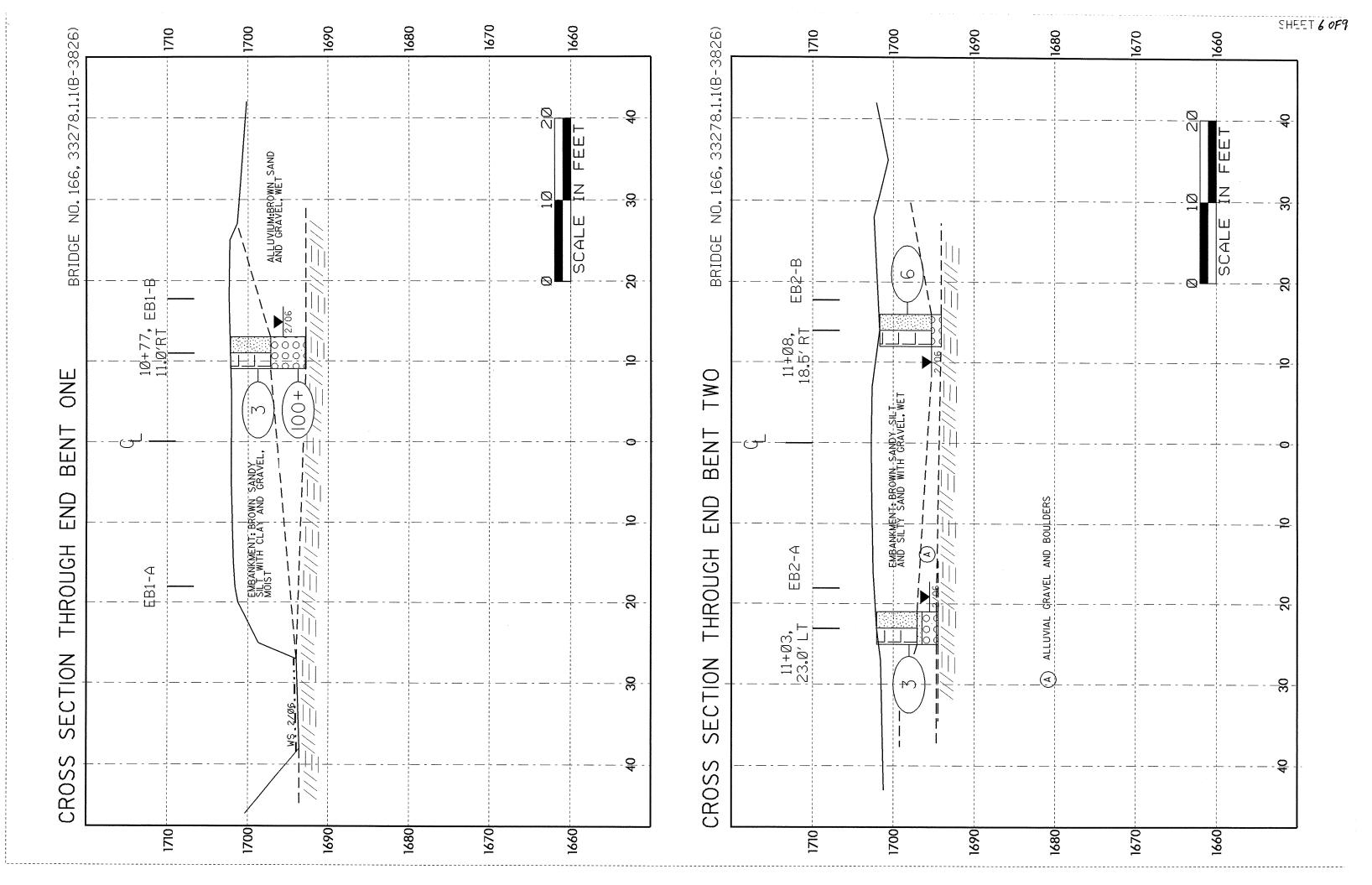
Embankment was encountered at the surface in both of the borings on this bent. This material is made of 5.0 to 6.4 feet of sandy silt and silty sand with gravel. The embankment was emplaced upon alluvium. The alluvium is a layer of sandy silt and gravel (with occasional boulders) 1.5 to 2.5 feet thick. Neither saprolite nor weathered rock was noted in either boring. Hollow auger refusal on rock occurred in EB2-A at 7.6 feet (elevation 1694.46); in EB2-B, it also occurred at 7.6 feet (elevation 1694.05). Static groundwater was measured in the boring for EB2-A at 6.5 feet (elevation 1695.56). In the boring for EB2-B, it was at 6.4 feet (elevation 1695.25).

Respectfully Submitted,

Charles A. Dunnagan, LG Project Engineering Geologist







NORTH CAROLINA DEPARTMENT OF TRANSPORTATION GEOTECHNICAL UNIT BORING LOG

PROJECT	ΓNO 3327	8.1.1			ID B-3	826	COU		EROKEE	OMINO	-	-	GIST T B DANIEL	
SITE DES	CRIPTIO	N BRI	IDGE	NO.	166 OI	N SR-1331					1020	20.	3101 I D DAIVILL	CNID WATER
BORING						HING 0.0				EASTING	0.00			GND WATER
ALIGNMI	ENT -L-				BORIN	IG LOCAT	TION 10+	-77.000		OFFSET		DT		0 HR N/A
COLLAR	ELEV 17	02.22	ft			L DEPTH			START D	TE 2/03/0		. 1	~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~	24 HR 6.60ft
DRILL M.	ACHINE (CME (550				1		D H.S. AL		, <u>o</u>		COMPLETION D.	
SURFACE	E WATER	DEPT	H				DEPTH			JOLINO	HAMMER TYP			AUTOMATIC
			LOW	CT	PEN	E	BLOWS F			SAMPLE		11	Log EB1-B, Page 1 of 1	ID ROCK
ELEV	DEPTH	6in	6in	6in	(ft)				75 10		MOI	ĮŌ	· ·	
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4 1 2	‡		٠.	l					-					
1702.22	<u> </u>						-Ground	Surface						
1700.00	Ł		ĺ									E	EMBANKMENT:	BROWN SANDY
_	3.40	2	1	2	1.0	3						E	SILT WITH CLAY	AND GRAVEL.
=	F.					X				SS-1		E.	MO	IST
	- 8.40	45									Y	0000	ALLUVIUM: SANI	O AND GRAVEL,
1692.82	- 0.40	15	50		0.4				65	SS-2		0000	W	ET
=	_					HOLLO	<u>W AUGE</u>	RREFU	SAL AT	1.0-2			WEATHER	PED BOCK /
_						ELE	V-1692.8	2 ON R	CK.				<u> </u>	LED ROOK
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NORTH CAROLINA DEPARTMENT OF TRANSPORTATION GEOTECHNICAL UNIT BORING LOG

						GEOT	ECHN	ICAL L	JNIT B	ORING	LOG			gar jegona		
PROJECT	NO 3327	8.1.1		1	D B-3	826	COU	NTY CHE	EROKEE	: .	GEO	LOC	GIST T B DANIEL			
SITE DES	CRIPTION	BRI	DGE	NO. 1	66 ON	I SR-1331	OVER E	BATES CI	REEK		.4			GND WATER		
BORING I	NO EB2-A			ľ	NORT	HING 0.0	0			EASTIN	G 0.00			0 HR N/A		
ALIGNMI	ENT -L-			I	BORIN	G LOCAT	TION 11-	+03.000		OFFSE	Γ 23.00ff	LT		24 HR 6.50ft		
COLLAR	ELEV 170)2.06f	t	TOTAL D			OTAL DEPTH 7.60ft START DATE 2				3/06		COMPLETION D	OMPLETION DATE 02/03/06		
DRILL M	ACHINE (OME 5	550				DRILL	METHOI	H.S. AL	JGERS			HAMMER TYPE AUTOMATIC			
SURFACE	WATER	DEPT.	H				DEPTH	TO ROC	K N/A			•	Log EB2-A, Page 1 of 1			
ELEV	DEPTH	BL	OW (CT	PEN	E	BLOWS PER FOOT SAMPLE Y L SOIL		SOIL AN	ND ROCK						
LLLV		6in	6in	6in	(ft)	0 2	25 5	50 7	75 10	NO NO	MO	OG I	DESCF	RIPTION		
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	E										2			· 4		
1702.06							Ground	Surface		H		LI				
1700.00_	1											Ŀ		BROWN SANDY WET		
_	4.00	3	2	1	1.0	3				SS-3		E	O.L.			
	-					X				33-3	▼	0000	ALLUVIUM: BRO	WN SANDY SILT,		
1694.46							7777755					0000		ET CIET,		
	L					-HOLLO	W-AUGE W-1694 #	R REFU 6 ON RE	6K	ŀ				GRAVEL AND		
	_			·										DERS		
	<u> </u>												\ WEATHER	RED ROCK		
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NORTH CAROLINA DEPARTMENT OF TRANSPORTATION GEOTECHNICAL UNIT BORING LOG

GEOTECHNICAL UNIT BORING LOG														
PROJECT	NO 3327	8.1.1		I	D B-38				ROKEE			LOG	IST T B DANIEL	
SITE DES	CRIPTION	BRI	DGE I	NO. 1	66 ON	SR-1331								GND WATER
BORING N						IING 0.00				EASTING	0.00			0 HR N/A
ALIGNME	ENT -L-			E	BORIN	G LOCAT	ION 11+	-08.000		OFFSET		RT		24 HR 6.40ft
COLLAR	ELEV 170)1.65fi	t			DEPTH			START DA	TE 2/03/0			COMPLETION DA	
DRILL MA	ACHINE C	ME 5	50						D H.S. AL				AUTOMATIC	
SURFACE								TO ROC	***************************************				Log EB2-B, Page 1 of 1	
ELEV	DEPTH	BL	.OW (СТ	PEN	E	LOWS F			SAMPLĘ	Y/	Ĺ		D ROCK
ELEV	DEPIR	6in	6in	6in	(ft)	0 2	5 5	0	75 10		MOI	G	DESCR	IPTION
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1701.65 –		1.					Ğround	Surface.						
1700.00_												E	EMBANKMENT: S	SANDY SILT AND
-	3.40	2	2	4	1.0	6						Ė	SILTY SAND WIT	H GRAVEL, WET
-	-					-X					_	Ė.		
1694.05											▼	0000	A. I. I. I. I. I. A. I.	00.0.451
_	_					-HOLLO	₩-AUGE	R-REFU	SAL-AT-				ALLUVIAL	GRAVEL
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WORK	SH
NORTH CAROLINA DEPARTMENT OF TRANSPORTATION	FIELD



SCOUR REPORT

PROJECT:	33278.1.1	ID:	B-3826	COUNTY: Cherokee	
DESCRIPTION(1): E	Bridge No. 166 o	n SR-1331	over Bates Creek.		

	EXISTING BRIDGE
Information from:	Field Inspection X Microfilm (reel pos:) Other (explain)
	Length: 24 Total Bents: 3 Bents in Channel: 1 Bents in Floodplain: 2
EVIDENCE OF SCOUP Abutments or End Be	R(2) ent Slopes: Minor amount around EB2 footing.
Interior Bents: None	noted.
Channel Bed: None	noted.
Channel Bank: None	noted.
EXISTING SCOUR PR Type(3): Pile an	
Extent(4): Five fe	eet beyond width of bridge.
Effectiveness(5): Good	
Obstructions(6): Chann	nel has been restricted to +/- 2.0 feet by rock walls, 25 - 35 feet upstream.

INSTRUCTIONS

- 1 Describe the specific site's location, including route number and body of water crossed.
- 2 Note scour evidence at existing end bents or abutments (e.g. undermining, sloughing, degradations).
- 3 Note existing scour protection (e.g. rip rap).
- 4 Describe extent of existing scour protection.
- 5 Describe whether or not the scour protection appears to be working.
- 6 Note obstructions such as dams, fallen trees, debris at bents, etc.
- Describe the channel bed material based on observation and/or samples. Include any lab results with report.
- 8 Describe the channel bank material based on observation and/or samples. Include any lab results with report.
- 9 Describe the material covering the banks (e.g. grass, trees, rip rap, none).
- 10 Determine the approximate floodplain width from field observation or a topographic map.
- 11 Describe the material covering the floodplain (e.g. grass, trees, crops).
- 12 Use professional judgement to specify if the stream is degrading, aggrading, or static.
- 13 Describe potential and direction of the stream to migrate laterally during the bridge's life (approx. 100 years).
- 14 Give the geotechnically adjusted scour elevation (GASE) expected over the life of the bridge (approx. 100 years). This elevation can be given as a range across the site, or for each bent. Discuss the relationship between the Hydraulics Unit theoritical scour and the GASE. If the GASE is dependent on scour counter measures, explain (e.g. rip rap armoring on slopes). The GASE is based on the erodability of materials, giving consideration to the influence of joints, foliation, bedding characteristics, % core recovery, % RQD, differential weathering, shear strength, observations at existing structures, other tests deemed appropriate, and overall geologic conditions at the site.

Channel Bo	ed Material(7): Rock w			IFORMA ts (sand.			es and	bould	lers) in	places.	
	`										-7	
Channel Ba	nk Material(8): <u>Silty sa</u>	nd.									P OF PROPERTY OF SERVICE AND AND A SERVICE OF
Channel E	Bank Cover(9): Grass v	vith occas	sional sn	nall trees.					andresses considerada balantere ar anos	and an analysis of the second second second second	
Floodpl	ain Width(10): <u>EB1-A</u> :	and EB2-	A +/- 50	feet; EB1	-b an	d EB2-E	3 > 100) feet.			•
Floodpl	ain Cover(11): <u>Grass v</u>	vith occas	sional sn	nall trees.	***************************************			nder obder in medical der medical des	n a regar von dense de d'antière es		
	Stream is(12): A	ggrading		Degra	nding _	ng in gang magandahan di danan		Sta	tic X		
annel Migration	Tendency(13): North.			allegan yang sa sagata sagata, panasa nebanjagan taun sag					- en elle frança en . No fil e les hands	described described as a second	
Observations an	d Other Com	ments:										
GEOTECHNICA	LLY ADJUS	TED SCO	UR ELEV	/ATION	S(14)	Fe	et		Mete	ers		
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	BENT B1		Da	D.4								
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	es, Rt	raulics Un	it theoreti	ical scou	ır:							
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NB Lan Comparison of C	GASE to Hyd						ERIAL					
NB Lan Comparison of C SOIL ANALYSI: Bed or Bank	GASE to Hyd						ERIAL					
NB Lan Comparison of C SOIL ANALYSI: Bed or Bank Sample No.	GASE to Hyd						ERIAL					
NB Lan Comparison of C SOIL ANALYSI Bed or Bank Sample No. Retained #4	GASE to Hyd						ERIAL					
NB Lan Comparison of C SOIL ANALYSI: Bed or Bank Sample No.	GASE to Hyd						ERIAL					
SOIL ANALYSIS Bed or Bank Sample No. Retained #4 Passed #10	GASE to Hyd						ERIAL					
NB Lan Comparison of C SOIL ANALYSI: Bed or Bank Sample No. Retained #4 Passed #10 Passed #40	GASE to Hyd						ERIAL					
SOIL ANALYSIS Bed or Bank Sample No. Retained #4 Passed #10 Passed #200	GASE to Hyd						ERIAL					
SOIL ANALYSI Bed or Bank Sample No. Retained #4 Passed #10 Passed #40 Passed #200 Coarse Sand	GASE to Hyd						ERIAL					
SOIL ANALYSI Bed or Bank Sample No. Retained #4 Passed #10 Passed #40 Passed #200 Coarse Sand Fine Sand	GASE to Hyd						ERIAL					
SOIL ANALYSI: Bed or Bank Sample No. Retained #4 Passed #10 Passed #200 Coarse Sand Fine Sand Silt	GASE to Hyd						ERIAL					
SOIL ANALYSIS Bed or Bank Sample No. Retained #4 Passed #10 Passed #200 Coarse Sand Fine Sand Silt Clay	GASE to Hyd						ERIAL					
SOIL ANALYSI: Bed or Bank Sample No. Retained #4 Passed #10 Passed #200 Coarse Sand Fine Sand Silt Clay LL	GASE to Hyd						ERIAL					
SOIL ANALYSI: Bed or Bank Sample No. Retained #4 Passed #10 Passed #200 Coarse Sand Fine Sand Silt Clay LL PI AASHTO Station	GASE to Hyd						ERIAL					
Comparison of Co	GASE to Hyd						ERIAL					
SOIL ANALYSI: Bed or Bank Sample No. Retained #4 Passed #10 Passed #200 Coarse Sand Fine Sand Silt Clay LL PI AASHTO Station	GASE to Hyd						ERIAL					

Reported by:		Date:	2/7/2006
	C A Dunnagan	-	