STATE OF NORTH CAROLINA DIVISION OF HIGHWAYS

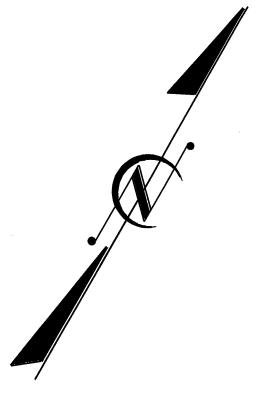


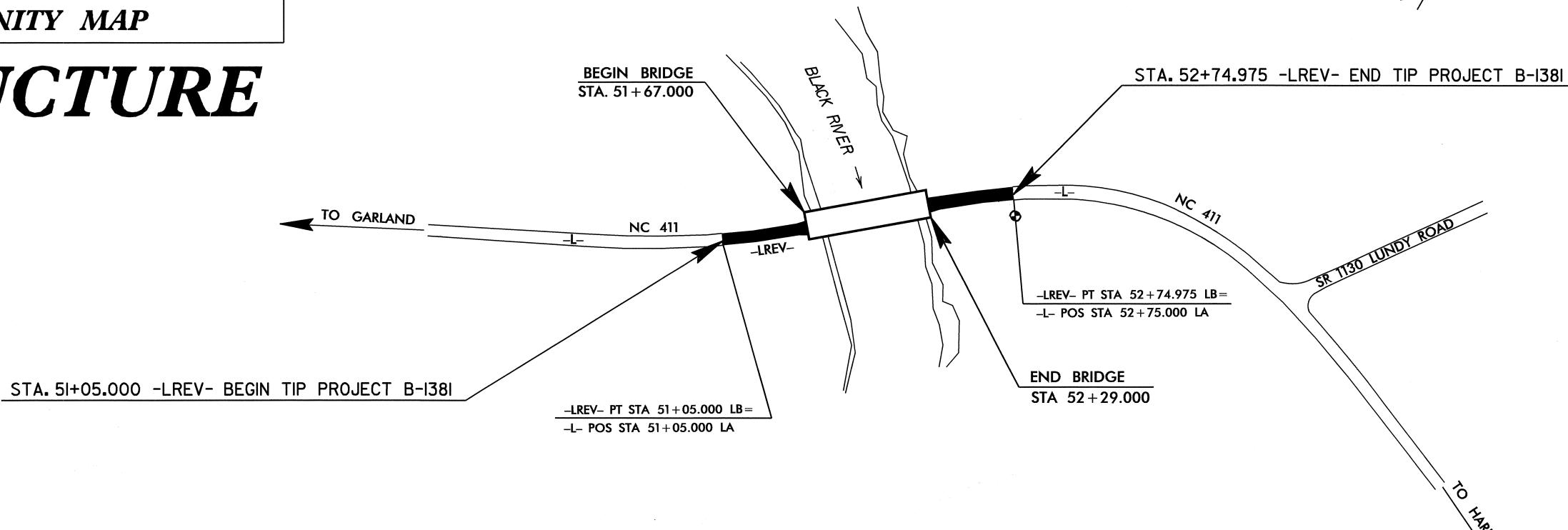
SIAIB	TIB FROMECT REPERENCE NO.	NO.	SHEETS
N.C.	B-1381		
STATE PROJ.NO.	F. A. PROJ. NO.	DESCRIP	rion
32594.1.1	BRSTP-411(1)	P.E.	
32594.2.1	BRSTP-411(1)	R/W &	UTIL.
32594.3.1	BRSTP-411(15)	CON	ST.

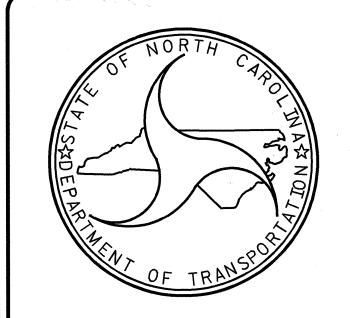
SAMPSON COUNTY

LOCATION: BRIDGE 14 OVER BLACK RIVER AND APPROACHES ON NC 411 (AT CLEAR RUN) WEST OF HARRELLS

TYPE OF WORK: GRADING, PAVING, DRAINAGE, AND STRUCTURE







Roseboro

OFF SITE DETOUR

VICINITY MAP

STRUCTURE

DESIGN DATA

ADT 2007 = 1,600

ADT 2030 = 3,200DHV = 10%

D = 60%

*T = 5 %V = 70 km/h

Clinton (24)

PROJECT

* (TTST 2% + DUAL 3%)

PROJECT LENGTH

LENGTH ROADWAY TIP PROJECT B-1381 = 0.108 km

LENGTH STRUCTURE TIP PROJECT B-1381 = 0.062 km

TOTAL LENGTH OF TIP PROJECT B-1381 = 0.170 km

Prepared in the Office of: **DIVISION OF HIGHWAYS**

1000 Birch Ridge Dr., Raleigh, NC 27610

2002 STANDARD SPECIFICATIONS

LETTING DATE: MARCH 20, 2007

PROJECT ENGINEER

T.G. PAYNE, P.E. PROJECT DESIGN ENGINEER

B.C. HUNT, P.E.

STRUCTURE DESIGN



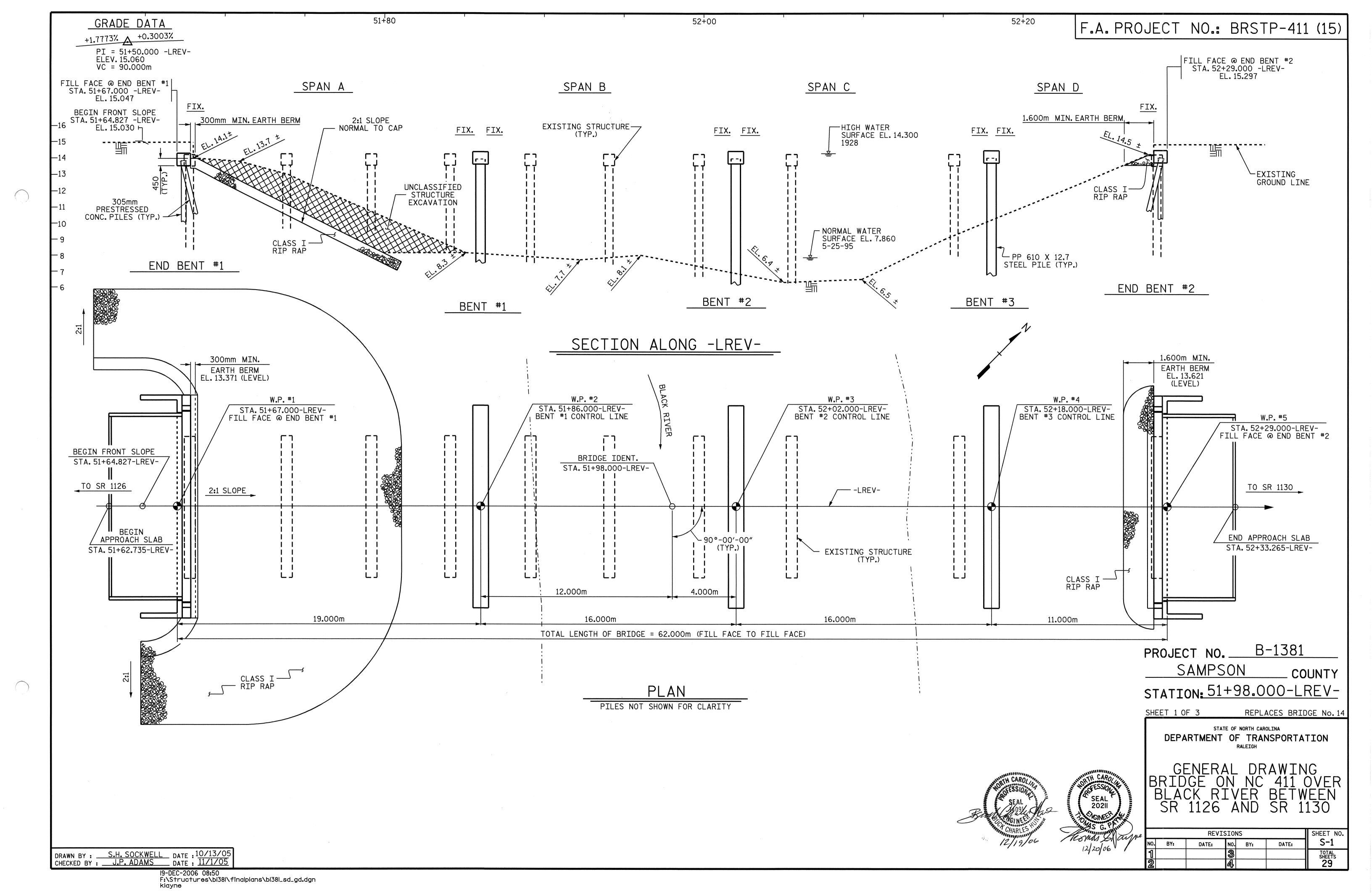
DIVISION OF HIGHWAYS STATE OF NORTH CAROLINA

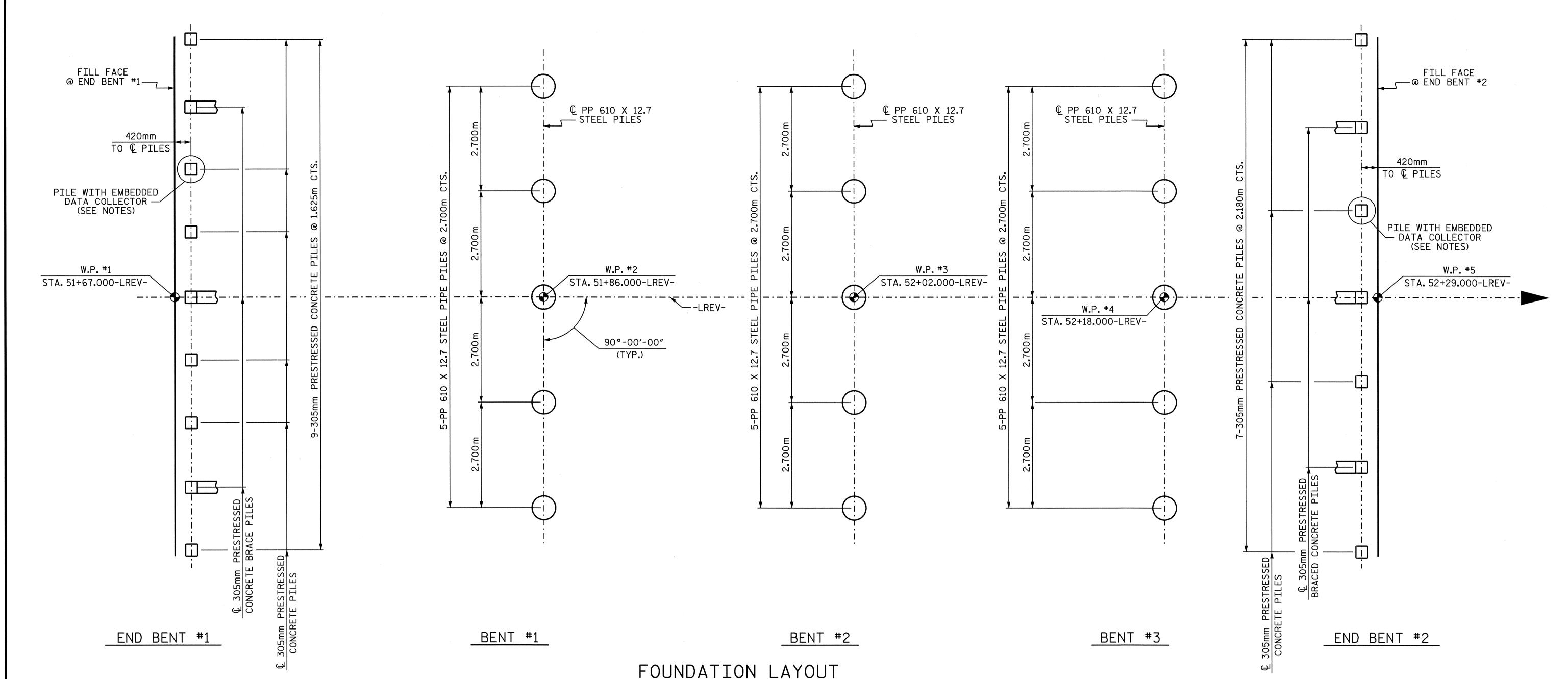
STATE DESIGN ENGINEER

DEPARTMENT OF TRANSPORTATION FEDERAL HIGHWAY ADMINISTRATION

DATE

APPROVED
DIVISION ADMINISTRATOR





DRIVE PILES AT END BENT #1 AND END BENT #2 TO A REQUIRED BEARING CAPACITY OF 900 KILONEWTONS EACH. THE REQUIRED BEARING CAPACITY IS EQUAL TO THE ALLOWABLE BEARING CAPACITY WITH A MINIMUM FACTOR OF SAFETY OF TWO.

IT HAS BEEN ESTIMATED THAT A HAMMER WITH AN EQUIVALENT RATED ENERGY IN THE RANGE OF 31 TO 67 KN-m PER BLOW WILL BE REQUIRED TO DRIVE THE PILES AT END BENT #1 AND END BENT #2. THIS ESTIMATED ENERGY RANGE DOES NOT RELEASE THE CONTRACTOR FROM THE BEARING PILE SPECIAL PROVISION (NOVEMBER 20, 2006).

DRIVE PILES AT BENT #1 TO A REQUIRED BEARING CAPACITY OF 2430 KILONEWTONS. THE REQUIRED BEARING CAPACITY IS EQUAL TO THE ALLOWABLE BEARING CAPACITY WITH A MINIMUM FACTOR OF SAFETY OF TWO PLUS ANY ADDITIONAL CAPACITY TO ACCOUNT FOR DOWN DRAG OR NEGATIVE SKIN FRICTION AND SCOUR.

DRIVE PILES AT BENT #2 TO A REQUIRED BEARING CAPACITY OF 2170 KILONEWTONS. THE REQUIRED BEARING CAPACITY IS EQUAL TO THE ALLOWABLE BEARING CAPACITY WITH A MINIMUM FACTOR OF SAFETY OF TWO PLUS ANY ADDITIONAL CAPACITY TO ACCOUNT FOR DOWN DRAG OR NEGATIVE SKIN FRICTION AND SCOUR.

DRIVE PILES AT BENT #3 TO A REQUIRED BEARING CAPACITY OF 1920 KILONEWTONS. THE REQUIRED BEARING CAPACITY IS EQUAL TO THE ALLOWABLE BEARING CAPACITY WITH A MINIMUM FACTOR OF SAFETY OF TWO PLUS ANY ADDITIONAL CAPACITY TO ACCOUNT FOR DOWN DRAG OR NEGATIVE SKIN FRICTION AND SCOUR.

DRIVE PILES AT BENT #1, BENT #2, AND BENT #3 TO A TIP ELEVATION NO HIGHER THAN -1.000.

IT HAS BEEN ESTIMATED THAT A HAMMER WITH AN EQUIVALENT RATED ENERGY IN THE RANGE OF 31 TO 64 kN-m PER BLOW WILL BE REQUIRED TO DRIVE THE PILES AT BENT #1, BENT #2, AND BENT #3. THIS ESTIMATED ENERGY RANGE DOES NOT RELEASE THE CONTRACTOR FROM THE BEARING PILE SPECIAL PROVISION (NOVEMBER 20, 2006).

THE SCOUR CRITICAL ELEVATION FOR BENT #1, BENT #2, AND BENT #3 IS ELEVATION 3.000. SCOUR CRITICAL ELEVATIONS ARE USED TO MONITOR POSSIBLE SCOUR PROBLEMS DURING THE LIFE OF THE STRUCTURE.

THE BEARING PILE SPECIAL PROVISION (NOVEMBER 20.2006) REPLACES SECTION 450 OF THE 2002 STANDARD SPECIFICATIONS.

GALVANIZED STEEL PILES ARE REQUIRED IN ACCORDANCE WITH THE BEARING PILE SPECIAL PROVISION (NOVEMBER 20, 2006) FOR PILES AT BENT #1, BENT #2, AND BENT #3.

DRIVING PRESTRESSED CONCRETE PILES MAY REQUIRE RESTRIKING OR REDRIVING AT END BENT #1 AND END BENT #2. SEE BEARING PILE SPECIAL PROVISION (NOVEMBER 20, 2006).

DRIVING PIPE PILES MAY REQUIRE RESTRIKING OR REDRIVING AT BENT #1, BENT #2, OR BENT #3. SEE BEARING PILE SPECIAL PROVISION (NOVEMBER 20, 2006).

TESTING THE PRODUCTION 305mm PRESTRESSED CONCRETE PILE CONTAINING EMBEDDED DATA COLLECTORS WITH THE PILE DRIVING ANALYZER (PDA) DURING DRIVING, RESTRIKING OR REDRIVING IS REQUIRED AT END BENT #1 AND END BENT #2. SEE EMBEDDED DATA COLLECTORS SPECIAL PROVISION AND PILE DRIVING ANALYZER SPECIAL PROVISION.

TESTING THE FIRST PRODUCTION PP 610 imes 12.7 STEEL PILE WITH THE PILE DRIVING ANALYZER (PDA) DURING DRIVING. RESTRIKING OR REDRIVING IS REQUIRED AT BENT #1, BENT #2, OR BENT #3. SEE PILE DRIVING SPECIAL PROVISIONS.

PIPE PILE PLATES MAY BE REQUIRED FOR THE PIPE PILES AT BENT #1, BENT #2, AND BENT #3. THE ENGINEER WILL DETERMINE THE NEED FOR PIPE PILE PLATES AFTER DRIVING TEST PILES OR A FEW INITAL PRODUCTION PILES AS DIRECTED BY THE ENGINEER. USE PIPE PILE PLATES WITH A DIAMETER EQUAL TO THE PIPE PILE DIAMETER. SEE THE BEARING PILE SPECIAL PROVISION (NOVEMBER 20, 2006).

A WAITING PERIOD IS NOT REQUIRED BEFORE BEGINNING ANY WORK FOR END BENT CONSTRUCTION AFTER COMPLETION OF THE EMBANKMENT AT EACH END BENT.

B-1381 PROJECT NO. SAMPSON COUNTY STATION: 51+98.000-LREV-

SHEET 2 OF 3

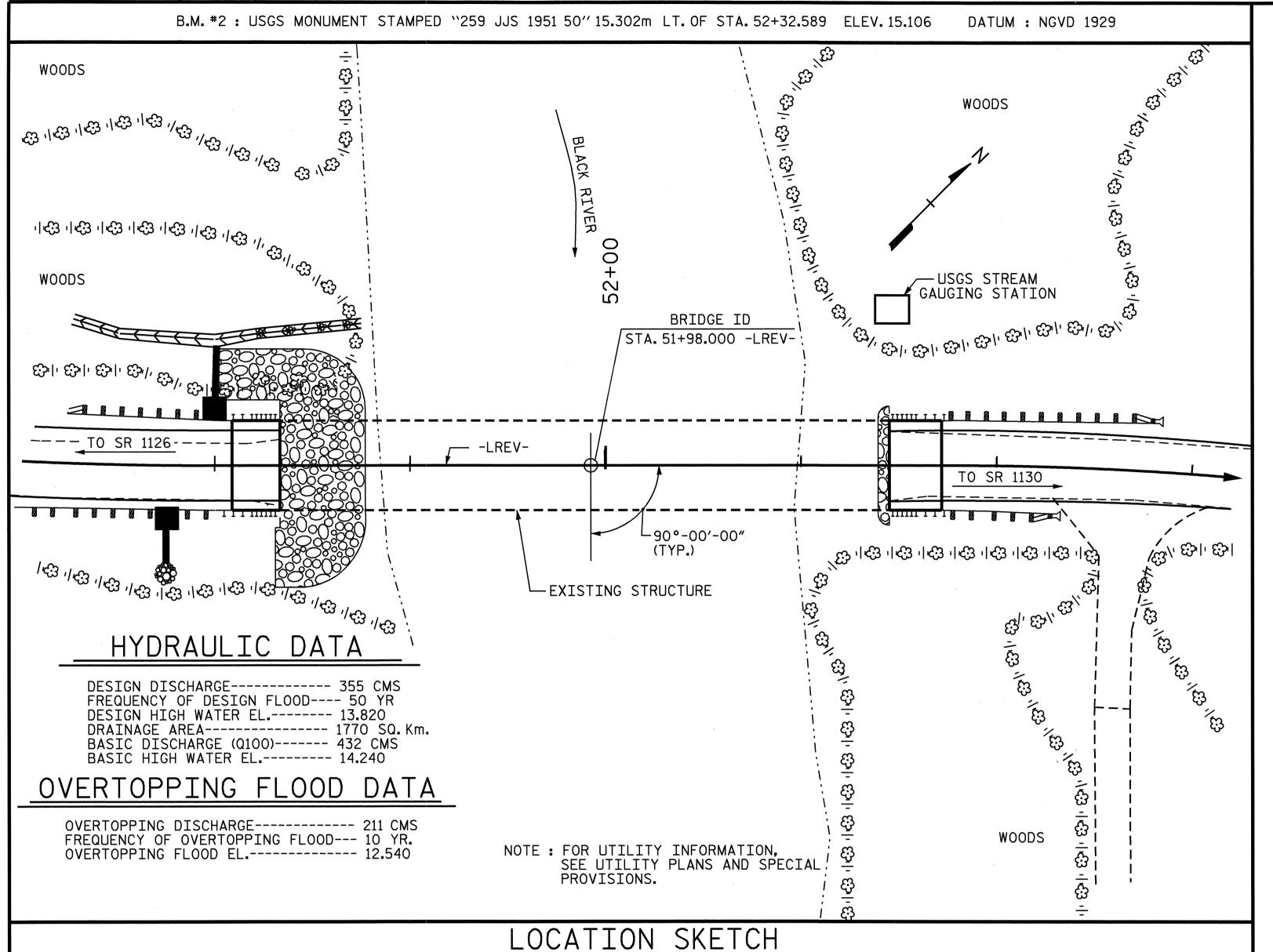
SEAL 20211

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH

> FOUNDATION LAYOUT

REVISIONS SHEET NO. S-2 DATE: DATE: BY: BY: TOTAL SHEETS 29

_ DATE :10/13/05 S.H. SOCKWELL DRAWN BY : DATE: 11/1/05 J.P. ADAMS CHECKED BY :



NOTES

ASSUMED LIVE LOAD = MS18 OR ALTERNATE LOADING, EXCEPT THAT CORED SLAB UNITS IN SPANS B. C. AND D HAVE BEEN DESIGNED FOR MS22.5.

ALL DIMENSIONS ARE IN MILLIMETERS UNLESS OTHERWISE NOTED.

ALL ELEVATIONS ARE IN METERS.

FOR OTHER DESIGN DATA AND GENERAL NOTES, SEE SHEET SNSM.

FOR EROSION CONTROL MEASURES SEE EROSION CONTROL PLANS.

THIS BRIDGE HAS BEEN DESIGNED BY THE STRENGTH DESIGN METHOD AS SPECIFIED IN AASHTO STANDARD SPECIFICATIONS.

THE EXISTING STRUCTURE, 59.740m TOTAL LENGTH, CONSISTING OF 9 SPANS (1 @ 5.480m, 1 @ 5.080m, 2 @ 5.280m, 1 @ 5.180m, 1 @ 5.200m, 1 @ 5.120m, 1 @ 10.780m, AND 1 @ 12.340m); WITH A BRIDGE DECK WIDTH OF 7.700m WITH A REINFORCED CONCRETE DECK ON TIMBER JOISTS AND I-BEAMS ON TIMBER END BENTS AND TIMBER BENTS 1-6, REINFORCED CONCRETE CAP/TIMBER PILES AT BENT 7, AND STEEL CAP/STEEL PILES AT BENT 8 AND LOCATED AT THE PROPOSED SITE SHALL BE REMOVED. EXISTING TIMBER PILES AND A TIMBER RETAINING WALL FROM A PREEXISTING BRIDGE LOCATED AT THE SITE OF THE PROPOSED BRIDGE SHALL ALSO BE REMOVED AS PART OF THE REMOVAL OF EXISTING STRUCTURE PAY ITEM.

INASMUCH AS THE PAINT SYSTEM ON THE EXISTING STRUCTURAL STEEL CONTAINS LEAD, THE CONTRACTOR'S ATTENTION IS DIRECTED TO ARTICLE 107-1 OF THE STANDARD SPECIFICATIONS. ANY COST RESULTING FROM COMPLIANCE WITH APPLICABLE STATE OR FEDERAL REGULATIONS PERTAINING TO HANDLING OF MATERIALS CONTAINING LEAD BASED PAINT SHALL BE INCLUDED IN THE BID FOR "REMOVAL ON EXISTING STRUCTURE".

REMOVAL OF THE EXISTING BRIDGE SHALL BE PERFORMED SO AS NOT TO ALLOW DEBRIS TO FALL INTO THE WATER. THE CONTRACTOR SHALL REMOVE THE BRIDGE AND SUBMIT PLANS FOR DEMOLITION IN ACCORDANCE WITH ARTICLE 402-2 OF THE STANDARD SPECIFICATIONS.

THE MATERIAL SHOWN IN THE CROSS-HATCHED AREA SHALL BE EXCAVATED FOR A DISTANCE OF 9.000m EACH SIDE OF CENTERLINE ROADWAY AS DIRECTED BY THE ENGINEER. THIS WORK WILL BE MEASURED AND PAID FOR AS UNCLASSIFIED STRUCTURE EXCAVATION.

THE SUBSTRUCTURE OF THE EXISTING BRIDGE INDICATED ON THE PLANS IS FROM THE BEST INFORMATION AVAILABLE. SINCE THIS INFORMATION IS SHOWN FOR THE CONVENIENCE OF THE CONTRACTOR, THE CONTRACTOR SHALL HAVE NO CLAIM WHATSOEVER AGAINST THE DEPARTMENT OF TRANSPORTATION FOR ANY DELAYS OR ADDITIONAL COST INCURRED BASED ON DIFFERENCES BETWEEN THE EXISTING BRIDGE SUBSTRUCTURE SHOWN ON THE PLANS AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

ASPHALT WEARING SURFACE IS INCLUDED IN ROADWAY QUANTITY ON ROADWAY PLANS.

THIS STRUCTURE HAS BEEN DESIGNED IN ACCORDANCE WITH HEC 18, "EVALUATING SCOUR AT BRIDGES", NOVEMBER, 1995.

THIS BRIDGE HAS BEEN DESIGNED IN ACCORDANCE WITH THE REQUIREMENTS OF THE AASHTO STANDARD SPECIFICATIONS FOR SEISMIC DESIGN OF HIGHWAY BRIDGES FOR SEISMIC PERFORMANCE CATEGORY A.

THE CONTRACTOR SHALL PROVIDE INDEPENDENT ASSURANCE SAMPLES OF REINFORCING STEEL AS FOLLOWS: FOR PROJECTS REQUIRING UP TO 360,000 kg OF REINFORCING STEEL, ONE 760mm SAMPLE OF EACH SIZE BAR USED, AND FOR PROJECTS REQUIRING OVER 360,000 kg OF REINFORCING STEEL, TWO 760mm SAMPLES OF EACH SIZE BAR USED. THE BARS FROM WHICH THE SAMPLES ARE TAKEN MUST THEN BE SPLICED WITH REPLACEMENT BARS OF THE SIZE AND LENGTH OF THE SAMPLE PLUS A MINIMUM LAP SPLICE OF THIRTY BAR DIAMETERS.

FOR SUBMITTAL OF WORKING DRAWINGS. SEE SPECIAL PROVISIONS.

FOR FALSEWORK AND FORMWORK, SEE SPECIAL PROVISIONS.

THIS BRIDGE SHALL BE CONSTRUCTED USING TOP-DOWN CONSTRUCTION METHODS. TOP-DOWN CONSTRUCTION SHALL PROCEED FROM SPAN D TOWARD SPAN A. A CRANE WILL NOT BE ALLOWED ON SPAN A AT ANY TIME DURING CONSTRUCTION. THE USE OF A TEMPORARY CAUSEWAY OR WORK BRIDGE IS NOT PERMITTED.

FOR CRANE SAFETY, SEE SPECIAL PROVISIONS.

FOR CONSTRUCTION OF SUPERSTRUCTURE, SEE SPECIAL PROVISIONS.

FOR CONSTRUCTION OF SUBSTRUCTURE, SEE SPECIAL PROVISIONS.

UNITED STATES GEOLOGICAL SURVEY HAS A GAUGING STATION LOCATED IN THE PROJECT AREA. THE CONTRACTOR WILL CONTACT GEOFFREY CARTANO AT 919-571-4064 OF USGS AND DOUG WALTERS AT 919-571-4069 OF USGS TWO WEEKS PRIOR TO BEGINNING WORK IN ORDER TO DISCUSS ANY POTENTIAL IMPACTS TO THE GAUGING STATION.

* PILES WITH EMBEDDED DATA COLLECTORS ARE INCLUDED IN THESE TOTALS.

GALVANIZED STEEL PILES ARE REQUIRED IN ACCORDANCE WITH THE BEARING PILE SPECIAL PROVISION (NOVEMBER 20, 2006) FOR PILES AT BENT #1, BENT #2, AND BENT #3.

		1				TO	TAL BILL OF	MATERIA	\L					
	REMOVAL OF EXISTING STRUCTURE	UNCLASSIFIED STRUCTURE EXCAVATION	BRIDGE APPROACH SLABS	PF C	305mm RESTRESSED ONC PILES		PP 610 X 12.7 STEEL PILES	RIP RAP CLASS I	CONSTRUCTION OF SUBSTRUCTURE	CONSTRUCTION OF SUPERSTRUCTURE	PDA TESTING	PDA ASSISTANCE	PIPE PILE PLATES	PILE REDRIVES
	LUMP SUM	CU. METERS	LUMP SUM	NO.	METERS	NO.	METERS	METRIC TONS	LUMP SUM	LUMP SUM	EACH	EACH	EACH	EACH
SUPERSTRUCTURE										LUMP SUM				
END BENT #1		435.0		* 9	76.5			427	LUMP SUM					9
BENT #1						5	92.5		LUMP SUM	·			5	5
BENT #2						5	85.0		LUMP SUM				5	5
BENT #3						5	82 . 5		LUMP SUM				5	5
END BENT #2				* 7	45.5			30	LUMP SUM					7
TOTAL	LUMP SUM	435.0	LUMP SUM	* 16	122.0	15	260.0	457	LUMP SUM	LUMP SUM	4	4	15	31

PROJECT NO. B-1381

SAMPSON COUNTY

STATION: 51+98.000-LREV-

SHEET 3 OF 3

STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION

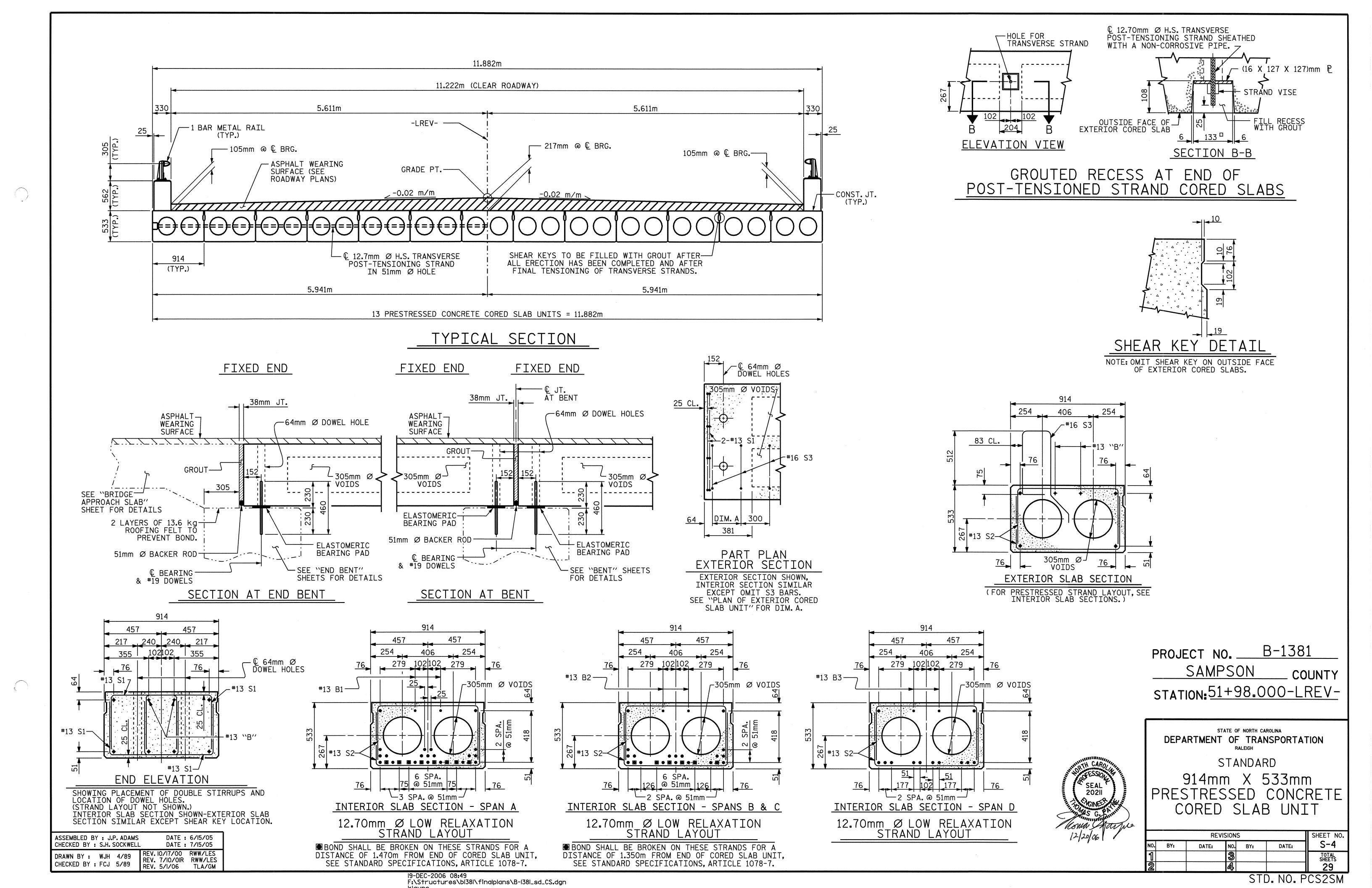
GENERAL DRAWING BRIDGE ON NC 411 OVER BLACK RIVER BETWEEN SR 1126 AND SR 1130

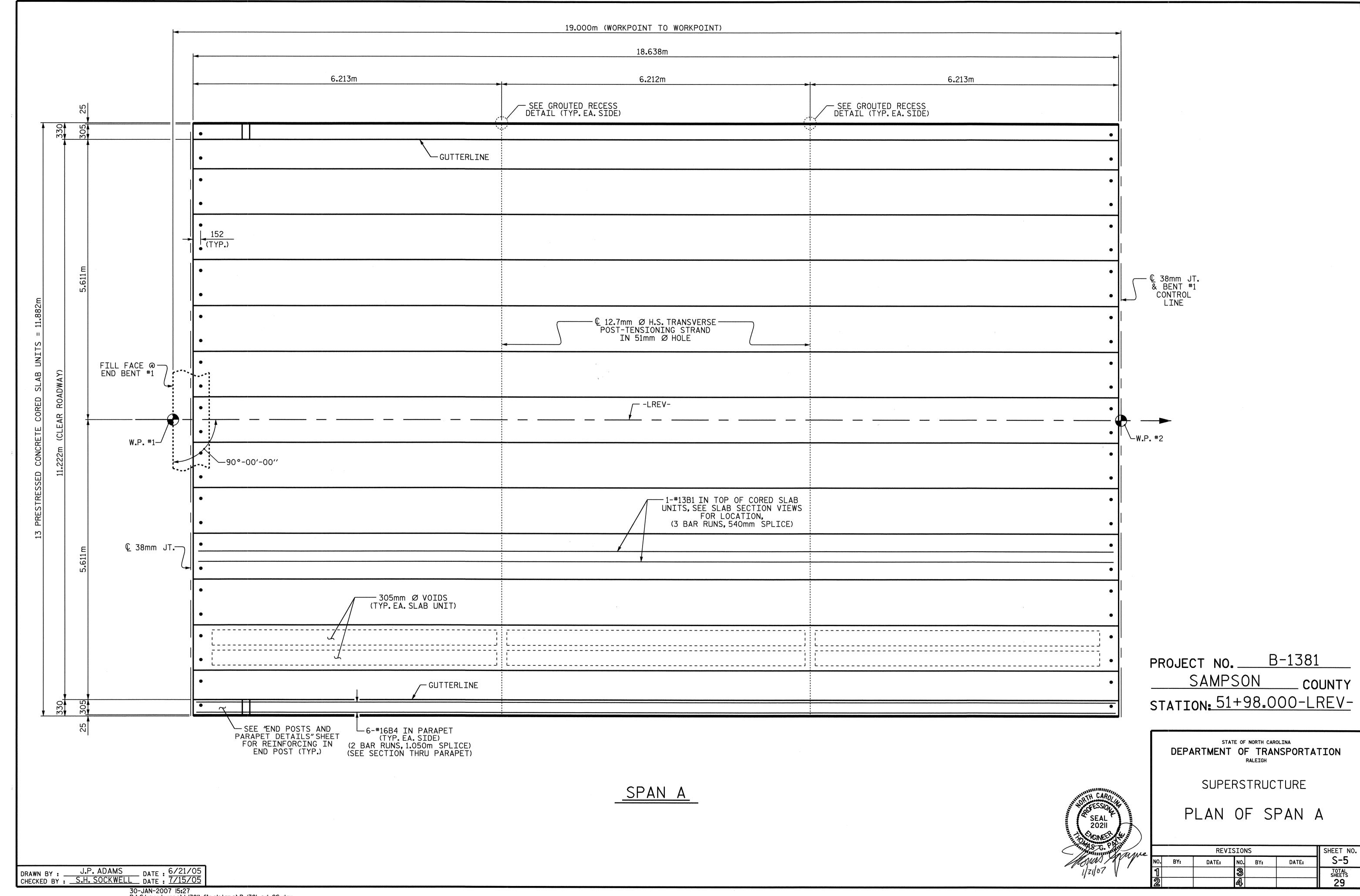
REVISIONS

NO. BY: DATE: NO. BY: DATE: S-3

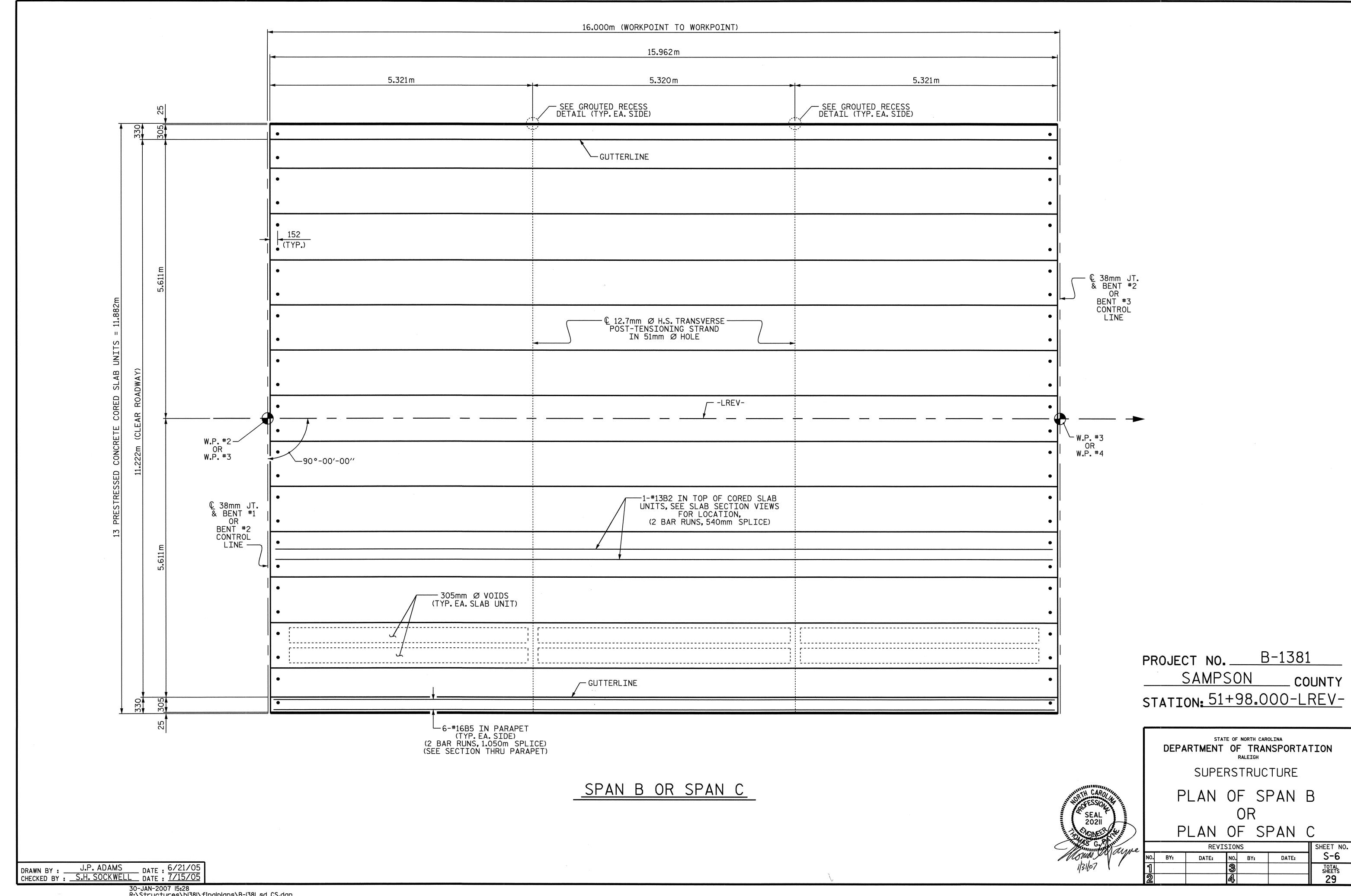
1 3 TOTAL SHEETS
2 4 29

DRAWN BY: S.H. SOCKWELL DATE: 5/10/05 CHECKED BY: J.P. ADAMS DATE: 11/1/05

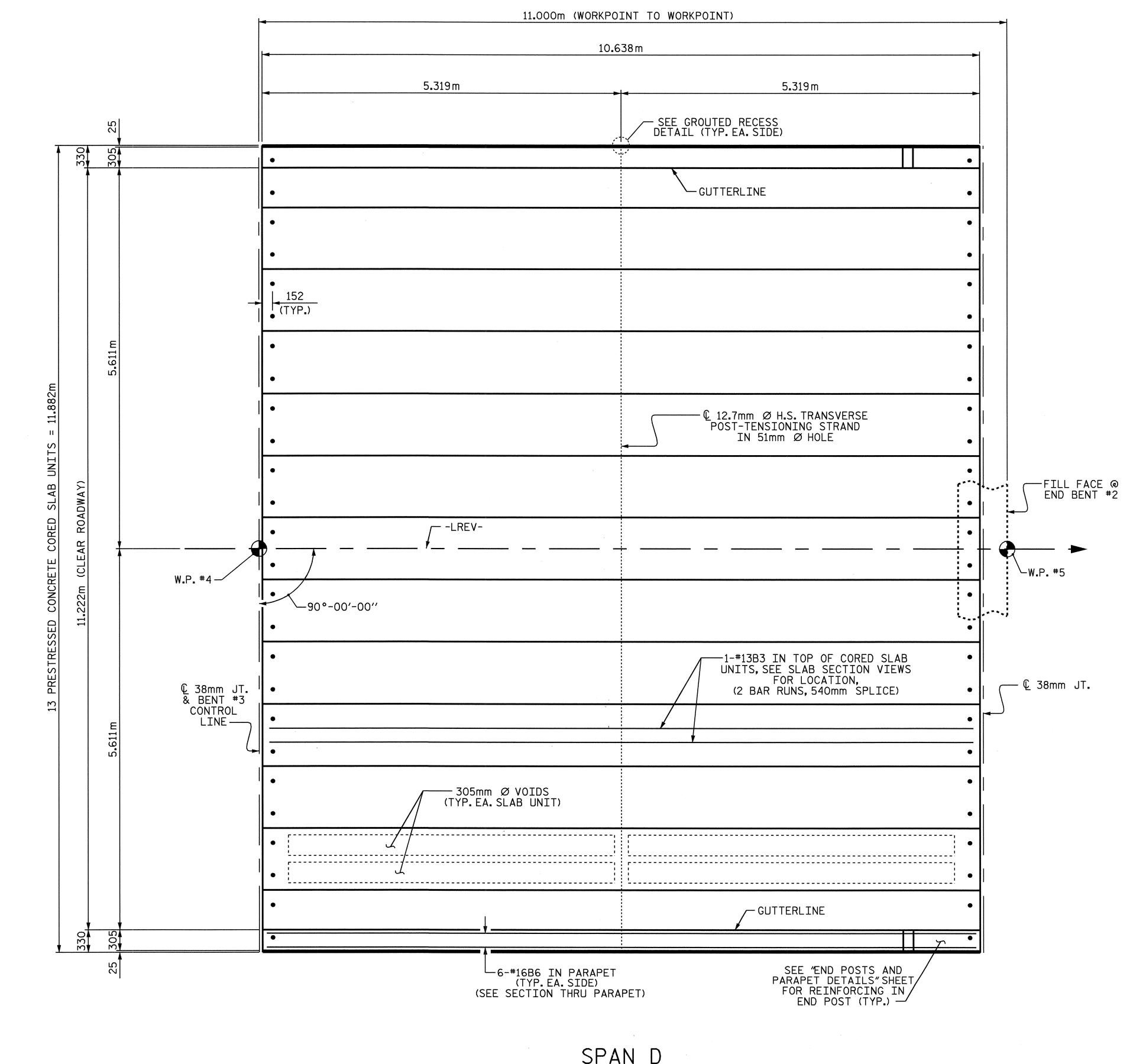




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30-JAN-2007 |5:28 R:\Structures\b|38|\fina|p|ans\B-|38|_sd_CS.dgn



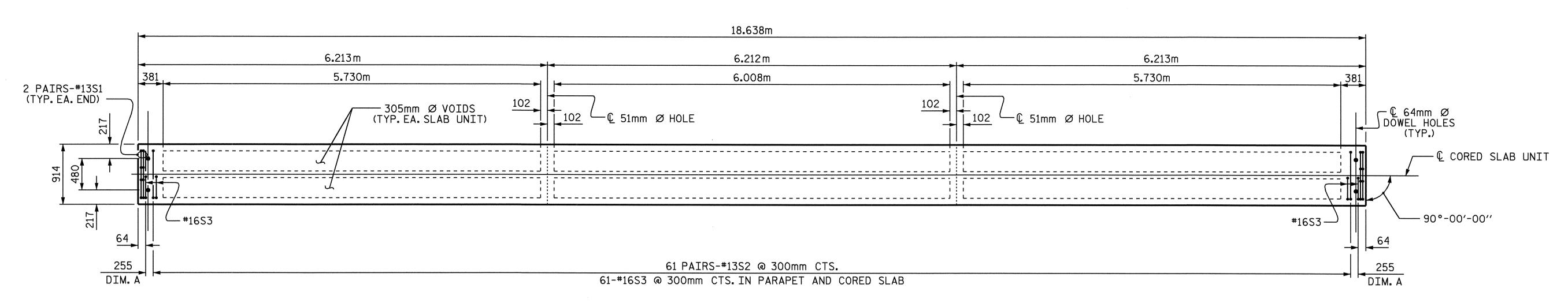
PROJECT NO. B-1381 SAMPSON ___ COUNTY STATION: 51+98.000-LREV-

> STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH

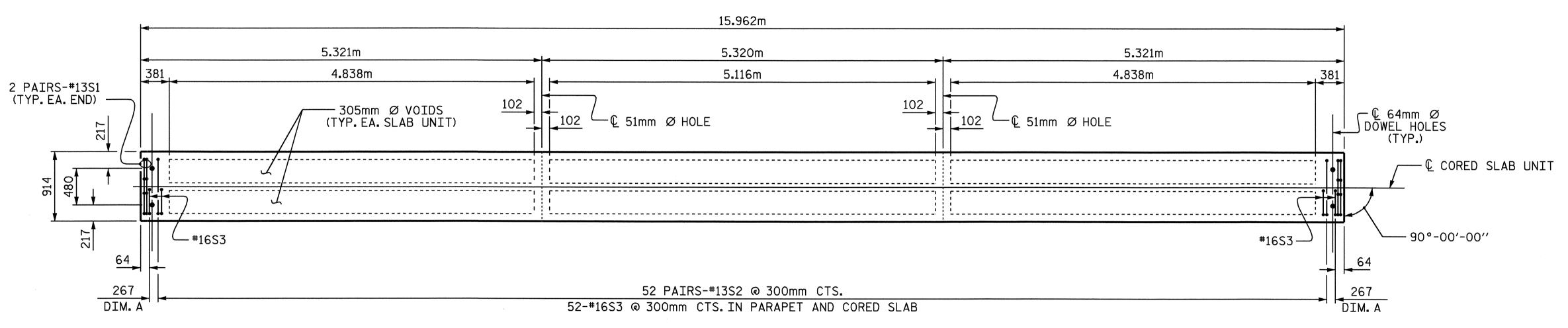
> > SUPERSTRUCTURE

PLAN OF SPAN D

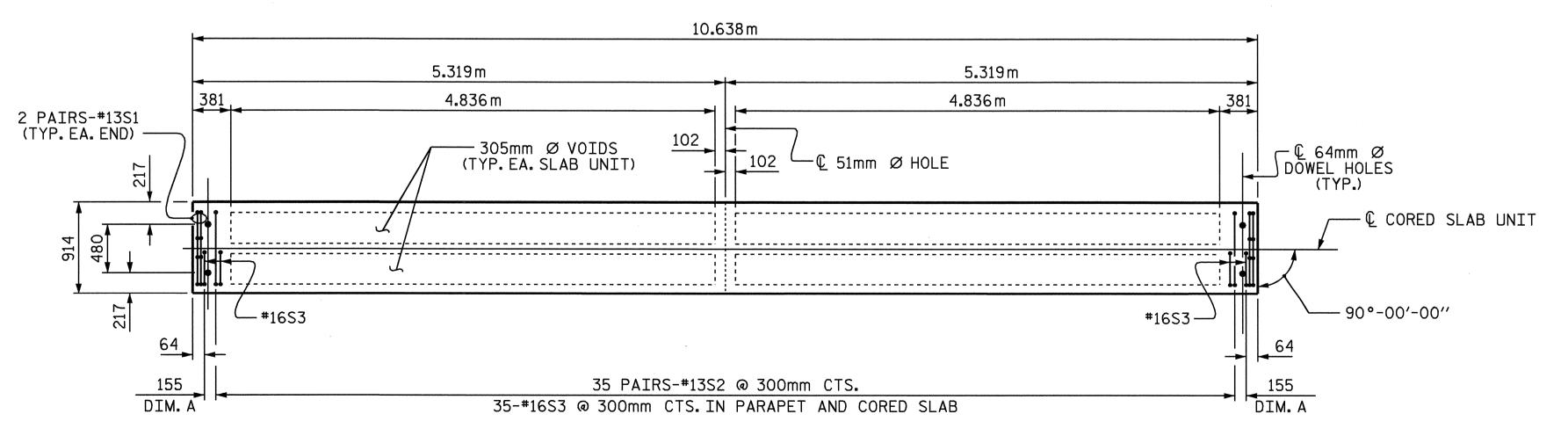
REVISIONS SHEET NO. S-7 DATE: DATE: BY: TOTAL SHEETS 29



PLAN OF EXTERIOR CORED SLAB UNIT - SPAN A



PLAN OF EXTERIOR CORED SLAB UNIT - SPAN B OR SPAN C



PLAN OF EXTERIOR CORED SLAB UNIT - SPAN D

NOTE: EXTERIOR CORED SLAB UNIT SHOWN, INTERIOR CORED SLAB UNIT SIMILAR EXCEPT #16S3 BARS ARE OMITTED. SEE PART PLAN-EXTERIOR SECTION FOR ADDITIONAL REINFORCEMENT (#13S1 BARS).

PROJECT NO. B-1381 SAMPSON _ COUNTY STATION: 51+98.000-LREV-

> STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

> > SUPERSTRUCTURE

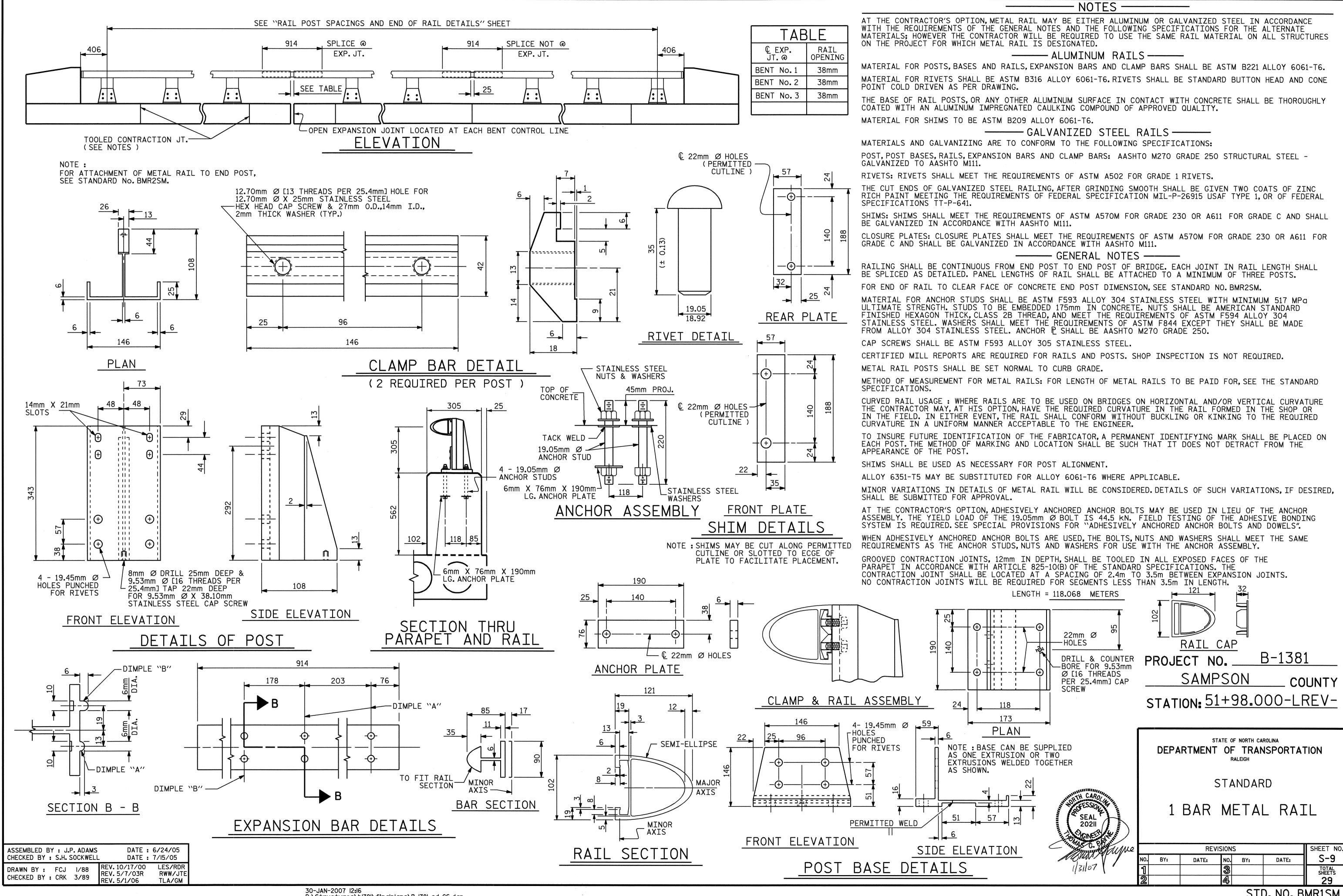
PLAN OF EXTERIOR CORED SLAB UNIT

SPAN A, B, C & D

REVISIONS S-8 BY: TOTAL SHEETS

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DRAWN BY: J.P. ADAMS
CHECKED BY: S.H. SOCKWELL DATE: 6/21/05



1.140m .406, 2 SPA.@ . 990 27 SPA. @ 1.980m CTS. 990 2 SPA. @ .406 1.140m 695mm CTS. 696mm CTS. | END POST € JT.@ END BENT #2 — -LREV-© JT. @ — TEND BENT #1 59.034m 1**.**140m 1.140m

PLAN OF RAIL POST SPACINGS

(TYPICAL FOR EACH SIDE)

TOP VIEW

ASSEMBLED BY : J.P. ADAMS

CHECKED BY : S.H. SOCKWELL

CHECKED BY: CRK 3/89

DATE: 6/24/05

DATE: 7/15/05

RWW/JTE

TLA/GM

REV. 10/17/00 LES/RDR

REV. 5/7/03

REV. 5/1/06

NOTES

STRUCTURAL CONCRETE INSERT

THE STRUCTURAL CONCRETE INSERT ASSEMBLY SHALL CONSIST OF THE FOLLOWING COMPONENTS:

- A. FERRULES SHALL BE MADE FROM STEEL MEETING THE REQUIREMENTS OF AASHTO M169, GRADE 12L14 AND SHALL HAVE A MINIMUM LENGTH OF THREADS OF 38mm.
- B. 1 19.05mm Ø X 41mm BOLT WITH WASHER.BOLT SHALL CONFORM TO THE REQUIREMENTS OF ASTM A307. BOLT AND WASHER SHALL BE GALVANIZED. (AT THE CONTRACTOR'S OPTION, STAINLESS STEEL BOLT AND WASHER MAY BE USED AS AN ALTERNATE FOR THE 19.05mm Ø X 41mm GALVANIZED BOLT AND WASHER. THEY SHALL CONFORM TO OR EXCEED THE MECHANICAL REQUIREMENTS OF ASTM A307. THE USE OF THIS ALTERNATE SHALL BE APPROVED BY THE ENGINEER.)
- C. WIRE STRUT SHOWN IN THE CONCRETE INSERT ASSEMBLY DETAIL IS THE MINIMUM ALLOWABLE SIZE AND SHALL HAVE A MINIMUM TENSILE STRENGTH OF 689 MPa. AS AN OPTION, A 11mm Ø WIRE STRUT WITH A MINIMUM TENSILE STRENGTH OF 620 MPa. IS ACCEPTABLE.

NOTES

METAL RAIL TO END POST CONNECTION

A. 12mm PLATES SHALL CONFORM TO AASHTO M270 GRADE 250 AND SHALL BE GALVANIZED AFTER FABRICATION.

THE METAL RAIL TO END POST CONNECTION SHALL CONSIST OF THE FOLLOWING COMPONENTS:

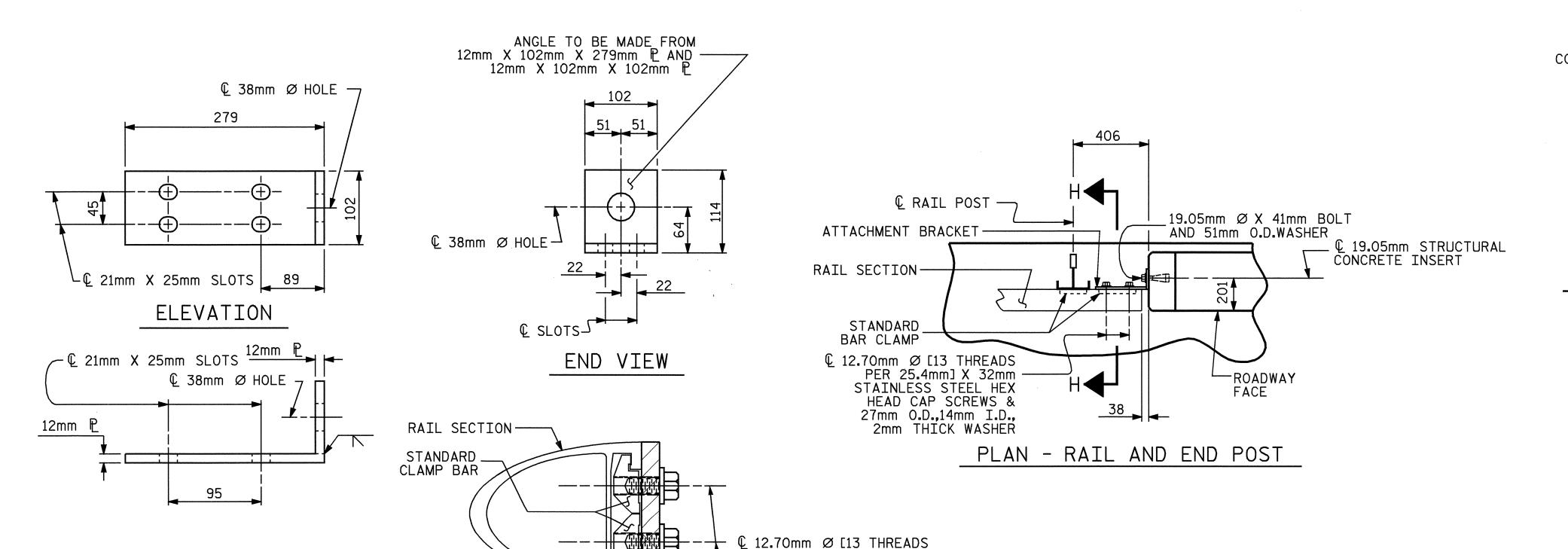
- B. 19.05mm STRUCTURAL CONCRETE INSERT SHALL HAVE A WORKING LOAD SHEAR CAPACITY OF 21.4 kN. THE FERRULES SHALL ENGAGE A 19.05mm Ø X 41mm BOLT WITH 51mm O.D. WASHER IN PLACE. THE 19.05mm Ø X 41mm BOLT SHALL HAVE N. C. THREADS.
- C. CAP SCREWS FOR RAIL ATTACHMENT TO ANGLE SHALL CONFORM TO THE REQUIREMENTS OF ASTM F593 ALLOY 305 STAINLESS STEEL. CAP SCREWS TO BE CENTERED IN SLOTS AT 16°C.
- D. STANDARD CLAMP BARS (SEE METAL RAIL SHEET).
- E. 13mm Ø PIPE SLEEVES (IF REQUIRED) TO BE GALVANIZED.

THE COST OF THE STANDARD CLAMP BARS AND CAP SCREWS USED IN THE METAL RAIL TO END POST CONNECTION SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR METERS OF 1 OR 2 BAR METAL RAILS.

THE 19.05mm STRUCTURAL CONCRETE INSERT WITH BOLT SHALL BE ASSEMBLED IN THE SHOP.

THE COST OF THE 19.05mm STRUCTURAL CONCRETE INSERT ASSEMBLY, AND THE 12mm PLATES COMPLETE IN PLACE SHALL BE INCLUDED IN THE VARIOUS PAY ITEMS.

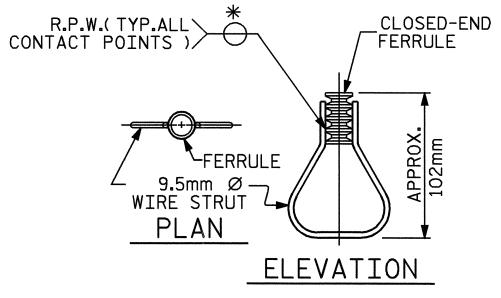
THE CONTRACTOR, AT HIS OPTION, MAY USE AN ADHESIVE BONDING SYSTEM IN LIEU OF THE STRUCTURAL CONCRETE INSERT EMBEDDED IN THE END POST. IF THE ADHESIVE BONDING SYSTEM IS USED, THE 19.05mm Ø X 41mm BOLT WITH WASHER SHALL BE REPLACED WITH A 19.05mm Ø X 165mm BOLT AND 51mm O.D. WASHER. ALL SPECIFICATIONS THAT APPLY TO THE 19.05mm Ø X 41mm BOLT SHALL APPLY TO THE 19.05mm Ø X 165mm BOLT. SEE SPECIAL PROVISIONS FOR "ADHESIVELY ANCHORED ANCHOR BOLTS AND DOWELS". FIELD TESTING OF THE ADHESIVE BONDING SYSTEM IS NOT REQUIRED.



PER 25.4mm] X 32mm - STAINLESS STEEL HEX

HEAD CAP SCREWS &

12mm P 27mm O.D.,14mm I.D., 2mm THICK WASHER



STRUCTURAL CONCRETE INSERT

*EACH WELDED ATTACHMENT OF WIRE TO FERRULE SHALL DEVELOP THE TENSILE STRENGTH OF THE WIRE.

PROJECT NO. B-1381

SAMPSON COUNTY

STATION: 51+98.000-LREV-

STATION: 51+98.000-LREV-



DEPARTMENT OF TRANSPORTATION
RALEIGH
STANDARD
RAIL POST SPACINGS
AND

END OF RAIL DETAILS
FOR ONE BAR METAL RAILS

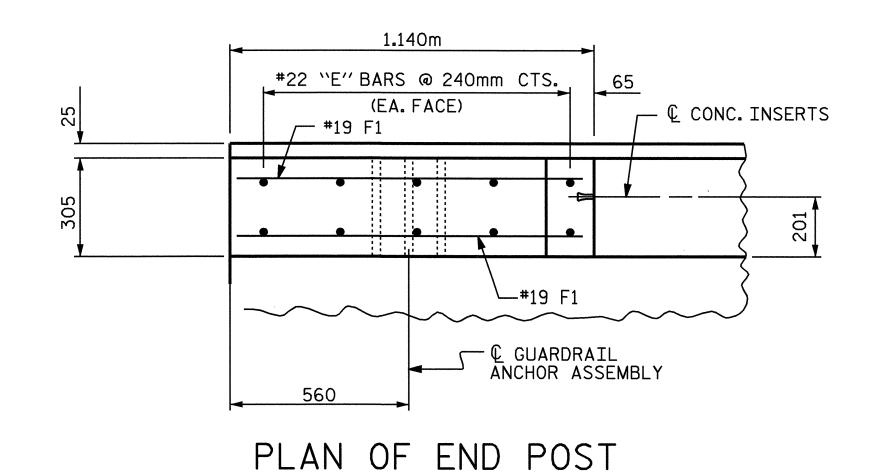
REVISIONS

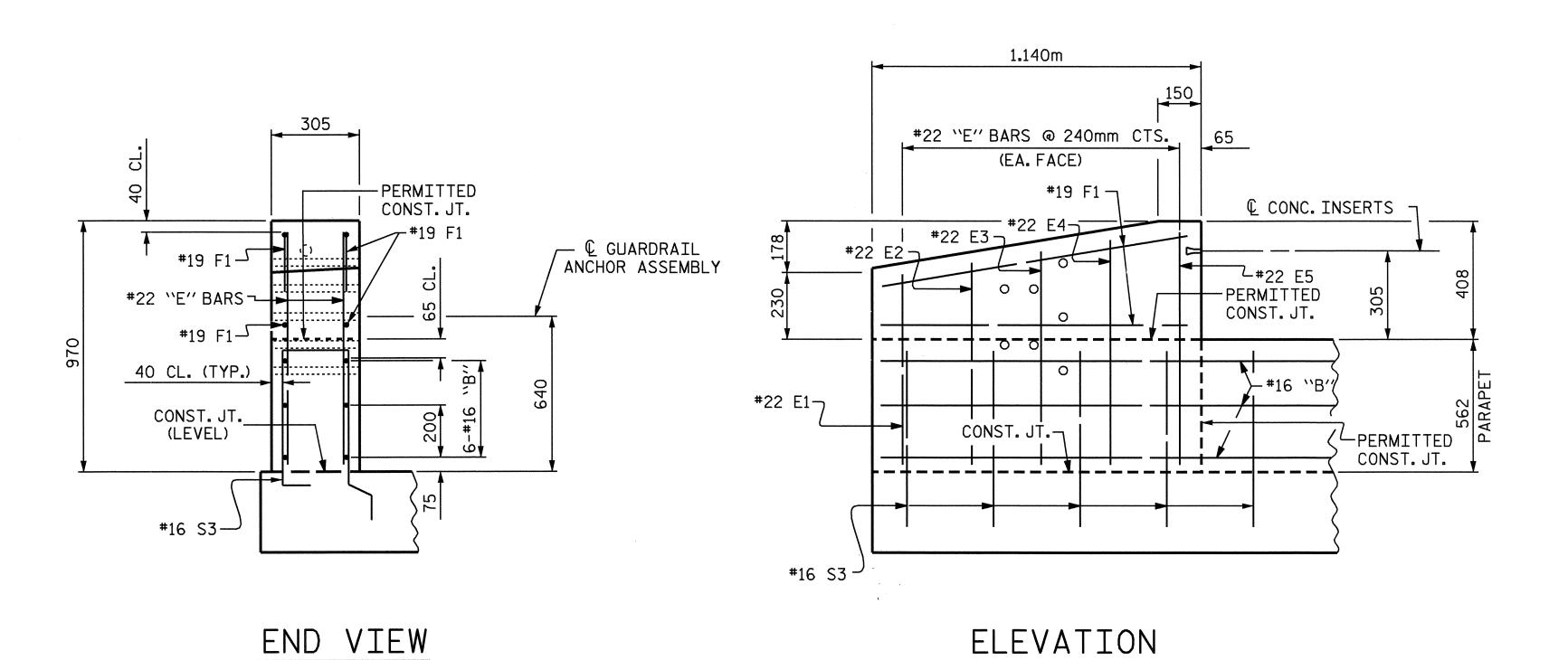
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1 3 TOTAL SHEETS
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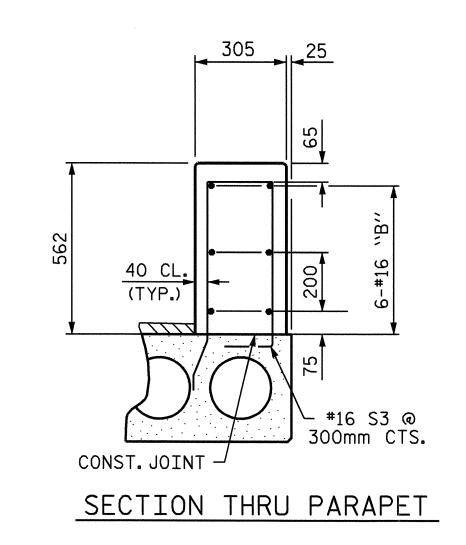
DETAILS FOR ATTACHING METAL RAIL TO END POST

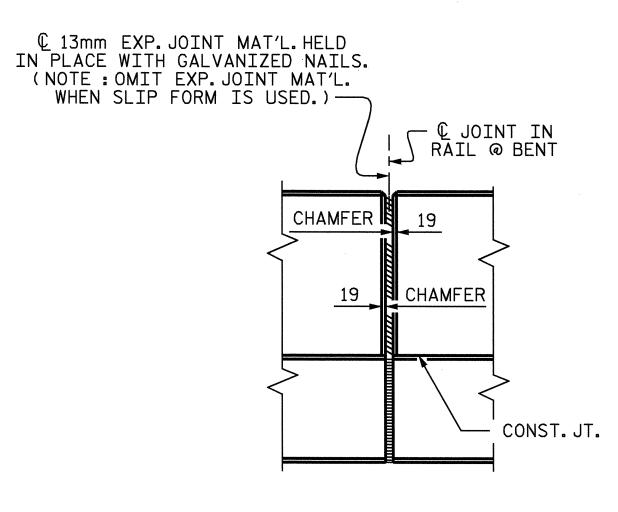
SECTION H-H





PARAPET AND END POST FOR ONE BAR RAIL





PROJECT NO. B-1381

SAMPSON COUNTY

STATION: 51+98.000-LREV-



DEPARTMENT OF TRANSPORTATION
RALEIGH

END POSTS

END POSTS AND PARAPET DETAILS

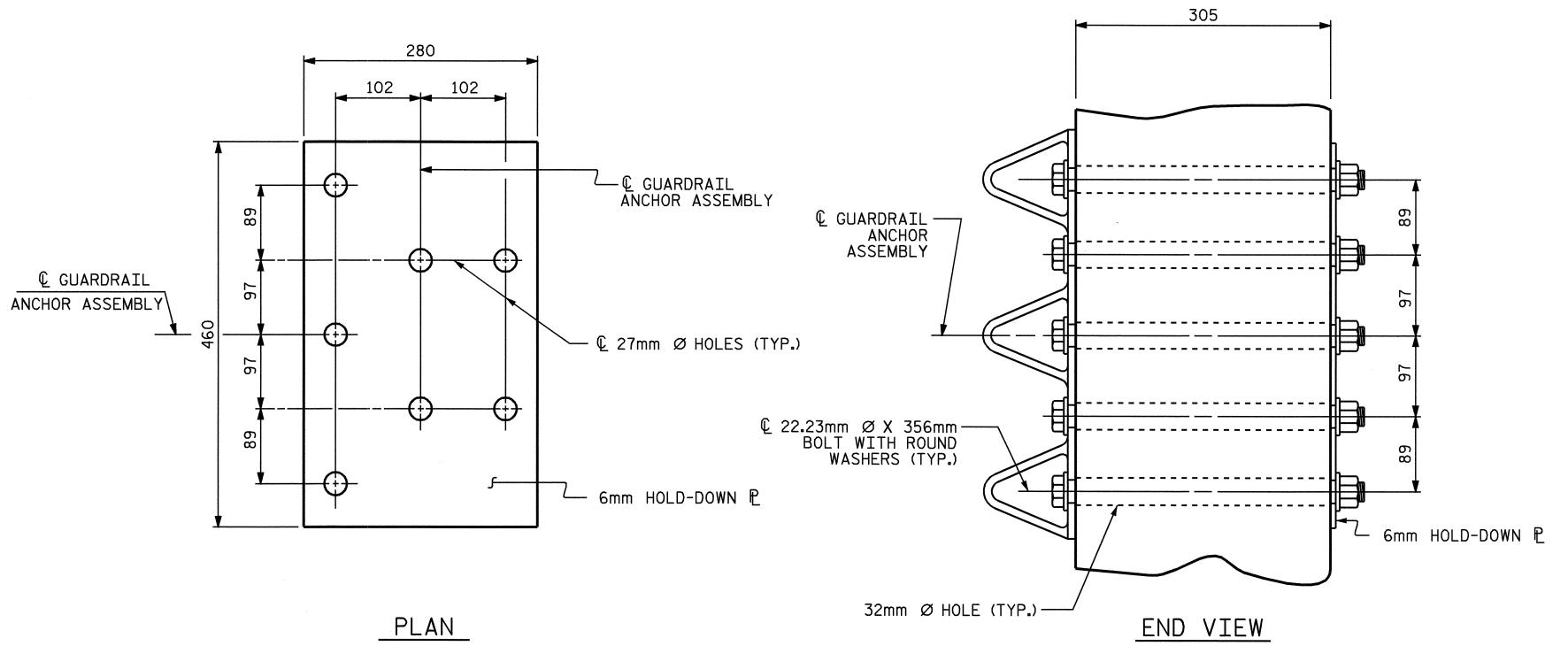
REVISIONS

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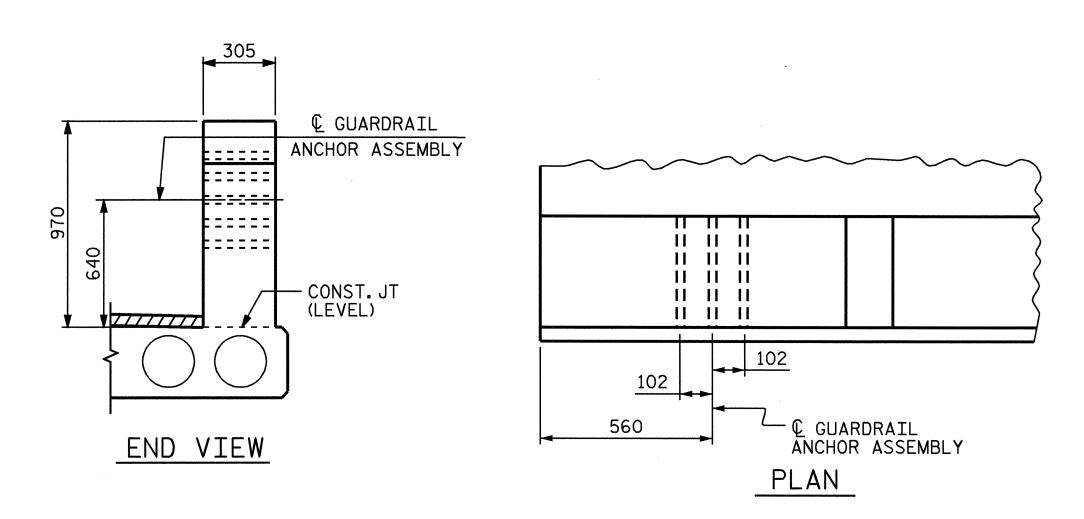
S-II

TOTAL
SHEETS
29

DRAWN BY: J.P. ADAMS DATE: 6/27/05 CHECKED BY: S.H. SOCKWELL DATE: 7/15/05



GUARDRAIL ANCHOR ASSEMBLY DETAILS



LOCATION OF GUARDRAIL ANCHOR AT END POST



THE GUARDRAIL ANCHOR ASSEMBLY SHALL CONSIST OF A 6mm HOLD DOWN PLATE AND 7 - 22.23mm Ø BOLTS WITH NUTS AND WASHERS.

THE HOLD-DOWN PLATE SHALL CONFORM TO AASHTO M270 GRADE 250. AFTER FABRICATION, THE HOLD-DOWN PLATE SHALL BE HOT-DIP GALVANIZED IN ACCORDANCE WITH AASHTO M111.

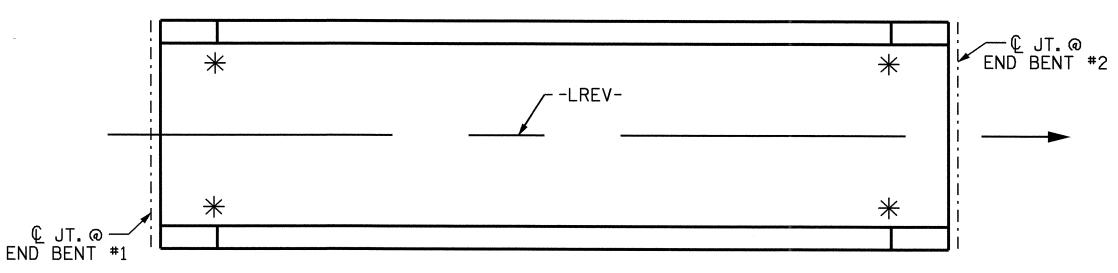
BOLTS SHALL CONFORM TO THE REQUIREMENTS OF ASTM A307 AND NUTS SHALL CONFORM TO THE REQUIREMENTS OF AASHTO M291M. BOLTS, NUTS AND WASHERS SHALL BE GALVANIZED. AT THE CONTRACTOR'S OPTION, STAINLESS STEEL BOLTS, NUTS AND WASHERS MAY BE USED AS AN ALTERNATE FOR THE 22.23mm Ø GALVANIZED BOLTS, NUTS AND WASHERS. THEY SHALL CONFORM TO OR EXCEED THE MECHANICAL REQUIREMENTS OF ASTM A307. THE USE OF THIS ALTERNATE SHALL BE APPROVED BY THE ENGINEER.

AFTER INSTALLATION, THE EXPOSED THREAD OF THE BOLT SHALL BE BURRED WITH A SHARP POINTED TOOL.

THE COST OF THE GUARDRAIL ANCHOR ASSEMBLIES WITH BOLTS, NUTS AND WASHERS COMPLETE IN PLACE, SHALL BE INCLUDED IN THE VARIOUS PAY ITEMS.

THE VERTICAL REINFORCING BARS MAY BE SHIFTED SLIGHTLY IN THE END POST TO CLEAR ASSEMBLY BOLTS.

THE 32mm Ø HOLES SHALL BE FORMED OR DRILLED WITH A CORE BIT. IMPACT TOOLS WILL NOT BE PERMITTED. ANY CONCRETE DAMAGED BY THIS WORK SHALL BE REPAIRED TO THE SATISFACTION OF THE ENGINEER.



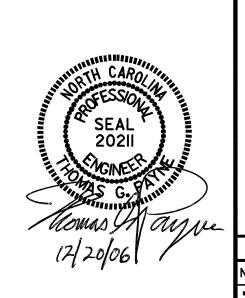
SKETCH SHOWING POINTS OF ATTACHMENT

*LOCATION OF GUARDRAIL ATTACHMENT

PROJECT NO. B-1381

SAMPSON COUNTY

STATION: 51+98.000-LREV-



STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION
RALEIGH

STANDARD

GUARDRAIL ANCHORAGE DETAILS FOR METAL RAILS

		REVI	SION	S		SHEET NO.
٥.	BY:	DATE:	NO.	BY:	DATE:	S-I2
			3			TOTAL SHEETS
2			4			29
						······································

ASSEMBLED BY: J. P. ADAMS DATE: 6/27/05 CHECKED BY: S.H. SOCKWELL DATE: 7/15/05

DRAWN BY: EEM 6/94 REV. 10/17/00 RWW/LES REV. 5/7/03 RWW/JTE REV. 5/1/06 TLA/GM

| 19-DEC-2006 08:48 | F:\Structures\b|38|\fina|p|ans\B-|38|_sd_CS.dgn

STD. NO. BMR8SM

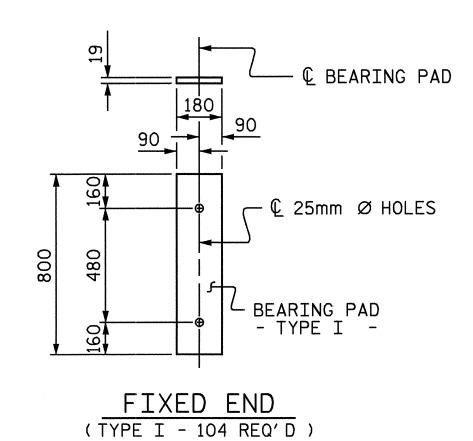
		IEKTA	AL FOI	R ONE	CORED	SLAB	UNIT	BILL	OF M	ATERIA	AL FOR	RONE	CORED	SLAB	UNIT	BILL	OF MA	TERI	AL FOI	RONE	CORED	SLAB	UNIT
			SP	AN A			·				SPAN	B OR	С						SP	AN D			
				EXTERIO	R UNIT	INTERIO	OR UNIT					EXTERIO	OR UNIT	INTERIO	OR UNIT					EXTERIO	R UNIT	INTERIO	OR UNIT
BAR I	NUMBER	SIZE	TYPE	LENGTH	WEIGHT	LENGTH	WEIGHT	BAR	NUMBER	SIZE	TYPE	LENGTH	WEIGHT	LENGTH	WEIGHT	BAR	NUMBER	SIZE	TYPE	LENGTH	WEIGHT	LENGTH	WEIGHT
B1	6	#13	STR	6560	39	6560	39	B2	4	#13	STR	8240	33	8240	33	В3	4	#13	STR	5580	22	5580	22
S1	8	#13	2	1300	10	1300	10	S1	8	#13	2	1300	10	1300	10	S1	8	#13	2	1300	10	1300	10
\$2 * \$3	122 63	#13 #16	1	1620 1840	196 180	1620	196	\$2 * \$3	104 54	#13 #16	1	1620 1840	167 154	1620	167	\$2 * \$3	70 37	#13 #16	1	1620 1840	113 106	1620	113
	ORCING S COATED		CTNC ST	- CCI	245kg 180kg		245kg		FORCING	STEEL REINFOF	OCTNC ST	<u> </u>	210kg 154kg	-	210kg		FORCING S Y COATED	·····	OCTNC ST		145kg 106kg		145kg
	Pa CONCR		CING 31		. METERS	6.5 CU.	METERS		MPa CON		CTING 31		. METERS	5.6 CU.	METERS	-	MPa CONC		CING 31		. METERS	3.8 CU.	METERS
12.70mr	m Ø L.R.	STRANDS	5		No. 28		No. 28	12.70r	mm Ø L.F	R. STRAND	S		No. 24		No. 24	12.70	mm Ø L.R.	STRAND	S		No. 12		No. 12

			·
DEAD LOAD DEFL	LECTION	AND CAM	IBER
	SPAN A	SPAN B OR C	SPAN D
	12.70mm Ø L.R. STRAND	12.70mm Ø L.R. STRAND	12.70mm Ø L.R. STRAND
CAMBER (SLAB ALONE IN PLACE)	77 🕴	62	14
DEFLECTION DUE TO SUPERIMPOSED DEAD LOAD **	15 . ♦	9 ♦	2 ♦
FINAL CAMBER	62	53 ` ♠	12

^{**}INCLUDES FUTURE WEARING SURFACE

BAR TYPES S1 532 S2 812 S3 82 S4 50 S5 532 S5 812 S6 50 S7 50 S8 50

NUMBER | LENGTH | TOTAL LENGTH SPAN A-EXTERIOR 2 18.638m 37.276m SPAN A-INTERIOR 205**.**018m 18.638m 15.962m SPAN B-EXTERIOR 31.924m 15.962m 175.582m SPAN B-INTERIOR 15.962m 31.924m SPAN C-EXTERIOR 15**.**962m 175.582m SPAN C-INTERIOR SPAN D-EXTERIOR 10.638m 21.276m 10.638m SPAN D-INTERIOR 117.018m TOTAL 52 795.600m



ELASTOMERIC BEARING DETAILS

GRADE 270 S	TRANDS
	12.70mm Ø L.R.
AREA (mm²)	98.71
ULTIMATE STRENGTH (KN PER STRAND)	183.7
APPLIED PRESTRESS (KN PER STRAND)	137.8

				ND POST					
BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT				

* B4	24	16	STR	9800	365				
★ B5	48	16	STR	8460	630				
* B6	12	16	STR	10520	196				
∗ E1	8	22	STR	700	17				
 ₩E2	8	22	STR	760	18				
 ₩E3	8	22	STR	820	20				
 ₩E4	8	22	STR	860	21				
 ₩E5	8	22	STR	880	21				
 ₩F1	16	19	STR	1040	37				
* EPOXY REINFOR	COATE CING S	D TEEL		13	25kg				
CLASS "AA" CONCRETE 21.5 m ³									
CONCRE	TE PARA	APET		122.628 ME	TERS				

DILL OF MATERIAL FOR

NOTES

ALL PRESTRESSING STRANDS SHALL BE 7-WIRE LOW-RELAXATION GRADE 270 STRANDS AND SHALL CONFORM TO AASHTO M203M EXCEPT FOR SAMPLING REQUIREMENTS WHICH SHALL BE IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

ALL REINFORCING STEEL CAST WITH THE CORED SLAB SECTIONS SHALL BE GRADE 420 AND SHALL BE INCLUDED IN THE LUMP SUM PRICE BID FOR CONSTRUCTION OF SUPERSTRUCTURE.

RECESSES FOR TRANSVERSE STRANDS SHALL BE GROUTED AFTER THE TENSIONING OF THE STRANDS.

THE 64mm \varnothing DOWEL HOLES AT FIXED ENDS OF SLAB SECTIONS SHALL BE FILLED WITH GROUT.

THE 51mm Ø BACKER ROD SHALL CONFORM TO THE REQUIREMENTS OF TYPE M BOND BREAKER. SEE SECTION 1028 OF THE STANDARD SPECIFICATIONS.

WHEN CORED SLABS ARE CAST, A POSITIVE HOLD-DOWN SYSTEM SHALL BE EMPLOYED TO PREVENT VOIDS FROM RISING OR MOVING SIDEWAYS. THIS SYSTEM SHALL BE DESIGNED TO BE LEFT IN PLACE UNTIL THE CONCRETE HAS REACHED RELEASE STRENGTH. AT LEAST THREE WEEKS PRIOR TO CASTING CORED SLABS, THE CONTRACTOR SHALL SUBMIT TO THE ENGINEER FOR REVIEW AND COMMENT, DETAILED DRAWINGS OF THE PROPOSED HOLD-DOWN SYSTEM. IN ADDITION TO STRUCTURAL DETAILS, LOCATION AND SPACING OF THE HOLD-DOWNS SHALL BE INDICATED.

THE TRANSFER OF LOAD FROM THE ANCHORAGES TO THE CORED SLAB UNIT SHALL BE DONE WHEN THE CONCRETE HAS REACHED A COMPRESSIVE STRENGTH OF NOT LESS THAN 34.5MPa FOR SPAN A AND 27.6 MPa.FOR SPANS B, C AND D.

ALL REINFORCING STEEL IN PARAPET AND END POSTS SHALL BE EPOXY COATED.

PRESTRESSED STRANDS SHALL BE CUT FLUSH WITH THE CORED SLAB UNIT ENDS.

APPLY EPOXY PROTECTIVE COATING TO CORED SLAB UNIT ENDS. FOR EPOXY PROTECTIVE COATING, SEE SPECIAL PROVISIONS.

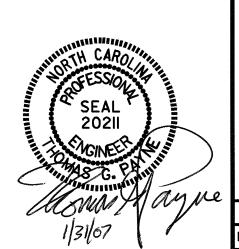
FOR ELASTOMERIC BEARINGS, SEE SPECIAL PROVISIONS.

FOR PRESTRESSED CONCRETE MEMBERS, SEE SPECIAL PROVISIONS.

PROJECT NO. B-1381

SAMPSON COUNTY

STATION: 51+98.000-LREV-



DEPARTMENT OF TRANSPORTATION
RALEIGH

STANDARD

914mm X 533mm

PRESTRESSED

CONCRETE CORED

SLAB UNIT

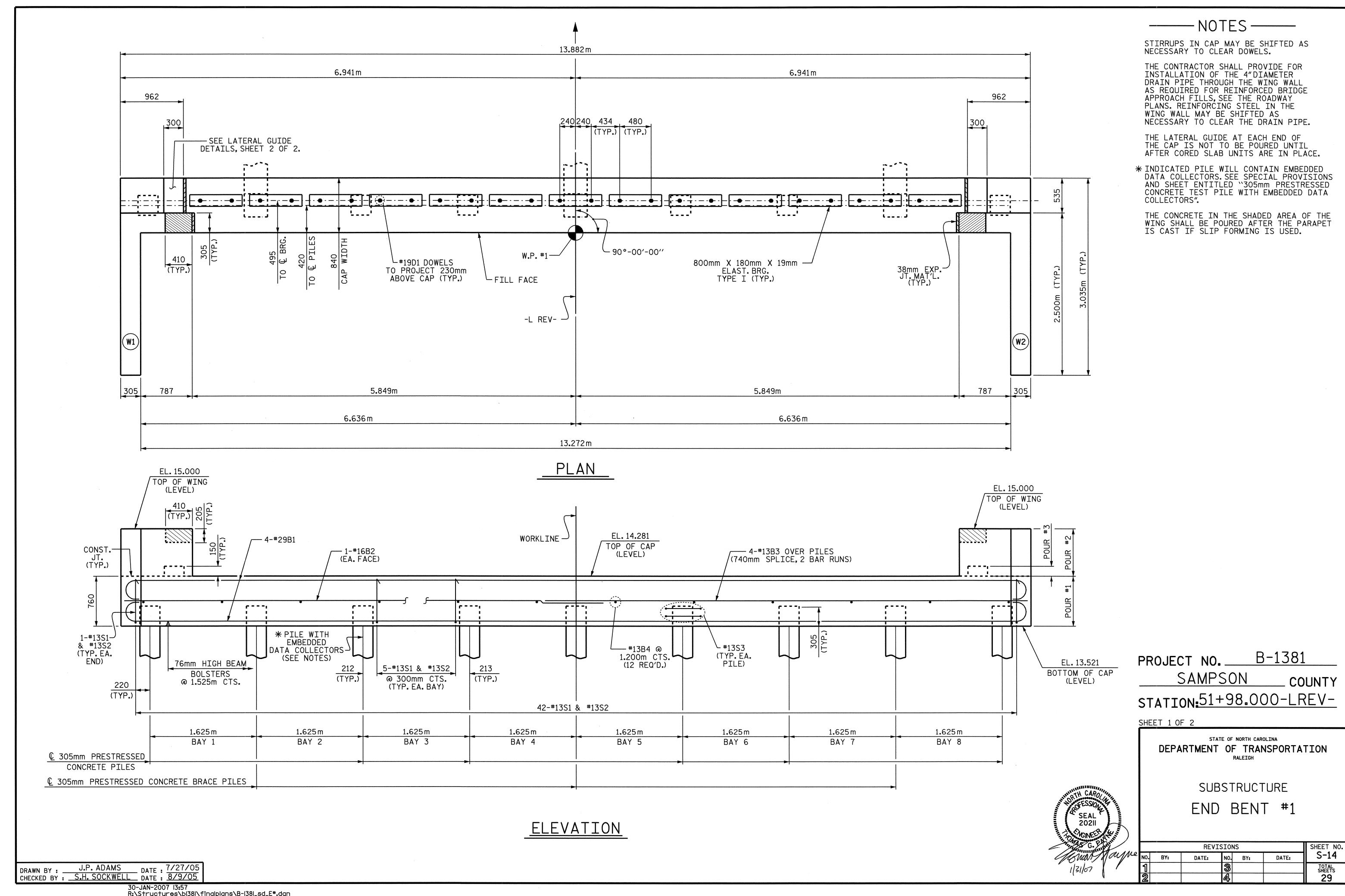
		RE\	/ISIONS			SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	S-13
1			3			TOTAL SHEETS
2			4			29

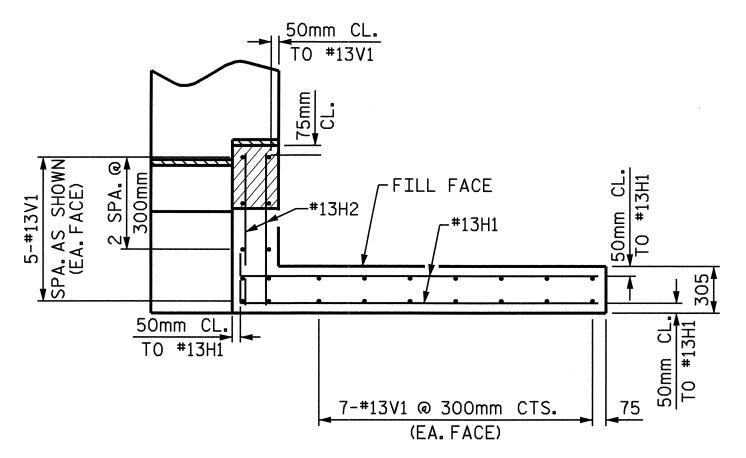
ASSEMBLED BY: J.P. ADAMS
CHECKED BY: S.H. SOCKWELL

DRAWN BY: WJH 4/89
CHECKED BY: FCJ 5/89

REV. 7/10/01
RWW/LES
REV. 5/7/03RR
RWW/JTE
REV. 5/1/06

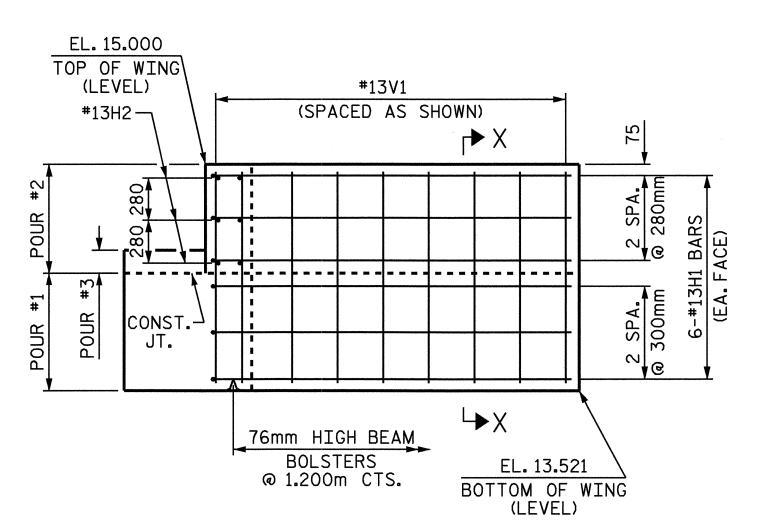
TLA/GM

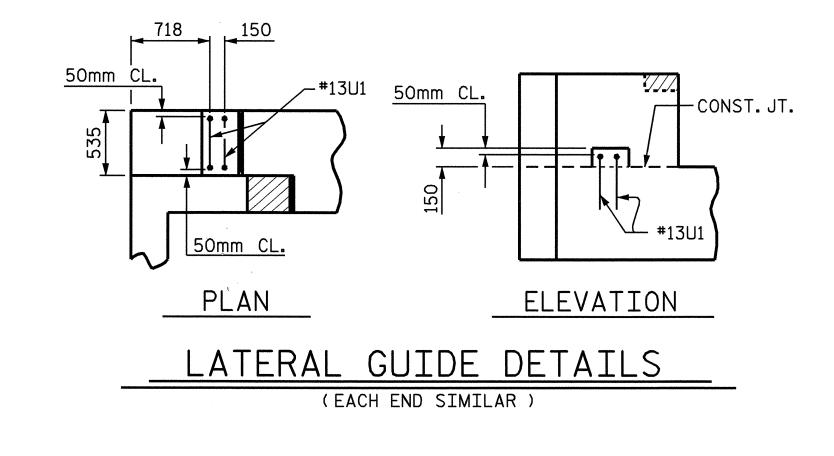


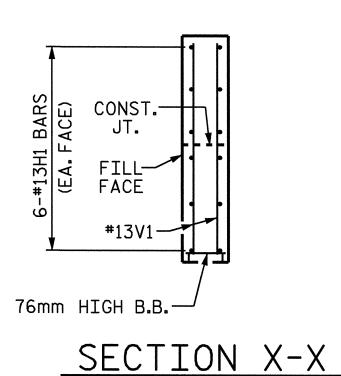


PLAN OF WING

WING 1 SHOWN, WING 2 SIMILAR.

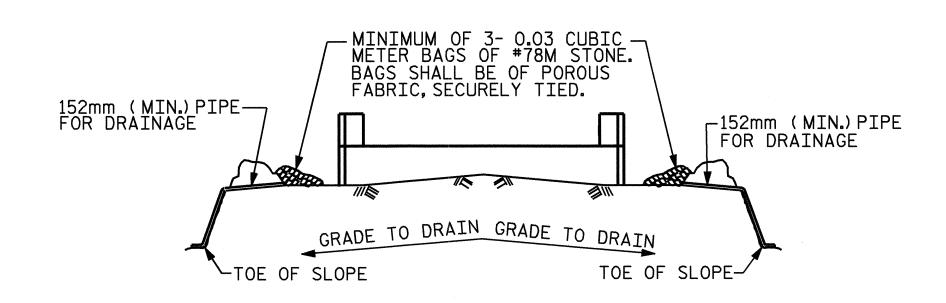






ELEVATION OF WING

WING 1 SHOWN, WING 2 SIMILAR.

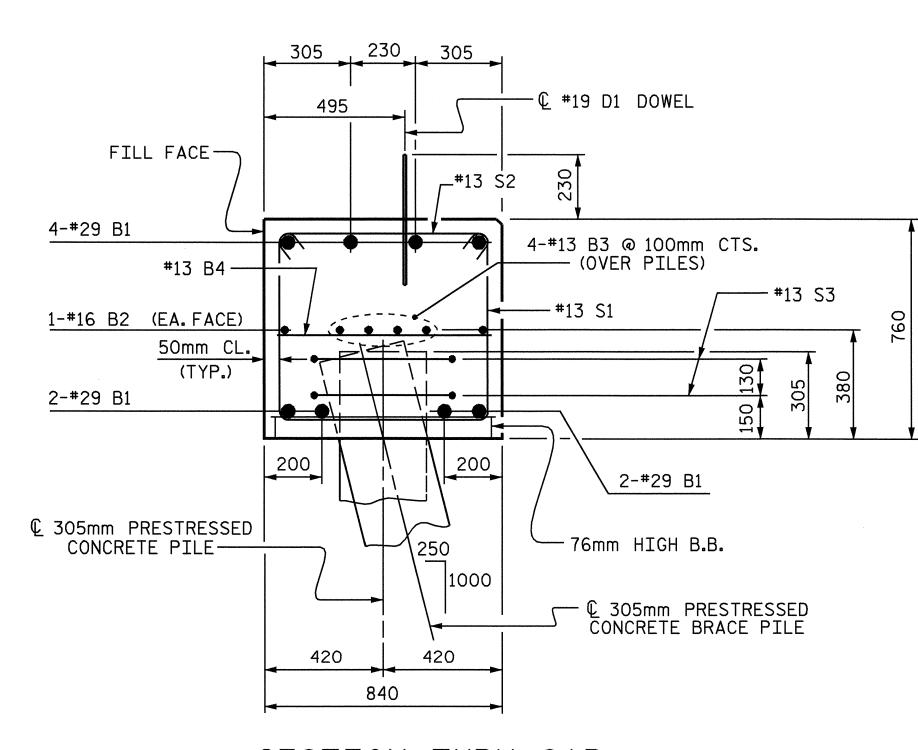


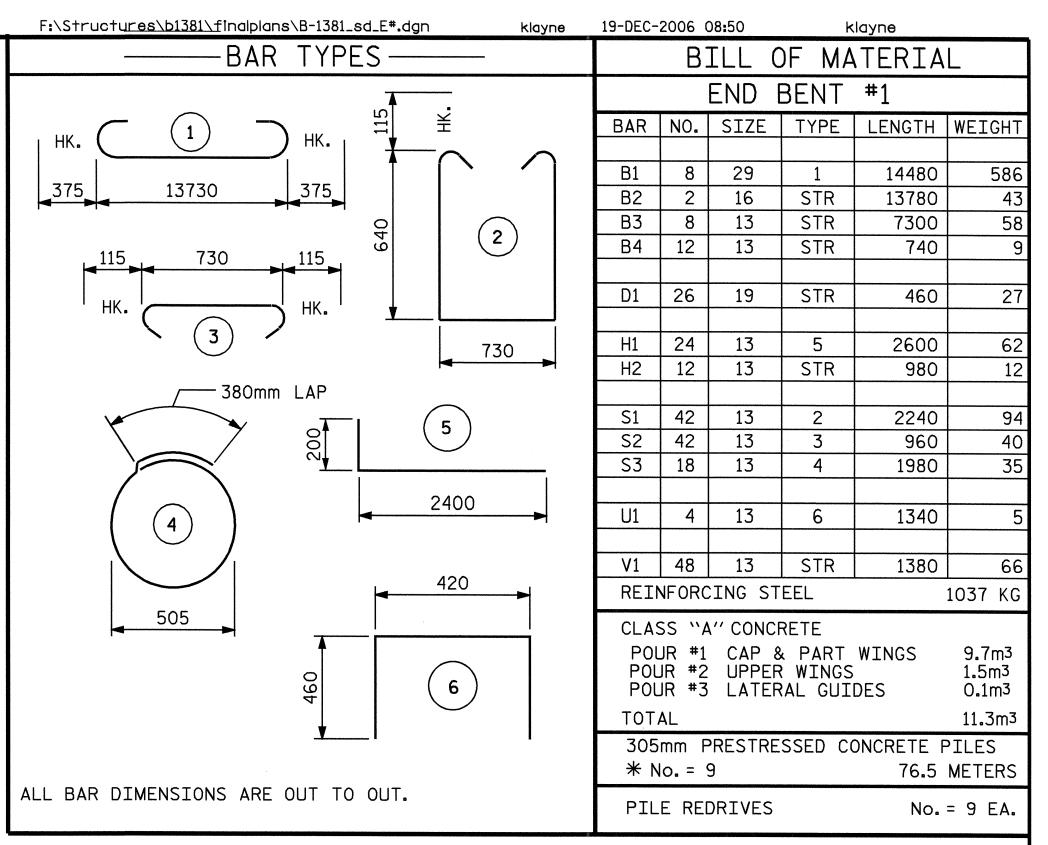
BAGGED STONE AND PIPE SHALL BE PLACED IMMEDIATELY AFTER COMPLETION OF END BENT EXCAVATION. PIPE MAY BE EITHER CONCRETE, CORRUGATED STEEL, CORRUGATED ALUMINUM ALLOY, OR CORRUGATED PLASTIC. PERFORATED PIPE WILL NOT BE ALLOWED.

BAGGED STONE SHALL REMAIN IN PLACE UNTIL THE ENGINEER DIRECTS THAT IT BE REMOVED. THE CONTRACTOR SHALL REMOVE AND DISPOSE OF SILT ACCUMULATIONS AT BAGGED STONE WHEN SO DIRECTED BY THE ENGINEER. BAGS SHALL BE REMOVED AND REPLACED WHENEVER THE ENGINEER DETER-MINES THAT THEY HAVE DETERIORATED AND LOST THEIR EFFECTIVENESS.

NO SEPARATE PAYMENT WILL BE MADE FOR THIS WORK AND THE ENTIRE COST OF THIS WORK SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE FOR THE SEVERAL PAY ITEMS.

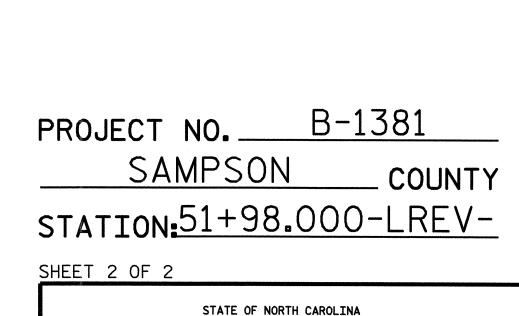
TEMPORARY DRAINAGE AT END BENT

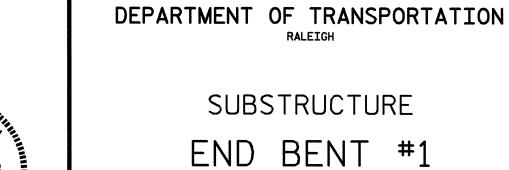




NOTE: CONCRETE DISPLACED BY 305mm PRESTRESSED CONCRETE PILES HAS BEEN DEDUCTED.

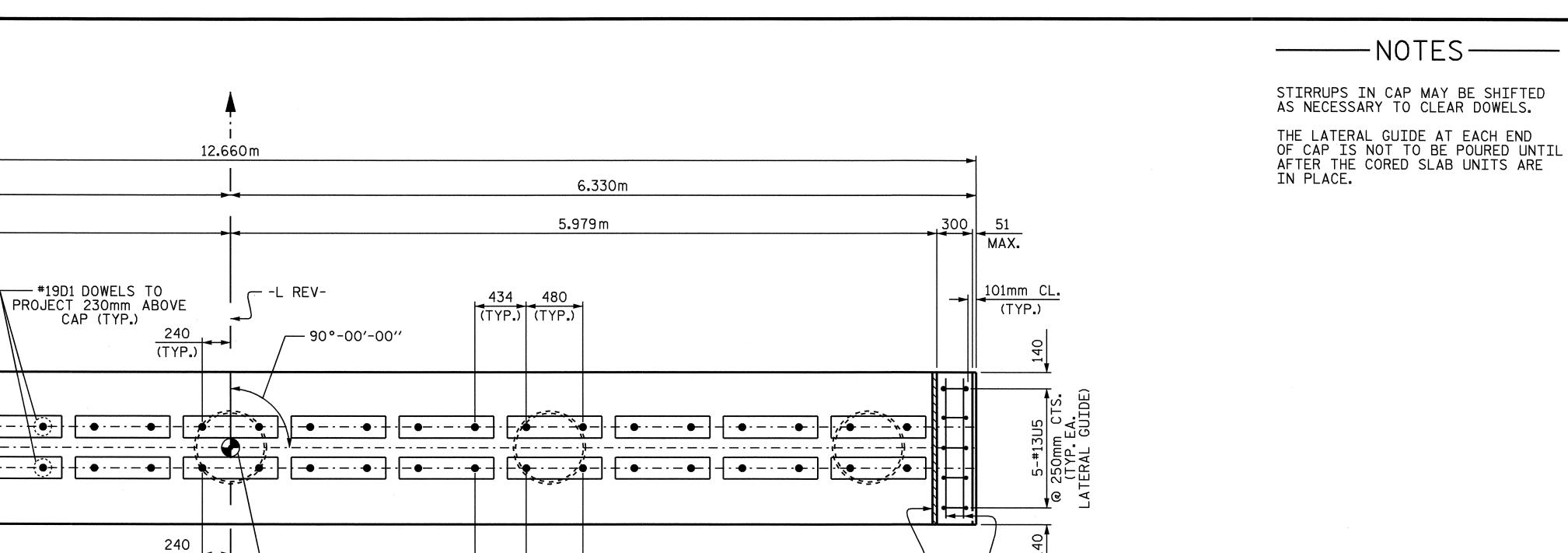
* PILE WITH EMBEDDED DATA COLLECTORS IS INCLUDED IN THIS TOTAL.





REVISIONS SHEET NO. S-15 DATE: BY: DATE: BY: TOTAL SHEETS

SECTION THRU CAP SEAL 20211 asmus Stague DRAWN BY: J.P. ADAMS DATE: 8/1/05 CHECKED BY: S.H. SOCKWELL DATE: 8/9/05



434 480 (TYP.) (TYP.)

38mm EXP JT. MAT'L.—
(TYP.)

#13B5 (TYP.EA. LATERAL GUIDE)

SEAL 20211

-const. ---- 4-#13U1 EL. 14.396 @ 150mm CTS. (TYP.) TOP OF CAP (TYP. OVER EA. -WORKLINE PIPE PILE) (LEVEL) OVER PILES
(2 BAR RUNS)
(740mm SPLICE) 2-#13S2 (TYP.EA. PIPE PILE) @ 1.200m CTS. (10 REQUIRED) #13B5 (TYP.EA. LATERAL GUIDE) ← 6-#29B2 #13U5 (TYP.EA. LATERAL GUIDE) — #13U3 (TYP. EA. END) .------4-#13U2 (TYP. EA. END) 4-#36B1*-*-— #29U4 — 1-#16B3 (EA. FACE) (TYP. EA. END) INVERT ALTERNATE STIRRUPS EL. 13.636 BOTTOM OF CAP 2-#16S1 @ 300mm CTS. 450 7-#16S1 @ 300mm CTS. (TYP.) (TYP. EACH BAY) (TYP. EA. END) 76mm HIGH B.B. @ 1.500 CTS. 2.700 m 2.700 m 2.700 m 2.700 m © PP 610 X 12.7 STEEL PILE

(TYP.)

PLAN

—— © CAP, PILES & BENT #1 CONTROL LINE

₩.P. #2

6.330m

- 800mm X 180mm X 19mm ELAST. BRG. PAD

TYPE I (TYP.)

└-800mm X 180mm X 19mm

ELAST. BRG. PAD TYPE I (TYP.)

5.979m

SAMPSON COUNTY STATION:51+98.000-LREV-SHEET 1 OF 2 STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION SUBSTRUCTURE BENT #1

PROJECT NO. ____

B-1381

-NOTES-

SHEET NO. **REVISIONS** S-16 NO. BY: BY: DATE: DATE: TOTAL SHEETS 29

ELEVATION

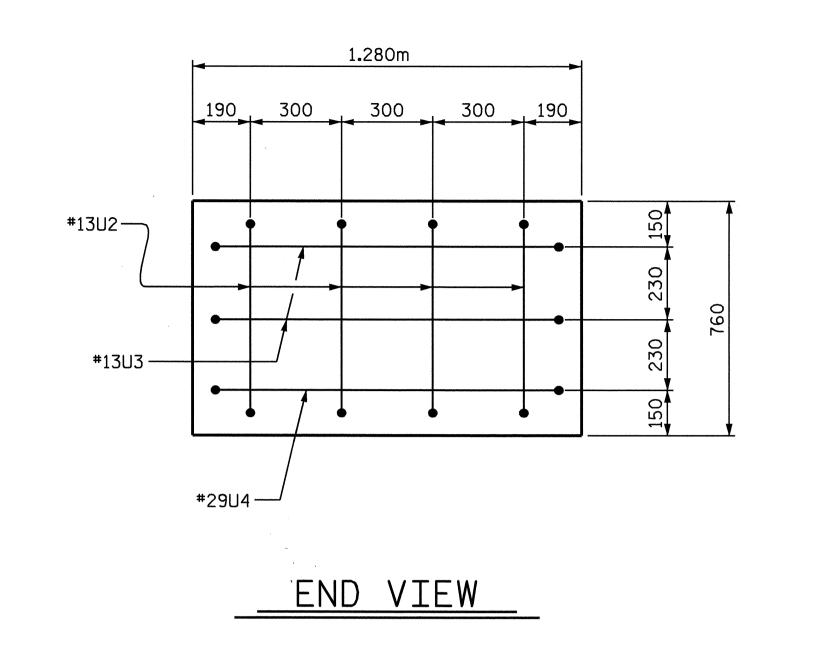
FOR REINFORCING STEEL AND OTHER DETAILS FOR PIPE PILE, SEE "PP 610 X 12.7 STEEL PILE" SHEET.

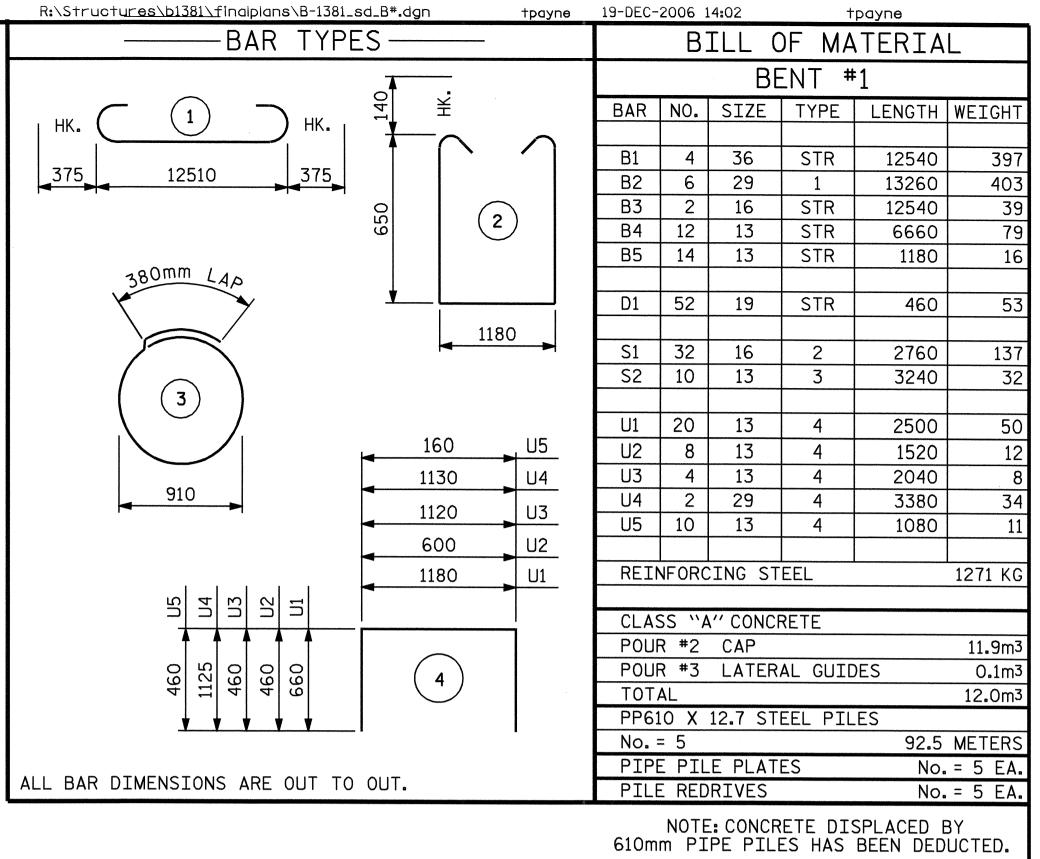
DRAWN BY: J.P. ADAMS DATE: 8/18/05 CHECKED BY: H.A. LOCKLEAR DATE: 8/26/05

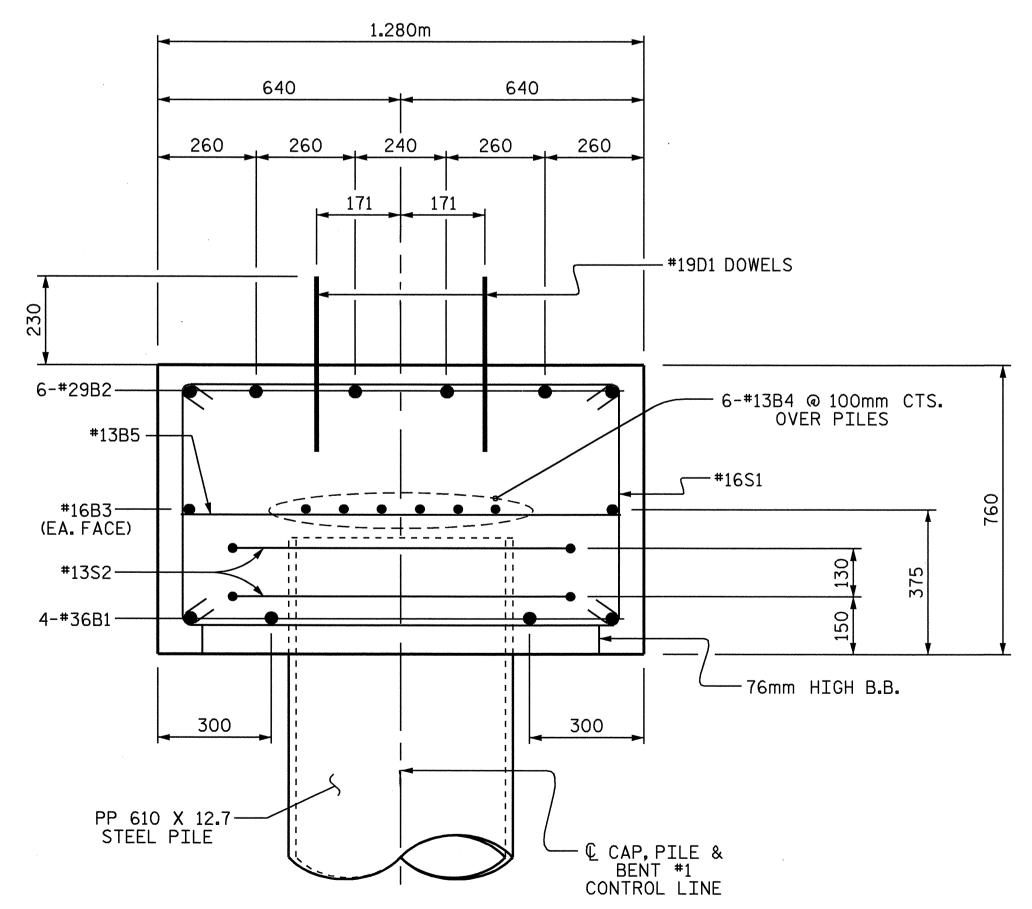
51 MAX.

LATERAL GUIDE — (TYP.)

300







SECTION THRU CAP REINFORCING STEEL IN PIPE PILE NOT SHOWN FOR CLARITY.

PROJECT NO. B-1381 SAMPSON _ COUNTY STATION:51+98.000-LREV-

SHEET 2 OF 2

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

> SUBSTRUCTURE BENT #1

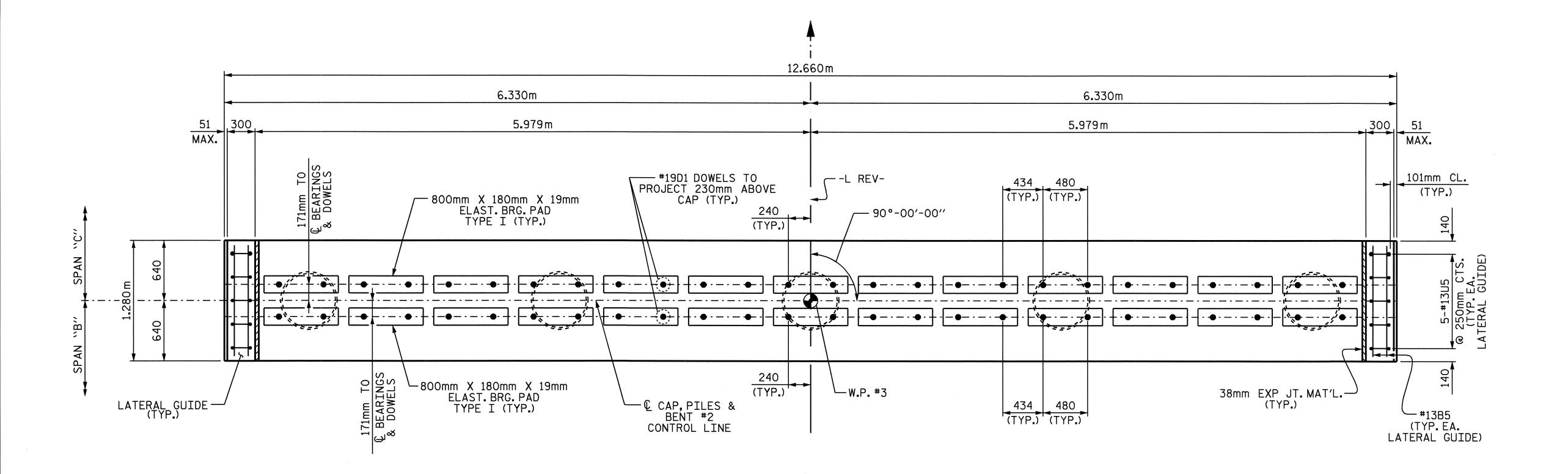
REVISIONS SHEET NO. S-17 NO. BY: DATE: BY: DATE: TOTAL SHEETS 29

DRAWN BY: J.P. ADAMS DATE: 8/19/05 CHECKED BY: H.A. LOCKLEAR DATE: 8/26/05



STIRRUPS IN CAP MAY BE SHIFTED AS NECESSARY TO CLEAR DOWELS.

THE LATERAL GUIDE AT EACH END OF CAP IS NOT TO BE POURED UNTIL AFTER THE CORED SLAB UNITS ARE IN PLACE.



-const. — 4-#13U1 EL. 14.450 @ 150mm CTS. (TYP. OVER EA. (TYP.) TOP OF CAP -WORKLINE PIPE PILE) (LEVEL) --- 6-#13B4 OVER PILES #13B5 @ 1.200m CTS. (10 REQUIRED) 2-#13S2 (TYP.EA. PIPE PILE) 150 (TYP.) (2 BAR RUNS) (740mm SPLICE) /- 6-#29В2 #13B5 (TYP. EA. LATERAL GUIDE) — #13U5 (TYP.EA. LATERAL GUIDE) ----- #13U3 (TYP.EA.END) -----4-#13U2 (TYP. EA. END) --- #29U4 4-#36B1*--*— 1-#16B3 (EA. FACE) (TYP. EA. END) INVERT ALTERNATE STIRRUPS EL. 13.690 BOTTOM OF CAP 2-#16S1 @ 300mm CTS. 7-#16S1 @ 300mm CTS. (TYP. EA. END) (TYP.EACH BAY) 76mm HIGH B.B. @ 1.500 CTS. 2.700 m 2.700 m 2.700 m 2.700 m © PP 610 X 12.7 STEEL PILE

<u>PLAN</u>

PROJECT NO. B-1381

SAMPSON COUNTY

STATION:51+98.000-LREV-

SHEET 1 OF 2

12/20/06

DEPARTMENT OF TRANSPORTATION

SUBSTRUCTURE

BENT #2

REVISIONS

NO. BY: DATE: NO. BY: DATE: S-18

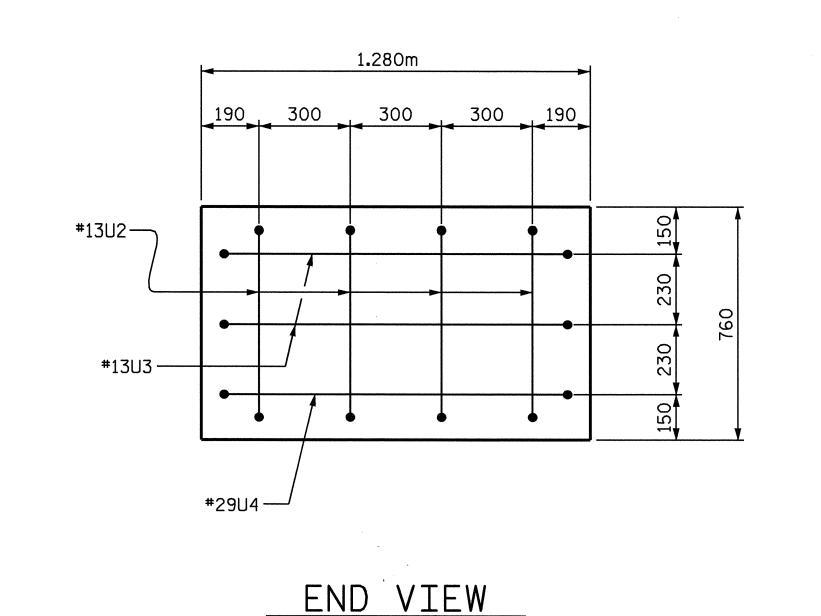
1 3 TOTAL SHEETS
29

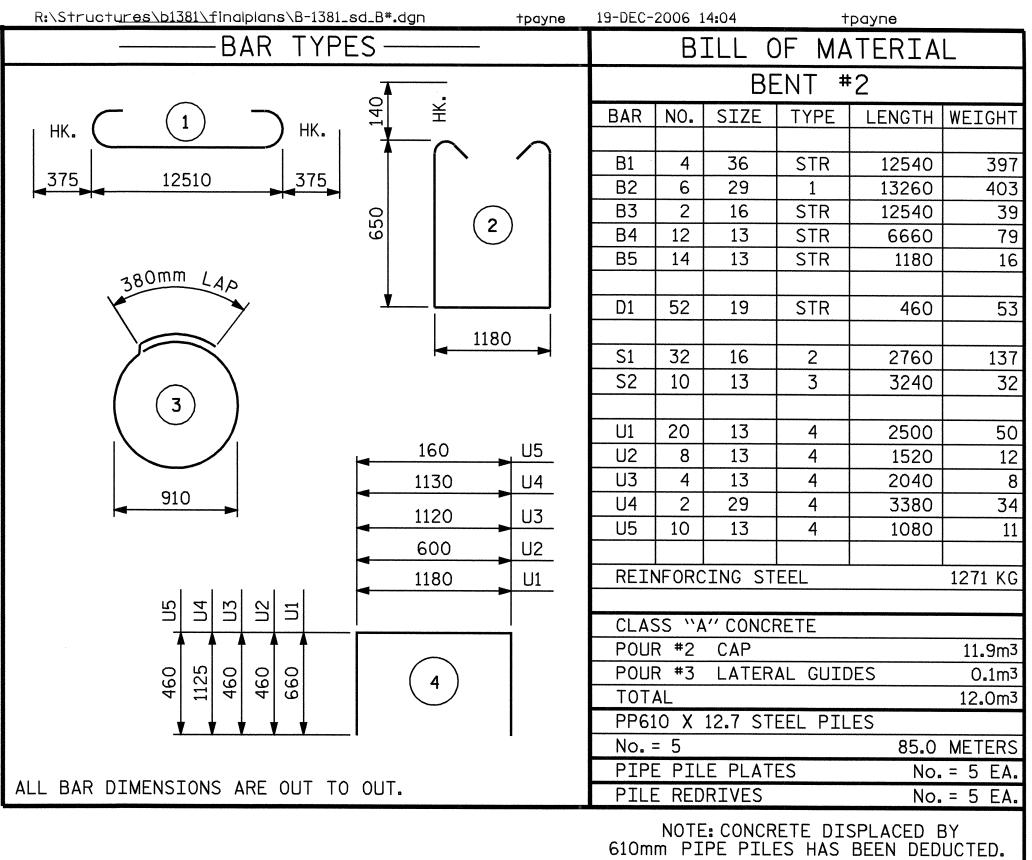
ELEVATION

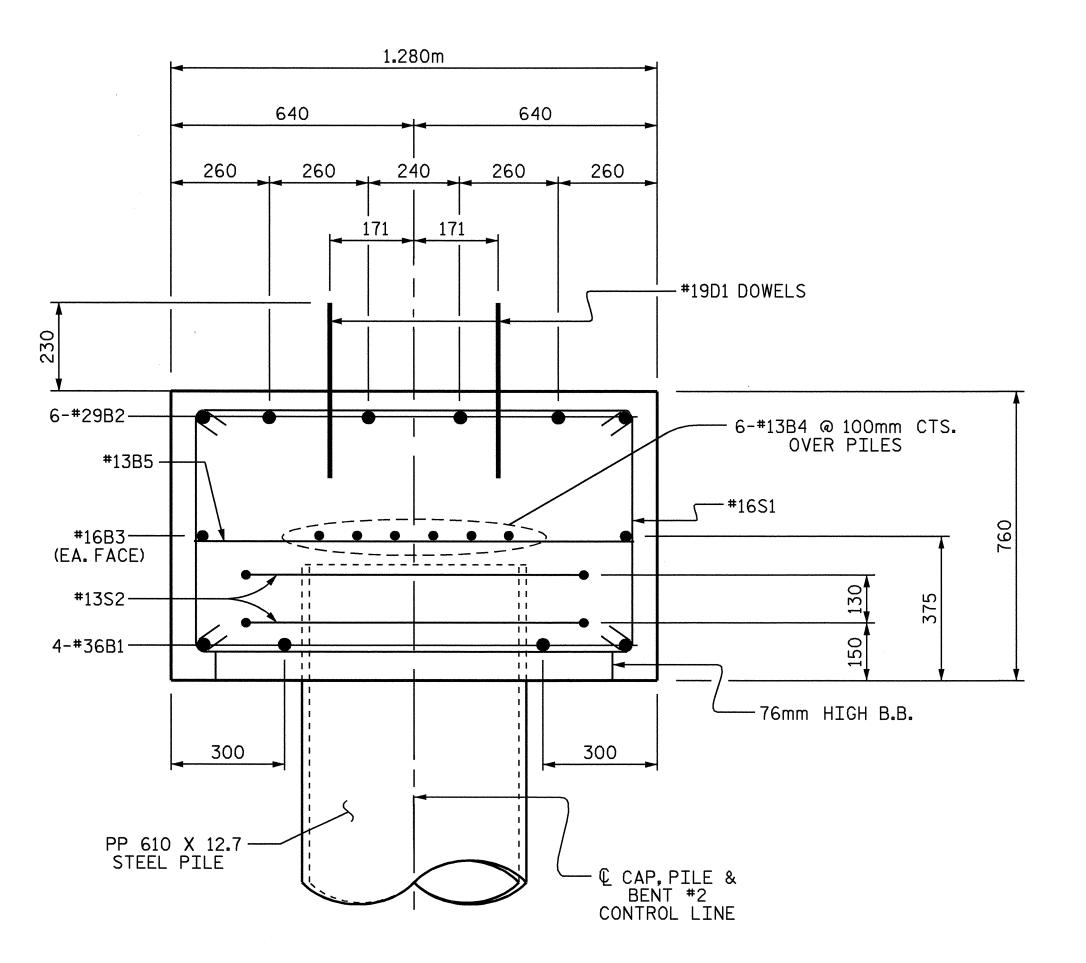
FOR REINFORCING STEEL AND OTHER DETAILS FOR PIPE PILE, SEE "PP 610 X 12.7 STEEL PILE" SHEET.

| 19-DEC-2006 | 14:2| R:\Structures\b|38|\fina|p|ans\B-|38|_sd_B#.dgn

DRAWN BY: J.P. ADAMS DATE: 8/18/05 CHECKED BY: H.A. LOCKLEAR DATE: 8/26/05







SECTION THRU CAP REINFORCING STEEL IN PIPE PILE NOT SHOWN FOR CLARITY.

B-1381 PROJECT NO. ____ SAMPSON _ COUNTY STATION:51+98.000-LREV-

SHEET 2 OF 2

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

> SUBSTRUCTURE BENT #2

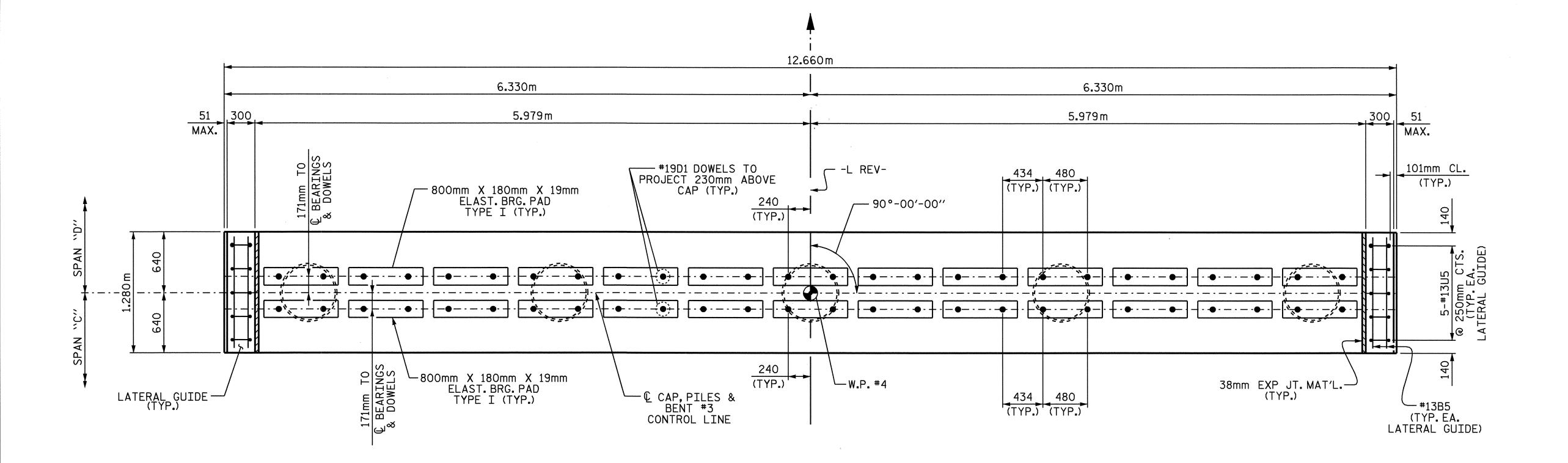
SHEET NO. S-19 **REVISIONS** NO. BY: DATE: NO. BY: DATE: TOTAL SHEETS 29

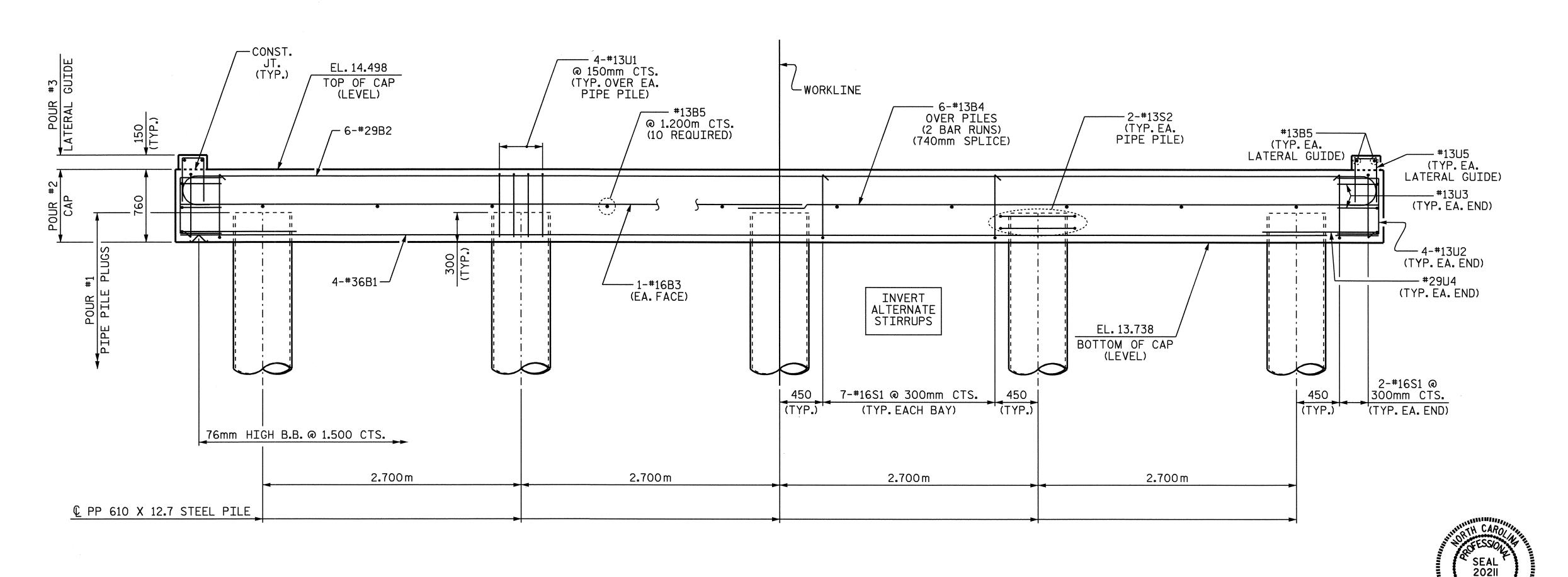
DRAWN BY: J.P. ADAMS DATE: 8/19/05 CHECKED BY: H.A. LOCKLEAR DATE: 8/26/05



STIRRUPS IN CAP MAY BE SHIFTED AS NECESSARY TO CLEAR DOWELS.

THE LATERAL GUIDE AT EACH END OF CAP IS NOT TO BE POURED UNTIL AFTER THE CORED SLAB UNITS ARE IN PLACE.





PLAN

PROJECT NO. B-1381

SAMPSON COUNTY

STATION:51+98.000-LREV-

SHEET 1 OF 2

STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION
RALEIGH

SUBSTRUCTURE

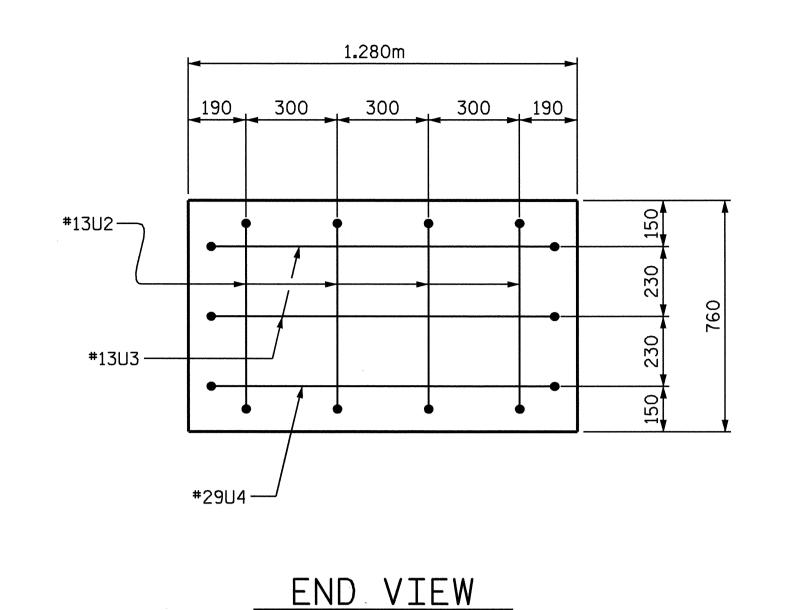
BENT #3

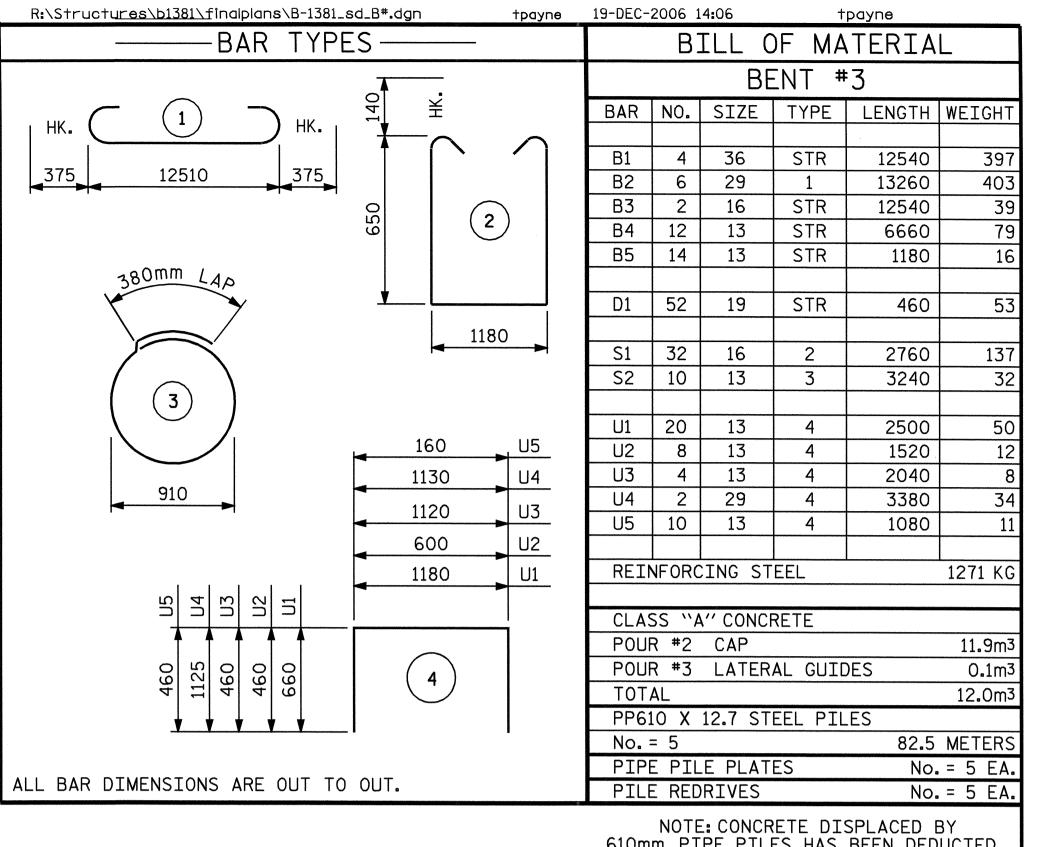
REVISIONS
SHEET NO. S-20
TOTAL SHEETS
29

DRAWN BY: J.P. ADAMS DATE: 8/18/05
CHECKED BY: H.A. LOCKLEAR DATE: 8/26/05

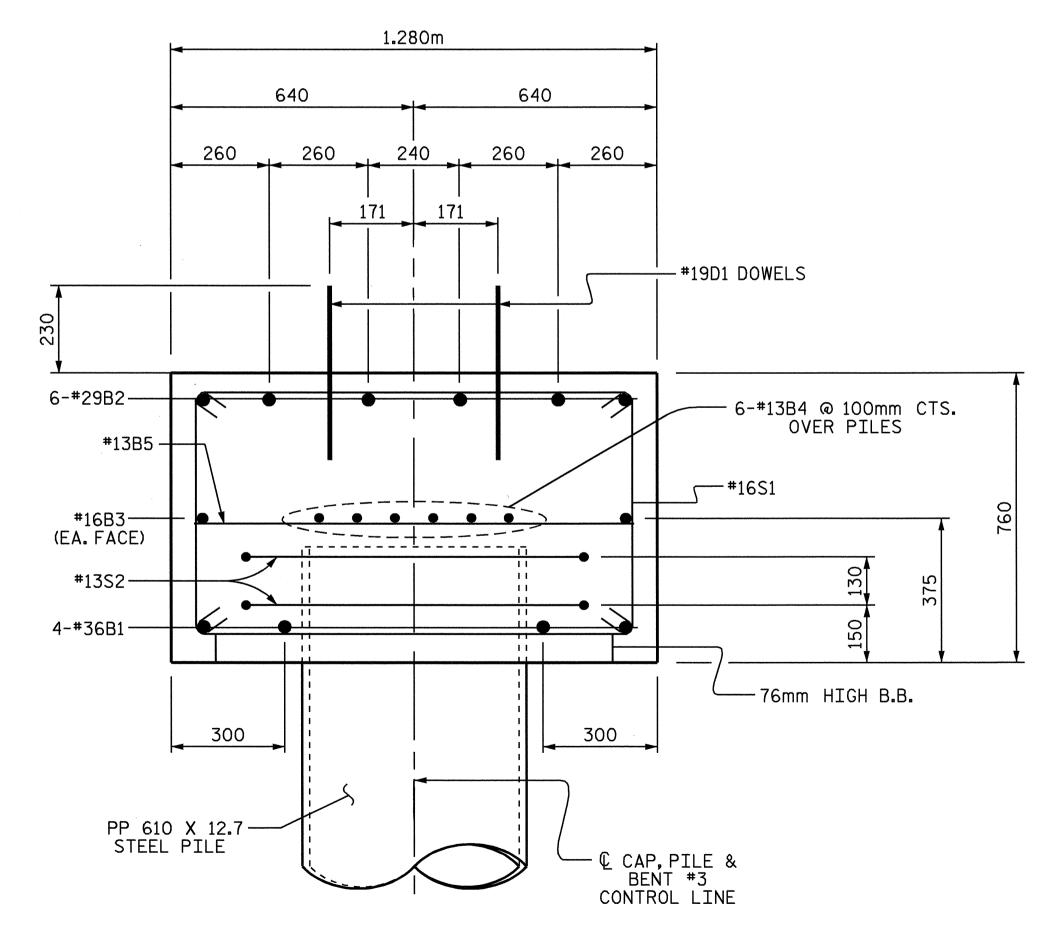
ELEVATION

FOR REINFORCING STEEL AND OTHER DETAILS FOR PIPE PILE, SEE "PP 610 X 12.7 STEEL PILE" SHEET.





NOTE: CONCRETE DISPLACED BY 610mm PIPE PILES HAS BEEN DEDUCTED.



SECTION THRU CAP

B-1381 PROJECT NO. ____ SAMPSON _ COUNTY STATION:51+98.000-LREV-

SHEET 2 OF 2

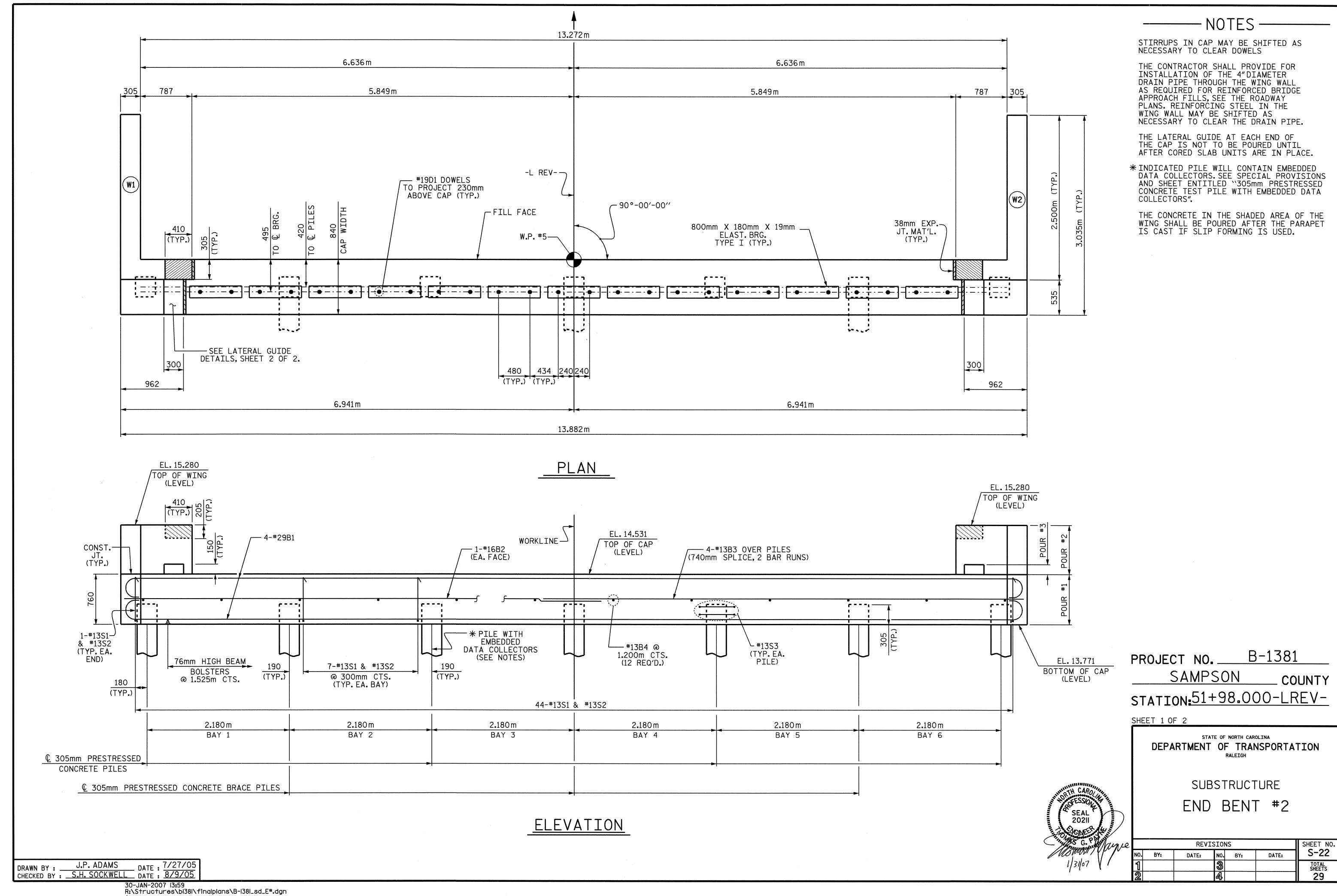
STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

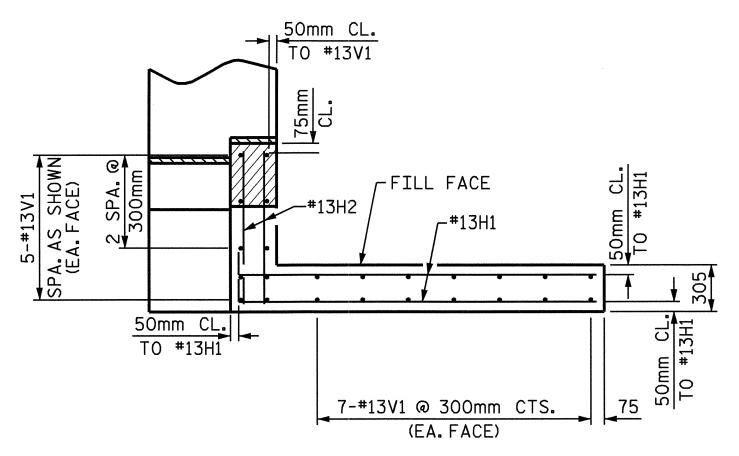
> SUBSTRUCTURE BENT #3

SHEET NO. S-21 **REVISIONS** DATE: DATE: BY: TOTAL SHEETS 29

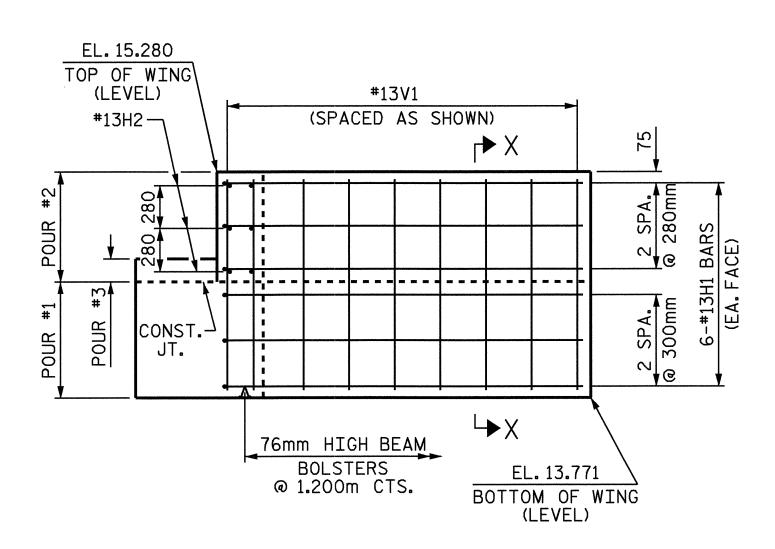
REINFORCING STEEL IN PIPE PILE NOT SHOWN FOR CLARITY.

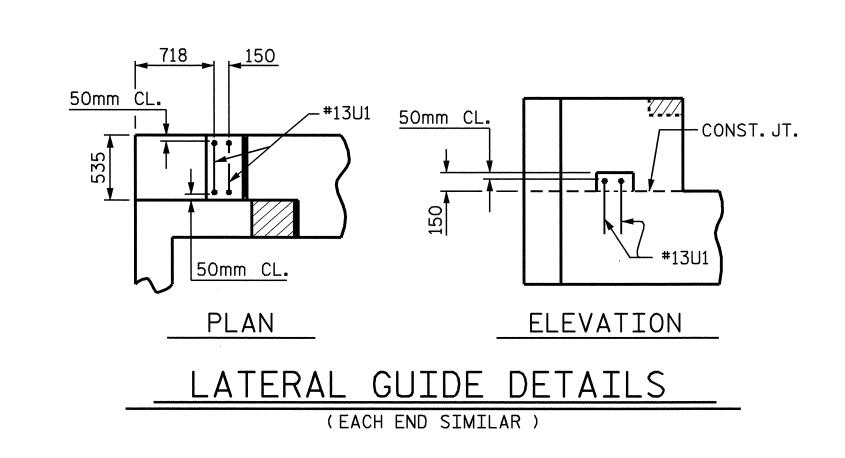
DRAWN BY: J.P. ADAMS DATE: 8/19/05 CHECKED BY: H.A. LOCKLEAR DATE: 8/26/05

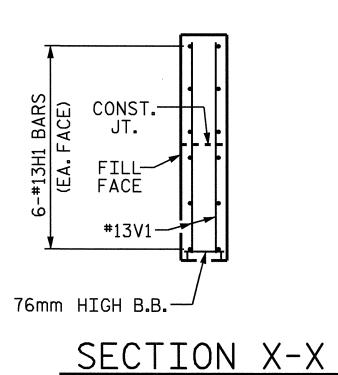




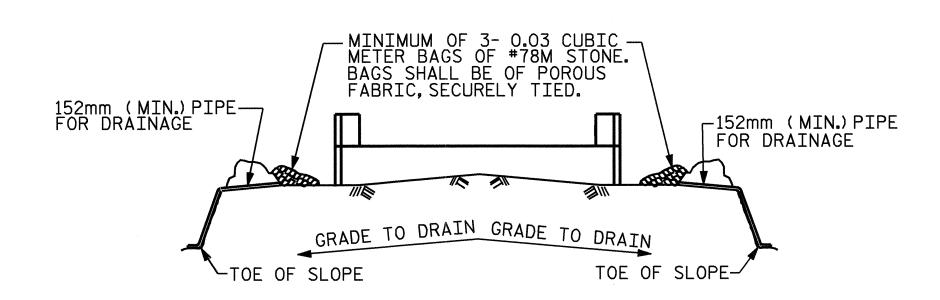
PLAN OF WING WING 2 SHOWN, WING 1 SIMILAR.







ELEVATION OF WING WING 2 SHOWN, WING 1 SIMILAR.

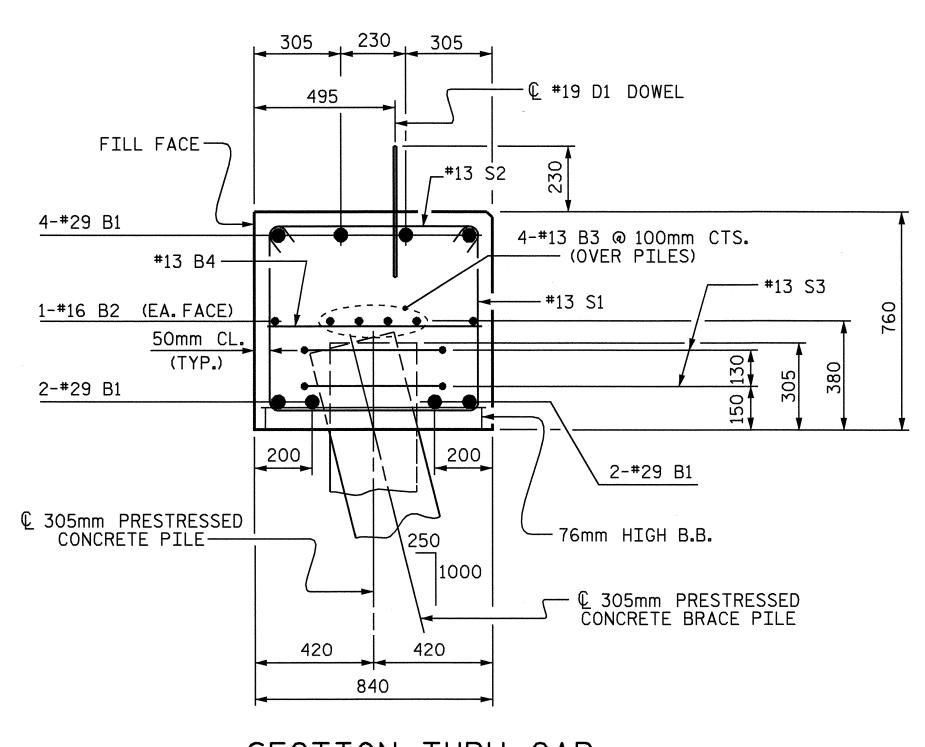


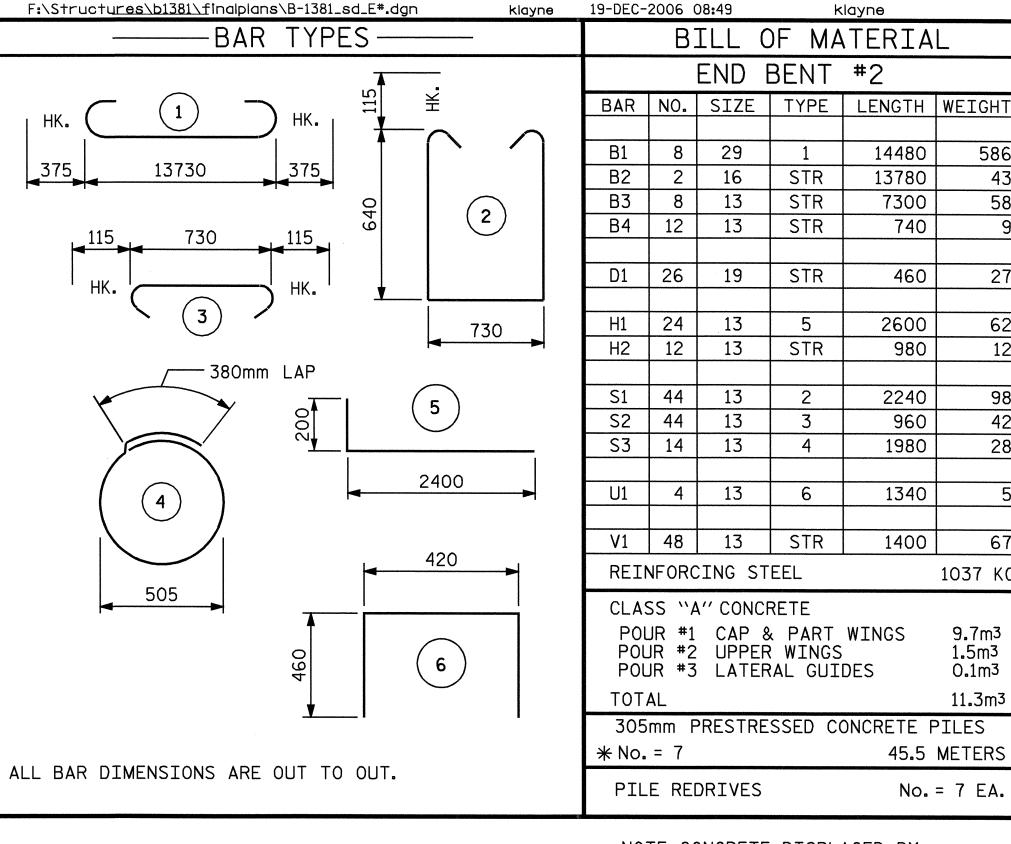
BAGGED STONE AND PIPE SHALL BE PLACED IMMEDIATELY AFTER COMPLETION OF END BENT EXCAVATION. PIPE MAY BE EITHER CONCRETE, CORRUGATED STEEL, CORRUGATED ALUMINUM ALLOY, OR CORRUGATED PLASTIC. PERFORATED PIPE WILL NOT BE ALLOWED.

BAGGED STONE SHALL REMAIN IN PLACE UNTIL THE ENGINEER DIRECTS THAT IT BE REMOVED. THE CONTRACTOR SHALL REMOVE AND DISPOSE OF SILT ACCUMULATIONS AT BAGGED STONE WHEN SO DIRECTED BY THE ENGINEER. BAGS SHALL BE REMOVED AND REPLACED WHENEVER THE ENGINEER DETERMINES THAT THEY HAVE DETERIORATED AND LOST THEIR EFFECTIVENESS.

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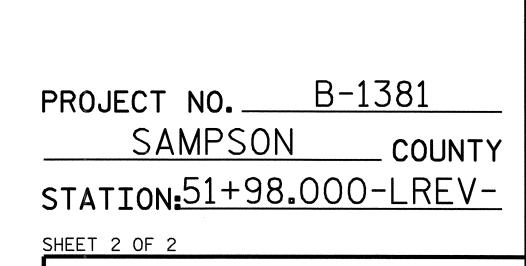
TEMPORARY DRAINAGE AT END BENT





NOTE: CONCRETE DISPLACED BY 305mm PRESTRESSED CONCRETE PILES HAS BEEN DEDUCTED.

* PILE WITH EMBEDDED DATA COLLECTORS IS INCLUDED IN THIS TOTAL.



DEPARTMENT OF TRANSPORTATION
RALEIGH

SUBSTRUCTURE END BENT #2

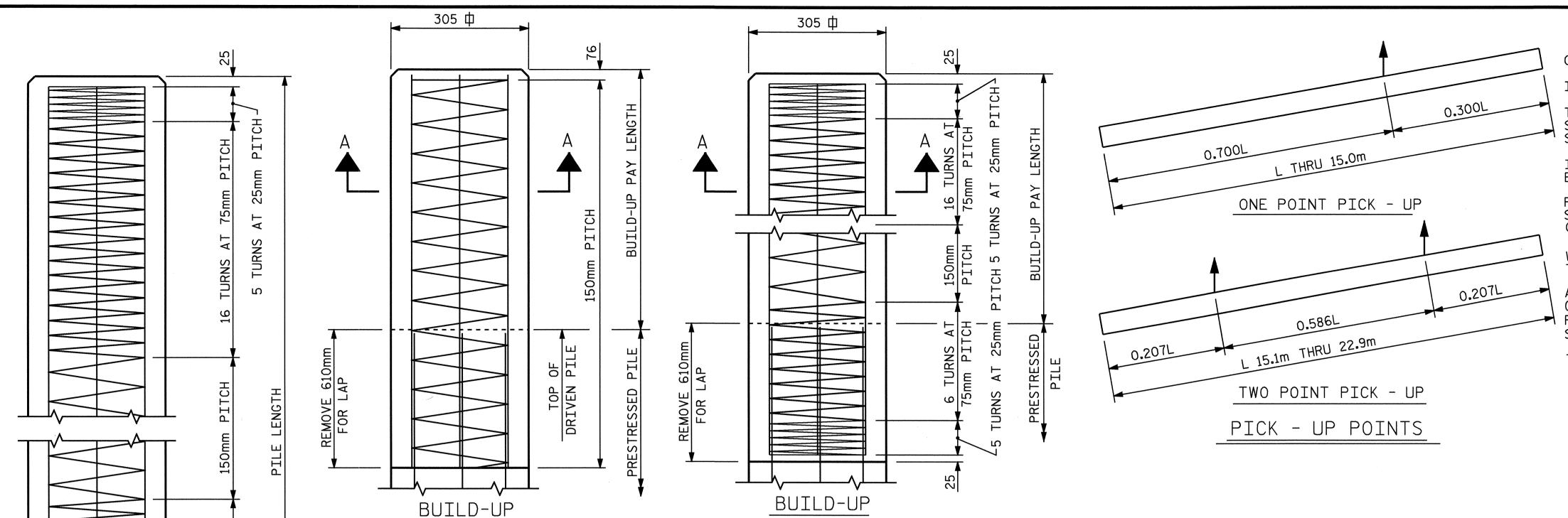
REVISIONS

No. BY: DATE: No. BY: DATE: S-23

1 3 TOTAL SHEETS
2 4 29

SECTION THRU CAP

DRAWN BY: J.P. ADAMS
CHECKED BY: S.H. SOCKWELL DATE: 8/1/05



WITH DRIVING

WITHOUT DRIVING

— 5 PRESTRESS STRANDS

∠ W3.5 COLD DRAWN STEEL WIRE SPIRAL

PRESTRESS STRANDS

TYP.

∠ W3.5 COLD DRAWN STEEL WIRE SPIRAL —

50 CL.

TYP.

PI

TYP.

PIT

305

TYPICAL SECTION

TYPICAL SECTION

QUAI	NTITIES	FOR ON	IE 305mr	m PREST	RESSED	PILE
LENGTH	CONCRETE	PILE WT.	ONE PICK	-UP POINT	TWO PIC	K-UP POINT
m	m ³	kg	0.300L m	0.700L m	0.207L m	0.586L m
7 . 5	0.70	1680	2.25	5.25		
9.0	0.84	2020	2.70	6.30		
10.5	0.98	2350	3 . 15	7.35		
12.0	1.12	2690	3 . 60	8.40		
13 . 5	1.26	3030	4.05	9.45		
15.0	1.40	3360	4.50	10.50		
16 . 5	1 . 53	3680			3.42	9.67
18.0	1.67	4010			3.73	10.55
19.5	1.81	4350			4.04	1 1.43
21.0	1.95	4690			4.35	12.31
22.5	2.09	5020			4.66	13.19
					:	
				1		

NOTES

CONCRETE DESIGN DATA: f'c = 34.5 MPa; fc = 13.8 MPa

IMPACT IN HANDLING = 50%

THE TRANSFER OF LOAD FROM THE ANCHORAGES TO THE PILE SHALL BE DONE WHEN THE CONCRETE HAS REACHED A COMPRESSIVE STRENGTH OF NOT LESS THAN 24.1 MPa.

IN DRIVING PILES, A METHOD APPROVED BY THE ENGINEER SHALL BE USED. WHEREBY THE HEAD OF THE PILE IS NOT DAMAGED.

PROPOSED DEVICES FOR LIFTING PILES, RECESS DETAILS, AND PATCHING MATERIAL SHALL BE DETAILED IN SHOP DRAWINGS. AFTER ATTACHMENTS HAVE BEEN REMOVED, OPENINGS SHALL BE REPAIRED SUCH THAT THE APPEARANCE OF THE PILE IS UNIFORM.

WHERE CAST - IN - PLACE LIFTING DEVICES ARE NOT USED. PICK-UP POINTS TO BE INDICATED WITH A BLACK MARK 50mm WIDE.

ALL PRESTRESSING STRANDS SHALL BE 7-WIRE LOW RELAXATION GRADE 270 STRANDS AND SHALL CONFORM TO AASHTO M203M EXCEPT FOR SAMPLING REQUIREMENTS WHICH SHALL BE IN ACCORDANCE WITH STANDARD SPECIFICATIONS.

THE CONTRACTOR MAY USE EITHER OF THE FOLLOWING STRAND CONFIGURATIONS:

SIZE	GRADE	NUMBER OF STRANDS	AREA mm ²	ULTIMATE STRENGTH KN	APPLIED PRESTRESS FORCE kN
12.70	270	4	98.71	183.7 PER STRAND	137.8 PER STRAND
12.70	270	5	98.71	183.7 PER STRAND	137.8 PER STRAND

THE SLIP-FORM METHOD OF CASTING PILES WILL NOT BE PERMITTED.

IF STRAND STRESS IS RELIEVED BY BURNING, THE STRANDS SHALL BE BURNED IN PAIRS, EXCEPT WHERE 5 STRANDS ARE USED THE LAST STRAND MAY BE BURNED SINGLY, ACCORDING TO BURNING PATTERNS SHOWN. NOT MORE THAN 4 STRANDS MAY BE BURNED AT ANY ONE SECTION BEFORE THE SAME STRANDS ARE BURNED AT BOTH ENDS OF THE BED AND BETWEEN EACH PAIR OF PILES IN THE BED.

BUILD-UPS SHALL BE 'CLASS A' CONCRETE WITH 20% ADDITIONAL CEMENT. NO DRIVING OF THE BUILT-UP PILE WILL BE PERMITTED UNTIL THE CONCRETE HAS REACHED A COMPRESSIVE STRENGTH OF 20.7 MPa AND UNTIL A PERIOD OF SEVEN DAYS HAS ELAPSED SINCE CASTING OF THE BUILD-UP.

ALL CORNERS TO BE CHAMFERED 19mm.

FOR PRESTRESSED CONCRETE MEMBERS, SEE SPECIAL PROVISIONS.

FOR PRESTRESSED CONCRETE PILES, SEE SPECIAL PROVISIONS.

FOR 305mm PRESTRESSED CONCRETE PILE WITH EMBEDDED DATA COLLECTORS, SEE SHEET ENTITLED "305mm PRESTRESSED CONCRETE TEST PILE WITH EMBEDDED DATA COLLECTORS".

> PROJECT NO. B-1381 SAMPSON COUNTY STATION: 51+98.000-LREV-

> > STATE OF NORTH CAROLINA



305mm PRESTRESSED CONCRETE PILE

		RE\	/ISIONS			SHEET NO.
10.	BY:	DATE:	NO.	BY:	DATE:	S-24
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<u>න</u>						ll 29

LENGTH	CONCRETE	PILE WT.	ONE PICK	-UP POINT	TWO PICK	C-UP POINT
m	m ³	kg	0.300L m	0.700L m	0.207L m	0.586L m
7.5	0.70	1680	2.25	5.25		
9.0	0.84	2020	2.70	6.30		
10.5	0.98	2350	3 . 15	7 . 35		
12.0	1.12	2690	3.60	8.40		
13 . 5	1.26	3030	4.05	9.45		
15.0	1.40	3360	4.50	10.50		
16.5	1 . 53	3680			3.42	9.67
18.0	1.67	4010			3.73	10.55
19.5	1.81	4350			4.04	1 1.43
21.0	1.95	4690			4.35	12.31
22.5	2.09	5020			4.66	13.19
					1	

ASSEMBLED BY : J.P. ADAMS DATE: 9/20/05 CHECKED BY : S.H.SOCKWELL DATE: 9/21/05 DRAWN BY: FCJ 7/88 REV. 7/17/98 RWW/LES RECKED BY: CRK 3/89 REV. 5/1/06 RWW/LES REV. 5/1/06

305 中

ELEVATION

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SECTION "A-A" FOR BURNING STRANDS

- 5-#16 BARS

SECTION "A-A" FOR BURNING STRANDS

-4- #16 BARS

12.70mm Ø GRADE 270 L.R. PRESTRESS STRANDS

12.70mm Ø GRADE 270 L.R. PRESTRESS STRANDS

TYPICAL PATTERN

TYPICAL PATTERN

5 TURNS @ 25mm PITCH

ELEVATION

NOTES

FOR ADDITIONAL NOTES AND DETAILS, SEE "305mm PRESTRESSED CONCRETE PILE" SHEET. ONLY THE 4 STRAND PATTERN CONFIGURATION IS PERMITTED FOR THE TEST PILES. BUILD-UP OF TEST PILES IS NOT PERMITTED.

EMBEDDED DATA COLLECTORS (EDC) ARE REQUIRED FOR THE PRESTRESSED CONCRETE PILES TESTED WITH THE PILE DRIVING ANALYZER (PDA) AT END BENTS No. 1 AND No. 2. FOR EMBEDDED DATA COLLECTORS, SEE SPECIAL PROVISIONS.

THE EDC CONSULTANT WILL PROVIDE AND INSTALL THE EDC DURING PILE FABRICATION. NOTIFY THE ENGINEER OF THE PILE FABRICATION SCHEDULE A MINIMUM OF 14 CALENDAR DAYS IN ADVANCE.

WRAP THE SENSOR WIRES, USING PLASTIC OR METAL TIES, AGAINST THE PRESTRESS STRANDS EVERY 300mm WHERE APPROPRIATE.

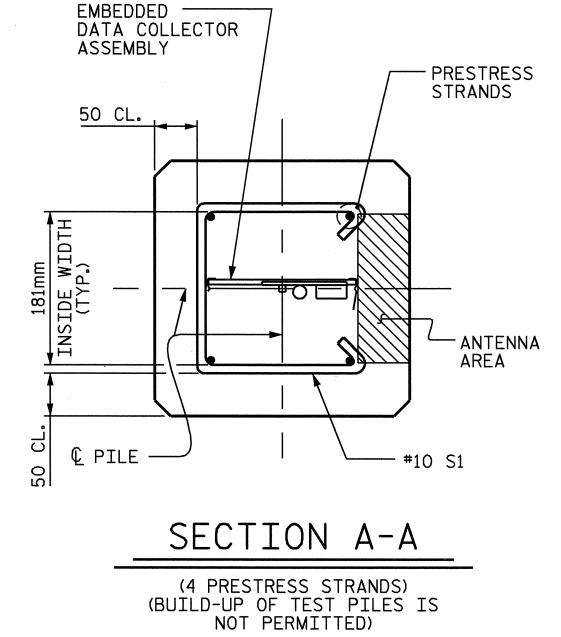
CLEARLY MARK THE LOCATION OF THE RADIO ANTENNA IN THE FRAME.

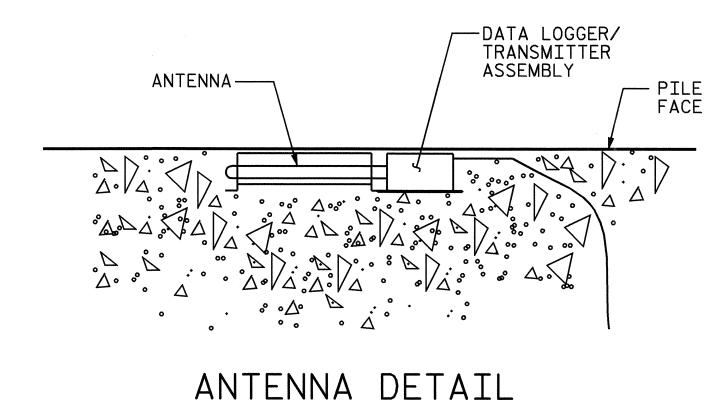
DURING THE PLACEMENT AND SETTING OF THE CONCRETE, ENSURE THAT VIBRATION SETTLING IS NOT PERFORMED WITHIN 100mm ON EITHER SIDE OF THE ANTENNA AND TRANSDUCER PACKAGES. (CAUTION: DO NOT ALLOW VIBRATOR TO COME IN CONTACT WITH EMBEDDED DATA COLLECTORS.)

COMPLETE THE PILE FINISHING PROCESS BY PLACING AND SCRIBING / MARKING EMBEDDED DATA COLLECTOR SERIAL NUMBER ALONG WITH OTHER PILE DETAILS (E.G. PILE LENGTH, CASTING DATA, JOB NUMBERS) ON THE HAND-FINISHED SURFACE AT THE TOP OF THE PILE.

THE REINFORCING ARRANGEMENT DETAILED CAN ONLY BE USED FOR THE TEST PILES (2 TOTAL, 1 AT EACH END BENT). FOR PRODUCTION PILE DETAILS, SEE "305mm PRESTRESSED CONCRETE PILE" SHEET.

THE 305mm PRESTRESSED CONCRETE PILES WITH EMBEDDED DATA COLLECTORS SHALL BE FABRICATED TO THE LENGTH SHOWN ON THIS SHEET.





DEPARTMENT OF TRANSPORTATION
RALEIGH

305mm PRESTRESSED
CONCRETE TEST DILE

305mm PRESTRESSED CONCRETE TEST PILE WITH EMBEDDED DATA COLLECTORS

STATE OF NORTH CAROLINA

200

#10S1 BAR

 REVISIONS
 SHEET NO.

 NO.
 BY:
 DATE:
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 1
 3
 TOTAL SHEETS

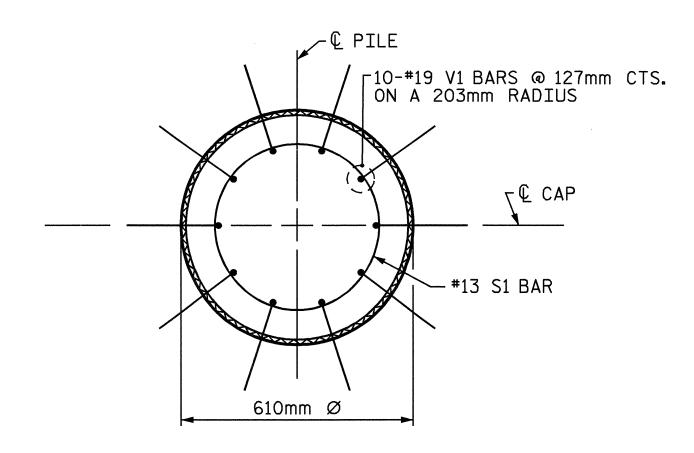
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PROJECT NO. B-1381

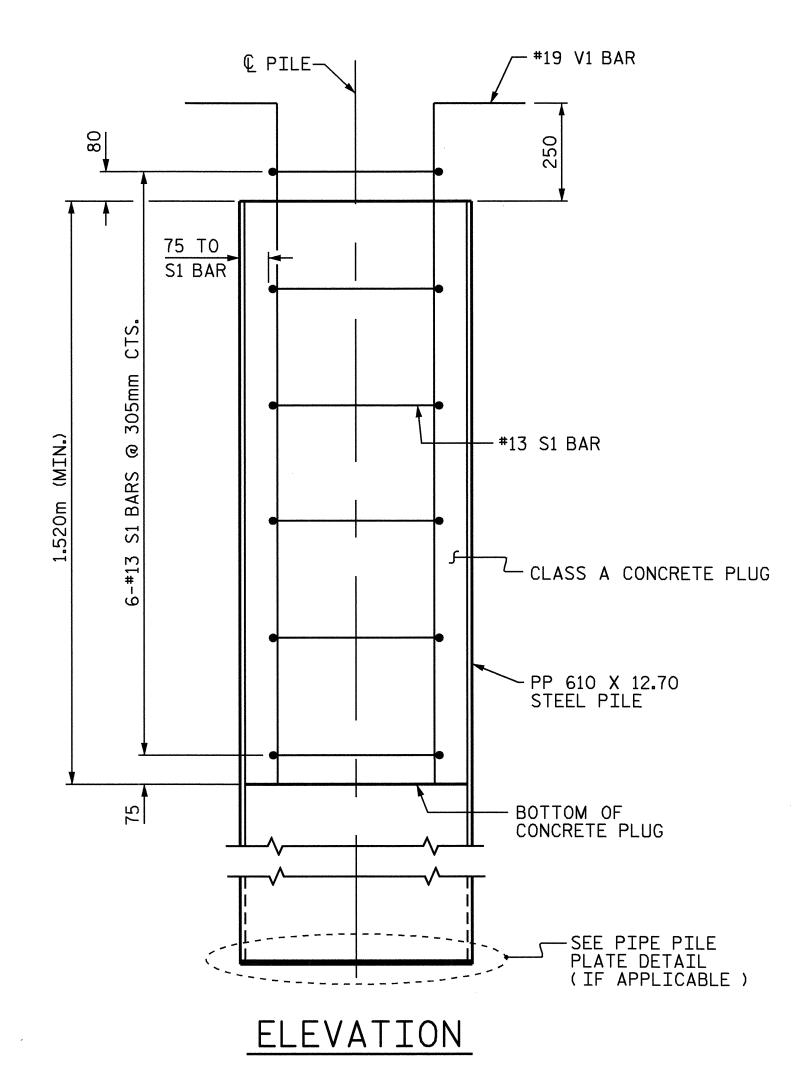
SAMPSON COUNTY

STATION: 51+98.000 -LREV.-





<u>PLAN</u>



PP 610 X 12.70 STEEL PILE

NOTES

STEEL PIPE PILES SHALL BE OF UNIFORM DIAMETER AND MEET THE REQUIREMENTS OF ASTM A252, GRADE 3 MODIFIED (344.7 MPa YIELD STRENGTH).

GALVANIZE STEEL PIPE PILES IN ACCORDANCE WITH THE BEARING PILE SPECIAL PROVISION (NOVEMBER 20, 2006). GALVANIZING PIPE PILE PLATES IS NOT REQUIRED.

REMOVE AND REPLACE OR REPAIR TO THE SATISFACTION OF THE ENGINEER PILES THAT ARE DAMAGED, DEFORMED OR COLLAPSED DURING INSTALLATION OR DRIVING.

PILE SPLICES SHALL BE IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS AND AWS D1.1.

FOR CLOSED END PIPE PILES, REMOVE ALL SOIL AND WATER FROM INSIDE THE PILES JUST PRIOR TO PLACING REINFORCING STEEL AND CONCRETE FOR THE CONCRETE PLUG.

FOR OPEN END PIPE PILES, REMOVE ENOUGH SOIL AND WATER FROM INSIDE THE PILES TO CONSTRUCT THE CONCRETE PLUG WITHOUT FOULING THE CONCRETE.

FORM THE CONCRETE PLUG SUCH THAT THE REINFORCING STEEL OR CONCRETE DOES NOT MOVE AND THE CLEARANCE FROM THE REINFORCING STEEL TO THE INSIDE OF THE PILE IS MAINTAINED AFTER CONCRETE PLACEMENT. DO NOT PLACE CONCRETE IN THE BENT CAP UNTIL THE CONCRETE PLUG HAS ATTAINED A MINIMUM COMPRESSIVE STRENGTH OF 10.3 MPa.

THE REINFORCING STEEL, CLASS A CONCRETE AND GALVANIZING ARE CONSIDERED INCIDENTAL TO THE CONTRACT UNIT PRICE BID PER METER FOR PP 610 X 12.70 STEEL PILES.

BILL OF MATERIAL FOR ONE PP 610 X 12.70 STEEL PILE					
BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT
S1	6	#13	1	1840	11
V1	10	#19	2	2020	45

CLASS A CONCRETE

1.520m MINIMUM PLUG

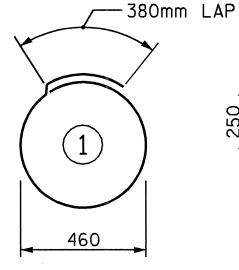
0.4 CU. METER

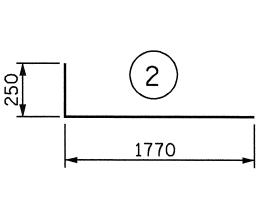
kg

56

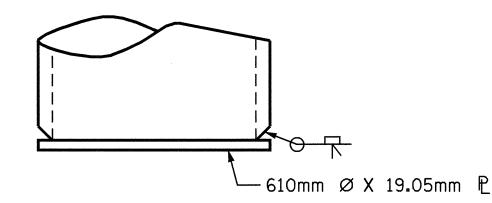
BAR TYPES

REINFORCING STEEL =



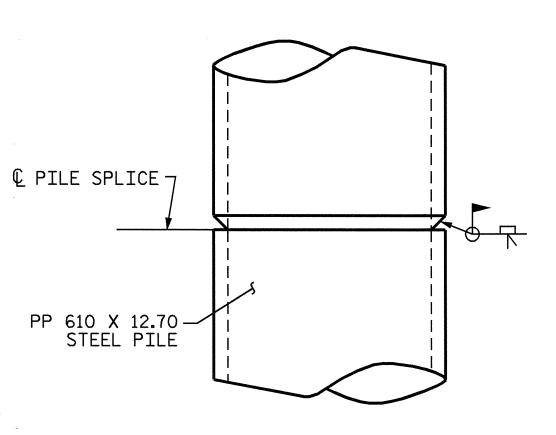


ALL BAR DIMENSIONS ARE OUT TO OUT.



PIPE PILE PLATE DETAIL

(IF APPLICABLE)



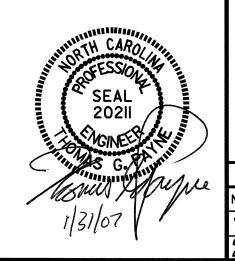
PIPE PILE SPLICE DETAIL

PROJECT NO. B-1381

SAMPSON COUNTY

STATION: 51+98.000-LREV-

DEPARTMENT OF TRANSPORTATION
RALEIGH
STANDARD
610mm STEEL
PIPE PILE



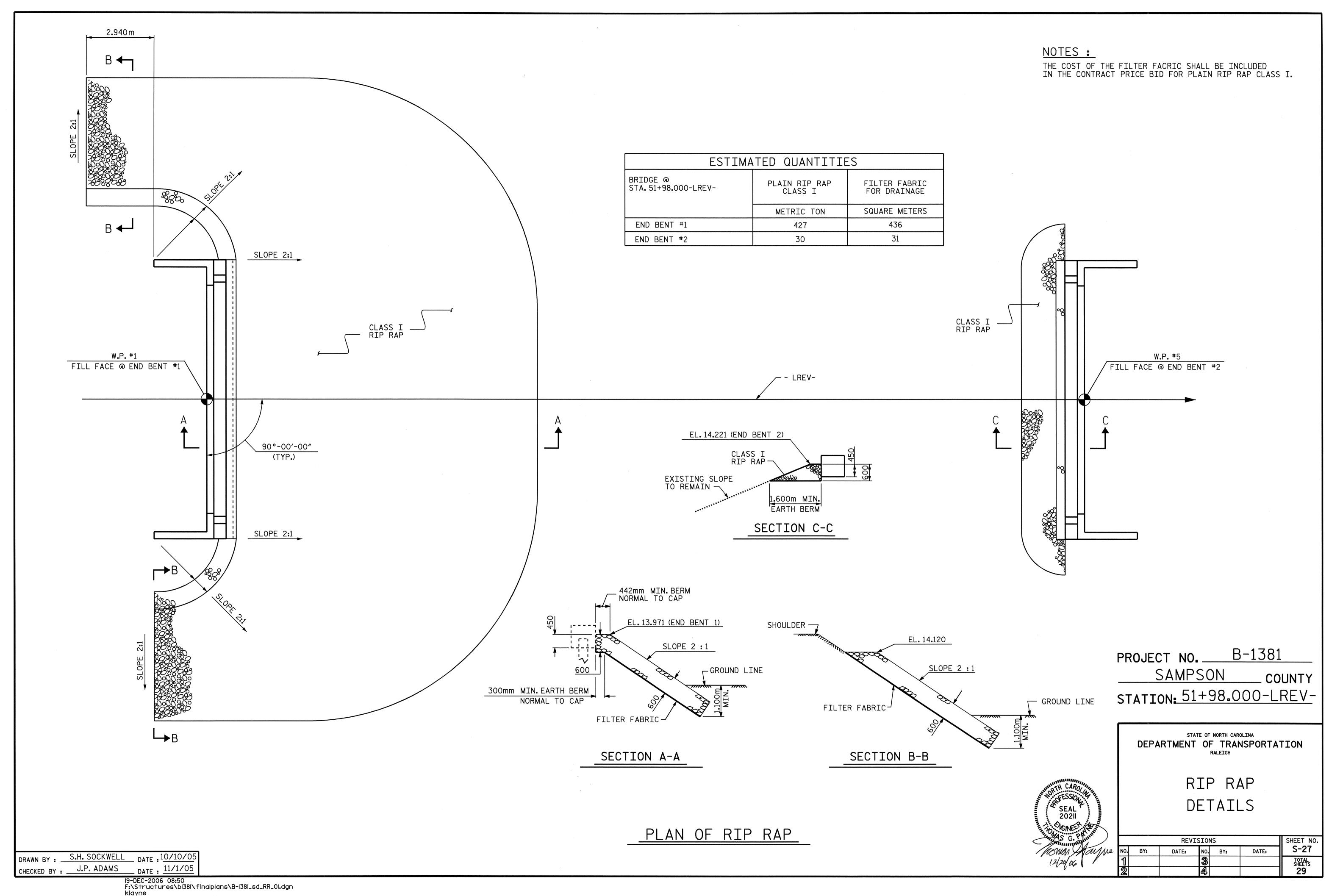
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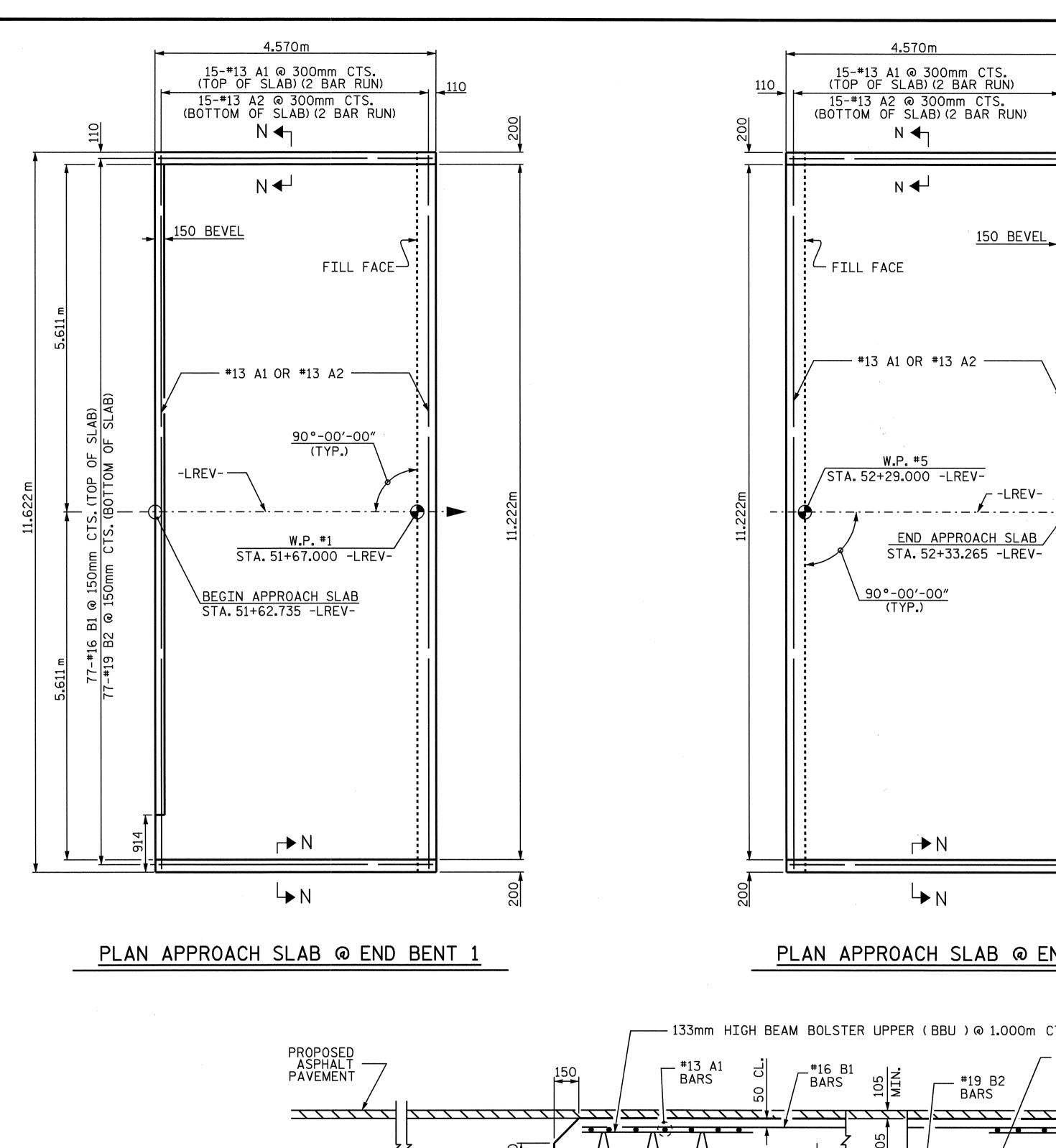
STD. NO. SPP4SM

ASSEMBLED BY: J.P. ADAMS DATE: 11/27/06 CHECKED BY: H. A. LOCKLEAR DATE: 11/27/06

DRAWN BY: TLA 8/05 CHECKED BY: GM 9/05

ADDED 10/1/05 REV. 5/1/06 TLA/GM





ASSEMBLED BY : A. CHAN

DRAWN BY: FCJ 6/87 CHECKED BY : EGA 6/87

CHECKED BY: K.D. LAYNE

DATE:10/27/06

DATE:10/27/06

REV. 10/17/00 RWW/LES REV. 7/10/01 LES/RDR

REV. 5/7/03R RWW/JTE

NOTES

FOR REINFORCED BRIDGE APPROACH FILL INCLUDING FABRIC, IMPERMEABLE GEOMEMBRANE, 102mm Ø DRAINAGE PIPE, #78M STONE, AND SELECT MATERIAL, SEE ROADWAY PLANS.

TEMPORARY DRAINAGE AND TEMPORARY BERM AND SLOPE DRAINS WILL BE PAID FOR UNDER THE LUMP SUM PRICE FOR BRIDGE APPROACH SLAB.

AREA BETWEEN THE WINGWALL AND APPROACH SLAB SHALL BE GRADED TO DRAIN THE WATER AWAY FROM THE FILL FACE OF THE BRIDGE AND SHALL BE PAVED. SEE ROADWAY PLANS.

THE 150mm COMP. A.B.C. SHALL EXTEND 3m BEYOND THE END OF THE APPROACH SLAB AND 300mm OUTSIDE OF EACH EDGE OF THE SLAB.

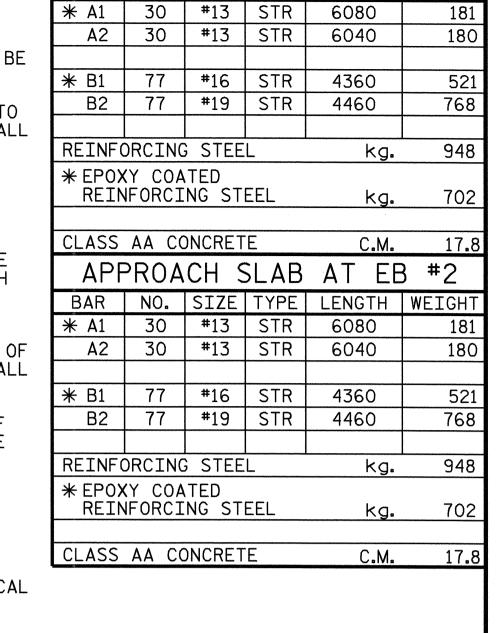
THE CONTRACTOR MAY USE 100mm TYPE B-25.0B ASPHALT CONCRETE COURSE IN LIEU OF 150mm COMP. A.B.C. IF THIS OPTION IS USED, THE BASE COURSE SHALL EXTEND 300mm BEYOND THE END OF THE APPROACH SLAB AND THE WIDTH SHALL BE THE SAME AS THAT OF THE APPROACH SLAB.

THE CONTRACTOR MAY USE 125mm CLASS "A" CONCRETE BASE IN LIEU OF 150mm COMP. A.B.C. IF THIS OPTION IS USED, THE CONCRETE BASE SHALL EXTEND 300mm BEYOND THE END OF THE APPROACH SLAB AND THE WIDTH SHALL BE THE SAME AS THAT OF THE APPROACH SLAB. THE CONCRETE SHALL BE FINISHED TO A SMOOTH SURFACE AND A LAYER OF 13.6 kg ROOFING FELT SHALL BE PLACED BETWEEN THE CONCRETE BASE AND THE APPROACH SLAB TO PREVENT BOND. THE APPROACH SLAB SHALL NOT BE CAST UNTIL THE CONCRETE BASE HAS REACHED AN AGE OF THREE CURING DAYS.

FOR JOINT DETAILS, SEE "PRESTRESSED CONCRETE CORED SLAB UNIT" SHEETS.

THE JOINT AT THE END BENT SHALL BE GROUTED AS SOON AS PRACTICAL AFTER THE CONSTRUCTION OF THE APPROACH SLAB.

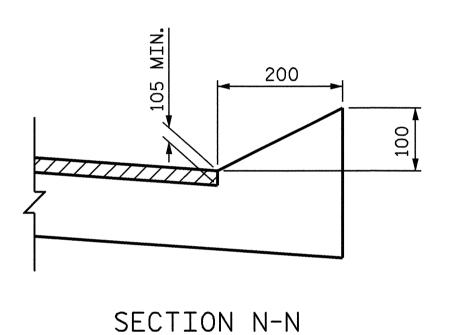
APPROACH SLAB GROOVING IS NOT REQUIRED.

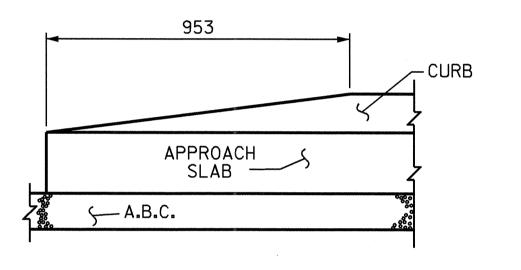


BILL OF MATERIAL

APPROACH SLAB AT EB #1

BAR NO. SIZE TYPE LENGTH WEIGHT





END OF CURB WITHOUT SHOULDER BERM GUTTER

CURB DETAILS

SPLICE CHART				
BAR	SIZE	SPLICE LENGTH		
A1	#13	610		
A 2	#13	540		

PROJECT NO. B-1381 SAMPSON _ COUNTY STATION: 51+98.00 -LREV

SHEET 1 OF 2

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

STANDARD

BRIDGE APPROACH SLAB FOR PRESTRESSED CONCRETE CORED SLAB

	REVISIONS					SHEET NO.
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PLAN APPROACH SLAB @ END BENT 2

---LREV-

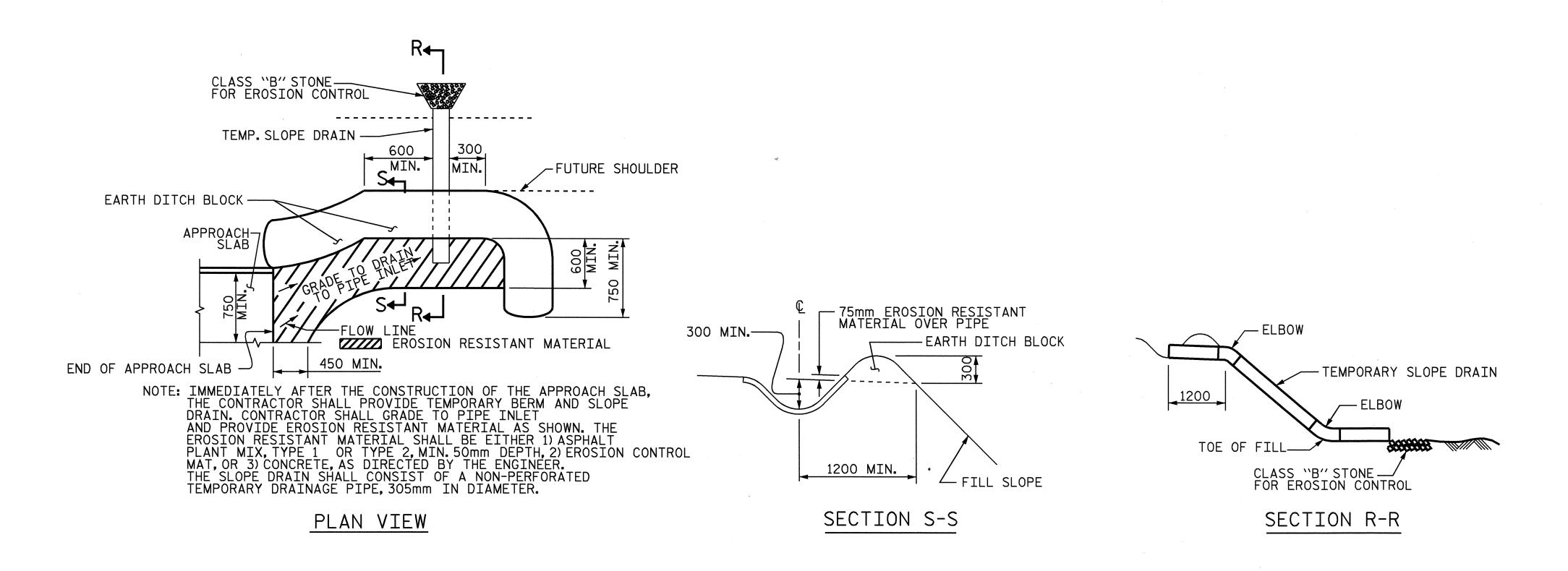
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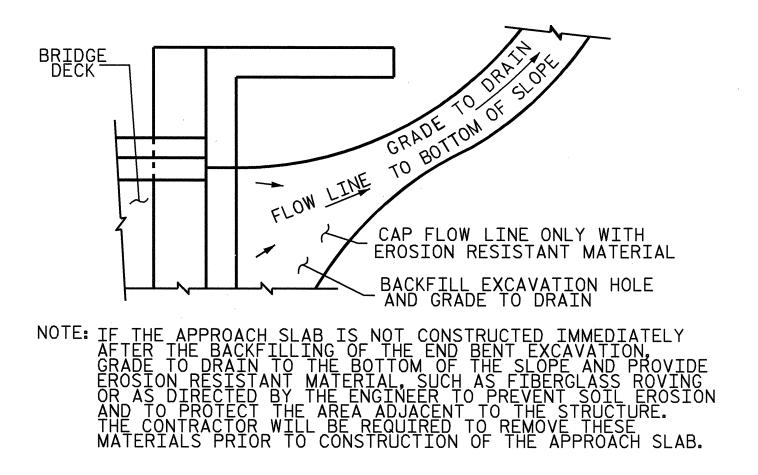
77-#16 | -#19 B2

PROPOSED ASPHALT PAVEMENT 15	133mm HIGH BEAM BOLSTER UPPER (BBU) @ 1.000m CTS. ACROSS SLAB *13 A2 BARS *19 B2 *2 :1 SLOPE CORED SLAB APPROVED WIRE BAR SUPPORTS @ 1.000m CTS. & G LIMITS OF REINFORCED BRIDGE APPROACH FILL (ROADWAY PAY ITEM, SEE NOTES) FABRIC SELECT MATERIAL FABRIC SELECT MATERIAL
†NORMAL TO END BENT	#78M STONE 2 LAYERS OF 13.6 kg ROOFING FELT TO PREVENT BOND PERFORATED DRAINAGE PIPE IMPERMEABLE GEOMEMBRANE

SECTION THRU SLAB



TEMPORARY BERM AND SLOPE DRAIN DETAILS



TEMPORARY DRAINAGE DETAIL

PROJECT NO. B-1381

SAMPSON COUNTY

STATION: 51+98.000-LREV-

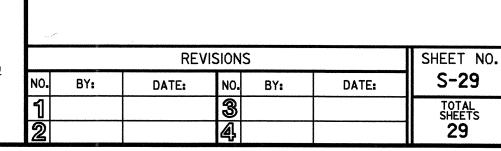
SHEET 2 OF 2

STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION
RALEIGH

STANDARD

BRIDGE APPROACH SLAB DETAILS



ASSEMBLED BY: A. CHAN DATE: 10/27/06 CHECKED BY: K.D. LAYNE DATE: 10/27/06 DRAWN BY: FCJ II/88 REV. 8/16/99 RAL/LES REV. 10/17/00 RWW/LES CHECKED BY: ARB II/88 REV. 5/7/03 RWW/JTE

STANDARD NOTES

DESIGN DATA:

---- A.A.S.H.T.O. (CURRENT) **SPECIFICATIONS** ----- SEE PLANS LIVE LOAD ---- SEE A.A.S.H.T.O. IMPACT ALLOWANCE STRESS IN EXTREME FIBER OF STRUCTURAL STEEL - AASHTO M270 GRADE 36 - 20,000 LBS. PER SQ. IN. - AASHTO M270 GRADE 50W - 27,000 LBS. PER SQ. IN. - AASHTO M270 GRADE 50 - 27,000 LBS. PER SQ. IN. REINFORCING STEEL IN TENSION GRADE 60 - - 24,000 LBS. PER SQ. IN. ---- 1,200 LBS. PER SQ. IN. CONCRETE IN COMPRESSION CONCRETE IN SHEAR ---- SEE A.A.S.H.T.O. STRUCTURAL TIMBER - TREATED OR ---- 1.800 LBS. PER SQ. IN. UNTREATED - EXTREME FIBER STRESS COMPRESSION PERPENDICULAR TO GRAIN 375 LBS. PER SQ. IN. OF TIMBER ----30 LBS. PER CU. FT. EQUIVALENT FLUID PRESSURE OF EARTH

MATERIAL AND WORKMANSHIP:

EXCEPT AS MAY OTHERWISE BE SPECIFIED ON PLANS OR IN THE SPECIAL PROVISIONS, ALL MATERIAL AND WORKMANSHIP SHALL BE IN ACCORDANCE WITH THE 2002 STANDARD SPECIFICATIONS "FOR ROADS AND STRUCTURES" OF THE N. C. DEPARTMENT OF TRANSPORTATION.

(MINIMUM)

STEEL SHEET PILING FOR PERMANENT OR TEMPORARY APPLICATIONS SHALL BE HOT ROLLED.

CONCRETE:

UNLESS OTHERWISE REQUIRED ON PLANS, CLASS A CONCRETE SHALL BE USED FOR ALL PORTIONS OF ALL STRUCTURES WITH THE EXCEPTION THAT: CLASS AA CONCRETE SHALL BE USED IN BRIDGE SUPERSTRUCTURES, ABUTMENT BACKWALLS, AND APPROACH SLABS; CLASS B CONCRETE SHALL BE USED FOR SLOPE PROTECTION AND RIP RAP; AND CLASS S SHALL BE USED FOR UNDERWATER FOOTING SEALS.

CONCRETE CHAMFERS:

UNLESS OTHERWISE NOTED ON THE PLANS, ALL EXPOSED CORNERS ON STRUCTURES SHALL BE CHAMFERED 3/4" WITH THE FOLLOWING EXCEPTIONS: TOP CORNERS OF CURBS MAY BE ROUNDED TO 1-1/2" RADIUS WHICH IS BUILT INTO CURB FORMS; CORNERS OF TRANSVERSE FLOOR EXPANSION JOINTS SHALL BE ROUNDED WITH A 1/4" FINISHING TOOL UNLESS OTHERWISE REQUIRED ON PLANS; AND CORNERS OF EXPANSION JOINTS IN THE ROADWAY FACES AND TOPS OF CURBS AND SIDEWALKS SHALL BE ROUNDED TO A 1/4" RADIUS WITH A FINISHING STONE OR TOOL UNLESS OTHERWISE REQUIRED ON PLANS.

DOWELS:

DOWELS WHEN INDICATED ON PLANS AS FOR CULVERT EXTENSIONS, SHALL BE EMBEDDED AT LEAST 12" INTO THE OLD CONCRETE AND GROUTED INTO PLACE WITH 1:2 CEMENT MORTAR.

ALLOWANCE FOR DEAD LOAD DEFLECTION, SETTLEMENT, ETC. IN CASTING SUPERSTRUCTURES:

BRIDGES SHALL BE BUILT ON THE GRADE OR VERTICAL CURVE SHOWN ON PLANS.
SLABS, CURBS AND PARAPETS SHALL CONFORM TO THE GRADE OR CURVE.

ALL DIMENSIONS WHICH ARE GIVEN IN SECTION AND ARE AFFECTED BY DEAD LOAD DEFLECTIONS ARE DIMENSIONS AT CENTER LINE OF BEARING UNLESS OTHERWISE NOTED ON PLANS. IN SETTING FORMS FOR STEEL BEAM BRIDGES AND PRESTRESSED CONCRETE GIRDER BRIDGES, ADJUSTMENTS SHALL BE MADE DUE TO THE DEAD LOAD DEFLECTIONS FOR THE ELEVATIONS SHOWN. WHERE BLOCKS ARE SHOWN OVER BEAMS FOR BUILDING UP TO THE SLAB, THE VERTICAL DIMENSIONS OF THE BLOCKS SHALL BE ADJUSTED BETWEEN BEARINGS TO COMPENSATE FOR DEAD LOAD DEFLECTIONS, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER. WHERE BOTTOM OF SLAB IS IN LINE WITH BOTTOM OF TOP FLANGES, DEPTH OF SLAB BETWEEN BEARINGS SHALL BE ADJUSTED TO COMPENSATE FOR DEAD LOAD DEFLECTION, VERTICAL CURVE ORDINATE, AND

IN SETTING FALSEWORK AND FORMS FOR REINFORCED CONCRETE SPANS, AN ALLOWANCE SHALL BE MADE FOR DEAD LOAD DEFLECTIONS, SETTLEMENT OF FALSEWORK, AND PERMANENT CAMBER WHICH SHALL BE PROVIDED FOR IN ADDITION TO THE ELEVATIONS SHOWN. AFTER REMOVAL OF THE FALSEWORK, THE FINISHED STRUCTURES SHALL CONFORM TO THE PROFILE AND ELEVATIONS SHOWN ON THE PLANS AND CONSTRUCTION ELEVATIONS FURNISHED BY THE ENGINEER.

DETAILED DRAWINGS FOR FALSEWORK OR FORMS FOR BRIDGE SUPERSTRUCTURE AND ANY STRUCTURE OR PARTS OF A STRUCTURE AS NOTED ON THE PLANS SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL BEFORE CONSTRUCTION OF THE FALSEWORK OR FORMS IS STARTED.

REINFORCING STEEL:

ALL REINFORCING STEEL SHALL BE DEFORMED WITH THE EXCEPTION OF #2
BARS WHICH MAY BE FABRICATED FROM COLD DRAWN STEEL WIRE. DIMENSIONS
RELATIVE TO PLACEMENT OF REINFORCING ARE TO CENTERS OF BARS UNLESS OTHERWISE
INDICATED IN THE PLANS. DIMENSIONS ON BAR DETAILS ARE TO CENTERS OF BARS
OR ARE OUT TO OUT AS INDICATED ON PLANS.

WIRE BAR SUPPORTS SHALL BE PROVIDED FOR REINFORCING STEEL WHERE
INDICATED ON THE PLANS. WHEN BAR SUPPORT PIECES ARE PLACED IN CONTINUOUS
LINES, THEY SHALL BE SO PLACED THAT THE ENDS OF THE SUPPORTING WIRES SHALL
BE LAPPED TO LOCK LEGS ON ADJOINING PIECES.

STRUCTURAL STEEL:

AT THE CONTRACTOR'S OPTION, HE MAY SUBSTITUTE 7/8% SHEAR STUDS FOR THE 3/4% STUDS SPECIFIED ON THE PLANS. THIS SUBSTITUTION SHALL BE MADE AT THE RATE OF 3-7/8% STUDS FOR 4-3/4% STUDS, AND STUD SPACING CHANGES SHALL BE MADE AS NECESSARY TO PROVIDE THE SAME EQUIVALENT NUMBER OF 7/8% STUDS ALONG THE BEAM AS SHOWN FOR 3/4% STUDS BASED ON THE RATIO OF 3-7/8% STUDS FOR 4-3/4% STUDS. STUDS OF THE LENGTH SPECIFIED ON THE PLANS MUST BE PROVIDED. THE MAXIMUM SPACING SHALL BE 2'-0%.

EXCEPT AT THE INTERIOR SUPPORTS OF CONTINUOUS BEAMS WHERE THE COVER PLATE IS IN CONTACT WITH BEARING PLATE, THE CONTRACTOR MAY, AT HIS OPTION, SUBSTITUTE FOR THE COVER PLATES DESIGNATED ON THE PLANS COVER PLATES OF THE EQUIVALENT AREA PROVIDED THESE PLATES ARE AT LEAST 5/16"IN THICKNESS AND DO NOT EXCEED A WIDTH EQUAL TO THE FLANGE WIDTH LESS 2"OR A THICKNESS EQUAL TO 2 TIMES THE FLANGE THICKNESS. THE SIZE OF FILLET WELDS SHALL CONFORM TO THE REQUIREMENTS OF THE CURRENT ANSI/AASHTO/AWS "BRIDGE WELDING CODE". ELECTROSLAG WELDING WILL NOT BE PERMITTED.

PLACEMENT OF BEAM OR GIRDER MEMBERS ON TRUCKS FOR HAULING SHALL
BE DONE IN COMPLIANCE WITH LIMITS SHOWN ON SKETCHES PROVIDED TO THE MATERIALS
AND TEST UNIT APPROVED BY THE STRUCTURE DESIGN UNIT DATED MAY 8,1991.
THESE SKETCHES PRIMARILY LIMIT THE UNSUPPORTED CANTILEVER LENGTH OF MEMBERS.
WHEN THE CONTRACTOR WISHES TO PLACE MEMBERS ON TRUCKS NOT IN ACCORDANCE
WITH THESE LIMITS, TO SHIP BY RAIL, TO ATTACH SHIPPING RESTRAINTS TO THE
MEMBERS OR TO INVERT MEMBERS, HE SHALL SUBMIT A SKETCH FOR APPROVAL
PRIOR TO SHIPPING. SEE ALSO ARTICLE 1072-11.

WITH THE SOLE EXCEPTION OF EDGES AT SURFACES WHICH BEAR ON OTHER SURFACES, ALL SHARP EDGES AND ENDS OF SHAPES AND PLATES SHALL BE SLIGHTLY ROUNDED BY SUITABLE MEANS TO A RADIUS OF APPROXIMATELY 1/16 INCH OR EQUIVALENT FLAT SURFACE AT A SUITABLE ANGLE PRIOR TO PAINTING, GALVANIZING, OR METALLIZING.

HANDRAILS AND POSTS:

METAL STANDARDS AND FACES OF THE CONCRETE END POSTS FOR THE METAL RAIL SHALL BE SET NORMAL TO THE GRADE OF THE CURB, UNLESS OTHERWISE SHOWN ON PLANS. THE METAL RAIL AND TOPS OF CONCRETE POSTS USED WITH THE ALUMINUM RAIL SHALL BE BUILT PARALLEL TO THE GRADE OF THE CURB.

METAL HANDRAILS SHALL BE IN ACCORDANCE WITH THE PLANS. RAILS SHALL BE AS MANUFACTURED FOR BRIDGE RAILING. CASTINGS SHALL BE OF A UNIFORM APPEARANCE. FINS AND OTHER DEFORMATIONS RESULTING FROM CASTING OR OTHERWISE SHALL BE REMOVED IN A MANNER SO THAT A UNIFORM COLORING OF THE COMPLETED CASTING SHALL BE OBTAINED. CASTINGS WITH DISCOLORATIONS OR OF NON-UNIFORM COLORING WILL NOT BE ACCEPTED. CERTIFIED MILL REPORTS ARE REQUIRED FOR METAL RAILS AND POSTS.

SPECIAL NOTES:

GENERALLY, IN CASE OF DISCREPANCY, THIS STANDARD SHEET OF NOTES SHALL GOVERN OVER THE SPECIFICATIONS, BUT THE REMAINDER OF THE PLANS SHALL GOVERN OVER NOTES HEREON, AND SPECIAL PROVISIONS SHALL GOVERN OVER ALL. SEE SPECIFICATIONS ARTICLE 105-4.

ENGLISH