B-4005

OJECT: 33373.1.1

STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION
DIVISION OF HIGHWAYS
GEOTECHNICAL ENGINEERING UNIT

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STRUCTURE SUBSURFACE INVESTIGATION

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CAUTION NOTICE

THE SUBSURFACE INFORMATION AND THE SUBSURFACE INVESTIGATION ON WHICH IT IS BASED WERE MADE FOR THE PURPOSE OF STUDY, PLANNING, AND DESIGN, AND NOT FOR CONSTRUCTION OR PAY PURPOSES. THE VARIOUS FIELD BORING LOGS, ROCK CORES, AND SOIL TEST DATA AVAILABLE MAY BE REVIEWED OR INSPECTED IN RALEIGH BY CONTACTING THE N. C. DEPARTHMENT OF TRANSPORTATION, GEOTECHNICAL ENGINEERING UNIT AT (919) 250-4088. NEITHER THE SUBSURFACE PLANS AND REPORTS, NOR THE FIELD BORING LOGS, ROCK CORES, OR SOIL TEST DATA ARE PART OF THE CONTRACT.

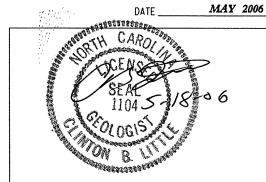
GENERAL SOIL AND ROCK STRATA DESCRIPTIONS AND INDICATED BOUNDARIES ARE BASED ON A GEOTECHNICAL INTERPRETATION OF ALL AVAILABLE SUBSUIFFACE DATA AND MAY NOT NECESSARILY REFLECT THE ACTUAL SUBSUIFFACE CONDITIONS BETWEEN BORINS'O RETWEEN SAMPLED STRATA WITHIN THE BOREHOLE. THE LABORATORY SAMPLE DATA AND THE IN SITU (IN-PLACE) TEST DATA CAN BE RELIED ON ONLY TO THE DEGREE OF RELIABILITY INHERENT IN THE STANDARD TEST METHOD. THE OBSERVED WATER LEVELS OR SOIL MOSITURE CONDITIONS NOICATED IN THE SUBSUIFFACE INVESTIGATIONS ARE AS RECORDED AT THE TIME OF THE INVESTIGATION. THESE WATER LEVELS OR SOIL MOSITURE CONDITIONS MOSITOR CONDITIONS TO CLUMBITE CONDITIONS INCLUDING TEMPERATURES, PRECIPITATION, AND WIND, AS WELL AS OTHER NON-CLIMATIC FACTORS.

THE BIDDER OR CONTRACTOR IS CAUTIONED THAT DETAILS SHOWN ON THE SUBSURFACE PLANS ARE PRELIMINARY ONLY AND IN MANY CASES THE FINAL DESIGN DETAILS ARE DIFFERENT, FOR BIDDING AND CONSTRUCTION PURPOSES, REFER TO THE CONSTRUCTION PLANS AND DOCUMENTS FOR FINAL DESIGN INFORMATION ON THIS PROJECT, THE DEPARTMENT DOES NOT WARRANT OR GUARANTEE THE SUFFICIENCY OR ACCURACY OF THE INVESTIGATION MADE, NOR THE INTERPRETATIONS MADE, OR OPINION OF THE DEPARTMENT AS TO THE TYPE OF MATERIALS AND COMPITIONS TO BE ENCOUNTERED. THE BIDDER OR CONTRACTOR IS CAUTIONED TO MAKE SUCH INDEPENDENT SUBSURFACE INVESTIGATIONS AS HE DEEMS NECESSARY TO SATISFY HIMSELF AS TO CONDITIONS TO BE ENCOUNTERED ON THIS PROJECT. THE CONTRACTOR SHALL HAVE NO CLAIM FOR ADDITIONAL COMPENSATION OR FOR AN EXTENSION OF TIME FOR ANY REASON RESULTING FROM THE ACTUAL CONDITIONS ENCOUNTERED AT THE SITE DIFFERING FROM THOSE INDICATED IN THE SUBSURFACE INFORMATION.

	J.K. STICKNEY	1"
	C.L. SMITH	
·	K. WISE	

INVESTIGATED (BY J.E. BEVERLY	
CHECKED BY	C.B. LITTLE	
SUBMITTED BY	C.B. LITTLE	

PERSONNEL



NOTE - THE INFORMATION CONTAINED HEREIN IS NOT IMPLIED OR GUARANTEED BY THE N.C. DEPARTMENT OF TRANSPORTATION AS BEING ACCURATE NOR IT IS CONSIDERED TO BE PART OF THE PLANS, SPECIFICATIONS, OR CONTRACT FOR THE PROJECT.

NOTE - BY HAVING REQUESTED THIS INFORMATION THE CONTRACTOR SPECIFICALLY WAIVES ANY CLAIMS
FOR INCREASED COMPENSATION OR EXTENSION OF TIME BASED ON DIFFÉRENCES BETWEEN THE
CONDITIONS INDICATED HEREIN AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

DIVISION OF HIGHWAYS

GEOTECHNICAL ENGINEERING UNIT

SUBSURFACE INVESTIGATION

The content of the	SOIL AND ROCK LEGEND, TERMS, SYMBOLS, AND ABBREVIATIONS
## 15 CONTROL OF THE	
Miles Mile	HARD ROUS PARTITION OF PARTICLES SIZES FRONT FIRE 10 COHSE. LES ARE ALL APPROXIMATELY THE SAME SIZE, (ALSO UNIFORM PARTICLES OF TWO OR MORE SIZES. ACLUVIUM (ALLUV) - SOILS THAT HAVE BEEN TRANSPORTED BY WATER. AGUIFER - A WATER BEARING FORMATION OR STRATA. ARENACEOUS - APPLIED TO ACL ROCKS OR SUBSTANCES COMPOSED OF CLAY MINERALS, AGUIFED SIZES. ARENACEOUS - APPLIED TO ALL ROCKS OR SUBSTANCES COMPOSED OF CLAY MINERALS, AGUIFED SIZES. OR HAVING A NOTABLE PROPORTION OF CLAY IN THEIR COMPOSITION, AS SHALE SLOTE FTO
Company Comp	ALOGICAL COMPOSITION ARTESIAN - GROUND WATER THAT IS UNDER SUFFICIENT PRESSURE TO RISE ABOVE THE LEVEL CRYSTALLINE ROCK (CR) FINE TO COARSE GRAIN IGNEOUS AND METAMORPHIC ROCK THAT WHICH IT IS ENCOUNTERED, BUT WHICH DOES NOT NECESSARILY RISE TO OR ABOVE THE GROUND SUFFACE. GROUND SUFFACE. GROUND SUFFACE. CALCAREOUS (CALC) - SOILS THAT CONTAIN APPRECIABLE AMOUNTS OF CALCIUM CARBONATE.
Part	COMPRESSIBILITY NON-CRYSTALLINE ROCK (NCR) LIQUID LIMIT LESS THAN 31 LIQUID LIMIT GREATER THAN 50 COASTAL PLAIN COASTAL PLAIN SEDIMENTARY ROCK THAT WOULD YELLO SPT REFUSAL IF TESTED, ROCK TYPE LIQUID LIMIT GOVAL TO 31-50 LIQUID LIMIT GREATER THAN 50 COASTAL PLAIN SEDIMENTARY ROCK SEDIMENTARY ROCK THAT WOULD YELLO SPT REFUSAL IF TESTED, ROCK TYPE INCLUDES PHYLLITE, SLATE, SANDSTONE, ETC. COASTAL PLAIN SEDIMENTARY ROCK SEDIMENTARY ROCK THAT WOULD YELLO SPT REFUSAL IF TESTED, ROCK TYPE SEDIMENTARY ROCK THAT WOULD YELLO SPT REFUSAL IF TESTED, ROCK TYPE SEDIMENTARY ROCK THAT WOULD YELLO SPT REFUSAL IF TESTED, ROCK TYPE SEDIMENTARY ROCK THAT WOULD YELLO SPT REFUSAL IF TESTED, ROCK TYPE SEDIMENTARY ROCK THAT WOULD YELLO SPT REFUSAL IF TESTED, ROCK TYPE SEDIMENTARY ROCK THAT WOULD YELLO SPT REFUSAL IF TESTED, ROCK TYPE SEDIMENTARY ROCK THAT WOULD YELLO SPT REFUSAL IF TESTED, ROCK TYPE SEDIMENTARY ROCK THAT WOULD YELLO SPT REFUSAL IF TESTED, ROCK TYPE SEDIMENTARY ROCK THAT WOULD YELLO SPT REFUSAL IF TESTED, ROCK TYPE SEDIMENTARY ROCK THAT WOULD YELLO SPT REFUSAL IF TESTED, ROCK TYPE SEDIMENTARY ROCK THAT WOULD YELLO SPT REFUSAL IF TESTED, ROCK TYPE SEDIMENTARY ROCK THAT WOULD YELLO SPT REFUSAL IF TESTED, ROCK TYPE SEDIMENTARY ROCK THAT WOULD YELLO SPT REFUSAL IF TESTED, ROCK TYPE SEDIMENTARY ROCK THAT WOULD YELLO SPT REFUSAL IF TESTED, ROCK TYPE SEDIMENTARY ROCK THAT WOULD YELLO SPT REFUSAL IF TESTED, ROCK TYPE SEDIMENTARY ROCK THAT WOULD YELLO SPT REFUSAL IF TESTED, ROCK TYPE SEDIMENTARY ROCK THAT WOULD YELLO SPT REFUSAL IF TESTED, ROCK TYPE SEDIMENTARY ROCK THAT WOULD YELLO SPT REFUSAL IF TESTED, ROCK TYPE SEDIMENTARY ROCK THAT WOULD YELLO SPT REFUSAL IF TESTED, ROCK TYPE SEDIMENTARY ROCK THAT WOULD YELLO SPT REFUSAL IF TESTED, ROCK TYPE SEDIMENTARY ROCK THAT WOULD YELLO SPT REFUSAL IF TESTED, ROCK TYPE SEDIMENTARY ROCK THAT WOULD YELLO SPT REFUSAL IF TESTED, ROCK TYPE SEDIMENTARY ROCK THAT WOULD YELLO SPT REFUSAL IF TESTED, ROCK TYPE SEDIMENTARY ROCK THAT WOULD YELLO SPT REFUSAL IF TESTED, ROCK TYPE SEDIMENTA
The part of the	SILT - CLAY SILT - CLAY OTHER MATERIAL DIKE - A TABULAR BODY OF IGNEOUS ROCK THAT CUTS ACROSS THE STRUCTURE OF ADJACENT
Description Note And A	FRESH MOCK FRESH, CRYSTALS BRIGHT, FEW JOINTS MAY SHOW SLIGHT STAINING, ROCK RINGS UNDER 3 - 5% TRACE 1 - 10% FRESH HOCK FRESH, CRYSTALS BRIGHT, FEW JOINTS MAY SHOW SLIGHT STAINING, ROCK RINGS UNDER DIP. THE ANGLE AT WHICH A STRATUM OR ANY PLANAR FEATURE IS INCLINED FROM THE
Married Mar	12 - 20%. SOME 20 - 35% VERY SLIGHT ROCK GENERALLY FRESH, JOINTS STAINED, SOME JOINTS MAY SHOW THIN CLAY COATINGS IF OPEN. 20%. HIGHLY 35% AND ABOVE VERY SLIGHT (V SLI.) CRYSTALS ON A BROKEN SPECIMEN FACE SHINE BRIGHTLY, ROCK RINGS UNDER HAMMER BLOWS IF OF A CRYSTALLINE NATURE. DIP DIRECTION OIP AZIMUTH) - THE DIRECTION OR BEARING OF THE HORIZONTAL TRACE OF THE LINE OF DIP, MEASURED CLOCKWISE FROM NORTH.
## SECURITY DOCUMENTS OF THE TOTAL OF THE PERSON OF THE PE	SLIGHT ROCK GENERALLY FRESH, JOINTS STAINED AND DISCOLORATION EXTENDS INTO ROCK UP TO SIDER HOLE IMMEDIATELY AFTER DRILLING (SLI.) 1 INCH. OPEN JOINTS MAY CONTAIN CLAY. IN GRANITOID ROCKS SOME OCCASIONAL FELDSPAR SIDES RELATIVE TO ONE ANOTHER PARALLEL TO THE FRACTURE.
## PACE OF THE PAC	MODERATE SIGNIFICANT PORTIONS OF ROCK SHOW DISCOLORATION AND MEATHERING EFFECTS. IN FLOAT - ROCK FRAGMENTS ON SURFACE NEAR THEIR ORIGINAL POSITION AND DISCOLORED FROM
PRIMARY COL. 1955 COLVENTION COLVENTION	DULL SOUND UNDER HAMMER BLOWS AND SHOWS SIGNIFICANT LOSS OF STRENGTH AS COMPARED WITH FRESH ROCK. MODERATELY ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED IN CRANITORD PROCESS OF STRENGTH AS COMPARED THE STREAM. MODERATELY ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED IN CRANITORD PROCESS OF STREAM.
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## 10 19 17	S - BULK SAMPLE SEVERE ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED, ROCK FABRIC CLEAR AND EVIDENT BUT REDUCED
September Sept	SS - 9PLIT SPOON EXTENT. SOME FRAGMENTS OF STRONG ROCK USUALLY REMAIN, ITS LATERAL EXTENT.
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TEXTURE OR GRAIN SIZE U.S. STO. SEVEN SIZE 4 10 49 69 209 279 FOR DESTRICTION OF SUBJECT SIZE 5 20 20 20 8.00 SILL CLAY GRAIN SIZE 5 20 8.00 SILL CLAY 5 20 8.	PIEZOMETER RT - RECOMPACTED TRIAXIAL SAMPLE SAMPLE SOLTIERE COMPLETE ROCK REDUCED TO SOIL, ROCK FABRIC NOT DISCERNIBLE, OR DISCERNIBLE ONLY IN SMALL AND SAMPLE SAMPLE SOLTIERE COMPLETE ROCK REDUCED TO SOIL, ROCK FABRIC NOT DISCERNIBLE ONLY IN SMALL AND SAMPLE SCATTERED CONCENTRATIONS, QUARTZ MAY BE PRESENT AS DIKES OR STRINGERS, SAPROLITE IS ROCK QUALITY DESIGNATION (ROD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF
SOUNDING FROM SOUNDING FRO	INSTALLATION CBR - CALIFORNIA BEARING ROCK HARDNESS RATIO SAMPLE ROCK HARDNESS EXPRESSED AS A PERCENTAGE,
BOULDER COBBLE GRAVEL COASE FINE SAID BY WITH CORPORT OF PICK, QUIES DR GROUPS TO AS MAND SELLT TO THE MAND SECURISE, CAR BE STANDARD BY WITH CORPORT OF PICK, QUIES DR GROUPS TO AS MAND SELLES. THE MAND AND SECURISE CAR BE EXCAVATED IN SMALL, CHIST OF PICK, QUIES DR GROUPS TO AS MAND SELLES. THE MAND AND SECURISE CAR BE EXCAVATED IN SMALL, CHIST OF PICK, QUIES DR GROUPS TO AS MAND SECURISE CAR BE EXCAVATED IN SMALL, CHIST OF PICK, QUIES DR GROUPS TO AS MAND SECURISE, CAR BE EXCAVATED IN SMALL, CHIST OF PICK, QUIES DR GROUPS TO AS MAND SECURISE, CAR BE EXCAVATED IN SMALL, CHIST OF PICK, QUIES DR GROUPS TO AS MAND SECURISE, CAR BE EXCAVATED IN SMALL, CHIST OF PICK, QUIES DR GROUPS TO AS MAND SECURISE, CAR BE EXCAVATED IN SMALL, CHIST OF PICK, QUIES DR GROUPS TO AS MAND SECURISE, CAR BE EXCAVATED IN SMALL, CHIST OF PICK, QUIES BY MAND BLOW OF THE PICK, MORE AND SECURISE, CAR BE EXCAVATED IN SMALL, CHIST OF PICK, DIGHT OF A RECORDER TO A SECURISE CAR BE EXCAVATED IN SMALL, CHIST OF PICK, DIGHT OF THE MAND SECURISE. SOIL MOSTURE CONTENT MODMOCRATELY V VERY VERY CAR BE SMALLDED BY WIRTE OR PICK, QUIES BY RORD BLOW OF THE MAND MAND SECURISE. CAR BE EXCAVATED IN SMALL, CHIST OF PICK, BUILDING SECURISE. CAR BE EXCAVATED IN SMALL, CHIST OF PICK, BUILDING SECURISE. TO THE MAND SECURISE. CAR BE EXCAVATED IN SMALL, CHIST OF PICK, BUILDING SECURISE. CAR BE EXCAVATED IN SMALL, CHIST OF PICK, BUILDING SECURISE. CAR BE EXCAVATED IN SMALL, CHIST OF PICK, BUILDING SECURISE. CAR BE EXCAVATED IN SMALL, CHIST OF PICK, BUILDING SECURISE. CAR BE EXCAVATED IN SMALL, CHIST OF PICK, BUILDING SECURISE. CAR BE EXCAVATED IN SMALL, CHIST OF PICK, BUILDING SECURISE. CAR BE EXCAVATED IN SMALL, CHIST OF PICK, BUILDING SECURISE. CAR BE EXCAVATED IN SMALL, CHIST OF PICK, BUILDING SECURISE. CAR BE EXCAVATED IN SMALL, CHIST OF PICK, BUILDING SECURISE. CAR BE EXCAVATED IN SMALL, CHIST OF PICK, BUILDING SECURISE. CAR BE EXCAVATED IN SMALL, CHIST OF PICK, BUILDING SECURISE. CAR BE EXCAVATED IN SMALL, CHIST OF PICK, BUILDING SECURISE. CAR BE E	VERY HARD CANNOT BE SCRATCHED BY KNIFE OR SHARP PICK, BREAKING OF HAND SPECIMENS REQUIRES SAPJ - RESIDUAL SOIL THAT RETAINS THE RELIC STRUCTURE OR FABRIC OF THE
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GATTERBERG LIMITS DESCRIPTION	NP - NON PLASTIC ORG ORGANIC PMT - PRESSUREMETER TEST MEDIUM CAN BE GROOVED OR GOUGED 0.05 INCHES DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT. CAN BE EXCAVATED IN SMALL CHIPS TO PEICES INCH MAXIMUM SIZE BY HARD BLOWS OF THE POINT OF A GEOLOGIST'S PICK. MEDIUM CAN BE GROOVED OR GOUGED 0.05 INCHES DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT. CAN BE EXCAVATED IN SMALL CHIPS TO PEICES INCH MAXIMUM SIZE BY HARD BLOWS OF THE POINT OF A GEOLOGIST'S PICK. STANDARD PENETRATION TEST (PENETRATION TEST (PENETRA
FRACE - FRACTURED, FRACTURES SLI SLIGHTLY PLASTIC RANGE OPTIMUM MOISTURE OPTIMUM MOISTURE SHINKAGE LIMIT LIOUD LIMIT SEMISOLID; REQUIRES DRYING TO A FRACTURED, FRACTURES SULL. SLIGHTLY SCHORTS THAN 10 EXPRESSER SA PERCENTAGE. TOTAL LENGTH OF STRATA AND EXPRESSED AS A PERCEN	SD SAND, SANDY FROM CHIPS TO SEVERAL INCHES IN SIZE BY MODERATE BLOWS OF A PICK POINT, SMALL, THIN STRAIG CORE RECOVERY (SREC.) - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY TOTAL LENGTH
PL PLASTIC LIMIT PRACTURE SPACING BEDDING IERM SPACING WORE THAN 10 FEET HAMMER TYPE: SPACING WORE THAN 10 FEET HICKLY BEDDED 1.5 - 4 FEET HICKLY BEDDED	TCR - TRICONE REFUSAL SOFT OR MORE IN THICKNESS CAN BE BROKEN BY FINGER PRESSURE. CAN BE SCRATCHED READILY BY FINGERNAIL. TOTAL LENGTH OF STRATA AND EXPRESSED AS A PERCENTAGE.
OM OPTIMUM MOISTURE - MOIST - (M) SOLID; AT OR NEAR OPTIMUM MOISTURE SL SHRINKAGE LIMIT - DRY - (D) REQUIRES ADDITIONAL WATER TO ATTAIN OPTIMUM MOISTURE - DRY - (D) REQUIRES ADDITIONAL WATER TO ATTAIN OPTIMUM MOISTURE - DRY - (D) REQUIRES ADDITIONAL WATER TO ATTAIN OPTIMUM MOISTURE - DRY - (D) REQUIRES ADDITIONAL WATER TO ATTAIN OPTIMUM MOISTURE - DRY - (D) REQUIRES ADDITIONAL WATER TO ATTAIN OPTIMUM MOISTURE - DRY - (D) REQUIRES ADDITIONAL WATER TO ATTAIN OPTIMUM MOISTURE - DRY - (D) REQUIRES ADDITIONAL WATER TO ATTAIN OPTIMUM MOISTURE - DRY - (D) REQUIRES ADDITIONAL WATER TO ATTAIN OPTIMUM MOISTURE - DRY - (D) REQUIRES ADDITIONAL WATER TO ATTAIN OPTIMUM MOISTURE - DRY - (D) REQUIRES ADDITIONAL WATER TO ATTAIN OPTIMUM MOISTURE - DRY - (D) REQUIRES ADDITIONAL WATER TO ATTAIN OPTIMUM MOISTURE - DRY - (D) REQUIRES ADDITIONAL WATER TO ATTAIN OPTIMUM MOISTURE - DRY - (D) REQUIRES ADDITIONAL WATER TO ATTAIN OPTIMUM MOISTURE - DRY - (D) RETURN TO A FEET THICKLY BEDDED TO A	TERM THICKNESS
- DRY - (D) REQUIRES ADDITIONAL WATER TO ATTAIN OPTIMUM MOISTURE - DRY - (D) REQUIRES ADDITIONAL WATER TO ATTAIN OPTIMUM MOISTURE - DRY - (D) BK-51 - BC-00TINUOUS FLIGHT AUGER CORE SIZE: CLOSE OLIS TO 1 FEET THICKLY LAMINATED OLOGO FEET THICKLY	ING TOULS: X AUTOMATIC MANUAL MANUAL MORE THAN 10 FEET MANUAL MORE THAN 10 FEET MORE THAN 1
DI ACTICITY INDIRATION	**CONTINUOUS FLIGHT AUGER CORE SIZE: CLOSE 0.16 TO 1 FEET VERY THINLY BEDDED 0.03 * 0.16 FEET NOTES:
CME-45C I THING PHOED FINGER BITS I I-N	ARD FACED FINGER BITS INDURATION
NONPLASTIC 0-5 VERY LOW X CMF-650	INGCARBIDE INSERTS -H
LOW PLASTICITY 6-15 SLIGHT CASING W/ ADVANCER HAND TOOLS: CASING W/ ADVANCER HAND TOOLS: GENTLE BLOW BY HAMMER DISINTEGRATES SAMPLE.	ASING WY ADVANCER HAND TOOLS: GENTLE BLOW BY HAMMER DISINTEGRATES SAMPLE.
HIGH PLASTICITY 26 OR MORE HIGH PORTABLE HOIST TRICONE STEEL TEETH POST HOLE DIGGER MODERATELY INDURATED GRAINS CAN BE SEPARATED FROM SAMPLE WITH STEEL PROBE; BREAKS EASILY WHEN HIT WITH HAMMER.	TIDOCHATEET TIDOCHATED
COLOR TRICONE TRICONE TRUCONE.	SQUINDING RDD INDURATED GRAINS ARE DIFFICULT TO SEPARATE WITH STEEL PROBE;
MODIFIERS SUCH AS LIGHT, DARK, STREAKED, ETC. ARE USED TO DESCRIBE APPEARANCE. WANE SHEAR TEST EXTREMELY INDURATED SHARP HAMMER BLOWS REQUIRED TO BREAK SAMPLE; SAMPLE BREAKS ACROSS GRAINS.	VANE SHEAR TEST EXTREMELY INDURATED SHARP HAMMER BLOWS REQUIRED TO BREAK SAMPLE;

PROJECT REFERENCE NO.

B-4005

SHEET NO.

2



STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

MICHAEL F. EASLEY P.O. BOX 25201, RALEIGH, N.C. 27611-5201 LYNDO TIPPETT

GOVERNOR

SECRETARY

May 15, 2006

STATE PROJECT: 33373.1.1 (B-4005)

COUNTY:

Alexander

DESCRIPTION:

Bridge #70 on SR 1331 (Little River Church Rd.) Over Grassy

Creek between SR 1329 and NC 16 (-L- Station 17+97)

SUBJECT:

Geotechnical Report – Bridge Foundation Investigation

This is a proposed bridge replacement for bridge number 70 on SR 1331 over Grassy Creek. The new structure will occupy the same location as the existing structure. The proposed structure will be a 122-foot single span steel girder bridge on a 50-degree skew angle. Recommended clear roadway width is 33 feet and proposed end bent slopes are 1.5:1 (H:V) with class II rip rap slope protection.

Four foundation test borings were performed utilizing both CME-45 and CME-550X drill machines, hollow stem augers, and an automatic drop hammers. The field investigation for this project was conducted in February and March of 2006.

Physiography/Geology

The project area is located in rural north central Alexander County. The site area is flat to gently sloping with the Grassy Creek floodplain extending to approximately 200 feet in width. The floodplain is a mixture of grassy fields and lightly wooded areas. Geologically the area is underlain by Inner Piedmont block Toluca Granite.

Site specific soils noted during our investigation were roadway fill, alluvial, and residual in nature. Roadway fill soils associated with Little River Church Rd. consist of very loose to loose micaceous clayey sandy silt (A-2-4) and very soft to medium stiff silty sandy clay (A-6). Alluvium is comprised of medium stiff to stiff sandy silt (A-4) and medium dense silty sand (A-2-4). Residual soils were found as loose to very dense micaceous sand (A-2-4, A-1-b).



2

Foundation Materials

End Bent 1:

Two borings were performed northwest of Grassy Creek and encountered 12 to 15 feet of roadway fill consisting of very soft to medium stiff brown silty sandy clay (A-6). Underlying fill between elevation 1119 to 1122 feet alluvial soil is encountered and consists of 3 to 5 feet of very loose brown silty sand (A-2-4). Residual soils are encountered next around elevation 1116.5 feet and consist of 2 to 3.5 feet of medium dense brown silty sand (A-1-b). Weathered rock lies at the base of residual soil. Boring EB1-B achieved auger refusal on crystalline rock.

The below elevation data delineates the boundaries for weathered and rock at each boring location:

Boring Location	Weathered Rock Elev. (feet)	Rock Elev. (feet)
EB1-A	1114.75	N/A
EB1-B	1112.85	1112.18

End Bent 2:

Two borings performed southeast of the creek encountered 13 to 14 feet of roadway fill. Fill material consists of very loose to loose brown micaceous clayey silty sand (A-2-4). Elevation 1119.5 feet marks the underlying alluvial boundary and the soil consists of 3 to 5 feet of medium stiff brown-gray sandy silt (A-4). Below alluvium between elevations 1114 and 1116 feet residual soil is encountered. Residual soil consists of 8 to 10.5 feet of loose to very dense brown-gray micaceous silty sand (A-2-4, A-1-b). Weathered rock lies at the base of residual soil. Crystalline rock was not encountered in either of these borings.

The below elevation data delineates the boundaries for weathered rock at each boring location:

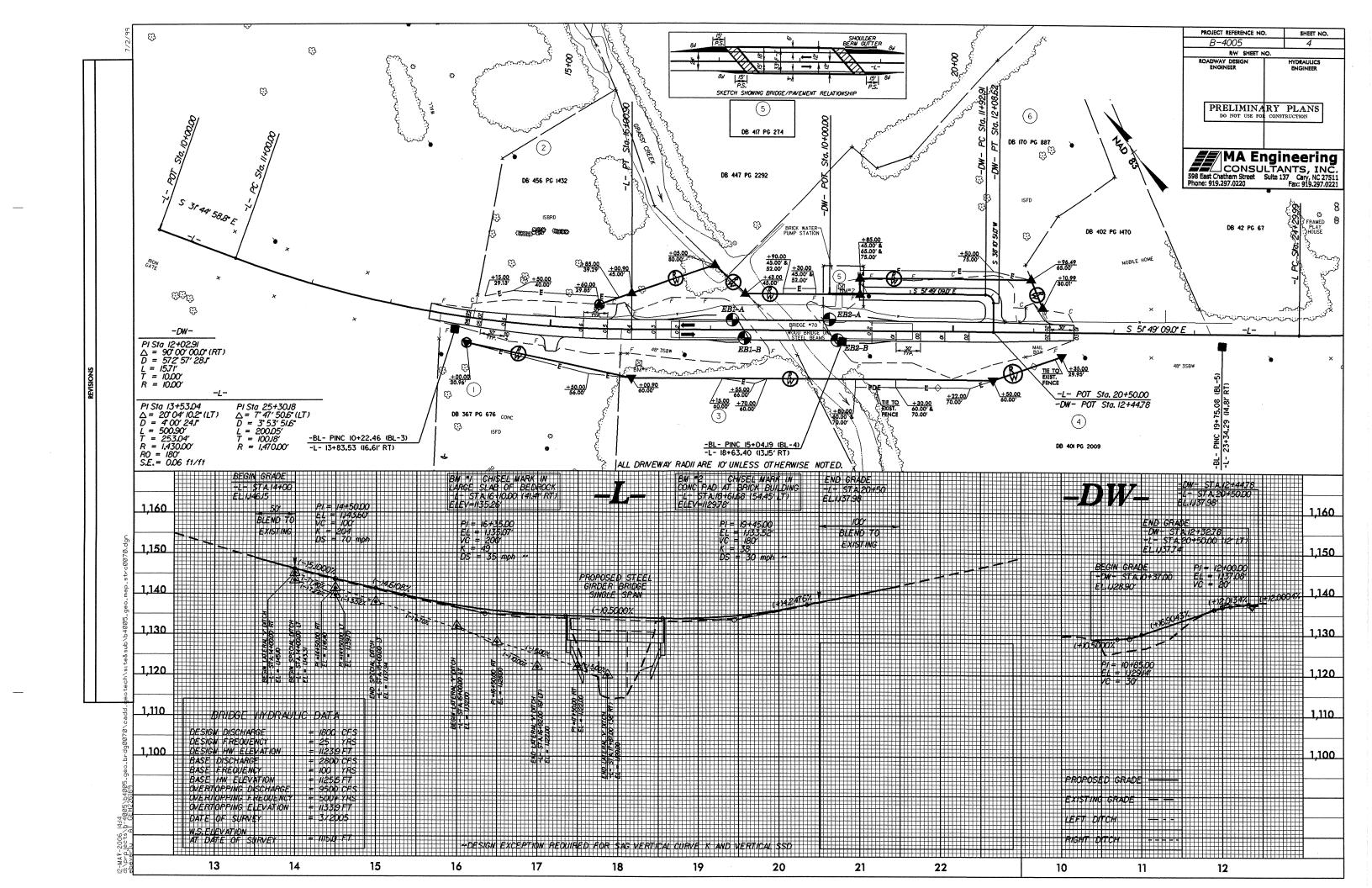
Boring Location	Weathered Rock Elev. (feet)	Rock Elev. (feet)
EB2-A	1103.40	N/A
EB2-B	1108.20	N/A

Groundwater

Based on groundwater measurements made at each boring location, and the water surface elevation of the creek, we determined that natural groundwater should be in the vicinity of elevation 1114 to 1116 feet.

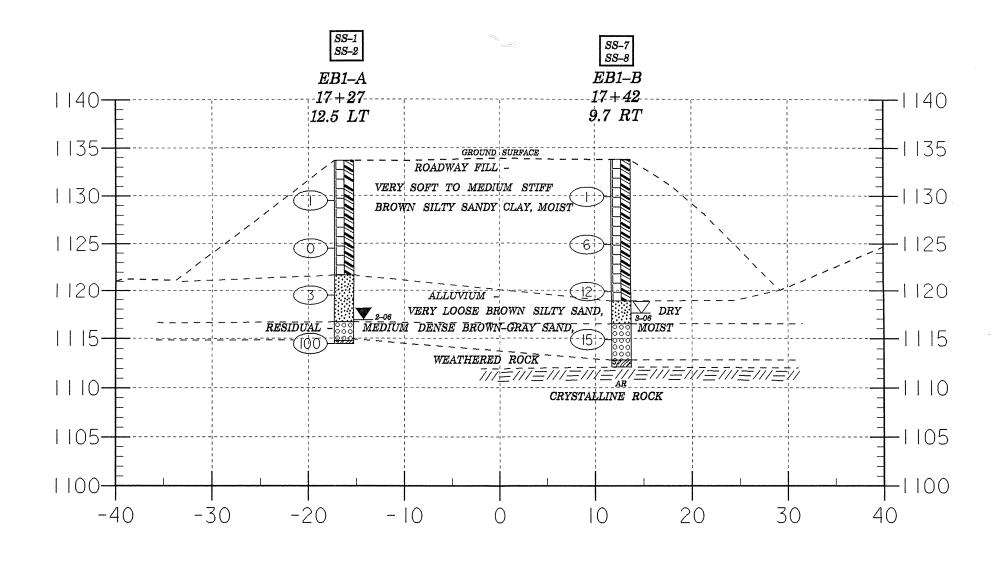
Respectfully submitted,

J.E. Beverly, Project Geologist

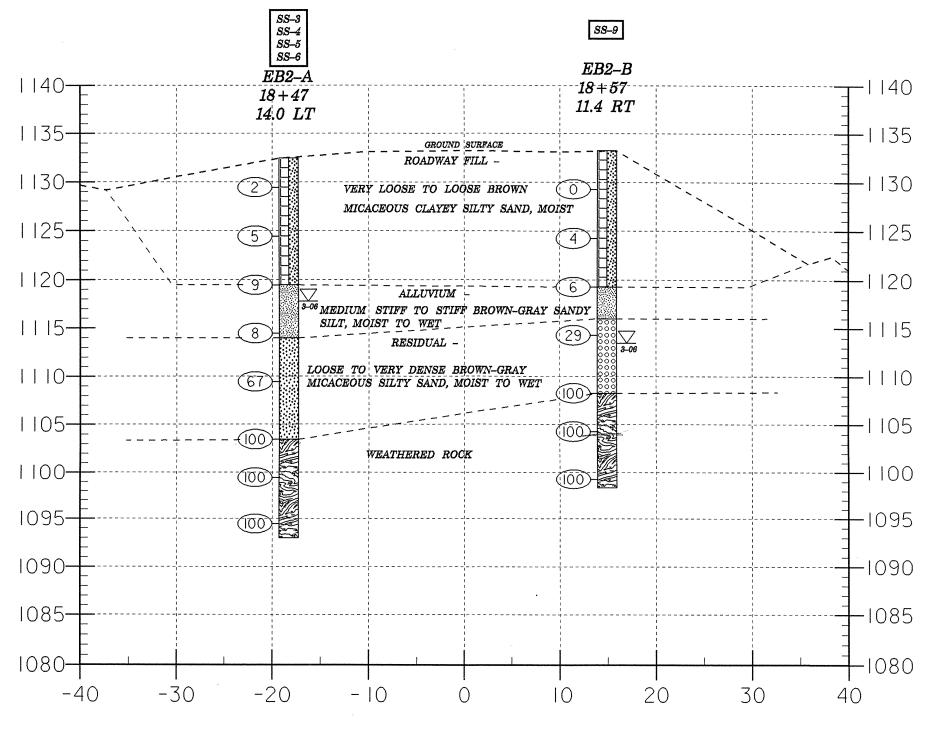


PROJECT REF. NO.	SHEET NO	TOTAL SHEETS
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B-4005	5	1 12 1

SECTION THROUGH EB1-A & EB1-B ALONG SKEW

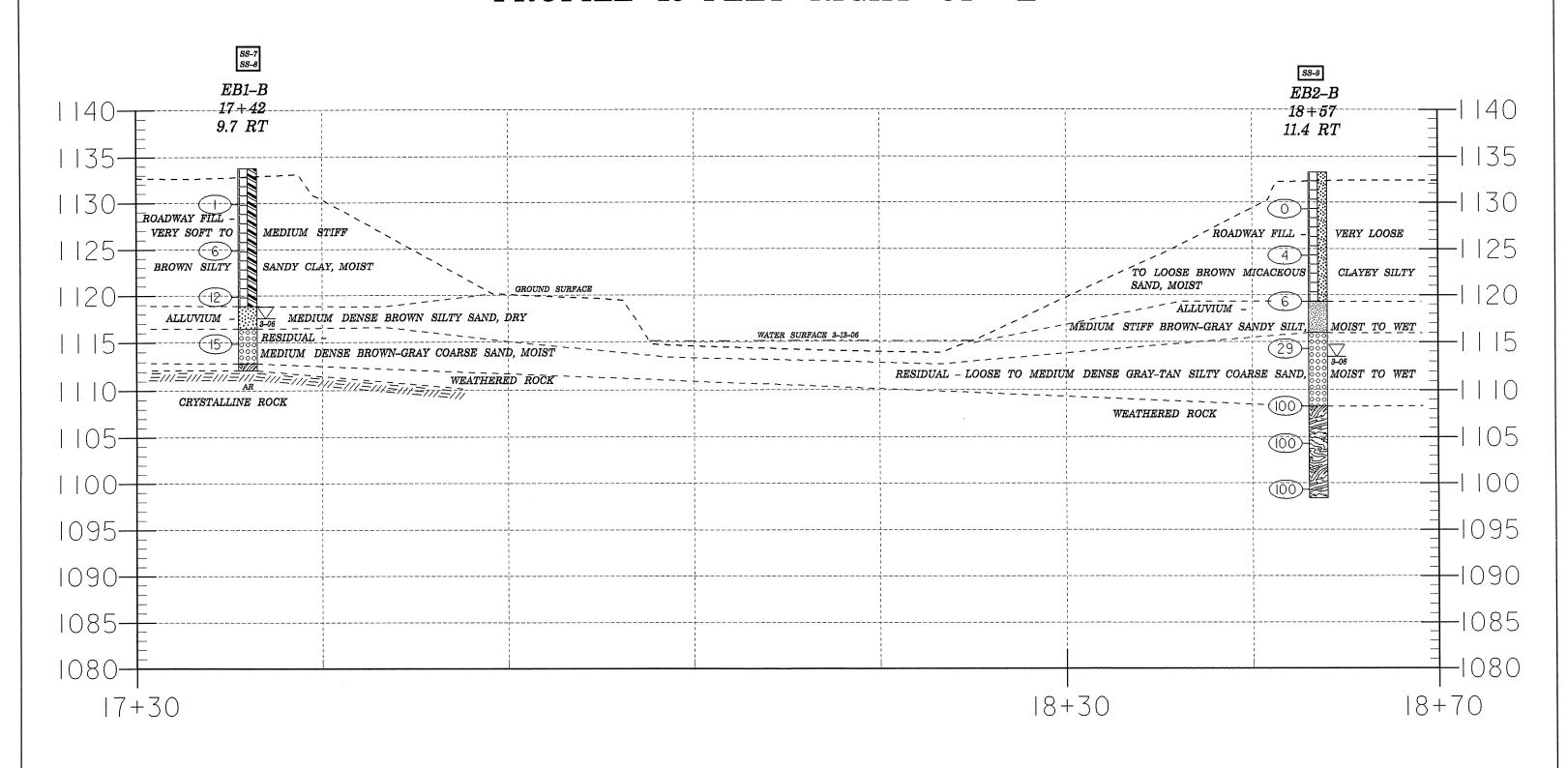


SECTION THROUGH EB2-A & EB2-B ALONG SKEW



PROJECT REF. NO. SHEET NO. TOTAL SHEETS
B-4005 7 12

PROFILE 13 FEET RIGHT OF -L-



NORTH CAROLINA DEPARTMENT OF TRANSPORTATION GEOTECHNICAL UNIT BORING LOG

						GEOT	ECHN	ICAL I	JNIT B	ORING	LOG			
PROJECT NO 33373.1.1				1	ID B-4005 COUNTY ALEXANDER GEOLOGIST					IST R.W. TODD				
SITE DESC	SITE DESCRIPTION BRIDGE #70 ON SR 1331 C							R GRASSY CREEK					GND WATER	
BORING NO EB1-A					NORTHING 0.00 EASTING 0.00					0 HR N/A				
ALIGNMENT L				I	BORING LOCATION 17+27.000 OFFSET 12.50ft LT					24 HR 16.80ft				
COLLAR	COLLAR ELEV 1133.68ft				ГОТАІ	DEPTH	19.20ft		START DA	TE 2/10/0	6		COMPLETION D.	
DRILL MA	ACHINE C	CME-4	5				DRILL	METHO	D H.S. AL	JGERS			HAMMER TYPE	AUTOMATIC
SURFACE	WATER							TO ROC			T	1	Log EB1-A, Page 1 of 1	
ELEV	DEPTH	1	OW		PEN		BLOWS F			SAMPLE	MOI	ᅵᅵ		ID ROCK
		6in	6in	6in	(ft)	0 2	5 5	50 	75 10	0 NO	MOI	G	DEŞCI	RIPTION
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	_						Ground	Surface						
1133.68 – –	=				 				======		 	EIS	ROADWAY FII	L - VERY SOFT
1120.00	_													Y SANDY CLAY
1130.00	- 4.20	1	1	0	1.0	J1				SS-1	MOIST			
-						1					10.01			
_	- - 9.20	1	0	0	1.0									
_	9.20	'	١	١	1.0	*					иоізт			
-	_								-					
	14.20	1	1	2	1.0	-3			-				ALLUVIUM - '	VERY LOOSE SILTY SAND
	-					X				SS-2	DRY		BROWNS	DILIT SAND
_									100-		-	0000	DESIDIAL M	EDIUM DENSE
1114.48 -	_ 19.20	100	<u> </u>		0.2		TIO FEE	7417.747		/		2000		GRAY SAND
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NORTH CAROLINA DEPARTMENT OF TRANSPORTATION GEOTECHNICAL UNIT BORING LOG

PROJECT NO 33373.1.1 ID B-4005 COUNTY ALEXANDER GEOLOGIST J.K. STICKNEY SITE DESCRIPTION BRIDGE #70 ON SR 1331 OVER GRASSY CREEK GND WATER **BORING NO EB1-B** NORTHING 0.00 EASTING 0.00 0 HR 16.20ft ALIGNMENT L **BORING LOCATION 17+42.000** OFFSET 9.70ft RT 24 HR N/A COLLAR ELEV 1133.78ft TOTAL DEPTH 21.60ft **START DATE 3/10/06** COMPLETION DATE 03/10/06 DRILL MACHINE CME-550X DRILL METHOD H.S. AUGERS **HAMMER TYPE AUTOMATIC** SURFACE WATER DEPTH DEPTH TO ROCK 21.60ft Log EB1-B, Page 1 of 1 **BLOW CT** PEN **BLOWS PER FOOT** SAMPLE V SOIL AND ROCK DEPTH **ELEV** MOI G 6in | 6in | 6in | (ft) NO DESCRIPTION 1133.78 ROADWAY FILL - VERY SOFT TO MEDIUM STIFF BROWN SILTY 1130.00 _____3.90 0 0 1 1.0 -1-SANDY CLAY MOIST SS-7 8.90 2 3 3 1.0 MOIST 6 _13.90 5 6 1.0 DRY ALLUVIUM - MEDIUM DENSE **BROWN SILTY SAND** MOIST 11 | 1.0 18.90 4 **RESIDUAL - MEDIUM DENSE** SS-8 **BROWN-GRAY COARSE SAND** 1112.18 WEATHERED ROCK - -AUGERREFU\$AL-ON ||ARD - -CRYSTALLINE ROCK AT - - ELEVATION-1112.18 FEET- -

Sheet 8

NORTH CAROLINA DEPARTMENT OF TRANSPORTATION GEOTECHNICAL UNIT BORING LOG

		,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,		GEOTECHNICAL UNIT BORING L										
PROJECT NO 33373.1.1					ID B-4				XANDER	 	GEOLOGIST J.K. STICKNEY			
SITE DESCRIPTION BRIDGE #70					0 ON SR 1331 OVER GRASSY CREEK						GND WATER			
				NORTHING 0.00 EASTING 0.										
ALIGNMENT L					BORING LOCATION 18+47.000 OFFSET 14.									
COLLAR ELEV 1132.45ft				TOTAI	DEPTH	1			TE 3/10/0)6		COMPLETION D.		
DRILL MACHINE CME-550X								H.S. AL	JGERS			HAMMER TYPE	AUTOMATIC	
SURFACE	WATER							TO ROC			·		Log EB2-A, Page 1 of 1	
ELEV	DEPTH	1	-OW (PEN		BLOWS F			SAMPLE	MOI			ID ROCK
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1120.00	F			_		- 						Ē		
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					'''	X				SS-5	M/W		RESIDUAL - MEI	NIIM DENSE TO
	E						S						VERY DENSE	
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_	F							X	[SS-6	MOIST			
_	F													
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eventurasing.	and the second s								<u> </u>	9			WEATHER	RED ROCK
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Moa-	- 33.00	14	4'	59	0.9									
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NORTH CAROLINA DEPARTMENT OF TRANSPORTATION GEOTECHNICAL UNIT RORING LOC

						GEOT	ECHNI	CALL	JNIT BO	DRING	LOG			
PROJECT					D B-4005 COUNTY ALEXANDER GEOLOGIST J.K. STICK							GIST J.K. STICKNE	Y	
			DGE#	#70 O	N SR 1331 OVER GRASSY CREEK								GND WATER	
BORING N				N	NORTHING 0.00 EASTING 0.00								0 HR 19.80ft	
ALIGNME				B	ORIN	G LOCAT	ION 18+	57.000		OFFSET	11.40ft	RT		24 HR N/A
COLLAR				T	OTAI	DEPTH :				TE 3/10/0	6		COMPLETION DA	ATE 03/10/06
DRILL MA				****		~~~			H.S. AU	GERS			HAMMER TYPE	AUTOMATIC
SURFACE	WATER		H N/A .OW (PEN	·	DEPTH			To	· /		Log EB2-B, Page 1 of 1	
ELEV	DEPTH	6in			1	0 2	LOWS P		' I '5 100	SAMPLE	MOI	Õ		ID ROCK
		OIII	OIII	OIII	(ft)	Ť	- J	-	1 1	NO	MOI	G	DESCR	RIPTION
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	-													
	-													
1133.28	_						 -Ground	Surface						***************************************
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1130.00	- 4.00	0	0	0	1.0							Ė	TO LOOSE BROV	NN MICACEOUS
	- 4.00				1.0	\mathbf{x}^{0}					моіѕт	E	CLAYEY S	ILTY SAND
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						<u>*</u>					MOIST	E		
1120.00_	- 14.00	2	2	4	1.0	-t						E		
_	- 14.00	2	_	4	1.0	X-6					м/w		ALLUVIUM - M	IEDILIM STIEE
	_													Y SANDY SILT
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	-						X			SS-9	M/W	0000	DENSE GRA	Y-TAN SILTY E SAND
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M & T Form 503

NORTH CAROLINA DEPARTMENT OF TRANSPORTATION **DIVISION OF HIGHWAY MATERIALS & TESTS UNIT** SOILS LABORATORY

T. I. P. No.	B-4005					
	REPORT ON SAM	IPLES OF	SOILS FOR QU	JALITY	7	
Project	33373.1.1	County	ALEXANDER		Owner	
Date: Sampled		Received	3/15/06		Reported	3/17/2006
Sampled from	BRIDGE	•		Ву	J E BEVER	LY
Submitted by	N WAINAINA				1995	Standard Specifications

728685 TO 728691 5/16/06

TEST RESULTS

Proj. Sample No.	SS-3	SS-4	SS-5	SS-6	SS-7	SS-8
Lab. Sample No.	728685	728686	728687	728688	728689	728690
Retained #4 Sieve %	_	-	18	11	-	28
Passing #10 Sieve %	100	100	73	85	100	60
Passing #40 Sieve %	73	99	52	64	81	34
Passing #200 Sieve %	35	40	25	28	47	9

MINUS NO. 10 FRACTION

SOIL MORTAR - 100%							
Coarse Sand Ret - #60	%	41.2	5.6	40.4	38.6	31.2	59.8
Fine Sand Ret - #270	%	28.6	63.6	30.6	35.4	24.9	27.6
Silt 0.05 - 0.005 mm	%	16.1	16.7	20.9	19.9	11.7	6.6
Clay < 0.005 mm	%	14.1	14.1	8.0	6.0	32.2	6.0
Passing #40 Sieve	%	-	-		-	-	-
LOCATION	%	EB2A	EB2A	EB2A	EB2A	EB1B	EB1B

L. L.	41	28	42	32	30	25
P. I.	7	NP	6	3	14	NP
AÁSHTO Classification	A-2-5(0)	A-4(0)	A-2-5(0)	A-2-4(0)	A-6(3)	A-1-b(0)
Station	18+47	18+47	18+47	18+47	17+42	17+42
OFFSET	14 LT	14 LT	14 LT	14 LT	9.7RT	9.7RT
ALIGNMENT	L	L	L	L	L	L
Depth (Ft)	3.50	13.50	18.50	23.50	4.40	19.40
to	4.50	14.50	19.50	24.50	5.40	20.40

cc: JE BEVERLY Soils File

> Soils Engineer Page 1

M & T Form 503

NORTH CAROLINA DEPARTMENT OF TRANSPORTATION **DIVISION OF HIGHWAY** MATERIALS & TESTS UNIT SOILS LABORATORY

T. I. P. No.	B-4005						
	REPORT ON SA	AMPLES OF	SOILS FO	R QUALIT	Y	~	
Project	33373.1.1	County	ALEXANI	DER	Owner		
Date: Sampled		Received	3/15/06		- Reported	3/17/2006	
Sampled from	BRIDGE	********	***************************************	By	J E BEVER	RLY	
Submitted by	N WAINAINA					Standard Spe	cifications
5/16/06 Proj. Sample No).	SS-9	EST RESU	LTS SS-1	SS-2		
Lab. Sample No		728691		728319	728320		
Retained #4 Si				720319	728320		
Passing #10 Si				98	100		
Passing #40 Si				77	90		
Passing #200 S	ieve %	19		46	22		
		MINU	S NO. 10 FR	ACTION			
SOIL MORTAR	R - 100%						
Coarse Sand				33.3	40.5		
Fine Sand Re				21.6	40.3		
Silt 0.05 - 0.0				15.0	9.2		
Clay < 0.005				30.1	10.0		
Passing #40 Si LOCATION	eve %			-	-		
LOCATION	% 0	LBZ-B					
L. L.		31		33	25		
P. I.		NP	T	15	NP	 	

NP

A-1-b(0)

18+57

11.4RT

L

19.50

20.50

to

AASHTO Classification

Station

OFFSET

Depth (Ft)

ALIGNMENT

15

A-6(3)

EB1-A

L

4.20

5.70

NP

A-2-4(0)

EB1-A

L

14.20

15.70



FIELD SCOUR REPORT

WBS:	33373.1.1	_ TIP:	B-4005	COUNTY: Alexand	ler
DESCRIPTION(1):	Bridge 70 on SI	R 1331 ov	er Grassy Creek		
			EXISTING I	BRIDGE	
Information from:	Field I Othe	nspection r (explain)	X Micr	ofilm (reel	_ pos:)
Bridge No.: Foundation Type:	70 Length	n: <u>106'</u> g with tim	Total Bents:4 ber posts & timber	Bents in Channel:	2 Bents in Floodplain: 2
EVIDENCE OF S Abutments or I		s: Erosion	under bridge betw	veen end bents & interior	bents
Interior Bents:	Erosion behind	interior be	ent one on upstrea	m side in bank.	
Channel Bed:	None noted.				
Channel Bank:	None noted.				
EXISTING SCO Type(3):			om bridge deck.		
Extent(4):	Upstream side	at both er	nd bents (under bri	dge).	
Effectiveness(5):	Fair				
Obstructions(6):	None				

INSTRUCTIONS

- 1 Describe the specific site's location, including route number and body of water crossed.
- 2 Note scour evidence at existing end bents or abutments (e.g. undermining, sloughing, degradations).
- 3 Note existing scour protection (e.g. rip rap).
- 4 Describe extent of existing scour protection.
- 5 Describe whether or not the scour protection appears to be working.
- 6 Note obstructions such as dams, fallen trees, debris at bents, etc.
- 7 Describe the channel bed material based on observation and/or samples. Include any lab results with report.
- 8 Describe the channel bank material based on observation and/or samples. Include any lab results with report.
- **9** Describe the material covering the banks (e.g. grass, trees, rip rap, none).
- 10 Determine the approximate floodplain width from field observation or a topographic map.
- 11 Describe the material covering the floodplain (e.g. grass, trees, crops).
- 12 Use professional judgement to specify if the stream is degrading, aggrading, or static.
- 13 Describe potential and direction of the stream to migrate laterally during the bridge's life (approx. 100 years).
- Give the design scour elevation (DSE) expected over the life of the bridge (approx. 100 years). This elevation can be given as a range across the site, or for each bent. Discuss the relationship between the Hydraulics Unit theoritical scour and the DSE. If the DSE is dependent on scour counter measures, explain (e.g. rip rap armoring on slopes). The DSE is based on the erodability of materials, giving consideration to the influence of joints, foliation, bedding characteristics, % core recovery, % RQD, differential weathering, shear strength, observations at existing structures, other tests deemed appropriate, and overall geologic conditions at the site.

			<u>DES</u>	<u>IGN IN</u>	FORM.	<u>ATION</u>					
Channel B	ed Material(7)	: Rock, bo									
Channel Ba	nk Material(8)	Material(8): Alluvial, loose silty sand (Ref:SS-2)									
Channel E	Bank Cover(9)										
Floodp	lain Width(10)	: 200'									
Floodp	lain Cover(11)	: Grass, tr	ees					<u></u>			
	Stream is(12)	: Ag	grading		Degr	ading	Χ	Sta	tic	_	
Channel Migration	Tendency(13)	: Slight									
Observations ar											
											
DESIGN SCOU	R ELEVATIO	NS(14)				Feet		Mete	ers		
	No sco	our predic	ted.								
		T		T	1	Γ		I	T	1	1
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	h		_								
Comparison of			_								
Hydraulics Unit	i neoretical Sc	cour = Nor	ie					· · · · · · · · · · · · · · · · · · ·			
DOE - IN/A							. , ,				
SOIL ANALYSI	S RESULTS I	FROM CH	IANNEL	BED AN	ID BANK	MATE	RIAL				
Bed or Bank											
Sample No.											
Retained #4					_					-	
Passed #10					-						
Passed #40 Passed #200					 					-	
Coarse Sand										 	
Fine Sand											
Silt											
Clay											
LL											
PI											
AASHTO											
Station										1	
Offset											
Depth _		····			<u> </u>						
										- 1.1	Pavisad 02/07

Template Revised 02/07/0

Reported by:	JKS/JEB/CBL	Date:	4/1/2006
Reported by:	JNO/JED/CDL	Date.	4/1/2000

33373.1.1 (B-4005) ALEXANDER COUNTY BRIDGE # 70 ON SR 1331 OVER GRASSY CREEK

SITE PHOTO



Looking West with creek flowing from right to left.