PROJECT SPECIAL PROVISIONS

ROADWAY

CLEARING AND GRUBBING – METHOD II:

(9-17-02)

SP2 R01

Perform clearing on this project to the limits established by Method "II" shown on Standard No. 200.02 of the *Roadway Standard Drawings*.

ROCK PLATING:

SPI

This work consists of rock plating at locations shown on the plans and as directed by the Engineer.

The fabric shall be placed by unrolling down the slope in a direction perpendicular to the centerline. Fabric shall be buried at the top and embedded at the bottom using dimensions and orientation as shown on the detail. It is preferable that the length of fabric down the slope be continuous. If length of fabric is not sufficient, such as at the end of a roll, an overlap of 5 feet is required with the upper fabric placed over the lower as shown on the detail.

The fabric shall be composed of strong rot-proof synthetic fibers formed into a fabric of the woven type. The fabric shall be free of any treatment or coating which might significantly alter its physical properties after installation. The fabric shall contain stabilizers and/or inhibitors to make the filaments resistant to deterioration resulting from ultraviolet or heat exposure. The fabric shall be a pervious sheet of synthetic fibers oriented into a stable network so that the fibers retain their relative position with respect to each other. The edges of the fabric shall be finished to prevent the outer yarn from pulling away from the fabric. The fabric shall be free of defects or flaws which significantly affect its physical and/or filtering properties.

During all periods of shipment and storage, the cloth shall be wrapped in a heavy duty protective covering to protect it from direct sunlight ultraviolet rays, mud, dust, dirt, and debris. The fabric shall not be exposed to temperatures greater than 140°F. After the protective wrapping has been removed, the fabric shall not be left uncovered under any circumstances for longer than one (1) week.

The fabric shall conform to the following physical requirements:

Fabric Property Test Method Requirements

Minimum Tensile

ASTM D4595-86 (Wide Strip Test) Warp Direction - 200 lb/in

Strength

Fill Direction - 200 lb/in

Maximum Elongation ASTM D4595-86 (Wide Strip Test) Warp Direction 35%

at Minimum Tensile

Fill Direction 35%

Strength

SP2 R15

Warp Direction ASTM D4595-86 (Wide Strip Test) 2200 lb/in Minimum

Tensile Stress-Strain Secant Modulus from 0 to 5% Strain

Minimum Puncture ASTM D 4833 130 lb

Strength

Minimum Bursting ASTM D 3786, (Diaphragm Method) 600 psi

Strength (Mullen)

EOS, U.S. STD. ASTM D 4751 30 min.- 100 max.

Sieve (mm)

The rock shall be plain rip rap meeting the size requirements for Class B rip rap in accordance with the Standard Specifications.

In placing the rock slope protection, the Contractor shall take care not to tear or damage the fabric and in no case shall the rock be allowed to fall from a height greater than 3 feet.

Measurement and Payment

Rock plating will be measured and paid for in square yards of surface, which has been completed and accepted. Such price shall be full compensation for all work and materials necessary to complete this work.

Payment will be made under:

Pay ItemPay UnitRock PlatingSquare Yard

BUILDING AND UNDERGROUND STORAGE TANK REMOVAL:

(1-1-02) (Rev.6-21-05)

Building Removal

Remove the buildings and appurtenances listed below in accordance with Section 215 of the *Standard Specifications* and the following:

Prior to removal of any building, comply with the notification requirements of *Title 40 Code of Federal Regulations*, Part 61, Subpart M, which are applicable to asbestos. Give notification to the North Carolina Department of Health and Human Services, Division of Public Health Epidemiology Branch and/or the appropriate county agency when the county performs enforcement of the Federal Regulation. Submit a copy of the notification to the Engineer prior to the building removal.

Perform removal and disposal of asbestos in accordance with the requirements of *Title 40 Code* of *Federal Regulations*; comply with all Federal, State and local regulations when performing building removal and/or asbestos removal and disposal. Any fines resulting from violations of any regulation are the sole responsibility of the Contractor and the Contractor agrees to indemnify and hold harmless the Department against any assessment of such fines.

The Department has performed asbestos assessments for building items identified below. Copies of this report may be obtained through the Division Right-of-Way Agent. When asbestos is discovered after the opening of bids for the project, the Engineer may have the work performed by others or the cost of asbestos removal and disposal will be paid for in accordance with Article 104-7 of the *Standard Specifications*. When a building has had or will have asbestos removed and the Contractor elects to remove the building such that it becomes a public area, the Contractor is responsible for any additional costs incurred including final air monitoring.

Underground Storage Tank Removal

Prior to removal of any Underground Storage Tank (UST), comply with the notification requirements of the *Title 40 Code of Federal Regulations*, Part 280.71(a). Give notification to the appropriate regional office of the North Carolina Department of Environment and Natural Resources, Division of Waste Management, UST Section. Submit a copy of the notification to the Engineer prior to the removal of the underground storage tank.

Permanently close UST systems by removal and disposal in compliance with the regulations set forth in *Title 40, Code of Federal Regulations*, Part 280.71 and *North Carolina Administrative Code (NCAC)* Title 15A, Chapter 2, Subchapter 2N and any applicable local regulations. Assess Underground Storage Tank sites at closure for the presence of contamination as required in *NCAC* Title 15A, Chapter 2, Subchapter 2N, Section .0803 and as directed by the appropriate Regional Office of the Division of Waste Management. Remove and dispose of UST systems and contents in a safe manner in conformance with requirements of *American Petroleum Institute Bulletin 1604*, Removal and Disposal of Used Underground Petroleum Storage Tanks, Chapters 3 through 6. (Note: As an exception to these requirements, the filling of the tank with water as a means of expelling vapors from the tank as described in Section 4.2.6.1 of *American Petroleum Institute Bulletin 1604*, will not be allowed. Comply with all Federal, State and local regulations when performing UST removal and contaminated material disposal. Any fines resulting from violations of any regulation are the sole responsibility of the Contractor and the Contractor agrees to indemnify and hold harmless the Department against any assessment of such fines.

Where underground storage tanks are indicated below, there will be no direct payment for the assessment or closure. When the contract does not indicate the presence of storage tanks and storage tanks are discovered after the opening of bids for the project, the Engineer may have the work performed by others or the cost of assessment, closure, and/or removal will be paid for in accordance with Article 104-7 of the *Standard Specifications*.

Disposition of any contaminated material associated with underground storage tanks will be made as provided in Article 107-26 of the *Standard Specifications*.

PARCEL	LOCATION	IMPROVEMENT
5	Right of Survey Line L –	1½ Story Frame Dwelling
	between Survey Station 20+60	
	and Survey Station 21+77	
5	Right of Survey Line L –	Garage
	between Survey Station 20+60	
	and Survey Station 21+77	
5	Right of Survey Line L –	Shed
	between Survey Station 20+60	
	and Survey Station 21+77	
5	Right of Survey Line L –	Well house
	between Survey Station 20+60	
	and Survey Station 21+77	
45	Left of Survey Line L –	1 Story Frame Dwelling
	between Survey Station	
	110+50 and 112+50	
45	Left of Survey Line L –	Storage Building (Chicken
	between Survey Station	Coup)
	110+50 and 112+50	

FALSE SUMPS:

(7-1-95) SP2 R40

Construct false sumps in accordance with the details in the plans and at locations shown in the plans or at other locations as directed by the Engineer.

Payment for the work of construction of the false sumps will be made at the contract unit price per cubic yard for *Unclassified Excavation* or *Borrow Excavation* depending on the source of material, or included in *Grading-Lump Sum*.

SHOULDER AND FILL SLOPE MATERIAL:

(5-21-02) SP2 R50

Description

C201667

Perform the required shoulder and slope construction for this project in accordance with the applicable requirements of Section 560 and Section 235 of the *Standard Specifications* except as follows:

Construct the top 6 inches of shoulder and fill slopes with soils capable of supporting vegetation.

Provide soil with a P.I. greater than 6 and less than 25 and with a pH ranging from 5.5 to 6.8. Remove stones and other foreign material 2 inches or larger in diameter. All soil is subject to test and acceptance or rejection by the Engineer.

Obtain material from within the project limits or approved borrow source.

Compensation

When the Contractor elects to obtain material from an area located beneath a proposed fill sections which does not require excavation for any reason other than to generate acceptable shoulder and fill slope material, the work of performing the excavation will be considered incidental to the item of *Borrow Excavation* or *Shoulder Borrow*. If there is no pay item for *Borrow* or *Shoulder Excavation* in the contract, this work will be considered incidental to *Unclassified Excavation*. Stockpile the excavated material in a manner to facilitate measurement by the Engineer. Fill the void created by the excavation of the shoulder and fill slope material with suitable material. Payment for material used from the stockpile will be made at the contract unit price for *Borrow Excavation* or *Shoulder Borrow*, then the material will be paid for at the contract unit price for *Unclassified Excavation*. The material used to fill the void created by the excavation of the shoulder and fill slope material will be made at the contract unit price for *Unclassified Excavation*, or *Shoulder Borrow*, depending on the source of the material.

Material generated from undercut excavation, unclassified excavation or clearing and grubbing operations that is placed directly on shoulders or slope areas, will not be measured separately for payment, as payment for the work requiring the excavation will be considered adequate compensation for depositing and grading the material on the shoulders or slopes.

When undercut excavation is performed at the direction of the Engineer and the material excavated is found to be suitable for use as shoulder and fill slope material, and there is no area on the project currently prepared to receive the material generated by the undercut operation, the Contractor may construct a stockpile for use as borrow at a later date. Payment for the material used from the stockpile will be made at the contract unit price for *Borrow Excavation* or *Shoulder Borrow*.

When shoulder material is obtained from borrow sources or from stockpiled material, payment for the work of shoulder construction will be made at the contract unit price per cubic yard for *Borrow Excavation* or *Shoulder Borrow* in accordance with the applicable provisions of Section 230 or Section 560 of the *Standard Specifications*.

NOTE TO CONTRACTOR:

All waste material not utilized as borrow material, shall be stockpiled by the Contractor between Station 79+00-80+00 -L- RT. This material will be removed by State Forces.

FLOWABLE FILL:

(9-17-02) (Rev 7-18-06)

SP3 R30

Description

This work consists of all work necessary to place flowable fill in accordance with these provisions, the plans, and as directed.

Materials

Provide flowable fill material in accordance with Article 340-3 of the Standard Specifications.

Construction Methods

Discharge flowable fill material directly from the truck into the space to be filled, or by other approved methods. The mix may be placed full depth or in lifts as site conditions dictate. The Contractor shall provide a method to plug the ends of the existing pipe in order to contain the flowable fill.

Measurement and Payment

At locations where flowable fill is called for on the plans and a pay item for flowable fill is included in the contract, *flowable fill* will be measured in cubic yards and paid for as the actual number of cubic yards that have been satisfactorily placed and accepted. Such price and payment will be full compensation for all work covered by this provision including but not limited to the mix design, furnishing, hauling, placing and containing the flowable fill.

Payment will be made under:

Pay Item Flowable Fill

Pay Unit Cubic Yard

PIPE ALTERNATES:

(7-18-06) SP3 R35

Description

The Contractor may substitute Aluminized Corrugated Steel Pipe, Type IR or HDPE Pipe, Type S or Type D up to 48 inches in diameter in lieu of concrete pipe in accordance with the following requirements.

Material

Item	Section
HDPE Pipe, Type S or D	1044-7
Aluminized Corrugated Steel Pipe, Type IR	1032-3(A)(7)

Aluminized Corrugated Steel Pipe will not be permitted in counties listed in Article 310-2 of the *Standard Specifications*.

Construction Methods

Aluminized Corrugated Steel Pipe Culverts and HDPE Pipe Culverts shall be installed in accordance with the requirements of Section 300 of the *Standard Specifications* for Method A, except that the minimum cover shall be at least 12 inches. Aluminized Corrugated Steel Pipe Culvert and HDPE Pipe Culvert will not be permitted for use under travelways, including curb and gutter.

Measurement and Payment

112000 VII VARIORI WARM I WY ALLVARY	
The quantity of "Aluminized Corrugated Steel Pipe Culvert to be number of linear feet installed and accepted. Measurement will be it 310-6 of the Standard Specifications.	•
The quantity of "HDPE Pipe Culvert to be paid for will be the a installed and accepted. Measurement will be in accordance with Sect Specifications.	
Payment will be made under:	
Pay Item " Aluminized Corrugated Steel Pipe Culverts, " Thick " HDPE Pipe Culverts	Pay Unit Linear Foot Linear Foot

AGGREGATE BASE COURSE:

12-19-06 SP5 R03

Revise the 2006 Standard Specifications as follows:

Page 5-11, Article 520-5 Hauling and Placing Aggregate Base Material, 6th paragraph, replace the first sentence with the following:

Base course that is in place on November 15 shall have been covered with a subsequent layer of pavement structure or with a sand seal. Base course that has been placed between November 16 and March 15 inclusive shall be covered within 7 calendar days with a subsequent layer of pavement structure or with a sand seal.

ASPHALT PAVEMENTS - SUPERPAVE:

(7-18-06) (Rev 9-19-06)

SP6 R01

Revise the 2006 Standard Specifications as follows:

Page 6-2, Article 600-9 Measurement and Payment

Delete the second paragraph.

Page 6-12, 609-5(C)2(c) add after (AASHTO T 209):

or ASTM D 2041

Page 6-13, last line on page & Page 6-14, Subarticle 609-5(C)(2)(e), delete and substitute the following:

- (e) Retained Tensile Strength (TSR) (AASHTO T 283 Modified), add subarticle (1) Option 1 before the first paragraph.
 - (1) Option 1

Add subarticle (2) Option 2 and the following sentence as the first sentence of the second paragraph:

(2) Option 2

Mix sampled from truck at plant with one set of specimens prepared by the Contractor and then tested jointly by QA and QC at a mutually agreed upon lab site within the first 7 calendar days after beginning production of each new mix design.

Page 6-28, 610-3(A) Mix Design-General, third sentence of the fourth paragraph:

Substitute 20% for 15%

First, second and third sentences of the fifth paragraph:

Substitute 20% for 15%

Page 6-44, 610-8, third full paragraph, replace the first sentence with the following:

Use the 30 foot minimum length mobile grade reference system or the non-contacting laser or sonar type ski with at least four referencing stations mounted on the paver at a minimum length of 24 feet to control the longitudinal profile when placing the initial lanes and all adjacent lanes of all layers, including resurfacing and asphalt in-lays, unless otherwise specified or approved.

Page 6-54, Article 620-4, add the following pay item:

Pay Item

Pay Unit

Asphalt Binder for Plant Mix, Grade PG 70-28

Ton

Page 6-69, Table 660-1 Material Application Rates and Temperatures, add the following:

Type of Coat	Grade of Asphalt	Asphalt Rate gal/yd ²	Application Temperature °F	Aggregate Size	Aggregate Rate lb./sq. yd. Total	
Sand Seal	CRS-2 or CRS-2P	0.22-0.30	150-175	Blotting Sand	12-15	

Page 6-75, 660-9(B), add the following as sub-item (5)

(5) Sand Seal

Place the fully required amount of asphalt material in one application and immediately cover with the seal coat aggregate. Uniformly spread the fully required amount of aggregate in one application and correct all non-uniform areas prior to rolling.

Immediately after the aggregate has been uniformly spread, perform rolling.

When directed, broom excess aggregate material from the surface of the seal coat.

When the sand seal is to be constructed for temporary sealing purposes only and will not be used by traffic, other grades of asphalt material meeting the requirements of Articles 1020-6 and 1020-7 may be used in lieu of the grade of asphalt required by Table 660-1 when approved.

Page 10-41, Table 1012-1, add the following:

Mix	Course Aggregate	Fine Aggregate Angularity	Sand Equivalent	Flat & Elongated 5:1 Ratio
Туре	Angularity (b) ASTM	% Minimum AASHTO	% Minimum	% Maximum ASTM
	D5821	T304 Method A	AASHTO T176	D4791 Section 8.4
S 9.5 D	100/100	45	50	10

Page 10-45, Replace Table 1012-2 with the following:

TABLE 1012-2 NEW SOURCE RAP GRADATION and BINDER TOLERANCES

(Apply Tolerances to Mix Design Data)

Mix Type	0	0-20% RAP		0-20% RAP 21-25% RAP				26%+ RAP		
Sieve (mm)	Base	Inter.	Surf.	Base	Inter.	Surf.	Base	Inter.	Surf.	
P _b , %		± 0.7%			± 0.4%			± 0.3%		
1 1/2" (37.5)	±10	-	-	±7	-	-	±5	-	-	
3/4" (19.0)	±10	±10	_	±7	±7	-	±5	±5	-	
1/2" (12.5)	-	±10	±6	-	±7	±3	-	±5	±2	
3/8" (9.5)	-	-	±8		-	±5	-	-	<u>±</u> 4	
No. 4 (4.75)	±10	· —	±10	<u>±</u> 7	***	±7	±5	-	±5	
No. 8 (2.36)	±8	±8	±8	±5	±5	±5	±4	<u>+</u> 4	<u>+</u> 4	
No.16 (1.18)	±8	±8	±8	±5	±5	±5	±4	<u>±</u> 4	<u>±</u> 4	
No. 30 (0.600)	±8	±8	±8	±5	±5	±5	±4	±4	±4	
No. 50 (0.300)	_		±8	-	•	±5	-	-	±4	
No. 200 (0.075)	±4	±4	±4	±2	±2	±2	±1.5	±1.5	±1.5	

ASPHALT BINDER CONTENT OF ASPHALT PLANT MIXES:

(11-21-00)

SP6 R15

The approximate asphalt binder content of the asphalt concrete plant mixtures used on this project will be as follows:

Asphalt Concrete Base Course	Type B 25.0	4.3%
Asphalt Concrete Intermediate Course	Type I 19.0	4.7%
Asphalt Concrete Surface Course	Type S 4.75A	7.0%
Asphalt Concrete Surface Course	Type SF 9.5A	6.5%
Asphalt Concrete Surface Course	Type S 9.5	6.0%
Asphalt Concrete Surface Course	Type S 12.5	5.5%

The actual asphalt binder content will be established during construction by the Engineer within the limits established in the *Standard Specifications*.

ASPHALT PLANT MIXTURES:

(7-1-95) SP6 R20

Place asphalt concrete base course material in trench sections with asphalt pavement spreaders made for the purpose or with other equipment approved by the Engineer.

PRICE ADJUSTMENT - ASPHALT BINDER FOR PLANT MIX:

(11-21-00) SP6 R25

Price adjustments for asphalt binder for plant mix will be made in accordance with Section 620 of the *Standard Specifications*.

The base price index for asphalt binder for plant mix is \$312.14 per ton.

This base price index represents an average of F.O.B. selling prices of asphalt binder at supplier's terminals on January 1, 2006.

CONVERT EXISTING DROP INLET TO JUNCTION BOX WITH MANHOLE:

(1-1-02) (Rev. 7-18-06)

SP8 R50

At the proper phase of construction, convert the existing drop inlet at locations indicated in the plans or where directed, to junction box with manhole in accordance with the details in the plans and the applicable requirements of Sections 840 and 859 of the *Standard Specifications*.

Convert Existing Drop Inlet to Junction Box with Manhole will be measured and paid for as each, completed and accepted. Such price and payment is considered full compensation for all equipment, materials, labor, tools, and incidentals necessary to complete each conversion satisfactorily.

Payment will be made under:

Pay Item

Pay Unit

Convert Existing Drop Inlet to Junction Box

Each

with Manhole

GUARDRAIL ANCHOR UNITS, TYPE 350:

(4-20-04)

SP8 R65

Description

Furnish and install guardrail anchor units in accordance with the details in the plans, the applicable requirements of Section 862 of the *Standard Specifications*, and at locations shown in the plans.

Materials

The Contractor may at his option, furnish any one of the guardrail anchor units.

Guardrail anchor unit (ET-2000) as manufactured by:

Trinity Industries, Inc. 2525 N. Stemmons Freeway Dallas, Texas 75207 Telephone: 800-644-7976

The guardrail anchor unit (SKT 350) as manufactured by:

Road Systems, Inc. 3616 Old Howard County Airport Big Spring, Texas 79720 Telephone: 915-263-2435

Prior to installation the Contractor shall submit to the Engineer:

- FHWA acceptance letter for each guardrail anchor unit certifying it meets the requirements of NCHRP Report 350, Test Level 3, in accordance with Section 106-2 of the Standard Specifications.
- (B) Certified working drawings and assembling instructions from the manufacturer for each guardrail anchor unit in accordance with Section 105-2 of the Specifications.

No modifications shall be made to the guardrail anchor unit without the express written permission from the manufacturer. Perform installation in accordance with the details in the plans, and details and assembling instructions furnished by the manufacturer.

Construction Methods

Guardrail end delineation is required on all approach and trailing end sections for both temporary and permanent installations. Guardrail end delineation consists of yellow reflective sheeting applied to the entire end section of the guardrail in accordance with Section 1088-3 of the Standard Specifications and is incidental to the cost of the guardrail anchor unit.

Measurement and Payment

Measurement and payment will be made in accordance with Articles 862-6 of the Standard Specifications.

Payment will be made under:

Pay Item

Pay Unit

Guardrail Anchor Units, Type 350

Each

AGGREGATE PRODUCTION:

(11-20-01)

SP10 R05

Provide aggregate from a producer who uses the current Aggregate Quality Control/Quality Assurance Program that is in effect at the time of shipment.

No price adjustment is allowed to contractors or producers who use the program. Participation in the program does not relieve the producer of the responsibility of complying with all requirements of the *Standard Specifications*. Copies of this procedure are available upon request from the Materials and Test Unit.

CONCRETE BRICK AND BLOCK PRODUCTION:

(11-20-01)

SP10 R10

Provide concrete brick and block from a producer who uses the current Solid Concrete Masonry Brick/Unit Quality Control/Quality Assurance Program that is in effect on the date that material is received on the project.

No price adjustment is allowed to contractors or producers who use the program. Participation in the program does not relieve the producer of the responsibility of complying with all requirements of the *Standard Specifications*. Copies of this procedure are available upon request from the Materials and Test Unit.

GLASS BEADS:

(7-18-06)

SP10 R35

Revise the *Standard Specifications* as follows:

Page 10-223, 1087-4(C) Gradation & Roundness

Replace the second sentence of the first paragraph with the following:

All Drop-On and Intermixed Glass Beads shall be tested in accordance with ASTM D1155.

Delete the last paragraph.

ENGINEERING FABRICS TABLE 1056-1:

(7-18-06)

SP10 R40

Revise the *Standard Specifications* as follows:

Page 10-100, Table 1056-1, replace the values for Trapezoidal Tear Strength with the following:

Physical Property ASTM Test Method		Type 1	Type 2	Тур	e 3	Type 4
				Class A	Class B	**************************************
Typical Applications	:	Shoulder Drain	Under Riprap	Tempor Fer	•	Soil Stabilization
Trapezoidal Tear Strength	D4533	<i>45</i> lb	<i>75</i> lb			75 lb

TEMPORARY SHORING FOR MAINTENANCE OF TRAFFIC:

(1-15-02) (Rev.7-18-06)

SP11 R01

Description

Furnish, install, and remove sheeting, shoring, and bracing necessary to maintain traffic at locations shown on the plans, and other locations determined during construction. Shoring required to maintain traffic is defined as shoring necessary to provide lateral support to the side of an excavation or embankment parallel to an open travelway when a theoretical 2:1 or steeper slope from the bottom of the excavation or embankment intersects the existing ground line closer than 5 feet from the edge of pavement of the open travelway. The Contractor has option of submitting their own shoring design or using the standard shoring design, unless otherwise noted in the plans.

Materials

Sheet piling shall be hot rolled and conform to the requirements of ASTM A328.

Steel piles shall conform to the requirements of ASTM A36.

Timber and lumber shall conform to the requirements of Article 1082-3 in *Standard Specifications*.

Include all materials proposed for use in temporary shoring in the shoring design submittal described below.

Provide a Type 7 Contractor's Certification for all shoring materials used.

Contractor Shoring Design

Submit shoring design for review and approval by the Engineer prior to beginning construction.

Submit calculations and detail drawings in accordance with Article 400-3 of the *Standard Specifications*.

Design all temporary shoring in accordance with the latest edition of AASHTO's *Guide Design Specifications for Bridge Temporary Works*.

If temporary concrete barrier is to be located within 3 feet of the top of the shoring, measured to the back face of the barrier, design the temporary shoring to resist the lateral movement of the barrier when struck by a vehicle and extend the shoring out of the ground at least to the top elevation of the temporary concrete barrier. Design the temporary shoring to resist an impact load of 2 kips/foot applied at one and half feet above ground.

Standard Shoring Design

Select the appropriate shoring design from the *Standard Temporary Shoring for Maintenance of Traffic* detail drawing as shown in the plans.

Submit a Standard Shoring Selection Form to the Engineer a minimum of 14 days prior to beginning construction of shoring.

Forms are located at website:

http://www.ncdot.org/doh/preconstruct/highway/geotech/formprovdet/

Criteria for the Standard Shoring Designs

Maximum height of shoring excavation is 11 feet.

Groundwater table is not above bottom of shoring excavation.

Traffic surcharge equal to 240 psf

Soldier pile spacing is 6 feet.

Soldier pile embedment depths are for driven piles.

Timber lagging shall have minimum thickness of 3 inches

Timber shall have a minimum allowable bending stress of 1000 psi.

If conditions at the shoring location do not meet the criteria of the standard shoring design as shown above and in the plans, the Contractor shall submit a shoring design to the Engineer for approval.

Construction Methods

Install and interlock steel sheet piles to a tolerance of not more than 3/8 inch per foot from vertical.

If soldier piles are used, then install piles to a tolerance of not more than 1/4 inch per foot from vertical.

If soldier piles are to be installed in drilled holes, set piles in drilled holes and fill the holes as soon as practical after installing the piles.

Excavate or auger the soil and rock in 2 foot diameter holes to the required embedment depth as shown on the approved design. Maintain holes, if required, by casing or other means. Set soldier piles to bottom of the hole prior to backfilling. Backfill holes with Class A concrete to the bottom of excavation. Fill remainder of hole with a lean sand-grout mixture to the ground surface. Remove mixture as necessary to install timber lagging.

Use timber lagging with a minimum 3 inch thickness perpendicular to the pile flange. Install timber lagging with a minimum bearing distance of 3 inches on each pile flange. Backfill voids behind lagging with granular material or compacted excavated material to the satisfaction of the Engineer.

Backfill and compact fill for shoring excavation prior to removal of shoring.

If the design embedment depth is not achieved, then notify the Engineer immediately.

Measurement and Payment

Temporary Shoring will be measured and paid for as the actual number of square feet of exposed face of the shoring measured from the bottom of the shoring excavation or embankment to the top of the shoring, with the upper limit for pay purposes not to exceed 1 foot above the retained ground elevation.

Temporary Shoring - Barrier Supported will be measured and paid for will as the actual number of square feet of exposed face of the shoring measured from the bottom of the excavation or embankment to the top of the shoring, with the upper limit for pay purposes not to exceed 1 foot above the retained ground elevation. Payment for temporary shoring will be made only at locations where it is required in order to maintain traffic.

Such payment will include, but not limited to boring, furnishing all labor, tools, equipment, and all incidentals necessary to install shoring and complete the work. Trench boxes are not considered temporary shoring for the maintenance of traffic and will not be paid for under this special provision.

Payment will be made under:

Pay ItemPay UnitTemporary ShoringSquare FeetTemporary Shoring - Barrier SupportedSquare Feet

PAVEMENT MARKING LINES MEASUREMENT AND PAYMENT:

 $\overline{(11-21-06)}$

SP 12 R01

Revise the 2006 Standard Specifications as follows:

Page 12-14, Subarticle 1205-10, delete the first sentence of the first paragraph and replace with the following:

Pavement Marking Lines will be measured and paid for as the actual number of linear feet of pavement marking lines per application that has been satisfactorily placed and accepted by the Engineer.

PORTLAND CEMENT CONCRETE (ALKALI-SILICA REACTION):

2-20-07

SP10 R16

Revise the 2006 Standard Specifications as follows:

Article 1024-1(A), replace the 2nd paragraph with the following:

Certain combinations of cement and aggregate exhibit an adverse alkali-silica reaction. The alkalinity of any cement, expressed as sodium-oxide equivalent, shall not exceed 1.0 percent. For mix designs that contain non-reactive aggregates and cement with an alkali content less than 0.6%, straight cement or a combination of cement and fly ash, cement and ground granulated blast furnace slag or cement and microsilica may be used. The pozzolan quantity shall not exceed the amount shown in Table 1024-1. For mixes that contain cement with an alkali content between 0.6% and 1.0%, and for mixes that contain a reactive aggregate documented by the Department, regardless of the alkali content of the cement, use a pozzolan in the amount shown in Table 1024-1.

Obtain the list of reactive aggregates documented by the Department at:http://www.ncdot.org/doh/operations/materials/pdf/quarryasrprob.pdf

	Table 1024-1		
Pozzolans for Use in Portland Cement Concrete			
Pozzolan	Rate		
Class F Fly Ash	20% by weight of required cement content, with 1.2 lbs Class F fly ash per lb of cement replaced		
Ground Granulated Blast Furnace Slag	35%-50% by weight of required cement content with 1 lb slag per lb of cement replaced		
Microsilica	4%-8% by weight of required cement content, with 1 lb microsilica per lb of cement replaced		