BENCH MARK #2: RAILROAD SPIKE SET IN 17" Ø MAPLE TREE, STA. 23+54.00-BL-, 128.4' RIGHT, EL. 34.000, NAVD 88 HYDRAULIC DATA DESIGN DISCHARGE = 3,000 CFS. = 25 YR. FREQUENCY OF DESIGN FLOOD DESIGN HIGH WATER ELEVATION = 38.870 DRAINAGE AREA = 53.6 SQ. MI. = 4,500 CFS. BASIC DISCHARGE (Q100) Θ BASIC HIGH WATER ELEVATION = 40.370 OVERTOPPING FLOOD DATA ننحننحنن OVERTOPPING DISCHARGE = 7,000 CFS.CLASS II FREQUENCY OF OVERTOPPING FLOOD = 500+YR. RIP RAP OVERTOPPING FLOOD ELEVATION STA. 17+62.50 -L-= 42.800 TO SR 1001 ----- TO SR 1154 90°-00′-00<u>″\</u> (TYP.) EXISTING _CLASS II ^{5;} STRUCTURE— RIP RAP > TO BE REMOVED NOTE: FOR UTILITY INFORMATION, SEE UTILITY PLANS AND SPECIAL PROVISIONS.

—— LOCATION SKETCH ——

NOTES

ASSUMED LIVE LOAD = HS20 OR ALTERNATE LOADING, EXCEPT THAT CORED SLAB UNITS HAVE BEEN DESIGNED FOR HS25.

FOR OTHER DESIGN DATA AND GENERAL NOTES, SEE SHEET SN.

FOR EROSION CONTROL MEASURES SEE EROSION CONTROL PLANS.

THIS BRIDGE HAS BEEN DESIGNED BY THE STRENGTH DESIGN METHOD AS SPECIFIED IN AASHTO STANDARD SPECIFICATIONS.

THE EXISTING STRUCTURE CONSISTING OF 4 SPANS 1 @ 30'-6", 2 @ 30'-0", AND 1 @ 30'-6", PRESTRESSED CONCRETE CHANNELS WITH A 3.5" ASPHALT WEARING SURFACE ON PRECAST CONCRETE CAPS ON TIMBER PILES WITH A CLEAR ROADWAY WIDTH OF 24'-1" AND LOCATED AT THE PROPOSED SITE SHALL BE REMOVED. THE EXISTING BRIDGE IS PRESENTLY POSTED BELOW THE LEGAL LOAD LIMIT. FOR SALVAGE OF MATERIAL, SEE SPECIAL PROVISIONS FOR "REMOVAL OF EXISTING STRUCTURE AT

REMOVAL OF THE EXISTING BRIDGE SHALL BE PERFORMED SO AS NOT TO ALLOW DEBRIS TO FALL INTO THE WATER. THE CONTRACTOR SHALL REMOVE THE BRIDGE AND SUBMIT PLANS FOR DEMOLITION IN ACCORDANCE WITH ARTICLE 402-2 OF THE STANDARD SPECIFICATIONS.

STA. 17+62.50-L-."

THE MATERIAL SHOWN IN THE CROSS-HATCHED AREA SHALL BE EXCAVATED FOR A DISTANCE OF 25 FT. EACH SIDE OF CENTERLINE ROADWAY AS DIRECTED BY THE ENGINEER. THIS WORK WILL BE PAID FOR AT THE CONTRACT LUMP SUM PRICE FOR UNCLASSIFIED STRUCTURE EXCAVATION. FOR UNCLASSIFIED STRUCTURE EXCAVATION. FOR UNCLASSIFIED STRUCTURE EXCAVATION. SEE SPECIAL PROVISIONS.

THE SUBSTRUCTURE OF THE EXISTING BRIDGE INDICATED ON THE PLANS IS FROM THE BEST INFORMATION AVAILABLE. SINCE THIS INFORMATION IS SHOWN FOR THE CONVENIENCE OF THE CONTRACTOR, THE CONTRACTOR SHALL HAVE NO CLAIM WHATSOEVER AGAINST THE DEPARTMENT OF TRANSPORTATION FOR ANY DELAYS OR ADDITIONAL COST INCURRED BASED ON DIFFERENCES BETWEEN THE EXISTING BRIDGE SUBSTRUCTURE SHOWN ON THE PLANS AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

ASPHALT WEARING SURFACE IS INCLUDED IN ROADWAY QUANTITY ON ROADWAY PLANS.

FOR CRANE SAFETY, SEE SPECIAL PROVISIONS.

THIS STRUCTURE HAS BEEN DESIGNED IN ACCORDANCE WITH HEC 18 "EVALUATING SCOUR AT BRIDGES", NOVEMBER, 1995.

FOR FALSEWORK AND FORMWORK, SEE SPECIAL PROVISIONS.

THIS BRIDGE HAS BEEN DESIGNED IN ACCORDANCE WITH THE REQUIREMENTS OF THE AASHTO STANDARD SPECIFICATIONS FOR SEISMIC DESIGN OF HIGHWAY BRIDGES FOR SEISMIC PERFORMANCE CATEGORY A.

THE STANDARD THREE MONTH WAITING PERIOD AFTER THE COMPLETION OF THE EMBANKMENT PRIOR TO THE CONSTRUCTION OF THE APPROACHSLAB, HAS BEEN WAIVED.

FOR SUBMITTAL OF WORKING DRAWINGS, SEE SPECIAL PROVISIONS.

THE CONTRACTOR SHALL PROVIDE INDEPENDENT ASSURANCE SAMPLES OF REINFORCING STEEL AS FOLLOWS: FOR PROJECTS REQUIRING UP TO 400 TONS OF REINFORCING STEEL, ONE 30 INCH SAMPLE OF EACH SIZE BAR USED, AND FOR PROJECTS REQUIRING OVER 400 TONS OF REINFORCING STEEL, TWO 30 INCH SAMPLES OF EACH SIZE BAR USED. THE BARS FROM WHICH THE SAMPLES ARE TAKEN MUST THEN BE SPLICED WITH REPLACEMENT BARS OF THE SIZE AND LENGTH OF THE SAMPLE, PLUS A MINIMUM LAP SPLICE OF THIRTY BAR DIAMETERS.

THIS BRIDGE SHALL BE CONSTRUCTED USING TOP-DOWN CONSTRUCTION METHODS. CRANE ACCESS MAY BE RESTRICTED TO SPAN A AND SPAN C REQUIRING WORK FROM BOTH ENDS. THE USE OF A TEMPORARY CAUSEWAY OR WORK BRIDGE IS NOT PERMITTED.

DRIVE PILES AT END BENT NO.1 AND NO.2 TO A MINIMUM BEARING CAPACITY OF 50 TONS EACH.

DRIVE PILES AT BENT NO.1 AND NO.2 TO AN ELEVATION NO HIGHER THAN EL.2.000 AND A MINIMUM BEARING CAPACITY OF 60 TONS EACH PLUS CAPACITY TO ACCOUNT FOR DOWN DRAG OR NEGATIVE SKIN FRICTION AND SCOUR.

THE SCOUR CRITICAL ELEVATION FOR BENT NO.1 AND NO.2 IS ELEVATION 16.000 FEET. BRIDGE MAINTENANCE USES SCOUR CRITICAL ELEVATIONS TO MONITOR POSSIBLE SCOUR PROBLEMS DURING THE LIFE OF THE STRUCTURE.

WHEN DRIVING PILES, THE MAXIMUM BLOW COUNT SHALL NOT BE EXCEEDED.

PREDRILLING OF PILES TO ELEVATION 16.000 MAYBE UTILIZED TO INSTALL PILES AT BENTS NO.1 AND NO.2. SEE PREDRILLING OF PILES SPECIAL PROVISIONS.

FOR CONSTRUCTION OF SUPERSTRUCTURE, SEE SPECIAL PROVISIONS.

FOR CONSTRUCTION OF SUBSTRUCTURE, SEE SPECIAL PROVISIONS.

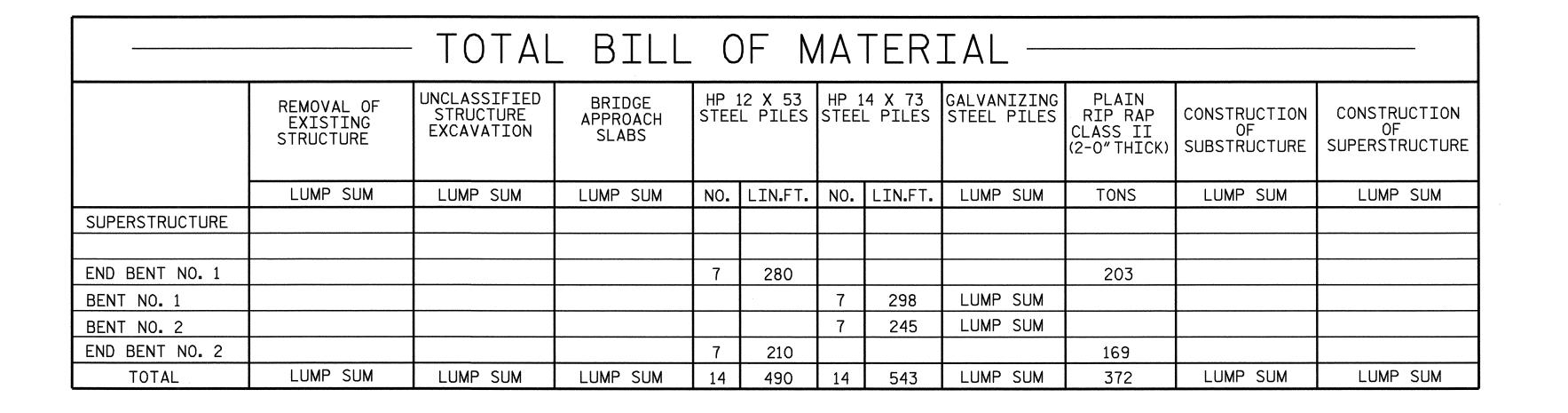
JETTING IS NOT ALLOWED TO INSTALL PILES.

PROVIDE GALVANIZED STEEL PILES AT INTERIOR BENTS, IN ACCORDANCE WITH SECTION 1076 OF THE STANDARD SPECIFICATIONS AND THE GALVANIZING STEEL PILES SPECIAL PROVISIONS.

FOR STEEL H PILES, SEE SPECIAL PROVISIONS.

PILINGS FROM THE EXISTING BRIDGE, AS WELL AS ANY REMNANT PILINGS FROM PREVIOUS BRIDGES SHALL BE REMOVED IN THEIR ENTIRETY. IN THE EVENT THAT A PILING BREAKS DURING THE REMOVAL AND CANNOT BE REMOVED IN ITS ENTIRETY, THE PILING MAY BE CUT OFF FLUSH WITH THE BED OF THE WATER BODY IF PRIOR APPROVAL IS RECEIVED FROM NORTH CAROLINA DIVISION OF COASTAL MANAGEMENT. NO SEPARATE PAYMENT WILL BE MADE FOR REMOVAL OF EXISTING PILINGS AS IT IS CONSIDERED TO BE INCLUDED IN THE PAY ITEM REMOVAL OF EXISTING STRUCTURE AT STA. 17+62.50-L-.

THE COST OF THE 2 BAR METAL RAIL AND CONCRETE PARAPET SHALL BE INCLUDED IN THE LUMP SUM PRICE BID FOR CONSTRUCTION OF SUPERSTRUCTURE. SEE SPECIAL PROVISIONS FOR "CONSTRUCTION OF SUPERSTRUCTURE."



PROJECT NO. B-4224

PENDER/DUPLIN COUNTY

STATION: 17+62.50 -L-

SHEET 3 OF 3

STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION

GENERAL DRAWING

FOR BRIDGE OVER DOCTOR'S CREEK ON SR 1305/SR 1155 BETWEEN SR 1001 AND SR 1154

REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	S-3
1			3			TOTAL SHEETS
2			4			28

DRAWN BY: S.P. LAM DATE: 10/12/05
CHECKED BY: H. T. BARBOUR DATE: 10/25/05