CONTENTS:
SHEET DESCRIPTION

7-9

10

11

12

TITLE SHEET

SITE PLAN

BORE LOGS

CROSS SECTIONS

SCOUR REPORT

SOIL TEST RESULTS

SITE PHOTOGRAPH

STRUCTURE INVENTORY REPORT

**LEGEND** 

**PROFILE** 

### STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION

### TATE OF NORTH CAROLIN.

DIVISION OF HIGHWAYS
GEOTECHNICAL UNIT

## STRUCTURE SUBSURFACE INVESTIGATION

STATE PROJECT 33612.1.1 I.D. NO. B-4271

F.A. PROJECT BRZ-1246(2)

COUNTY SAMPSON

PROJECT DESCRIPTION BRIDGE NO. 98 ON

SR 1246 (BUTLER ISLAND RD.) OVER

BIG SWAMP AT -L- STATION 17+15

INVENTORY

			P.E. CONS	<del>-</del>
STATE	PROJ. NO.	F, A. PROJ. NO.	DESCRIP	TION
N.C.	33612	2.1.1(B-4271)	1	12
STATE	STATE PRO	OJECT REFERENCE NO.	NO.	SHEE

#### **CAUTION NOTICE**

THE SUBSURFACE INFORMATION AND THE SUBSURFACE INVESTIGATION ON WHICH IT IS BASED WAS MADE FOR THE PURPOSE OF STUDY, PLANNING AND DESIGN, AND NOT FOR CONSTRUCTION OR PAY PURPOSES. THE VARIOUS FIELD BORNING LOGS, ROCK CORES, AND SOIL TEST DATA AVAILABLE MAY BE REVIEWED OR INSPECTED IN RALEIGH BY CONTACTING THE N. C. DEPARTMENT OF TRANSPORTATION, GEOTECHNICAL UNIT @ 1993 250-4088. NEITHER THE SUBSURFACE PLANS AND REPORTS, NOR THE FIELD BORNING LOGS, ROCK CORES, OR SOIL TEST DATA IS PART OF THE CONTRACT.

GENERAL SOIL AND ROCK STRATA DESCRIPTIONS AND INDICATED BOUNDARIES ARE BASED ON A GEOTECHNICAL INTERPRETATION OF ALL AVAILABLE SUBSURFACE DATA AND MAY NOT NECESSARILY REFLECT THE ACTUAL SUBSURFACE CONDITIONS BETWEEN BORINGS OR BETWEEN SAMPLED STRATA WITHIN THE BOREHOLE. THE LABORATORY SAMPLE DATA AND THE IN SITU (IN-PLACE) TEST DATA CAN BE RELIED ON ONLY TO THE DEGREE OF RELIABILITY INHERENT IN THE STRANDARD TEST METHOD. THE OBSERVED WATER LEVELS OR SOIL MOISTURE CONDITIONS OF THE INVESTIGATION. THESE WATER LEVELS OR SOIL MOISTURE CONDITIONS ARE AS RECORDED AT THE TIME OF THE INVESTIGATION. THESE WATER LEVELS OR SOIL MOISTURE CONDITIONS MAY VARY CONSIDERABLY WITH TIME ACCORDING TO CLIMATIC CONDITIONS INCLUDING TEMPERATURES, PRECEITATION AND WIND, AS WELL AS OTHER NON-CLIMATIC FACTORS.

THE BIDDER OR CONTRACTOR IS CAUTIONED THAT DETAILS SHOWN ON THE SUBSURFACE PLANS ARE PRELMMARY ONLY AND IM MANY CASES THE FINAL DESIGN DETAILS ARE DIFFERENT, FOR BIDDING AND CONSTRUCTION PURPOSES, REFER TO THE CONSTRUCTION PLANS AND DOCUMENTS FOR FINAL DESIGN INFORMATION ON THIS PROJECT. THE DEPARTMENT DOES NOT WARRANT OR GUARANTEE THE SUFFICIENCY OR ACCURACY OF THE INVESTIGATION MADE, NOR THE INTERPRETATIONS MADE OR OPINION OF THE DEPARTMENT AS TO THE TYPE OF MATERIALS AND CONDITIONS TO BE ENCOUNTERED. THE BIDDER OR CONTRACTOR IS CAUTIONED TO MAKE SUCH INDEPENDENT SUBSURFACE INVESTIGATIONS AS HE DEEMS NECESSARY TO SATISTY HIMSELF AS TO CONDITIONS TO BE ENCOUNTERED ON THIS PROJECT. THE CONTRACTOR SHALL HAVE NO CLAIM FOR ADDITIONAL COMPENSATION OR FOR AN EXTENSION OF TIME FOR ANY REASON RESULTING FROM THE ACTUAL CONDITIONS ENCOUNTERED AT THE SITE DIFFERING FROM THOSE INDICATED IN THE SUBSURFACE INFORMATION,

INVESTIGATED BY O.B. OTT

CHECKED BY N.T. ROBERSON

SUBMITTED BY N.T. ROBERSON

DATE AUGUST 2005

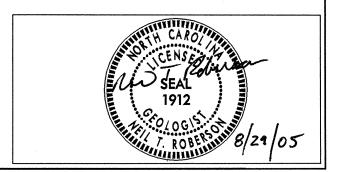
BY N.T. ROBERSON

S.M. SMITH

S.W. MILLER

R.R HELD

J.I. MILKOVITS JR.



NOTE - THE INFORMATION CONTAINED HEREIN IS NOT IMPLIED OR GUARANTEED BY THE N. C. DEPARTMENT OF TRANSPORTATION AS BEING ACCURATE NOR IT IS CONSIDERED TO BE PART OF THE PLANS, SPECIFICATIONS, OR CONTRACT FOR THE PROJECT.

NOTE - BY HAVING REQUESTED THIS INFORMATION THE CONTRACTOR SPECIFICALLY WAIVES ANY CLAIMS FOR INCREASED COMPENSATION OR EXTENSION OF TIME BASED ON DIFFERENCES BETWEEN THE CONDITIONS INDICATED HEREIN AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

#### NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

### DIVISION OF HIGHWAYS GEOTECHNICAL UNIT

## SUBSURFACE INVESTIGATION

SOIL AND ROCK LEGEND, TERMS, SYMBOLS, AND ABBREVIATIONS SOIL DESCRIPTION TERMS AND DEFINITIONS <u>IELL GRADED</u>- INDICATES A GOOD REPRESENTATION OF PARTICLE SIZES FROM FINE TO COARSI NIFDRM- INDICATES THAT SOIL PARTICLES ARE ALL APPROXIMATELY THE SAME SIZE. (ALSO HARD ROCK IS NON-COASTAL PLAIN MATERIAL THAT WHEN TESTED, WOULD YIELD SPT REFUSAL. AN INFERRED ROCK LINE INDICATES THE LEVEL AT WHICH NON-COASTAL PLAIN MATERIAL WOULD YIELD SPT REFUSAL. SPT REFUSAL IS PENETRATION BY A SPLIT SPOON SAMPLER EDUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS. IN NON-COASTAL PLAIN MATERIAL, THE TRANSITION BETWEEN SOIL AND ROCK IS OFTEN REPRESENTED BY A ZONE SOIL IS CONSIDERED TO BE THE UNCONSOLIDATED, SEMI-CONSOLIDATED OR WEATHERED EARTH MATERIALS ALLUVIUM (ALLUV.) - SOILS WHICH HAVE BEEN TRANSPORTED BY WATER WHICH CAN BE PENETRATED WITH A CONTINUOUS FLIGHT POWER AUGER, AND WHICH YIELDS LESS THAN 100 BLOWS PER FOOT ACCORDING TO STANDARD PENETRATION TEST (AASHTO 1206, ASTM D-1586). SOIL ADUIFER - A WATER BEARING FORMATION OR STRATA. P-GRADED- INDICATES A MIXTURE OF UNIFORM PARTICLES OF TWO OR MORE SIZES. CLASSIFICATION IS BASED ON THE AASHTD SYSTEM AND BASIC DESCRIPTIONS GENERALLY SHALL INCLUDE ANGULARITY OF GRAINS ARENACEOUS - APPLIED TO ROCKS THAT HAVE BEEN DERIVED FROM SAND OR THAT CONTAIN SAND. CONSISTENCY, COLOR, TEXTURE, MOISTURE, AASHTO CLASSIFICATION, AND OTHER PERTINENT FACTORS SUCH ROCK MATERIALS ARE TYPICALLY DIVIDED AS FOLOWS: ARGILLACEOUS - APPLIED TO ALL ROCKS OR SUBSTANCES COMPOSED OF CLAY MINERALS. THE ANGULARITY OR ROUNDNESS OF SOIL GRAINS ARE DESIGNATED BY THE TERMS: ANGULAR, AS MINERALOGICAL COMPOSITION, ANGULARITY, STRUCTURE, PLASTICITY, ETC. EXAMPLE WEATHERED ROCK (WR) R HAVING A NOTABLE PROPORTION OF CLAY IN THEIR COMPOSITION, AS SHALE, SLATE, ETC. NON-COASTAL PLAIN MATERIAL THAT YIELDS SPT N VALUES > 100 BLOWS VERY STIFF, GRAY SBTY CLAY, MORST, WITH INTERREDUED FINE SAND LAYERS HIGHLY PLASTIC, A-7-6 SUBANGULAR, SUBROUNDED, OR ROUNDED. <u> ARTESIAN</u> - GROUND WATER THAT IS UNDER SUFFICIENT PRESSURE TO RISE ABOVE THE LEVEL PER FOOT. MINERALOGICAL COMPOSITION SOIL LEGEND AND AASHTO CLASSIFICATION T WHICH IS IS ENCOUNTERED, BUT WHICH DOES NOT NECESSARILY RISE TO OR ABOVE THE FINE TO COARSE GRAIN IGNEOUS AND METAMORPHIC ROCK THAT CRYSTALLINE ROCK (CR) MINERAL NAMES SUCH AS QUARTZ, FELDSPAR, MICA, TALC, KAOLIN, ETC. ARE USED IN DESCRIPTIONS FINE TO COURSE OFFAIR TOROUGH AND AND MELTAMORPHIL HOLK THAT WOULD YIELD SPT REFUSAL IF TESTED. ROCK TYPE INCLUDES GRANITE, GNEISS, GABBRO, SCHIST, ETC. FINE TO COARSE GRAIN METAMORPHIC AND NON-COASTAL PLAIN GENERA GROUND SURFACE. GRANIII AR MATERIAI S SILT-CLAY MATERIALS ORGANIC MATERIALS >85% PASSING \*200) WHENEVER THEY ARE CONSIDERED OF SIGNIFICANCE. CALCAREOUS (CALC.) - SOILS WHICH CONTAIN APPRECIABLE AMOUNTS OF CALCIUM CARBONATE. NON-CRYSTALLINE ROCK (NCR) A-1 A-3 A-4 A-5 A-6 A-7 GROUP COLLUVIUM - ROCK FRAGMENTS MIXED WITH SOIL DEPOSITED BY GRAVITY ON SLOPE OR AT BOTTOM SEDIMENTARY ROCK THAT WOULD YEILD SPT REFUSAL IF TESTED, ROCK TYPE A-6, A-7 SLIGHTLY COMPRESSIBLE MODERATELY COMPRESSIBLE HIGHLY COMPRESSIBLE LIOUID LIMIT LESS THAN 30 LIOUID LIMIT 31-50 LIOUID LIMIT GREATER THAN 50 NCLUDES PHYLLITE, SLATE, SANDSTONE, ETC.
COASTAL PLAIN SEDIMENTS CEMENTED INTO ROCK, BUT MAY NOT YIELD CLASS. 1-a A-1-b A-2-4 A-2-5 A-2-5 A-2-7 A-3 F SLOPE. COASTAL PLAIN SEDIMENTARY ROCK SYMBOL <u>CORE RECOVERY (REC.)</u> - TOTAL LENGTH OF ALL MATERIAL RECOVERED IN THE CORE BARREL DIVIDED BY TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE. SPT REFUSAL. ROCK TYPE INCLUDES LIMESTONE, SANDSTONE, CEMENTED PERCENTAGE OF MATERIAL PASSING SILT-DIKE - A TABULAR BODY OF IGNEOUS ROCK THAT CUTS ACROSS THE STRUCTURE OF ADJACENT WEATHERING MUCK. RANULA SILT- CLA CLAY ROCKS OR CUTS MASSIVE ROCK. ORGANIC MATERIAL PEAT SOILS OTHER MATERIAL SOILS SOILS SOILS ROCK FRESH, CRYSTALS BRIGHT, FEW JOINTS MAY SHOW SLIGHT STAINING, ROCK RINGS UNDER FRESH DIP - THE ANGLE AT WHICH A STRATUM OR ANY PLANAR FEATURE IS INCLINED FROM THE RACE OF ORGANIC MATTER 2 - 3% 3 - 5% HAMMER IF CRYSTALLINE. ITTLE ORGANIC MATTER
MODERATELY ORGANIC HORIZONTAL. LITTLE 10 - 20% 40 MX41 MN 40 MX41 MN 40 MX41 MN 40 MX41 M TOUID LIMI ROCK GENERALLY FRESH, JOINTS STAINED, SOME JOINTS MAY SHOW THIN CLAY COATINGS IF OPEN, 5 - 10% 12 - 20% SOME DIP DIRECTION (DIP AZIMUTH) - THE DIRECTION OR BEARING OF THE HORIZONTAL TRACE OF N.P. 10 MX 10 MX 11 MN 11 MN 10 MX 10 MX 11 MN 11 MN CRYSTALS ON A BROKEN SPECIMEN FACE SHINE BRIGHTLY. ROCK RINGS UNDER HAMMER BLOWS IF HIGHLY DRGANIC >20% (V. SLI.) >10% THE LINE OF DIP, MEASURED CLOCKWISE FROM NORTH. OF A CRYSTALLINE NATURE. GROUP INDEX а 0 0 4 MX 8 MX 12 MX 16 MX No MX MODERATE USUAL TYPES STONE FRAGS. FINE GRAVEL AND SAND GROUND WATER FAULT - A FRACTURE OR FRACTURE ZONE ALONG WHICH THERE HAS BEEN DISPLACEMENT OF THE ROCK GENERALLY FRESH, JOINTS STAINED AND DISCOLORATION EXTENDS INTO ROCK UP TO AMOUNTS OF SOILS IDES RELATIVE TO ONE ANOTHER PARALLEL TO THE FRACTURE.  $\nabla$ WATER LEVEL IN BORE HOLE IMMEDIATELY AFTER DRILLING. 1 INCH, OPEN JOINTS MAY CONTAIN CLAY, IN GRANITOID ROCKS SOME OCCASIONAL FELDSPAR STLTY OR CLAYEY CLAYEY ORGANIC (SL1.) MATTER CRYSTALS ARE DULL AND DISCOLORED, CRYSTALLINE ROCKS RING UNDER HAMMER BLOWS. FISSILE - A PROPERTY OF SPLITTING ALONG CLOSELY SPACED PARALLEL PLANES. STATIC WATER LEVEL AFTER 24 HOURS MODERATE SIGNIFICANT PORTIONS OF ROCK SHOW DISCOLORATION AND WEATHERING EFFECTS. IN  $\underline{\mathsf{FLOAT}}$  - ROCK FRAGMENTS ON SURFACE NEAR THEIR ORIGINAL POSITION AND DISLODGED FROM PARENT MATERIAL. VPW. GRANITOID ROCKS, MOST FELDSPARS ARE DULL AND DISCOLORED, SOME SHOW CLAY, ROCK HAS PERCHED WATER, SATURATED ZONE OR WATER BEARING STRATA (MOD.) POOR EXCELLENT TO GOOD FAIR TO POOR UNSUITABL POOR DULL SOUND UNDER HAMMER BLOWS AND SHOWS SIGNIFICANT LOSS OF STRENGTH AS COMPARED SUBGRADE FLOOD PLAIN (F.P.) - LAND BORDERING A STREAM, BUILT OF SEDIMENTS DEPOSITED BY THE STREAM. OM-SPRING OR SEEPAGE P.I. OF A-7-5 < L.L. - 30 : P.I. OF A-7-6 > L.L. ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. IN GRANITOID ROCKS, ALL FELDSPARS DULL AND DISCOLORED AND A MAJORITY SHOW KAOLINIZATION. ROCK SHOWS SEVERE LOSS OF STRENGTH MODERATEL CONSISTENCY OR DENSENESS MISCELLANEOUS SYMBOLS <u>FORMATION (FM.) -</u> A MAPPABLE GEOLOGIC UNIT THAT CAN BE RECOGNIZED AND TRACED IN THE FIELD. RANGE OF UNCONFINED (MOD, SEV.) AND CAN BE EXCAVATED WITH A GEOLOGIST'S PICK, ROCK GIVES 'CLUNK' SOUND WHEN STRUCK. COMPACTNESS OR CONSISTENCY PRIMARY SOIL TYPE PENETRATION RESISTENCE COMPRESSIVE STRENGTH ROADWAY EMBANKMENT DET DAT TEST BORING SAMPLE DESIGNATIONS IF TESTED, WOULD YIELD SPT REFUSAL JOINT - FRACTURE IN ROCK ALONG WHICH NO APPRECIABLE MOVEMENT HAS OCCURRED. WITH SOIL DESCRIPTION ALL ROCKS EXCEPT QUARTZ DISCOLORED OR STAINED, ROCK FABRIC CLEAR AND EVIDENT BUT REDUCED SEVERE LEDGE - A SHELF-LIKE RIDGE OR PROJECTION OF ROCK WHOSE THICKNESS IS SMALL COMPARED TO ITS LATERAL EXTENT. IN STRENGTH TO STRONG SOIL. IN GRANITOID ROCKS ALL FELDSPARS ARE KAOLINIZED TO SOME AUGER BORING GENERALLY S- BULK SAMPLE LOOSE 4 TO 10 EXTENT. 90ME FRAGMENTS OF STRONG ROCK USUALLY REMAIN. GRANUL AR MEDIUM DENSE N/A SS- SPLIT SPOON LENS - A BODY OF SOIL OR ROCK THAT THINS OUT IN ONE OR MORE DIRECTIONS ARTIFICIAL FILL OTHER THAN IF TESTED, YIELDS SPT N VALUES > 100 BPF CORE BORING 30 TO 50 (NON-COHESIVE) SAMPLE MOTTLED (MOT.) - IRREGULARLY MARKED WITH SPOTS OF DIFFERENT COLORS, MOTTLING IN VERY SEVERE ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED, ROCK FABRIC ELEMENTS ARE DISCERNIBLE BUT (V, SEV.) THE MASS IS EFFECTIVELY REDUCED TO SOIL STATUS, WITH ONLY FRAGMENTS OF STRONG ROCK VERY DENSE >50 ST- SHELBY TUBE SOILS USUALLY INDICATES POOR AERATION AND LACK OF GOOD DRAINAGE INFERRED SOIL BOUNDARIES VERY SOFT **\***\*O SAMPLE PERCHED WATER - WATER MAINTAINED ABOVE THE NORMAL GROUND WATER LEVEL BY THE PRESENCE OF AN REMAINING, SAPROLITE IS AN EXAMPLE OF ROCK WEATHERED TO A DEGREE SUCH THAT ONLY MINOR <0.25 MONITORING WELL GENERALLY 2 TO 4 0.25 TO 0.5 VESTIGES OF THE ORIGINAL ROCK FABRIC REMAIN. IF TESTED, YIELDS SPT N VALUES < 100 BPF INTERVENING IMPERVIOUS STRATUM SUFFICE INFERRED ROCK LINE RS- ROCK SAMPLE MEDJUM STIFF 4 TO 8 PIEZOMETER SILT-CLAY 0.5 TO 1 Δ ROCK REDUCED TO SOIL, ROCK FABRIC NOT DISCERNIBLE, OR DISCERNIBLE ONLY IN SMALL AND COMPLETE RESIDUAL SOIL - SOIL FORMED IN PLACE BY THE WEATHERING OF BOOK. INSTALLATION 8 TO 15 MATERIAL RT- RECOMPACTED ALLUVIAL SOIL BOUNDARY SCATTERED CONCENTRATIONS, QUARTZ MAY BE PRESENT AS DIKES OR STRINGERS, SAPROLITE IS ROCK QUALITY DESIGNATION (R.Q.D.) - A MEASURE OF ROCK QUALITY DESCRIBED BY: TOTAL LENGTH OF ROCK SEGMENTS GOULD TO OR OREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE. (COHESIVE) VERY STIFF 15 TO 30 SLOPE INDICATOR  $\bigcirc$ DIP/DIP DIRECTION OF INSTALLATION CBR - CBR SAMPLE ROCK HARDNESS ROCK STRUCTURES TEXTURE OR GRAIN SIZ - SPT N-VALUE SAPROLITE (SAP.) - RESIDUAL SOIL WHICH RETAINS THE RELIC STRUCTURE OR FABRIC OF THE CANNOT BE SCRATCHED BY KNIFE OR SHARP PICK. BREAKING OF HAND SPECIMENS REQUIRES VERY HARD SOUNDING ROD (REF)— SPT REFUSAL SEVERAL HARD BLOWS OF THE GEOLOGISTS PICK. OPENING (MM 4.76 2.0 0.42 0.25 0.075 0.053 <u>SILL</u> - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, WHICH HAS BEEN EMPLACED PARALLEL CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY, HARD HAMMER BLOWS REQUIRED HARD COARSE FINE TO DETACH HAND SPECIMEN. CORRL F GRAVEL TO THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS ROLL DER SILT CLAY (CL.) AR - AUGER REFUSAL PMT - PRESSUREMETER TEST (SL.) MODERATEI CAN BE SCRATCHED BY KNIFE OR PICK. GOUGES OR GROOVES TO 0.25 INCHES DEEP CAN BE SLICKENSIDE - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT OR SLIP PLANE. BORING TERMINATED SD. - SAND, SANDY EXCAVATED BY HARD BLOW OF A GEOLOGISTS PICK, HAND SPECIMENS CAN BE DETACHED CL. - CLAY SI - STIT STITY 2.0 0.25 0.05 0.005 BY MODERATE BLOWS. - CONE PENETRATION TEST S17F IN. 12\* STANDARD PENETRATION TEST (PENETRATION RESISTANCE) (SPT) - NUMBER OF BLOWS (N OR B.P.F.) OF MEDIUM CAN BE GROOVED OR GOUGED 0.05 INCHES DEEP BY FIRM PRESSURE OF KNIFF OR PICK POINT. CSE. - COARSE TCR - TRICONE REFUSAL 140 LB. HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF 1 FOOT INTO SOIL WITH EXCAVATED IN SMALL CHIPS TO PEICES I INCH MAXIMUM SIZE BY HARD BLOWS OF THE DMT - DILATOMETER TEST SOIL MOISTURE - CORRELATION OF TERMS  $\gamma$  - UNIT WEIGHT A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER, SPT REFUSAL IS LESS THAN 0.1 FOOT PENETRATION DPT - DYNAMIC PENETRATION TEST POINT OF A GEOLOGISTS PICK. SOIL MOISTURE SCALE 7d - DRY UNIT WEIGHT GUIDE FOR FIELD MOISTURE DESCRIPTIO CAN BE GROVED OR GOUGED READILY BY KNIFE OR PICK, CAN BE EXCAVATED IN FRAGMENTS - VOID RATIO SOFT DESCRIPTION FROM CHIPS TO SEVERAL INCHES IN SIZE BY MODERATE BLOWS OF A PICK POINT. SMALL, THIN PIECES CAN BE BROKEN BY FINGER PRESSURE. STRATA CORE RECOVERY (SREC.) - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY TOTAL LENGTH OF STRATUM AND EXPRESSED AS A PERCENTAGE. F. - FINE W - MOISTURE CONTENT FOSS. - FOSSILIFEROUS V. - VERY FRAC. - FRACTURED VST - VANE SHEAR TEST STRATA ROCK QUALITY DESIGNATION (S.R.O.D.) - A MEASURE OF ROCK QUALITY DESCRIBED BY: CAN BE CARVED WITH KNIFE. CAN BE EXCAVATED READILY WITH POINT OF PICK, PIECES 1 INCH OR MORE IN THICKNESS CAN BE BROKEN BY FINGER PRESSURE. CAN BE SCRATCHED READILY BY (SAT.) FROM BELOW THE GROUND WATER TABLE OTAL LENGTH OF ROCK SEGMENTS WITHIN A STRATUM EQUAL TO OR GREATER THAN 10 CENTIMETERS DIVIDED LIQUID LIMIT MED. - MEDIUM BY THE TOTAL LENGTH OF STRATA AND EXPRESSED AS A PERCENTAGE. ASTIC FINGERNATI SEMISOLID: REQUIRES DRYING TO - WET - (W) TOPSOIL (T.S.) - SURFACE SOILS USUALLY CONTAINING ORGANIC MATTER. EQUIPMENT USED ON SUBJECT PROJECT FRACTURE SPACING ATTAIN OPTIMUM MOISTURE (PI) PLASTIC LIMIT TERM THICKNESS BENCH MARK: TBM NO.I NAIL IN BACK OF 18" MAPLE AT STA. 17+51. TERM SPACING DRILL UNITS: ADVANCING TOOLS: HAMMER TYPE: VERY THICKLY BEDDED > 4 FEET VERY WIDE 10RE THAN 10 FEET - MOIST - (M) SOLID: AT OR NEAR OPTIMUM MOISTURE 45′ LT OPTIMUM MOISTURE AUTOMATIC MANUAL THICKLY BEDDED 1.5 - 4 FEET CLAY BITS 3 TO 10 FEET ELEVATION: 81.14' SHRINKAGE LIMIT MOBILE B-MODERATELY CLOSE 0.03 - 0.16 FEE VERY THINLY BEDDED 6º CONTINUOUS FLIGHT AUGER REQUIRES ADDITIONAL WATER TO CORE SIZE: CLOSE 0.16 TO 1 FEET NOTES: THICKLY LAMINATED 0.008 - 0.03 FFFT - DRY - (D) LESS THAN 0.16 FEET BK-51 VERY CLOSE ATTAIN OPTIMUM MOISTURE 8" HOLLOW AUGERS HINLY LAMINATED INDURATION PLASTICITY HARD FACED FINGER BITS CME-45C FOR SEDIMENTARY ROCKS, INDURATION IS THE HARDENING OF THE MATERIAL BY CEMENTING, HEAT, PRESSURE, ETC. PLASTICITY INDEX (PI) DRY STRENGTH TUNG-CARRIDE INSERTS -н\_\_\_\_ NONPLASTIC 0-5 VERY LOW RURRING WITH FINGER FREES NUMEROUS GRAINS FRIABLE LOW PLASTICITY MED. PLASTICITY CASING W/ ADVANCER 6-15 SLIGHT HAND TOOLS: GENTLE BLOW BY HAMMER DISINTEGRATES SAMPLE. MEDIUM 16-25 PORTABLE HOIST TRICONE 2 15/16 STEEL TEETH GRAINS CAN BE SEPARATED FROM SAMPLE WITH STEEL PROBE: HIGH PLASTICITY HIGH POST HOLE DIGGER MODERATELY INDURATED BREAKS EASILY WHEN HIT WITH HAMMER. TRICONE HAND AUGER COLOR OTHER GRAINS ARE DIFFICULT TO SEPARATE WITH STEEL PROBE; INDURATED SOUNDING ROD CORE BIT DESCRIPTIONS MAY INCLUDE COLOR OR COLOR COMBINATIONS (TAN, RED, YEL-BRN, BLUE-GRAY VANE SHEAR TEST OTHER MODIFIERS SUCH AS LIGHT, DARK, STREAKED, ETC. ARE USED TO DESCRIBE APPEARANCE. OTHER\_ EXTREMELY INDURATED SHARP HAMMER BLOWS REQUIRED TO BREAK SAMPLE: OTHER SAMPLE BREAKS ACROSS GRAINS

STATE PROJECT NO. SHEET NO. TOTAL SHEET!

B-4271



## STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

Michael F. Easley Governor

P.O. BOX 25201, RALEIGH, N.C. 27611-5201

Lyndo Tippett Secretary

August 29, 2005

STATE PROJECT:

33612.1.1 (B-4271)

FEDERAL PROJECT: COUTY:

BRZ-1246 (2) Sampson

DESCPRIPTION:

Bridge No. 98 on -L- (SR 1246) over Big Swamp

SUBJECT:

Geotechnical Report – Structure Inventory Report

#### **Project Description**

The project is located in west central Sampson County, west of the town of Clinton. Structure No. 98 on SR 1246 is scheduled to be replaced along the existing alignment. The proposed structure is 120 feet long with three spans at a skew angle of 90 degrees. A subsurface investigation was conducted during June of 1998. Two standard penetration test borings were performed at each end bent location and one SPT boring was performed at each interior bent. Soil samples were obtained for classification in the field and for laboratory analysis by the Materials and Tests Unit.

#### Physiography and Geology

The topography of the project area is typical of the coastal plain and is relatively flat resulting in a wide flood plain. The flood plain at the structure site is approximately 600 feet wide. Big Swamp flows toward the south into South River. Geologically, the project is located in the Coastal Plain Physiographic Province. Coastal Plain sediments are of the Black Creek Formation. These sediments typically consist of gray to black clay interlayered with gray to green sand. These sediments were deposited in a deltaic environment.

#### **Soil Properties**

Roadway embankment soils are present at the proposed End Bent locations. The embankment material consists of moist to wet, very loose to loose, fine to coarse sand (AASHTO classification A-2-4). Roadway embankment is underlain by alluvial soils

SHEET 3 33612.1.1 (B-4271)

Alluvial soils are present throughout the site and range in thickness from 8.9 to 13.9 feet. Very soft to soft fine-grained sandy silt (A-4) and silty clay (A-7-5) overlie loose to medium dense fine to coarse sand (A-2-4, A-3, and A-1-b). Organic contents range from trace amounts to highly organic in the alluvium.

Coastal Plain sediments underlie the alluvium. The sediments consists of black to gray, silty clay (A-7-5) and gray to green sand (A-2-4, a-2-6, A-1, b). The cohesive soils are generally stiff to very stiff and commonly contain laminae of very fine sand. The granular soils are medium to very dense, glauconitic, and contain laminae of clay. Lignite and mica are common.

#### Groundwater

Groundwater was encountered at all bent locations and ranges from elevation 80.7 feet to 78.7 feet across the site. Surface water in Big Swamp was measured at elevation 79.5 feet in December of 2004.

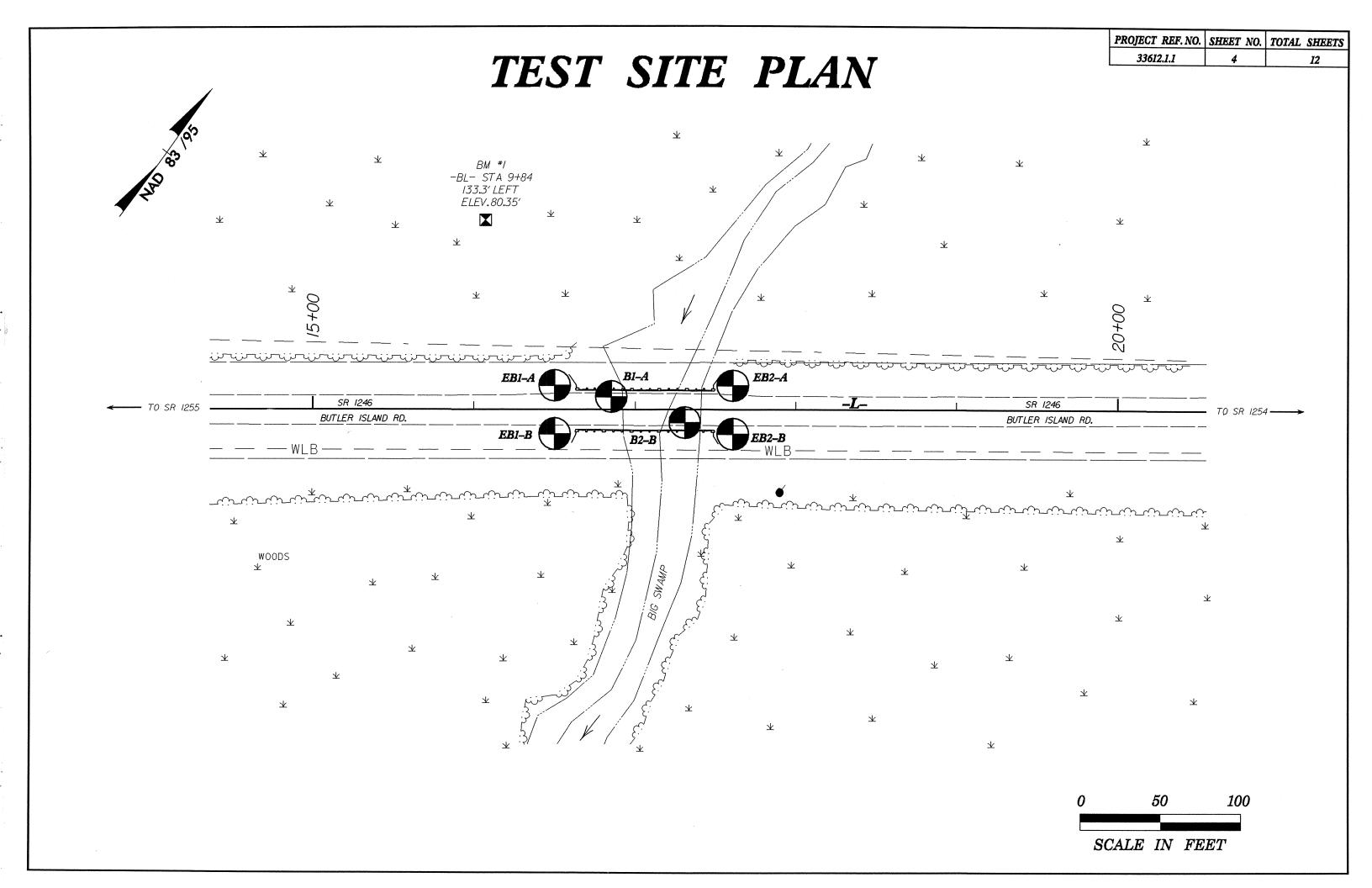
#### **Notice**

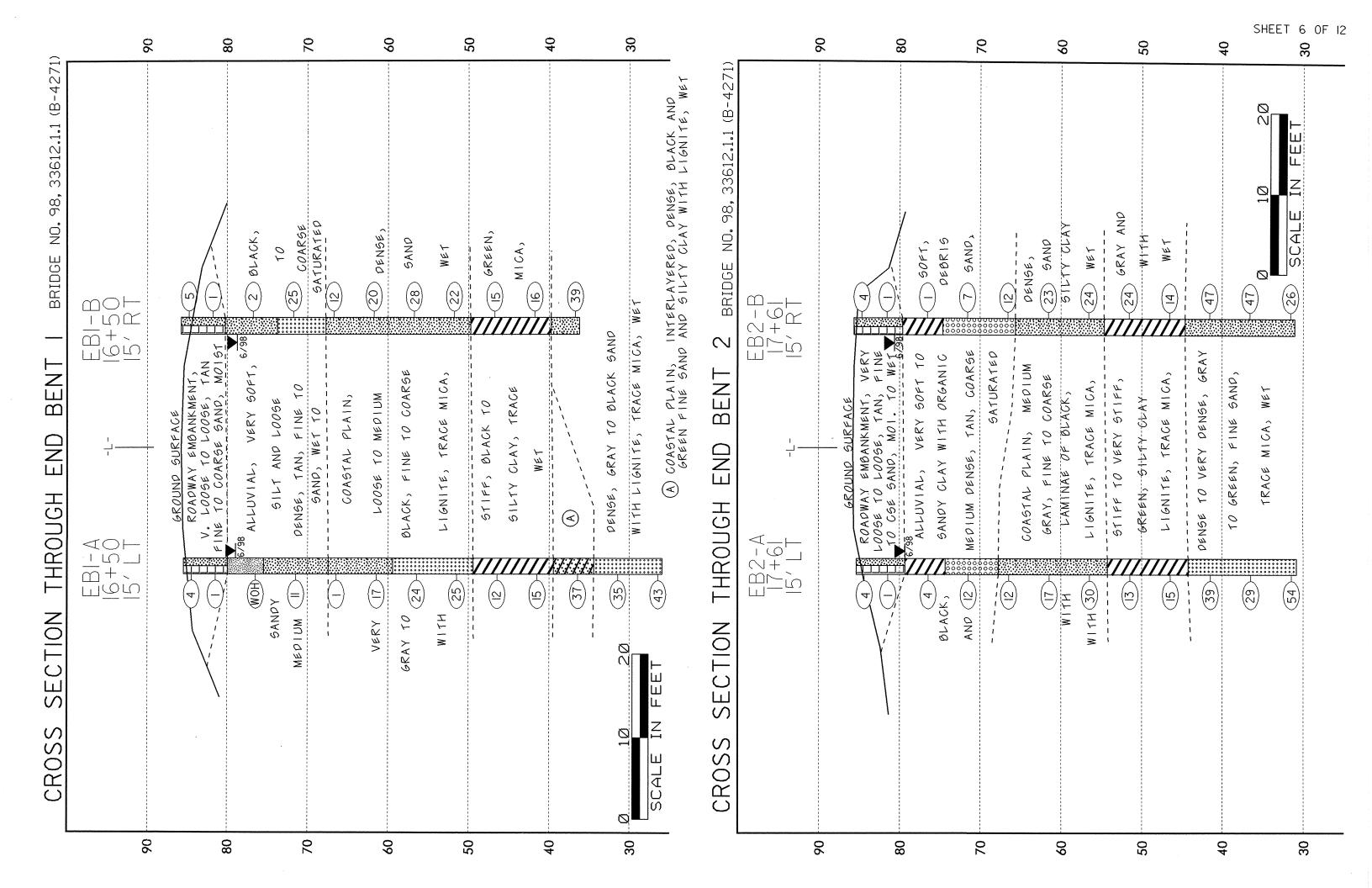
This Geotechnical structure inventory report is based on the bridge survey report for Big Swamp dated April 7 2005. If significant changes are made in the design or location of the proposed structure, the subsurface information should be reviewed and modified as necessary.

Respectfully submitted,

Onuoha B. Oti

Engineering Geologist





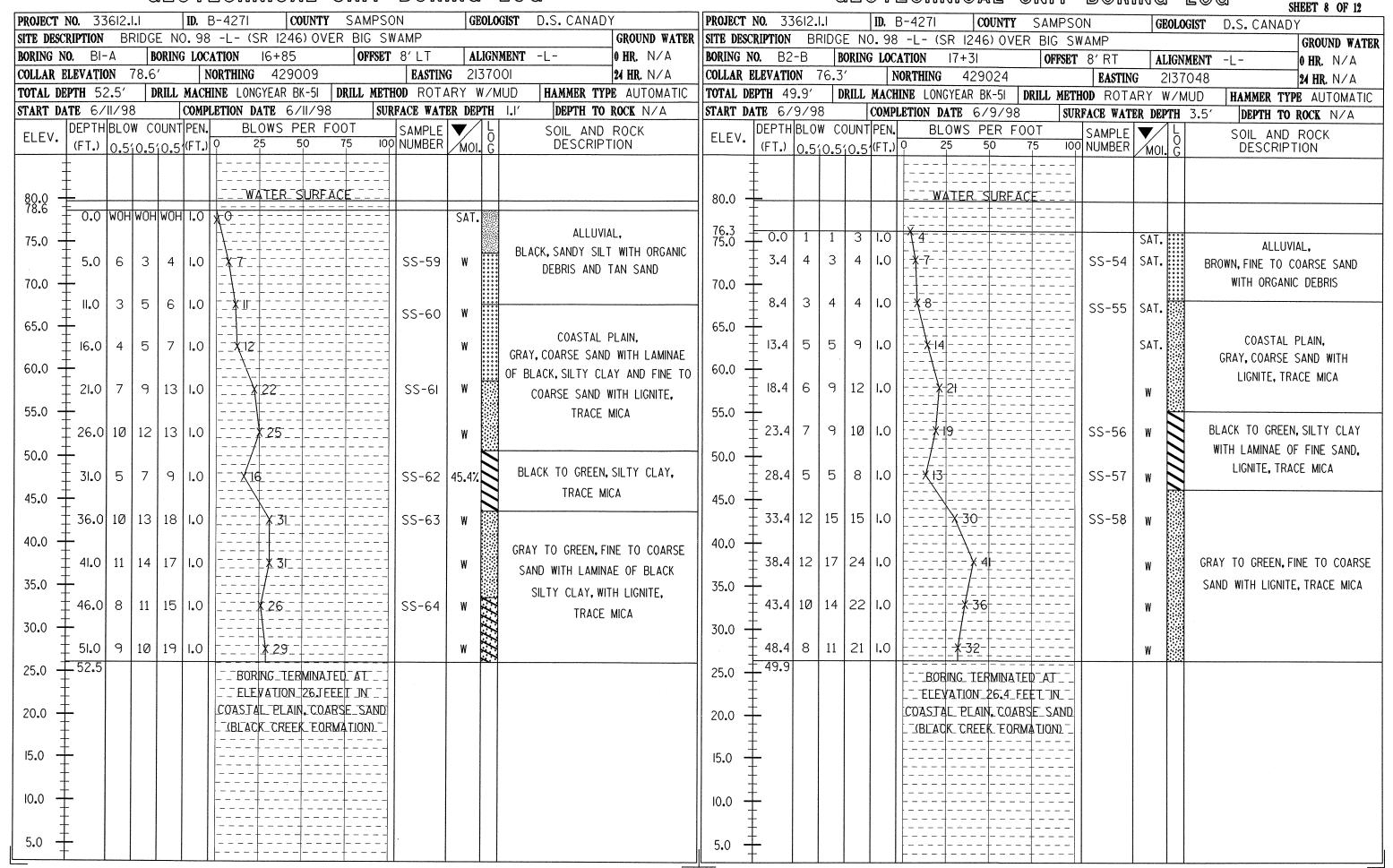
### NORTH CAROLINA DEPARTMENT OF TRANSPORTATION TO NORTH CAROLINA DEPARTMENT OF TRANSPORTATION GEOTECHNICAL UNIT BORING LOG

GEOTECHNICAL UNIT BORING LOG

TRANSPORT NA 77CIO LI VID D 407L GOVERNOV CANADON GRANDOV	SHEET 7 OF 12
PROJECT NO. 33612.1.1 ID. B-4271 COUNTY SAMPSON GEOLOGIST D.S. CANADY  SITE DESCRIPTION BRIDGE NO. 98 -L- (SR 1246) OVER BIG SWAMP GROUND WATER	PROJECT NO. 33612.1.1 ID. B-4271 COUNTY SAMPSON GEOLOGIST D.S. CANADY  SITE DESCRIPTION BRIDGE NO. 98 -L- (SR 1246) OVER BIG SWAMP GROWN WATER
SITE DESCRIPTION BRIDGE NO. 98 -L- (SR 1246) OVER BIG SWAMP  BORING NO. EBI-A BORING LOCATION 16+50 OFFSET 15' LT ALIGNMENT -L- OHR. N/A	OROCHO WATER
COLLAR ELEVATION 85.5' NORTHING 428993 EASTING 2136969 24 HR. 6.5'	COLLEGE COLUMN C
TOTAL DEPTH 59.9' DRILL MACHINE LONGYEAR BK-51 DRILL METHOD ROTARY W/MUD HAMMER TYPE AUTOMATIC	COLLAR ELEVATION 85.7' NORTHING 428969 EASTING 2136987 24 HR. 7.0'  TOTAL DEPTH 49.5' DRILL MACHINE LONGYEAR BK-51 DRILL METHOD ROTARY W/MUD HAMMER TYPE AUTOMATIC
START DATE 6/4/98   COMPLETION DATE 6/4/98   SURFACE WATER DEPTH N/A   DEPTH TO ROCK N/A	START DATE 6/4/98 COMPLETION DATE 6/4/98 SURFACE WATER DEPTH N/A DEPTH TO ROCK N/A
DEPTHIBLOW COLINTIPEN BLOWS PER FOOT SAMPLE TO LE SOU AND POCK	DEPTHRIOW COUNTRY DIOMS DED FOOT
ELEV. (FT.) 0.510.510.51(FT.) 0 25 50 75 100 NUMBER MOI. G DESCRIPTION	ELEV.   CFT.)   O.510.510.510.51 (FT.)   O.510.510.51 (FT.)   O.510.510.
+	+
85.5 + 0.0 1 2 2 1.0 7-4 M L ROADWAY SARAWAY	85.7 ± 0.0 2 3 2 1.0 7-5 M LIS
T	
TAN, FINE TO COARSE SAND	# 3.0 1 0 1 1.0 * F   W   TAN, FINE, COARSE SAND
80.0 +	-   80.0 <del> - </del>
# 8.0   WOH  WOH  I.0   *TOT	8.0 1 2 Ø 1.0 - X14 SS-46 SAT.
75.0 # ALLUVIAL, BLACK, SANDY SILT WITH ORGANIC	75.0 +                   ALLUVIAL,
TI3.0 1 5 6 1.0 - XII	
70.0 +	70.0 +
18.0   2   1   Ø   1.0	
65.0 +	
23.0 5 7 10 1.0X17 W COASTAL PLAIN.	23.0 6 9 11 1.0 X20 SS-49 W COASTAL PLAIN,
60.0 + GRAY TO BLACK, FINE TO COARSE	60.0 + GRAY TO BLACK, FINE TO COARSE
28.0 7   11   13   1.0   24	28.0 7 13 15 1.0
55.0 +	55.0 +
33.0 7 10 15 1.0 - 25 - 1-1-1 W	33.0 7 9 13 1.0 <del>22</del> W
50.0 +	] 50.0 +
38.0 4 5 7 1.0 - X12   W	38.0 5 7 8 1.0
45.0 # BLACK TO GREEN SILTY CLAY,	45.0 BLACK TO GREEN, SILTY CLAY
TRACE MICA	
40.0 — INTERLAYERED BLACK AND GREEN.	40.0 + 10.0   15.05
+ 48.0 11 16 21 1.0	48.0 8 15 25 1.0 SS-51 W GRAY TO BLACK SAND
35.0 + LIGNITE	35.0 + 49.5   BORING IERMINATED AT -
53.0 9 13 22 1.0 1 35 35 35 W	
30.0 + GRAY SAND WITH LIGNITE, TRACE MICA	
	TOBLACK_CREEK_FORMATIONS_
25.0 + 59.5    BORING_TERMINATED_AT	-   <sub>25.0</sub> - <del> </del>
- 55.5         - ELEVATION 25.6 FEET IN -	
20.0 COASTAL PLAIN, SAND	$\  \cdot \ _{200} = \  \cdot \ _{100} = $
20.0 +       CBLACK CREEK FORMATION) -	
15.0 +              +	15.0 +
<u> </u>	
10.0 + 1	10.0 +

## NORTH CAROLINA DEPARTMENT OF TRANSPORTATION GEOTECHNICAL UNIT BORING LOG

# NORTH CAROLINA DEPARTMENT OF TRANSPORTATION GEOTECHNICAL UNIT BORING LOG



## GEOTECHNICAL UNIT BORING LOG

NORTH CAROLINA DEPARTMENT OF TRANSPORTATION TO NORTH CAROLINA DEPARTMENT OF TRANSPORTATION GEOTECHNICAL UNIT BORING LOG

<b>PROJECT NO.</b> 33612.1.1 <b>ID.</b> B-4271 <b>COUNTY</b> SAMPSON	GEOLOGIST D.S. CANADY	PROJECT NO. 33612.1.1 ID. B	-427I <b>COUNTY</b> SAMPSON	GEOLOGIST D.S. CANADY
SITE DESCRIPTION BRIDGE NO. 98 -L- (SR 1246) OVER BIG SWAMF			-L- (SR 1246) OVER BIG SWAMP	GROUND WATER
BORING NO. EB2-A BORING LOCATION 17+61 OFFSET 15'		SORING NO. EB2-B BORING LOCA'		ALIGNMENT -L- 0 HR. N/A
COLLAR ELEVATION 85.3' NORTHING 429060 EA TOTAL DEPTH 54.4' DRILL MACHINE LONGYEAR BK-51 DRILL METHOD 1			NRTHING 429036 EASTIN  NE LONGYEAR BK-51 DRILL METHOD ROTA	IG         2137076         24 HR. 5.0'           ARY         W/MUD         HAMMER TYPE         AUTOMATIC
				ER DEPTH N/A DEPTH TO ROCK N/A
	PLE V SOIL AND ROCK	ELEV. DEPTH BLOW COUNT PEN. (FT.) 0.510.510.5 (FT.)	BLOWS PER FOOT SAMPLE	SOIL AND ROCK
		<u> </u>		·
85:8 = 0.0 1 2 2 1.0   X =	W 183	85.7 ± 0.0 1 1 3 1.0	<del></del>	M LES
85.0 <del>1</del> 0.0 1 2 2 1.0 <del>1</del> 2.9 1 1 Ø 1.0 <del>1</del> 7.13	M L ROADWAY EMBANKMENT,	± 3.l   1   Ø   1   l.0   3	/     	ROADWAY EMBANKMENT,
80.0 + 2.3   1   1   8   1.0		80.0 #		TAN, FINE TO COARSE SAND
7.9 0 2 2 1.0 7.4	W	± 8.1 WOH 1 0 1.0 1		SAT.
75.0	ALLUVIAL,	75.0 +		ALLUVIAL,
12.9 5 5 7 1.0 - 1.0	BLACK, SANDY CLAY WITH ORGANIC	± 13.1   4   4   3   1.0	<b>X7</b>	SAT. BLACK, SANDY CLAY WITH ORGANIC
70.0 +	"	70.0 +		DEBRIS AND TAN, COARSE SAND
	37   SAT.	± 18.1   3   4   8   1.0		SAT.
65.0 +	COASTAL PLAIN,	65.0 +		COASTAL PLAIN,
22.9 5 8 9 1.0 17	SAT. GRAY, COARSE SAND WITH LAMINAE	± 23.1   8   10   13   1.0	SS-43	GRAY, FINE TO COARSE SAND WITH
60.0 +		60.0 +		WITH LAMINAE OF BLACK, SILTY
+	38   SAT.   LIGNITE, TRACE MICA	28.1 9 10 14 1.0		SAND WITH LIGNITE, TRACE MICA
55.0 +		55.0 +		
	39 46.4%	33.1 9 12 12 1.0	T X 24	BLACK TO GREEN, SILTY CLAY,
50.0 +	GRAY TO BLACK, SILTY CLAY WITH LIGNITE, TRACE MICA	50.0 +	SS-44	TDACE MICA
45.0 + 37.9 5 7 8 1.0 - 1.5		45.0 + 0 0 1.0		71.1%
T	40   w :::	± 43.1   11   20   27   1.0   1	 	w 🐰
40.0 + 42.5   5   10   25   1.0     35	TU   11 0000	40.0 + 10.0   12   25   27   10   1		
47.9 13 12 17 1.0 X 29	GRAY TO GREEN, FINE SAND,	± 48.I   13   23   24   I.O		GREEN, FINE SAND, TRACE MICA
35.0	TRACE MICA	35.0 +		"
52.9 12 20 34 1.0	GRAY TO GREEN, FINE SAND,  W TRACE MICA	± 53.l   10   11   15   1.0		w
30.0 + 54.4  BORING_TERMINATED_AT	9 0 0 0	30.0 -54.6	BORING_TERMINATED_AT	1 (V) (V)
ELEVATION 30.9 FEET IN -		Ŧ	ELEVATION 3LIFEET_IN	
25.0		25.0 +	COASTAL PLAIN, FINE SAND	
T          (BLACK_CREEK_FORMATION)_		‡		
20.0 +		20.0 ‡		
<u> </u>		<u> </u>		
15.0 +		15.0 +		
		‡		
10.0 +		10.0 +		

PROJ. NO. - 33612.1.1 ID NO. - B-4271 COUNTY - SAMPSON

EB1-A

	SOIL TEST RESULTS														
SAMPLE			DEPTH	AASHTO				% BY W	/EIGHT		% PAS	SING (S	IEVES)	%	%
NO.	OFFSET	STATION	INTERVAL	CLASS.	L.L.	P.I.	C.SAND	F.SAND	SILT	CLAY	10	40	200	MOISTURE	ORGANIC
SS-52	15' LT	16+50	28.0-29.5	A-3(0)	24	NP	67.9	22.8	5.3	4.0	98	77	10	-	-
SS-53	15' LT	16+50	48.0-49.5	A-2-6(1)	31	15	37.8	33.7	10.3	18.2	97	81	29	-	-

EB1-B

			S	OIL T	TE.	ST	RE	SUL	LTS						
SAMPLE			DEPTH	AASHTO				% BY W	VEIGHT		% PAS	SING (S	IEVES)	%	%
NO.	OFFSET	STATION	INTERVAL	CLASS.	L.L.	P.I.	C.SAND	F.SAND	SILT	CLAY	10	40	200	MOISTURE	ORGANIC
SS-46	15' RT	16+50	8.0-9.5	A-2-4(0)	21	NP	17.6	51.5	18.8	12.1	100	98	35	•	•
SS-47	15' RT	16+50	13.0-14.5	A-3(0)	20	NP	69.1	25.9	3.0	2.0	80	52	5	•	•
SS-48	15' RT	16+50	18.0-19.5	A-2-4(0)	28	NP	36.0	47.1	8.9	8.1	100	91	18	-	
SS-49	15' RT	16+50	23.0-24.5	A-2-4(0)	26	NP	53.5	31.9	6.5	8.1	100	82	15		-
SS-50	15' RT	16+50	38.0-39.5	A-7-6(64)	91	63	2.4	10.3	28.7	58.6	100	99	89	•	•
SS-50	15' RT	16+50	48.0-49.5	A-2-4(0)	24	NP	28.7	51.1	12.1	8.1	100	89	21	-	-

*B1-A* 

$DI^{-}A$															
			S	OIL 7	TE	ST	RE	SUI	LTS						
SAMPLE			DEPTH	AASHTO				% BY V	VEIGHT		% PAS	SING (S	IEVES)	%	%
NO.	OFFSET	STATION	INTERVAL	CLASS.	L.L.	P.I.	C.SAND	F.SAND	SILT	CLAY	10	40	200	MOISTURE	ORGANIC
SS-59	8' LT	16+85	6.0-7.5	A-3(0)	22	NP	45.8	46.0	4.1	4.1	100	84	10	•	
SS-60	8' LT	16+85	11.0-12.5	A-3(0)	24	NP	57.2	33.7	5.1	4.1	100	80	10	-	
SS-61	8' LT	16+85	21.0-22.5	A-2-4(0)	30	NP	43.0	36.5	10.3	10.1	100	89	21	-	-
SS-62	8' LT	16+85	31.0-32.5	A-7-6(53)	83	58	4.3	13.6	31.4	50.7	100	98	84	45.4	•
SS-63	8' LT	16+85	36.0-37.5	A-2-4(0)	21	NP	46.0	35.7	8.1	10.1	95	78	18	-	
SS-64	8' LT	16+85	46.0-47.5	A-2-6(1)	34	18	45.4	23.3	13.0	18.3	89	60	29	-	•

*B2-B* 

			S	OIL T	TE.	ST	RE	SUL	TS						
SAMPLE			DEPTH	AASHTO				% BY W	/EIGHT		% PAS	SING (S	IEVES)	%	%
NO.	OFFSET	STATION	INTERVAL	CLASS.	L.L.	P.I.	C.SAND	F.SAND	SILT	CLAY	10	40	200	MOISTURE	ORGANIC
SS-54	8' RT	17+31	3.4-4.9	A-3(0)	22	NP	60.8	36.6	2.6	0.0	100	80	3	-	•
SS-55	8' RT	17+31	8.4-9.9	A-2-4(0)	25	NP	53.1	30.5	8.3	8.1	100	83	17	•	
SS-56	8' RT	17+31	23.4-24.9	A-7-6(17)	52	35	24.6	17.8	19.2	38.4	100	93	59	-	
SS-57	8' RT	17+31	28.4-29.9	A-7-6(50)	78	54	2.6	15.2	27.7	54.5	100	99	85		•
SS-58	8' RT	17+31	33.4-34.9	A-2-4(0)	23	NP	30.9	53.7	9.3	6.1	100	95	16	-	-

EB2-A

	SOIL TEST RESULTS														
SAMPLE			DEPTH	AASHTO				% BY W	VEIGHT		% PAS	SING (S	SIEVES)	%	%
NO.	OFFSET	STATION	INTERVAL	CLASS.	L.L.	P.I.	C.SAND	F.SAND	SILT	CLAY	10	40	200	MOISTURE	ORGANIC
SS-37	15' LT	17+61	17.9-19.4	A-2-4(0)	24	NP	64.2	19.2	10.5	6.1	100	77	17		
SS-38	15' LT	17+61	27.9-29.4	A-2-4(0)	28	NP	58.4	27.5	8.1	6.1	94	76	14	-	•
SS-39	15' LT	17+61	37.9-39.4	A-7-5(50)	84	54	4.4	14.5	285	52.5	100	98	83	46.4	-
SS-40	15' LT	17+61	42.9-44.4	A-3(0)	26	NP	17.0	75.8	5.3	2.0	100	99	8	-	-

SHEET 10 OF 12

EB2-B

	SOIL TEST RESULTS														
SAMPLE			DEPTH	AASHTO				% BY WE	IGHT		% PAS	SING (S	IEVES)	%	%
NO.	OFFSET	STATION	INTERVAL	CLASS.	L.L.	P.I.	C.SAND	F.SAND	SILT	CLAY	10	40	200	MOISTURE	ORGANIC
SS-41	15' RT	17+61	8.1-9.6	A-7-5(6)	43	13	20.6	24.0	35.2	20.2	100	93	59	-	•
SS-42	15' RT	17+61	13.1-14.6	A-1-b(0)	20	NP	94.6	3.8	1.6	0.0	68	14	1	-	
SS-43	15' RT	17+61	23.1-24.6	A-2-4(0)	34	NP	41.2	35.6	13.1	10.1	100	86	24	-	
SS-44	15' RT	17+61	38.1-39.6	A-7-6(57)	83	57	2.4	10.5	28.5	58.6	100	98	89	47.1	-
SS-45	15' RT	17+61	43.1-44.6	A-2-4(0)	25	NP	14.1	76.8	5.1	4.0	100	99	11	-	-



# FIELD SCOUR REPORT

WBS:	33612.1.1	TIP:	B-4271	COUNTY: SAMPSON
DESCRIPTION(1):	BRIDGE NO.98	ON SR 12	246 (BUTLER I	ISLAND RD) OVER BIG SWAMP.
			EXISTING	BRIDGE
Information from:		nspection (explain)	X Mid	icrofilm (reel pos: )
Bridge No.: Foundation Type:	98 Length TIMBER PILES	: 86.6'	Total Bents:	6 Bents in Channel: 3 Bents in Floodplain: 3
EVIDENCE OF SAbutments or E	SCOUR(2) End Bent Slopes	: NO EVID	ENCE OF SCO	OUR.
Interior Bents:	B1 MINIMAL TO AND B4 MINIM			R CONTRACTION SCOUR AT BENT LOCATION, B3
Channel Bed:	GENERAL DEC	GRADATIO	NAL SCOUR.	
Channel Bank:	NO EVIDENCE	OF SCOU	IR. BANKS AR	RE STABLE AND COVERED WITH VEGETATION.
EXISTING SCO Type(3):			MBER ABUTMI	ENTS AT WINGWALLS.
Extent(4):	EXTEND TO TO	DE OF SLO	OPE, BASE OF	F EMBANKMENT.
Effectiveness(5):	VERY EFFECT	IVE.		
Obstructions(6):	NONE OBSER	VED		

#### **INSTRUCTIONS**

- 1 Describe the specific site's location, including route number and body of water crossed.
- 2 Note scour evidence at existing end bents or abutments (e.g. undermining, sloughing, degradations).
- 3 Note existing scour protection (e.g. rip rap).
- 4 Describe extent of existing scour protection.
- 5 Describe whether or not the scour protection appears to be working.
- 6 Note obstructions such as dams, fallen trees, debris at bents, etc.
- 7 Describe the channel bed material based on observation and/or samples. Include any lab results with report.
- 8 Describe the channel bank material based on observation and/or samples. Include any lab results with report.
- **9** Describe the material covering the banks (e.g. grass, trees, rip rap, none).
- **10** Determine the approximate floodplain width from field observation or a topographic map.
- 11 Describe the material covering the floodplain (e.g. grass, trees, crops).
- 12 Use professional judgement to specify if the stream is degrading, aggrading, or static.
- 13 Describe potential and direction of the stream to migrate laterally during the bridge's life (approx. 100 years).
- Give the geotechnically adjusted scour elevation (GASE) expected over the life of the bridge (approx. 100 years). This elevation can be given as a range across the site, or for each bent. Discuss the relationship between the Hydraulics Unit theoritical scour and the GASE. If the GASE is dependent on scour counter measures, explain (e.g. rip rap armoring on slopes). The GASE is based on the erodability of materials, giving consideration to the influence of joints, foliation, bedding characteristics, % core recovery, % RQD, differential weathering, shear strength, observations at existing structures, other tests deemed appropriate, and overall geologic conditions at the site.

Comparison of GASE to Hydraulics Unit theoretical scour:
GASE IS UNCHANGE FROM HYDRAULICS UNIT'S THEORETICAL SCOUR FOR 100 YEAR EVENT

SOIL ANALYSIS RESULTS FROM CHANNEL BED AND BANK MATERIAL

SOIL ANAL I	SIS INCOULT	2 I IZOINI CITAL	AIALL DED WI	AD DAIAN INV	ILINAL		
Bed or Bank							
Sample No.	SS-54	SS-41	NS				
Retained #4							
Passed #10	100	100				and the second s	
Passed #40	- 80	93					
Passed #200	3	59	4				
Coarse Sand	60.8	20.6					
Fine Sand	36.6	24					
Silt	2.6	35.2					
Clay	0	20.2					
LL	22	43				4	
PI	NP	13					
AASHTO	A-3(0)	A-7-5(6)	A-4				
Station	17+31	17+31					
Offset	8' RT	15' RT					
Depth	3.4'-4.9'	8.1' -9.6'					
•							

Reported by: Onuoha B Oti

**Date:** 8/11/2005



